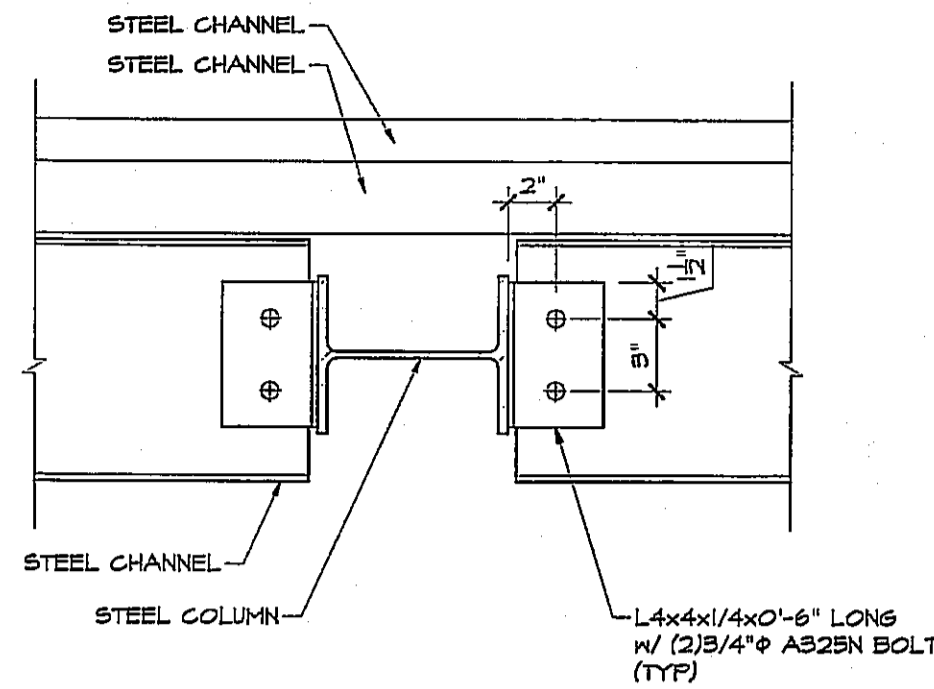
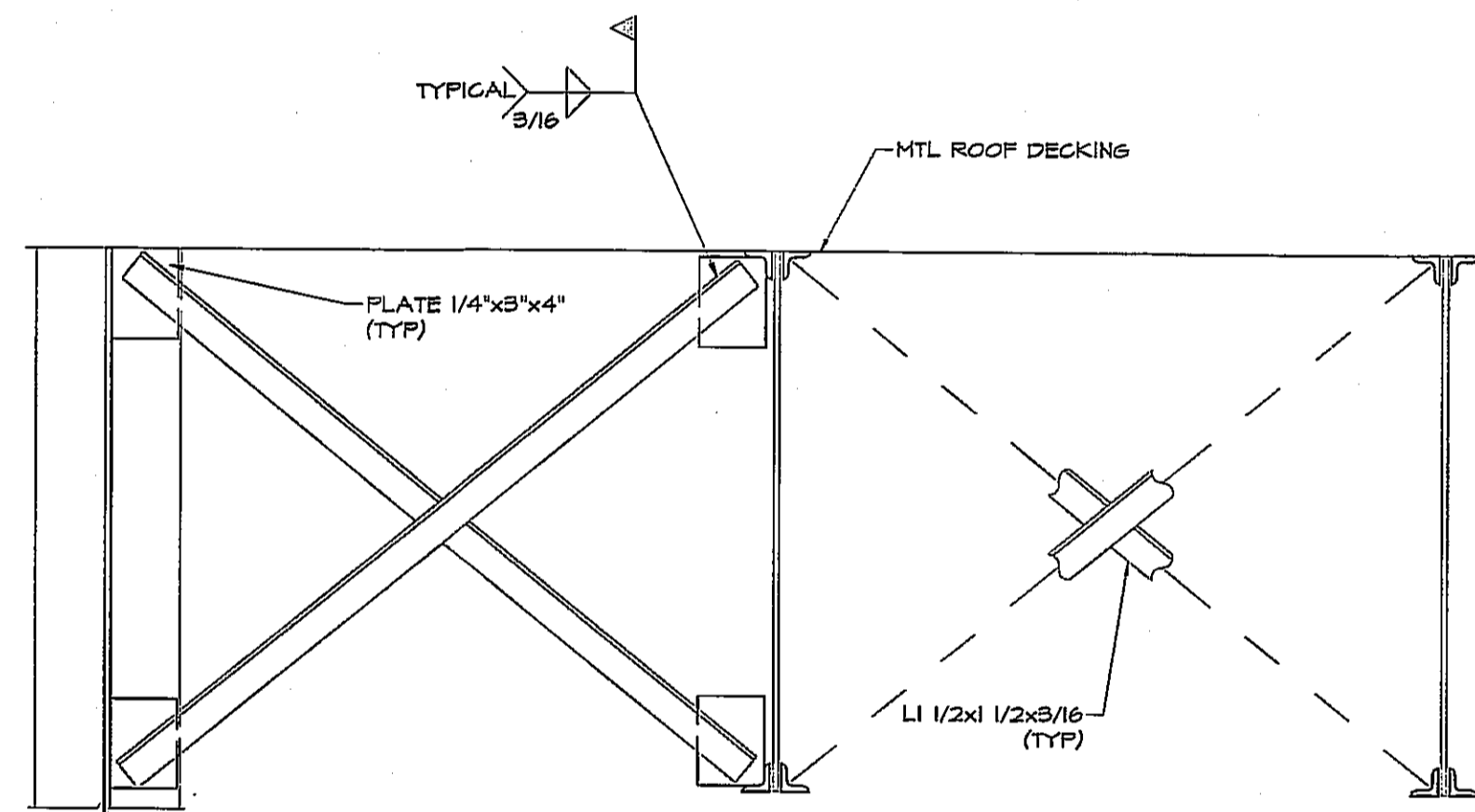


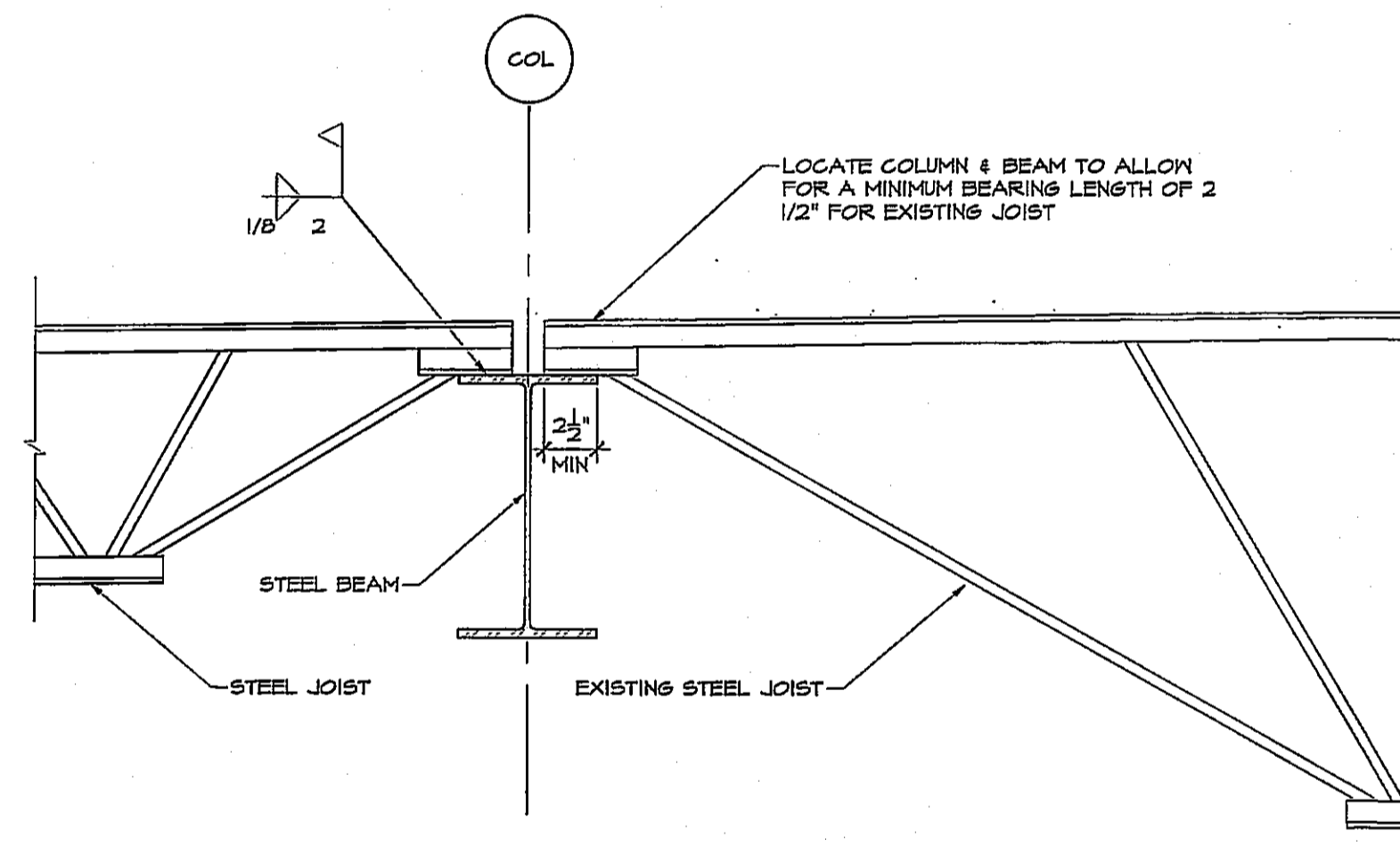
6 DETAIL @ MONUMENT
S1.2 SCALE: 1/2" = 1'-0"



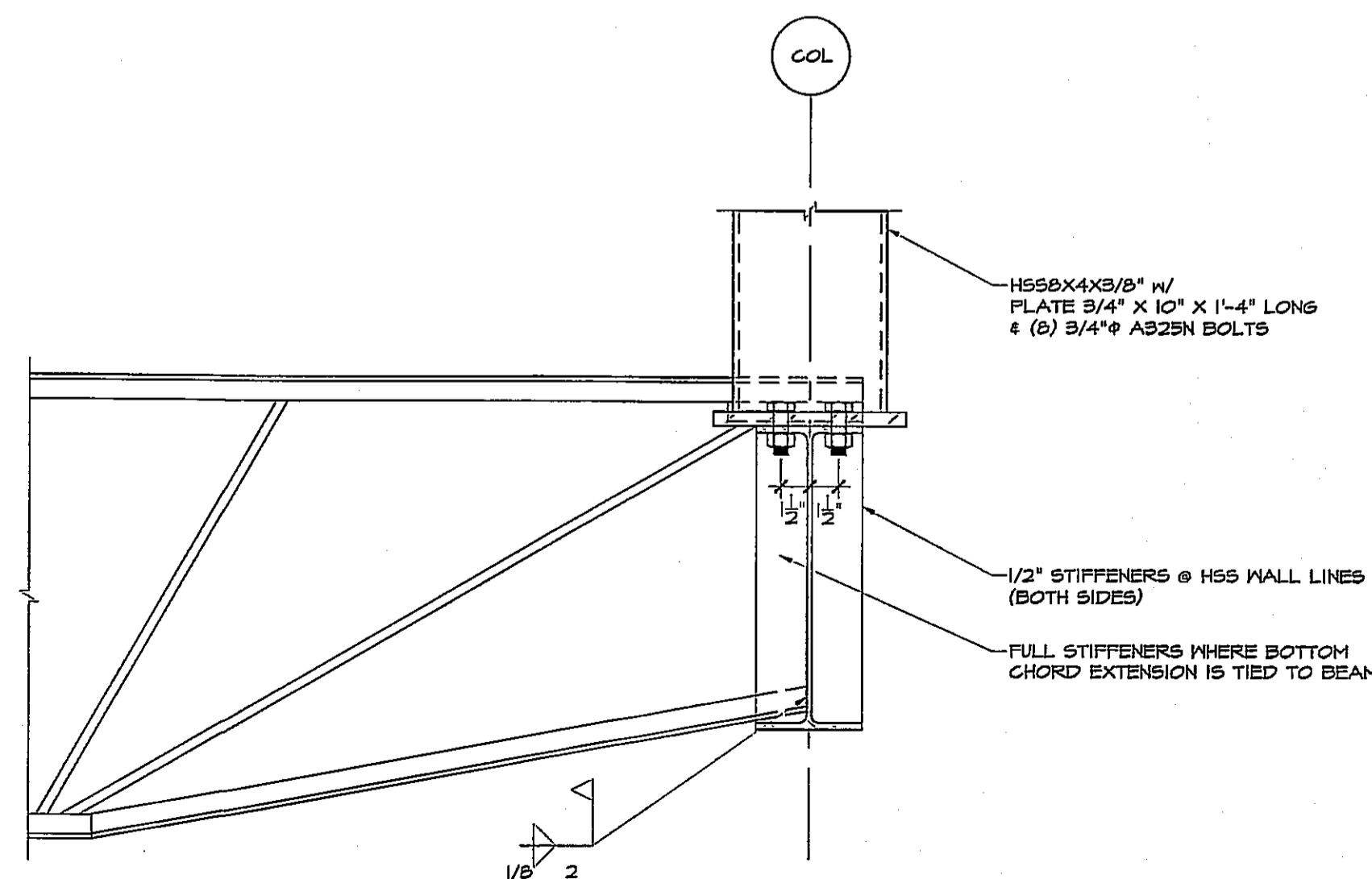
6 DETAIL @ CHANNEL CONNECTION
S1.2 SCALE: 1/2" = 1'-0"



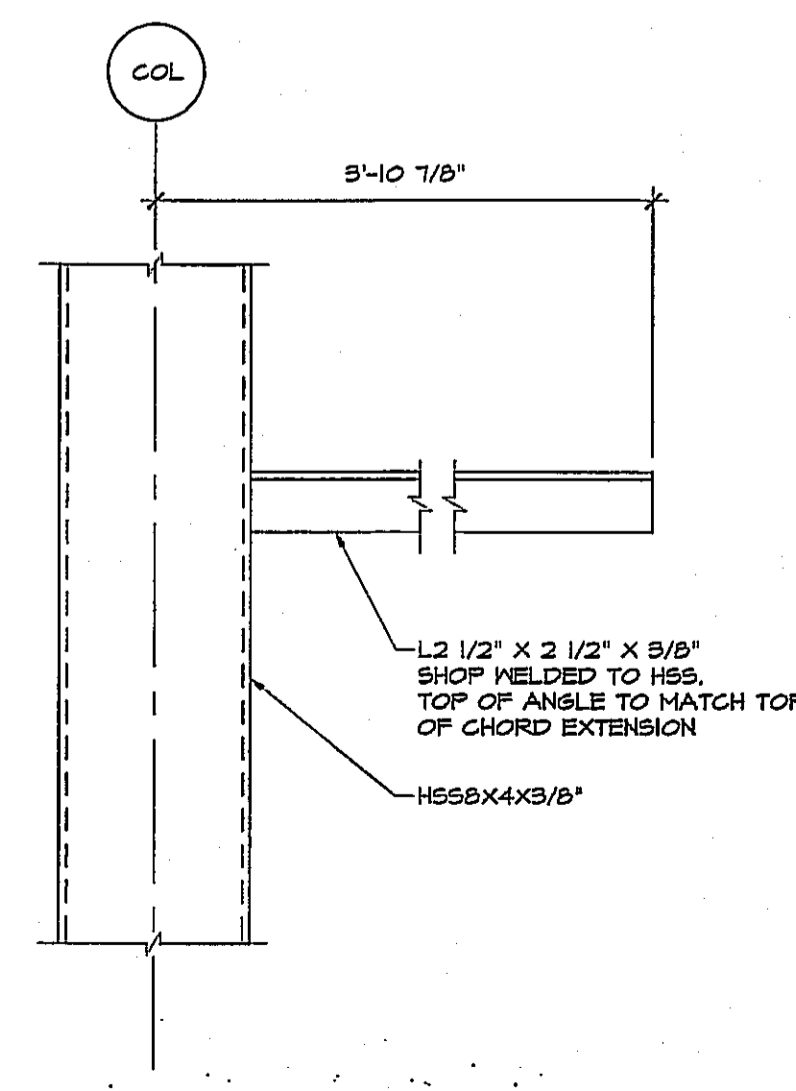
5 SECTION @ VERTICAL CROSS BRIDGING
S1.2 SCALE: 1/2" = 1'-0"



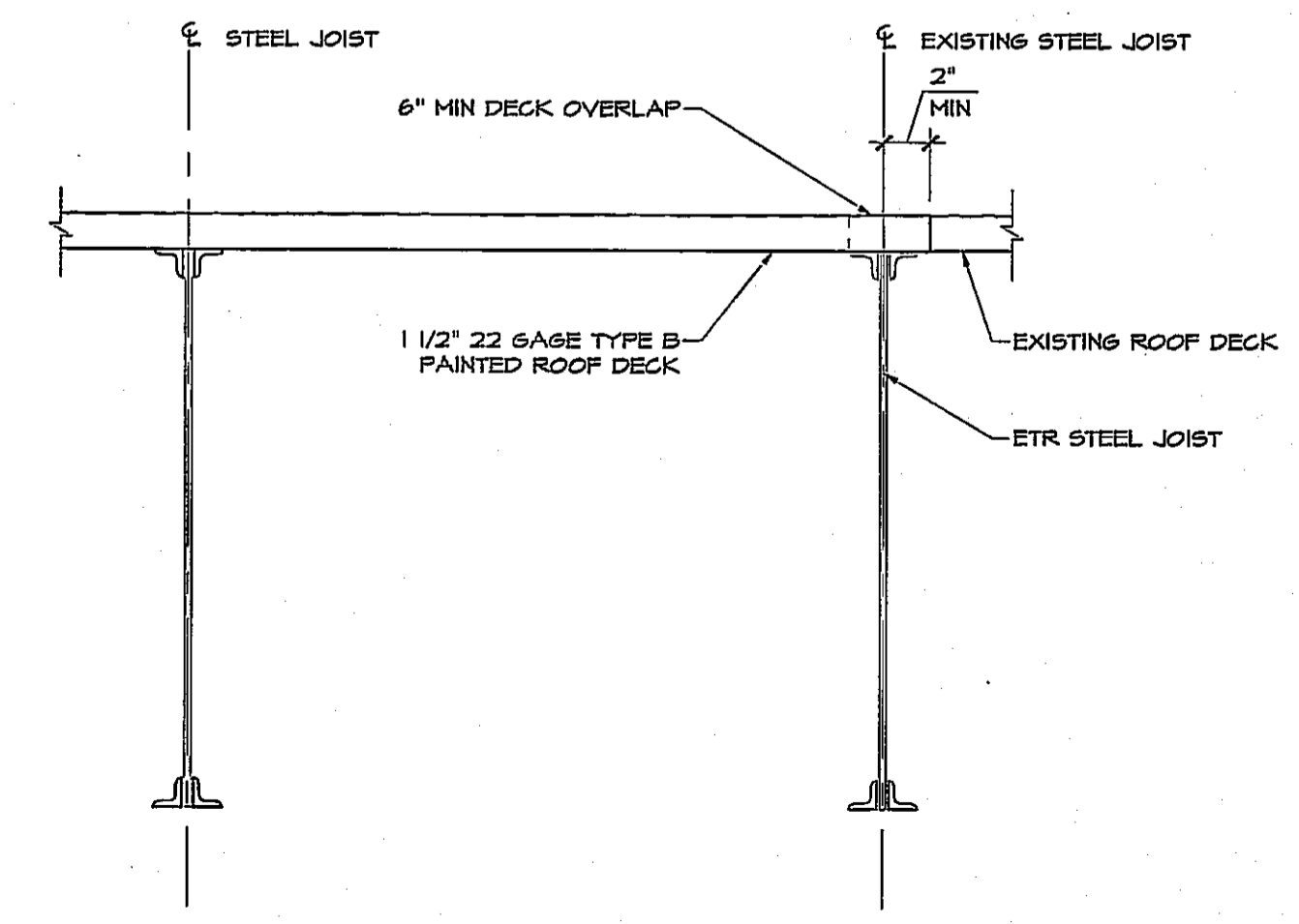
4 DETAIL @ EXISTING TO NEW STEEL JOIST
S1.2 SCALE: 1/2" = 1'-0"



3 DETAIL @ HSS STUB UP COLUMN
S1.2 SCALE: 1/2" = 1'-0"



2 DETAIL @ HSS COLUMN
S1.2 SCALE: 1/2" = 1'-0"



1 ROOF DECK DETAIL
S1.2 SCALE: 1/2" = 1'-0"

HANGER NOTES & SPECIFICATIONS

APPROVED JOIST HANGER SUPPLIERS

SIMPSON STRONG-TIE COMPANY, INC.
HOME OFFICE
5456 N. LAS FORTAS BLVD.
PLEASANTON, CALIFORNIA 94588
PHONE: (925) 560-1000
FAX: (925) 847-1511
WWW.STRONGTIE.COM

UNITED STEEL PRODUCTS COMPANY
(USP STRUCTURAL CONNECTORS)
BURNSVILLE, MN
PHONE: (800) 328-5434
FAX: (952) 848-8688
WWW.USPCONNECTORS.COM

MINIMUM METAL THICKNESS
HANGERS TO BE COMPRISED OF NO LIGHTER THAN 18 GAUGE STEEL. LIGHTER GAUGE HANGERS LACK SUFFICIENT STRENGTH AND RIGIDITY TO PROVIDE ADEQUATE SUPPORT, INCLUDING LATERAL STABILITY.

MINIMUM NAIL PENETRATION
SUFFICIENT NAIL PENETRATION MAXIMIZES THE CAPACITY OF THE CONNECTOR. THE FOLLOWING PENETRATION MUST BE ACHIEVED:

FACE MOUNT HANGERS
10D X 1" - 1-1/2" MIN.
10D (3") COMMON - 1-3/4" MIN.
16D (3") COMMON - 2" MIN.

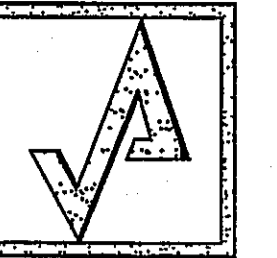
TOP FLANGE HANGERS
10D X 1" - 1-1/2" MIN.
10D (3") COMMON - 3" MIN.
16D (3") COMMON - 3-1/2" MIN.

THE REQUIRED PENETRATION FOR FACE MOUNT HANGERS IS 12 TIMES THE NAIL DIAMETER. REFER TO SUPPLIER'S LITERATURE FOR THE APPROPRIATE ADJUSTMENTS FOR REDUCED PENETRATION. CONTACT THE SUPPLIER FOR APPROPRIATE TOP FLANGE REDUCTIONS SINCE TOP FLANGE HANGER CAPACITIES ARE OFTEN BASED ON TESTING RATHER THAN CALCULATION. NOTE THAT THE SUPPORT WIDTH MUST BE LARGE ENOUGH TO ACCOMMODATE TOP FLANGE HANGERS.

LATERAL STABILITY
UNLESS SUPPLEMENTAL RESTRAINT IS PROVIDED, THE DEPTH OF THE HANGER SHOULD BE AT LEAST 60% OF THE SUPPORTED JOIST DEPTH.

GENERAL STRUCTURAL STEEL NOTES

- STRUCTURAL STEEL IS DESIGNED IN ACCORDANCE WITH AND WORK SHALL CONFORM TO THE LATEST EDITIONS OF "SPECIFICATIONS FOR DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS" (AISC), "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" (AISC), AND "STRUCTURAL WELDING CODE-STEEL (AWS) STRUCTURAL STEEL SHALL BE NEW STEEL CONFORMING TO THE FOLLOWING:
IN SECTIONS, ASTM A572 GRADE 50
HSS SECTION, ASTM A500 GRADE B
ALL OTHER STEEL ASTM A36
- ALL EXTERIOR EXPOSED STRUCTURAL STEEL, COMPONENTS AND FITTINGS SHALL BE HOT DIPPED GALVANIZED (2 OZ/ SQ. FT.) AFTER FABRICATION IN COMPLIANCE WITH ASTM-A123, A153, A584 OR A585 AS APPLICABLE. GALVANIZER SHALL FURNISH TO ENGINEER A NOTARIZED CERTIFICATE OF COMPLIANCE WITH THESE SPECIFICATIONS.
- STEEL FRAMING SHALL BE TRUED AND FLUMB BEFORE CONNECTIONS ARE PERMANENTLY BOLTED OR WELDED.
- CONNECTIONS:
 - BEAM CONNECTIONS SHALL BE STANDARD FRAMED BEAM CONNECTIONS.
 - CONNECTIONS SHALL BE BOLTED OR WELDED OR BOTH, AND FABRICATOR SHALL SUBMIT PROPOSED CONNECTION DETAILS FOR APPROVAL PRIOR TO FABRICATION.
 - BOLTED CONNECTIONS SHALL BE MADE WITH 3/4" DIAMETER ASTM A325 HIGH-STRENGTH STEEL BOLTS AND LOAD INDICATING DEVICES.
 - WELDED CONNECTIONS SHALL BE MADE BY A CERTIFIED WELDER IN ACCORDANCE WITH AWS D.11, USING CLASS E70 SERIES ELECTRODES, WELDS SHALL DEVELOP THE FULL STRENGTH OF THE MATERIALS BEING WELDED.
 - ANCHOR BOLTS SHALL BE HOT-DIPPED GALV. A-325 BOLTS.
- ANCHOR BOLTS AND BEARING PLATES SHALL BE LOCATED BY TEMPLATES OR SIMILAR METHOD. PLATES SHALL BE SET IN FULL BEDS OF NON-SHRINK GROUT. BOTTOM OF BASE PLATES SHALL BE SET APPROXIMATELY 3/4" ABOVE TOP OF BEARING. RESULTING SPACE SHALL BE FILLED WITH DRY PACKED NON-SHRINK GROUT.
- TEMPORARY ERECTION BRACING AND SUPPORTS SHALL BE PROVIDED TO HOLD STRUCTURAL STEEL FRAMING SECURELY IN POSITION. SUCH TEMPORARY BRACING AND SUPPORTS SHALL NOT BE REMOVED UNTIL PERMANENT BRACING HAS BEEN INSTALLED AND FLOOR SLABS HAVE ATTAINED 75% OF SPECIFIED CONCRETE STRENGTH.
- MINIMUM 3/8" WELDED WEB STIFFENERS SHALL BE PROVIDED ON EACH SIDE OF BEAM WEB AT ALL LOAD CONCENTRATIONS ON CONTINUOUS SUPPORTING BEAM MEMBERS OVER COLUMNS, AND WHERE SHOWN ON THE DRAWINGS.
- *** WELDING SHALL BE INSPECTED IN THE FIELD BY QUALIFIED WELDING INSPECTORS UNDER THE SUPERVISION OF AN APPROVED TESTING AGENCY.
- FIELD CUTTING OR ANY OTHER FIELD MODIFICATIONS OF STRUCTURAL STEEL SHALL NOT BE MADE WITHOUT APPROVAL FROM ENGINEER FOR EACH SPECIFIC CASE.
- SUBMIT SHOP DRAWINGS FOR REVIEW AND APPROVAL PRIOR TO CONSTRUCTION.



AHARONIAN & ASSOCIATES INC. ARCHITECTS

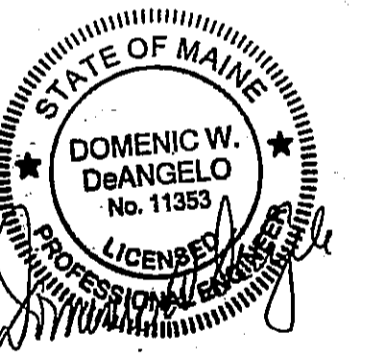
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REVISIONS

NUMBER	REMARKS	DATE



FOR CONSTRUCTION

PROJECT TITLE

Cumberland FARMS
100 Crossing Blvd, Framingham, Massachusetts
STORE # 5604
V1079

801 WASHINGTON AVE
PORTLAND, ME
Cumberland County

DRAWING TITLE

ROOF FRAMING DETAILS & NOTES

DATE JULY 19, 2012	PROJ NO 11112
DRAWN BY JD	CHECKED BY AZ

DRAWING NUMBER

S2.2