

MSE CALCULATIONS SHOP DRAWING SUBMITTAL  
FOR  
BAYVIEW CONDOMINIUMS  
MSE RETAINING WALL  
PORTLAND, ME

SUBMITTED ON June 6, 2014  
BY

A. H. GROVER, INC.

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Cumberland, ME 04021

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PREPARED BY:

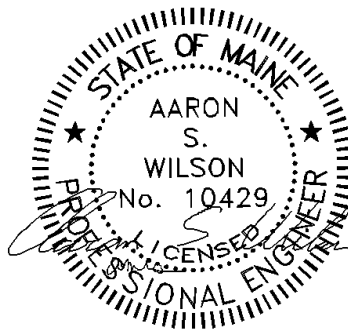
ASSOCIATED DESIGN PARTNERS INC.

80 LEIGHTON ROAD

FALMOUTH, MAINE 04105

(207) 878-1751 FAX (207) 878-1788

ADP# 14205



# CALCULATIONS

## **Bayview Condominiums ANCHORWALL MSE RETAINING WALL PORTLAND, MAINE**

- THESE CALCULATIONS ARE PROVIDED FOR AN "ANCHOR WALL SYSTEMS" MECHANICALLY STABILIZED EARTH RETAINING STRUCTURE FOR THE ABOVE REFERENCED PROJECT. THE ATTACHED ANALYSIS IS BASED UPON THE SITE CIVIL DRAWINGS DATED 6/10/13. SHOULD ANY VARIATIONS OR DISCREPANCIES IN THOSE DOCUMENTS BE IN EXISTENCE, OUR CALCULATIONS AND SUBMITTAL MAY CHANGE ACCORDINGLY. ASSOCIATED DESIGN PARTNERS, INC. MAKES NO CERTIFICATION TO THE ACCURACY OR COMPLETENESS OF CONTRACT DOCUMENTS PROVIDED BY OTHERS. PRESENTATION OF THIS SUBMITTAL IN NO WAY IMPLIES ACCEPTABILITY OR ADEQUACY OF THE COMPONENTS OR SYSTEMS USED WITHIN THE OVERALL STRUCTURE FOR PERFORMANCE OR CAPACITY OTHER THAN THAT, WHICH IS SPECIFICALLY REPRESENTED ON THE DRAWINGS AND CALCULATIONS HEREIN.
- THE WALL HAS BEEN DESIGNED FOR SUPPORT OF A UNIFORMLY DISTRIBUTED LIVE LOAD SURCHARGE OF 125PSF AT AREAS ABOVE THE WALL.
- THE ATTACHED COMPUTER CALCULATIONS ARE BASED UPON THE MODIFIED COULOMB METHOD WITH SEISMIC PROVISIONS CONSIDERED. THIS ANALYSIS ASSUMES  $A = 0.08$
- THIS ANALYSIS PROVIDES REASONABLE RESULTS FOR THE WALL DESIGN SCENARIOS AND GEOMETRY DEPICTED ON THE CIVIL SITE CONTRACT DRAWINGS. WALL DISPLACEMENT ANALYSIS, GLOBAL INSTABILITY, AND SOIL LIQUIFACTION ASSOCIATED WITH SEISMIC ANALYSIS ARE BEYOND THE SCOPE OF THIS ANALYSIS.



**ANCHOR**<sup>®</sup>  
BUILD SOMETHING BEAUTIFUL

## Project Information

<b>Client</b>	AH Grover Inc	<b>Number</b>	1
<b>Name</b>	Bayview MSE	<b>Designer</b>	ASW
<b>Site</b>	Ocean Ave, Portland ME	<b>Modified</b>	6/6/2014
<b>Revision</b>	1	<b>Created</b>	6/6/2014
<b>Standard</b>	National Concrete Masonry Association 3rd Edition		

### Selected Wall Unit

Diamond Pro®

Licensors: Anchor Wall Systems, Inc.

### Selected Reinforcement Types

SF35 - Synteen Geogrid SF35

Supplier: Synteen Technical Fabrics, Inc.

## Project Design Inputs

### Design Standard National Concrete Masonry Association 3rd Edition

#### Reinforced Analysis

Category	Factor	Variable	Default Minimum	Used Minimum
	Max. Reinforcement Separation	Rs	2.6667	2.0000
	Max. multiple of Wu at bottom	RsBottom	2.50	2.50
	Max. multiple of Wu at top	RsTop	2.50	2.50
	Min. Anchorage Length	La	0.9999	0.9999
	Min. L/H Ratio	L/H Ratio	0.6000	0.6000
	Min. Reinforcement Length	L	3.9993	4.0000
External	Base Sliding	FSsl	1.50	1.50
External	Bearing Capacity	FSbc	2.00	2.00
External	Crest Toppling	FSct	1.50	1.50
External	Overturning	FSot	2.00	2.00
Internal	Global Stability	FSGlobal	1.30	1.30
Internal	Internal Compound Stability	FSics	1.30	1.30
Internal	Internal Sliding	FSsl	1.50	1.50
Internal	Pullout	FSpO	1.50	1.50
Internal	Tensile Overstress	FSto	1.50	1.50
Local	Connection Strength	FScs	1.50	1.50
Local	Facing Shear	FSsc	1.50	1.50

#### Seismic

External	Base Sliding	FSslseismic	1.10	1.10
External	Bearing Capacity	FSbcseismic	1.10	1.10
External	Crest Toppling	FSctseismic	1.10	1.10
External	Overturning	FSotseismic	1.10	1.10
Internal	Global Stability	FSGlobalSeismic	1.30	1.30
Internal	Internal Compound Stability	FSicsseismic	1.10	1.10
Internal	Internal Sliding	FSslseismic	1.10	1.10
Internal	Pullout	FSpOseismic	1.10	1.10
Internal	Tensile Overstress	FStoseismic	1.10	1.10
Local	Connection Strength	FScsseismic	1.10	1.10
Local	Facing Shear	FSscseismic	1.10	1.10

#### Conventional Analysis

Category	Factor	Variable	Default Minimum	Used Minimum
External	Base Sliding	FSsl	1.50	1.50
External	Bearing Capacity	FSbc	2.00	2.00
External	Overturning	FSot	2.00	2.00
Internal	Global Stability	FSGlobal	1.50	1.50
Internal	Internal Compound Stability	FSics	1.50	1.50
Internal	Internal Sliding	FSsl	1.50	1.50
Internal	Shear Capacity	FSsc	1.50	1.50

#### Seismic

External	Base Sliding	FSslseismic	1.10	1.10
External	Bearing Capacity	FSbcseismic	1.10	1.10
External	Overturning	FSotseismic	1.10	1.10
Internal	Global Stability	FSGlobalSeismic	1.50	1.50
Internal	Internal Compound Stability	FSicsseismic	1.10	1.10
Internal	Internal Sliding	FSslseismic	1.10	1.10
Internal	Shear Capacity	FSscseismic	1.10	1.10

#### Common Criteria

Category	Factor	Variable	Default Minimum	Used Minimum
	Minimum Embedment	MinHemb	6.0000	6.0000
	Minimum Embedment	MinHemb	6.0000	6.0000
	Minimum Embedment %	Hemb	0.1000	0.1000
	Minimum Embedment %	Hemb	0.1000	0.1000

Category	Factor	Variable	Default Minimum	Used Minimum
	Minimum Embedment over 18.4° toe	Hemb2	0.1428	0.1428
	Minimum Embedment over 18.4° toe	Hemb2	0.1428	0.1428

**Seismic****Anchorplex Analysis**

Category	Factor	Variable	Default Minimum	Used Minimum
	Min. Anchorplex Depth	Anchorplex Depth	1.0000	1.0000
	Min. Anchorplex L/H Ratio	Anchorplex L/H Ratio	0.3000	0.3000
External	Base Sliding	FSsl	1.50	1.50
External	Bearing Capacity	FSbc	2.00	2.00
External	Overturning	FSot	1.50	1.50
Internal	Internal Compound Stability	FSics	1.50	1.50

**Seismic**

External	Base Sliding	FSslseismic	1.10	1.10
External	Bearing Capacity	FSbcseismic	1.50	1.50
External	Overturning	FSotseismic	1.10	1.10
Internal	Internal Compound Stability	FSicsseismic	1.10	1.10

**Selected Wall Unit****Diamond Pro®**

Licensor: Anchor Wall Systems, Inc.

Unit Height	Hu	0.67 ft
Coping Height	Hcu	0.33 ft
Face Width	Lu	1.50 ft
Unit Depth	Wu	1.00 ft
Unit Weight	Xu	129.91 lb/ft <sup>3</sup>
Center of Gravity	Gu	0.50 ft
Setback	u	0.08 ft
Batter		7.13 deg
Initial Shear Capacity	au	1,180.97 lb/ft
Apparent Shear Angle	u	45.00 deg
Maximum Shear Capacity	Vu(max)	2,660.97 lb/ft

**Selected Reinforcement Types****SF35 - Synteen Geogrid SF35**

Supplier: Synteen Technical Fabrics, Inc.

Coeff. of Direct Sliding	Cds	0.80
Coeff. of Interaction	Ci	0.80
Ultimate Tensile Strength	Tult	3,436.09 lb/ft
Reduction Factor - Creep	RFcr	1.54
Reduction Factor - Install. Damage	RFid	1.08
Reduction Factor - Durability	RFd	1.10
Long-term Allowable Design Strength	Ta	2,892.33 lb/ft
Long-term Allowable Design Strength (creep)	Ta(cr)	1,878.14 lb/ft

**Connection Properties for Wall Unit Diamond Pro®**

Minimum Connection Capacity	acs	1,184.38 lb/ft
Normal Load (IP-1)		1,944.62 lb/ft
Connection Capacity		1,667.53 lb/ft
Normal Load (IP-2)		1,944.62 lb/ft
Maximum Connection Capacity	Sc(max)	1,667.53 lb/ft
Initial Shear Capacity	au	1,181.38 lb/ft
Apparent Shear Angle	u	45.00 deg
Maximum Shear Capacity	Vu(max)	2,660.84 lb/ft

**Selected Soil Types**

Soil Zone	Soil Type	Friction Angle	In Situ	Friction Factor $\mu$	Cohesion $C_f$
			Density [lb/ft <sup>3</sup> ]		
Infill (i)	GW	32°	130.00	n/a	n/a
Retained (r)	GM	28°	125.00	n/a	n/a
Foundation (f)	SM	28°	120.00	0.60	500.00
Base (b)	GW	39°	140.02	0.70	n/a
Drainage (d)	GW	32°	130.00	0.70	n/a

### Soil Glossary

<b>CH:</b>	Inorganic clays, high plasticity
<b>CL:</b>	Inorganic clays, low to medium plasticity, gravelly, sandy, silty, lean clays
<b>GC:</b>	Clayey gravels, poorly graded gravel-sand-clay mixtures
<b>GM:</b>	Silty gravels, poorly graded gravel-sand-silt mixtures
<b>GP:</b>	Poorly-graded gravels, gravel-sand. Little or no fines.
<b>GW:</b>	Well-graded gravels, gravel-sand. Little or no fines.
<b>MH:</b>	Inorganic clayey silts, elastic silts
<b>ML:</b>	Inorganic silts, very fine sands, silty or clayey, slight plasticity
<b>SC:</b>	Clayey sands, poorly graded sand-clay mixtures
<b>SM:</b>	Silty sands, poorly graded sand-silt mixtures
<b>SP:</b>	Poorly-graded sands, gravelly sands. Little or no fines.
<b>SW:</b>	Well-graded sands, gravelly sands. Little or no fines.

## Panel Geometry

### Reinforcement Details

Panel	Course	Length [ft]	Area [ft <sup>2</sup> ]	Reinforcement
1	1	5.50	57.75	SF35 - Synteen Geogrid SF35
	4	5.50	57.75	SF35 - Synteen Geogrid SF35
	7	5.50	57.75	SF35 - Synteen Geogrid SF35

## Analysis Summary

### Lowest Values - Reinforced

#### Static Analysis

Test	Description	Panel	Layer/ Course	Minimum Requirement	Result	Status
Rs	Max. Reinforcement Separation	1		2.0000	2.0000	Pass
RsBottom	Max. multiple of Wu at bottom	1		2.50	0.67	Pass
RsTop	Max. multiple of Wu at top	1		2.50	1.33	Pass
La	Min. Anchorage Length	1		0.9999	1.5242	Pass
L/H Ratio	Min. L/H Ratio	1		0.6000	0.9167	Pass
L	Min. Reinforcement Length	1		4.0000	5.5000	Pass
Fssl	Base Sliding	1		1.50	3.30	Pass
FSbc	Bearing Capacity	1		2.00	24.03	Pass
FSct	Crest Toppling	1	7	1.50	7.42	Pass
FSot	Overturning	1		2.00	7.09	Pass
Fssl	Internal Sliding	1	1	1.50	5.03	Pass
Fspo	Pullout	1	3	1.50	5.06	Pass
Fsto	Tensile Overstress	1	1	1.50	6.81	Pass
Fscs	Connection Strength	1	1	1.50	4.92	Pass

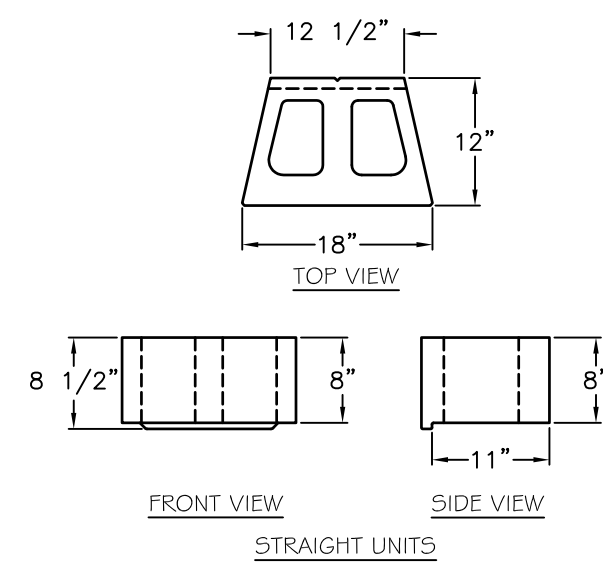
#### Seismic Analysis

Test	Description	Panel	Layer/ Course	Minimum Requirement	Result	Status
Fsslseismic	Base Sliding	1		1.10	3.01	Pass
FSbcseismic	Bearing Capacity	1		1.10	23.46	Pass
FSctseismic	Crest Toppling	1	7	1.10	3.55	Pass
FSotseismic	Overturning	1		1.10	6.19	Pass
Fsslseismic	Internal Sliding	1	1	1.10	4.70	Pass
Fsposeismic	Pullout	1	3	1.10	2.23	Pass
Fstoseismic	Tensile Overstress	1	1	1.10	5.74	Pass
Fscsseismic	Connection Strength	1	1	1.10	3.83	Pass

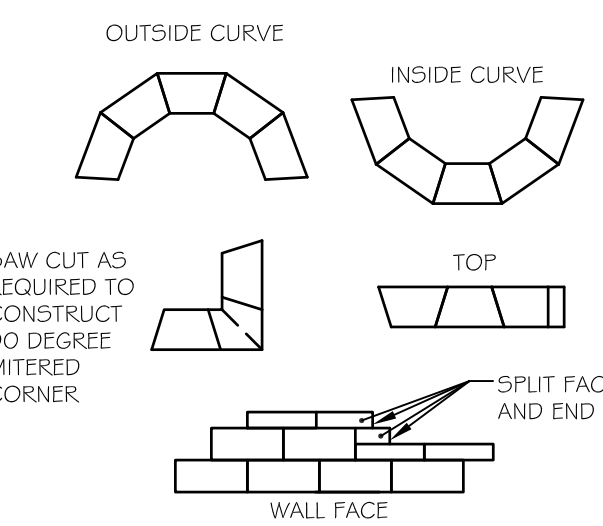
### Below Standard Values

None



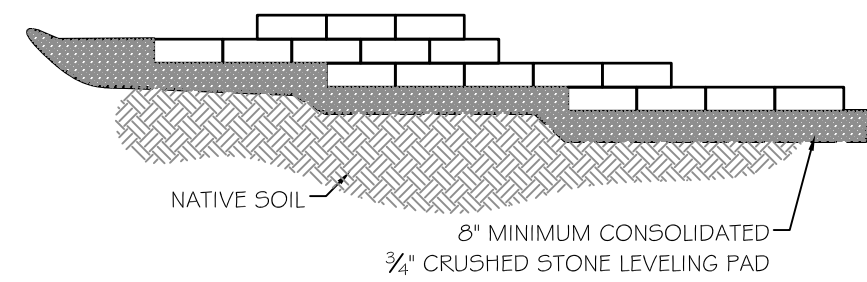


**ANCHOR DIAMOND PRO™  
3-WAY BLOCK VIEWS**

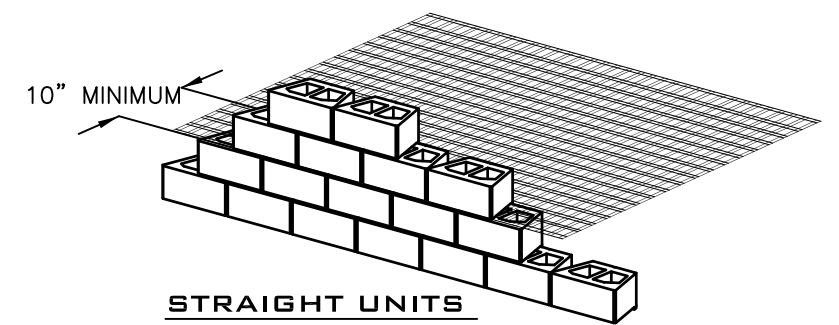


1. ALWAYS START CAPPING WALL FROM THE LOWEST ELEVATION.
2. LAYOUT CAPS PRIOR TO USING ADHESIVE.
3. CUT CAPS TO FIT. VARIOUS COMBINATIONS OF LONG AND SHORT CAP FACES WILL BE NECESSARY FOR RADII GREATER THAN THE MINIMUM.
4. ALTERNATE SHORT AND LONG CAP FACES EVERY OTHER CAP TO ACHIEVE A STRAIGHT ROW OF CAPS.
5. USE EXTERIOR-GRADE ELASTOMERIC POLYMER CONSTRUCTION ADHESIVE TO SECURE CAPS.

**ANCHOR DIAMOND PRO™  
3\"/>**



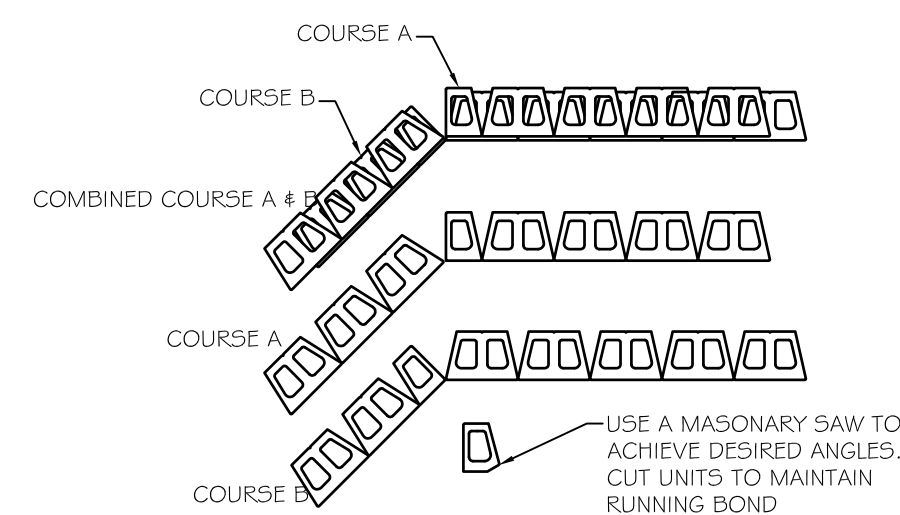
**ANCHOR DIAMOND PRO™  
TYPICAL STEP-UP DETAIL**



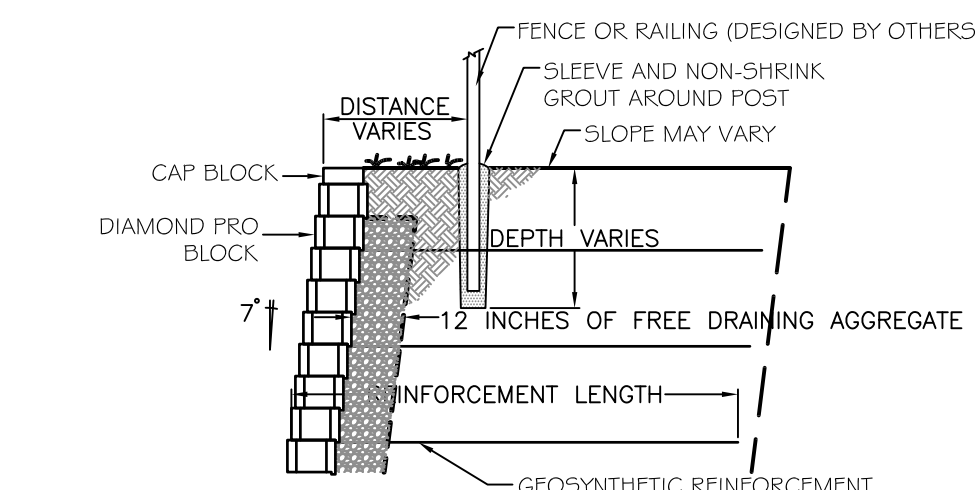
**STRAIGHT UNITS**

EXTEND GEOSYNTHETIC REINFORCEMENT TO WITHIN 2\"/>

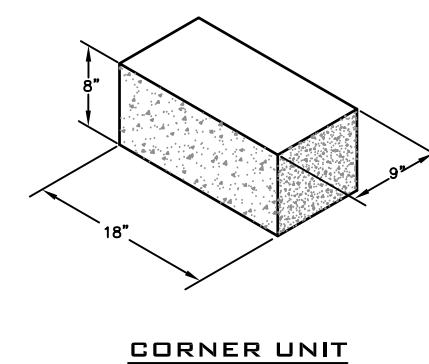
**ANCHOR DIAMOND PRO™  
REINFORCEMENT CONNECTION DETAIL**



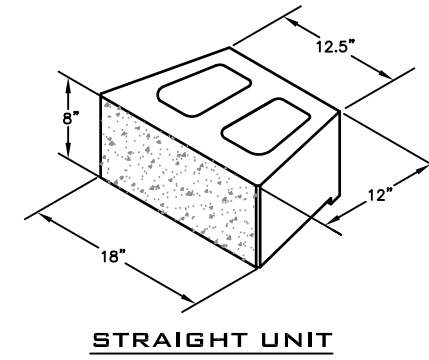
**ANCHOR DIAMOND PRO™  
45\"/>**



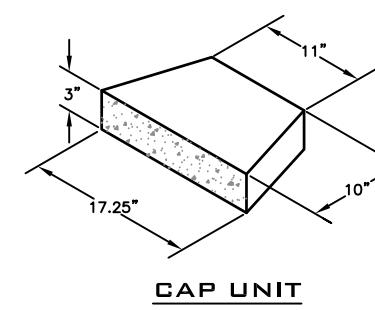
**ANCHOR DIAMOND PRO™  
FENCE BEHIND WALL**



**CORNER UNIT**

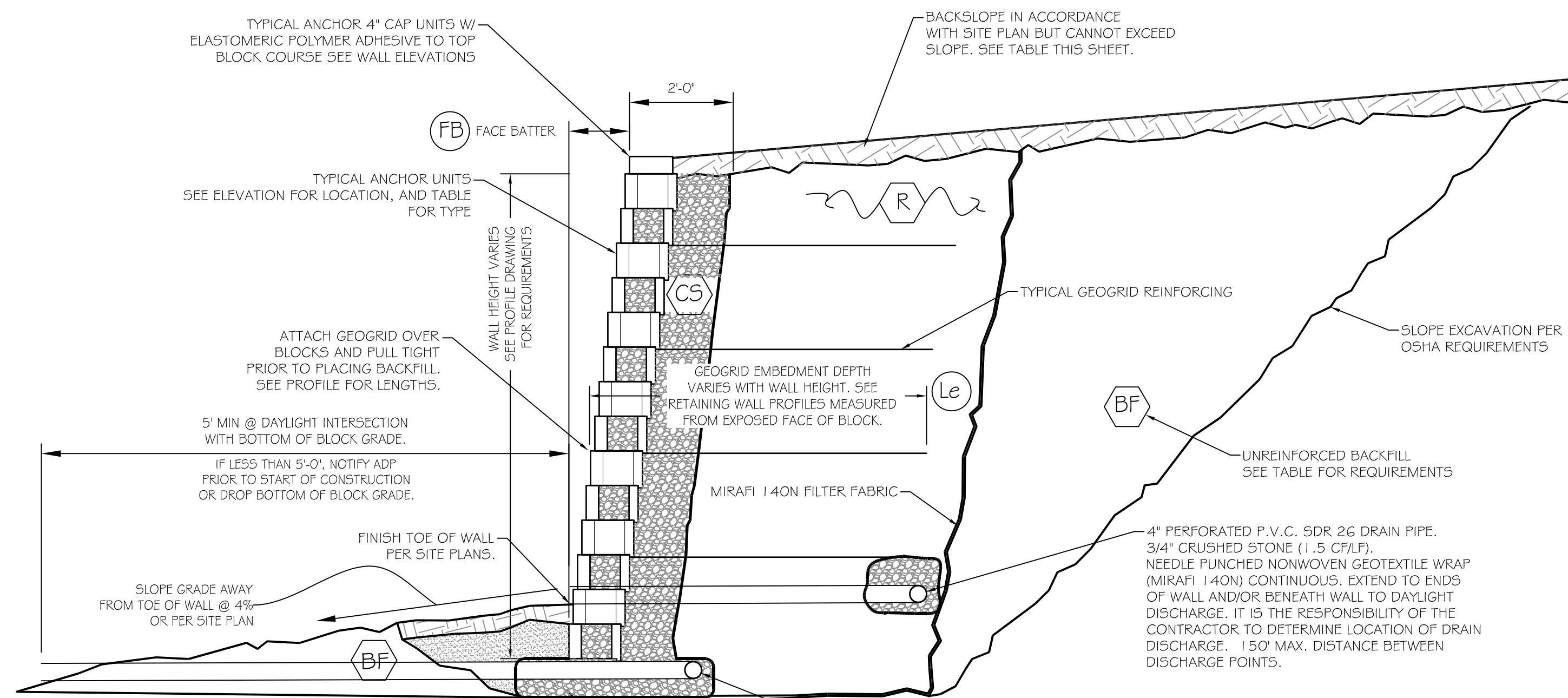


**STRAIGHT UNIT**



**CAP UNIT**

**ANCHOR DIAMOND PRO™  
150 BLOCK VIEWS**



**NOTES:**

1. CONSTRUCT WALL AND FILL MATERIALS ON ENGINEER APPROVED, UNDISTURBED NATIVE SOILS. ANY REQUIRED BACKFILL DUE TO UNSUITABLE MATERIALS OR OVER EXCAVATIONS WILL BE REPLACED WITH COMPACTED GRAVEL TO 95% MODIFIED PROCTOR DENSITY.
2. PLACE BACKFILL IN MAX. 8\"/>
3. CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DAMAGE INCURRED TO THE WALLS FROM ANY ONSITE BLASTING CONDUCTED SUBSEQUENT TO START OF WORK ON WALLS.

**TYPICAL ANCHOR WALL SECTION**

SCALE: NTS

**1. SHOP DRAWING ACTION STAMP:**

1.1. THIS IS AN ENGINEER SHOP DRAWING DESIGN BASED UPON INFORMATION PROVIDED BY OTHERS. THE WALL DESIGN DEPICTED HEREIN SHOULD BE REVIEWED BY THE SITE DESIGNER OR OTHERWISE RESPONSIBLE ENTITY TO VERIFY COMPLIANCE WITH THE GENERAL INTENT OF THE SITE DESIGN WITH RESPECT TO GRADING, WALL ALIGNMENT AND GEOMETRY, WALL STEPS, ETC.  
1.2. THIS DESIGN IS BASED UPON INFORMATION PROVIDED BY OTHERS. SHOULD VARIATIONS BE ENCOUNTERED THE CONTRACTOR, SITE DESIGNER OF RECORD, OR OTHER RESPONSIBLE ENTITY SHALL NOTIFY THE OWNER/ENGINEER AND ASSOCIATED DESIGN PARTNERS, INC. (ADP) TO MAKE APPROPRIATE ADJUSTMENTS.

**2. WALL DESIGN NOTES:**

2.1 THE WALL DESIGN(S) REPRESENTED HEREIN ARE BASED ON THE PROCEDURES DESCRIBED IN THE INDUSTRY STANDARD PUBLICATION NCMA TR127A "DESIGN MANUAL FOR SEGMENTAL RETAINING WALLS, 2ND ED".

2.2 IN ACCORDANCE WITH NCMA TR127A "DESIGN MANUAL FOR SEGMENTAL RETAINING WALLS, 2ND ED" SEC. 5.1.4, EXTERNAL GLOBAL STABILITY HAS NOT BEEN ADDRESSED AS PART OF THIS RETAINING WALL DESIGN.

2.3 THE WALL STABILITY ANALYSES IS BASED ON ANTICIPATED SOIL DESIGN VALUES AS REPRESENTED IN THE TABLE BELOW. THE PROJECT GEOTECHNICAL ENGINEER SHOULD REVIEW THE VALUES REPRESENTED HEREIN, AND NOTIFY ASSOCIATED DESIGN PARTNERS AND/OR THE SITE CONTRACTOR IF MODIFICATIONS TO THE SOIL DESIGN PARAMETERS IS NECESSARY. A GEOTECHNICAL REPORT HAS NOT BEEN PROVIDED AT THIS TIME.

**3. FILL SOIL COMPACTION:**

3.1. ALL GRANULAR SOIL FILL SHALL BE COMPACTED TO 95% OF THE MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D698 STANDARD PROCTOR.

3.2. STONE IN WALL BASE PAD AND IN FILL LOCATIONS TO BE CONSOLIDATED TO 100% OF DRY RODDED UNIT WEIGHT PER ASTM C-29. ROUNDED ROCK OR PEA STONE IS SPECIFICALLY NOT ALLOWED AT CRUSHED STONE FILL LOCATIONS.

**4. GENERAL:**

4.1. PLACE ANCHOR BLOCKS ON A 8\"/>

4.3. INSTALL GEOGRID REINFORCING FABRIC AT LOCATIONS AND ELEVATIONS SHOWN ON THE WALL ELEVATION DRAWINGS.

4.4. ROLL GEOGRID OUT WITH STRONG FIBER (MACHINE DIRECTION) DIRECTION PERPENDICULAR TO WALL FACE TO EMBEDMENT LENGTH (LE) AS SPECIFIED ON THE PROFILE ELEVATIONS. IMPORTANT: GRID MUST BE LAID SMOOTH, FREE OF WRINKLES, PULLED TIGHT AND STAKED PRIOR TO FILL PLACEMENT.

4.5. PLACE A MINIMUM OF 6\"/>

4.6. GENERAL SOIL COMPACTION GUIDELINES: SITE EXCAVATION CONTRACTOR IS RESPONSIBLE FOR THE METHODS AND RESULTS OF THE COMPACTION PROCESS. THE FOLLOWING IS A SUGGESTED METHOD OF INSTALLATION.

4.6.1. PLACE SOIL IN MAXIMUM 8\"/>

4.7. LAY SUCCESSIVE COURSES OF BLOCK AND LAYERS OF GEOGRID ACCORDING TO PLANS AND PROFILE ELEVATIONS.

4.8. NOTIFY ENGINEER IMMEDIATELY IF ACTUAL SITE GRADES/CONTOURS DIFFER BY MORE THAN 1\"/>

4.9. ALL SOIL TESTING TO FOLLOW CONTRACT SPECIFICATIONS. PROVIDE SEVE ANALYSIS, SHEAR TEST, AND COMPACTION TEST REPORTS TO ASSOCIATED DESIGN PARTNERS INC, AND THE OWNER.

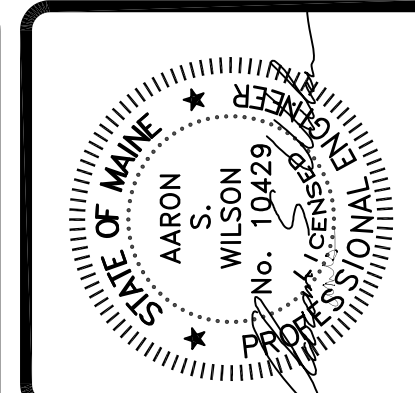
4.10 MAX STATIC BEARING PRESSURE = 840 PSF

**PROJECT SPECIFIC DESIGN VALUES**

DETAIL REFERENCE LETTER	DESIGN VALUES DESCRIPTION	E/P A/C	VALUE	UNITS	CONSTRUCTION COMPLIANCE CONFIRMED
NS	NATIVE SOIL SUBGRADE ALLOWABLE BEARING CAPACITY	E/A	2000	P.S.F.	
	NATIVE SOIL SUBGRADE INTERNAL FRICTION ANGLE	E/A	28°	DEGREES	
	NATIVE SOIL SUBGRADE UNIT WEIGHT TOTAL ±5 P.C.F.	E/A	120	P.C.F.	
	NATIVE SOIL SUBGRADE COHESION	E/A	500	P.C.F.	
	NATIVE RETAINED SOIL INTERNAL FRICTION ANGLE	E/A	28°	DEGREES	
	NATIVE RETAINED SOIL UNIT WEIGHT TOTAL ±5 P.C.F.	E/A	125	P.C.F.	
R	REINFORCED FILL MATERIAL INTERNAL FRICTION ANGLE	P	32	DEGREES	
	REINFORCED FILL MATERIAL UNIT WEIGHT TOTAL ±5 P.C.F.	P	130	P.C.F.	
	REINFORCED FILL MATERIAL MAXIMUM PARTICLE SIZE	P	3	INCHES	
BF	REINFORCED FILL MATERIAL MAXIMUM FINES PASSING 200 SIEVE	P	5	PERCENT	
	UNREINFORCED BACKFILL MATERIAL INTERNAL FRICTION ANGLE	P	28	DEGREES	
	UNREINFORCED BACKFILL UNIT WEIGHT	P	125	P.C.F.	
CS	UNREINFORCED BACKFILL MATERIAL MAXIMUM PARTIAL SIZE	P	6	INCHES	
	UNREINFORCED BACKFILL MATERIAL MAXIMUM FINES PASSING 200 SIEVE	P	20	PERCENT	
	CRUSHED STONE UNIT FILL MEDIAN PARTICLE SIZE	P	3/4	INCHES	
BS	TOP OF WALL MAXIMUM BACKSLOPE ANGLE	P	14	DEGREES	
FB	FACE BATTER	P	7.13	DEGREES	
K	SIZE / TYPE OF UNITS	P	STRAIGHT-FACE DIAMOND-PRO	BLOCK	

**LEGEND**

- E - EXISTING CONDITION OR VALUE
- P - PROPOSED CONDITION OR VALUE
- A - ASSUMED VALUE BASED UPON ANTICIPATED SITE CONDITIONS
- D - DERIVED VALUE GIVEN BY OTHERS BASED UPON EXPLORATION, TESTING, OR OBSERVATION
- PCF - POUNDS PER CUBIC FOOT
- PSF - POUNDS PER SQUARE FOOT



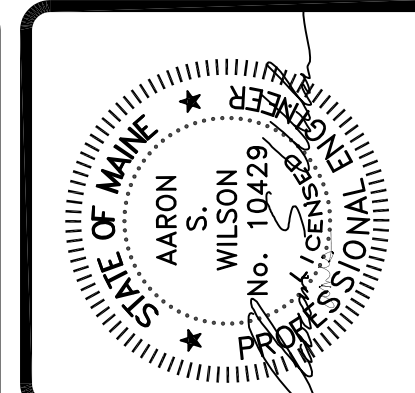
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PROJECT: **BAYVIEW CONDOMINIUMS MSE PORTLAND, ME**  
FOR: A.H. GROVER  
SHEET TITLE: **ANCHOR WALL SECTION, DESIGN VALUES CONSTRUCTION NOTES, & DETAILS**

NO.	BY	DATE	REVISIONS DESCRIPTION

DATE : 6-9-14  
SCALE : AS NOTED  
DESIGN BY: ASW  
DRAWN BY: RSC  
PROJECT NUMBER:  
**14205**  
SHEET NO:  
**RT1**



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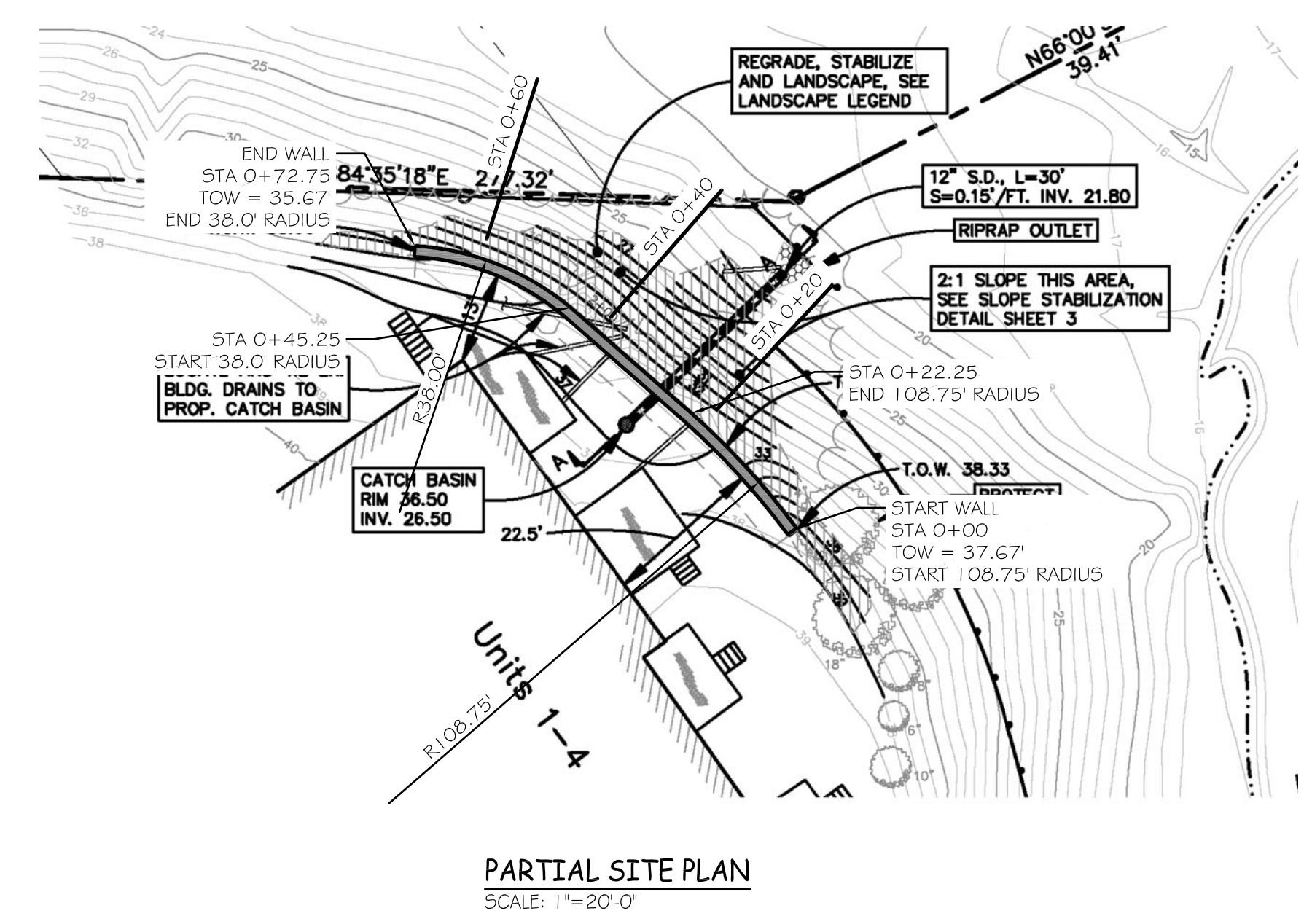
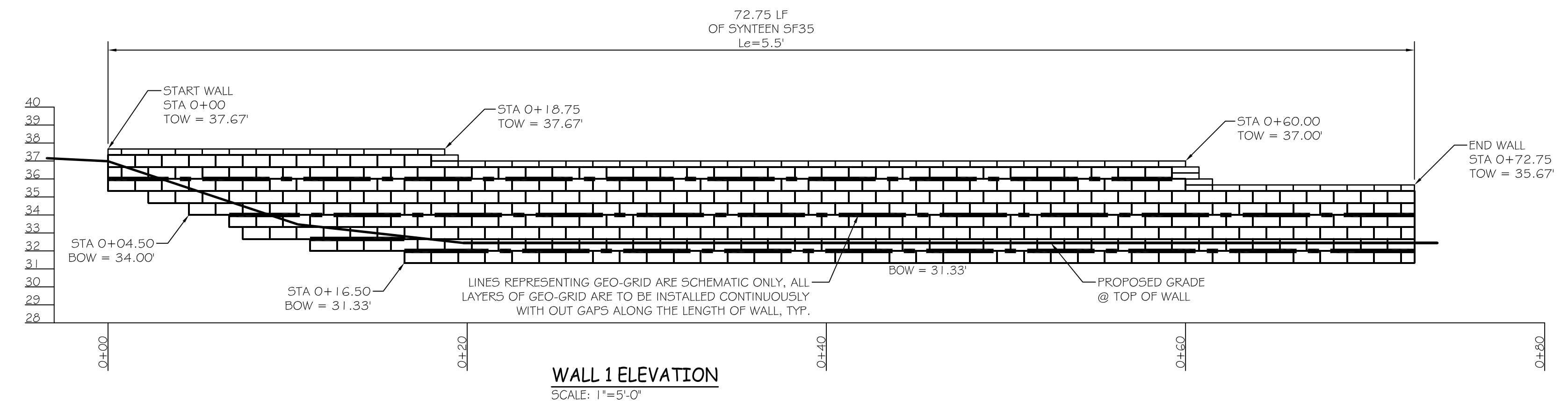
PROJECT: **BAYVIEW CONDOMINIUMS MSE**  
 PORTLAND, ME  
 FOR: A.H. GROVER  
 SHEET TITLE: **ANCHOR WALL ELEVATION AND PARTIAL SITE PLAN**

REVISIONS	DATE
No.	BY

DATE :	6-9-14
SCALE :	AS NOTED
DESIGN BY:	ASW
DRAWN BY:	RSC
FILE #:	11302-RT.DWG
PROJECT NUMBER:	<b>14205</b>
SHEET NO.:	<b>RT2</b>

**WALL TOTALS**

101.5 - 3" CAPS
1131.5 - ANCHOR DIAMOND PRO UNITS
504 - 5Y OF SYNTEEN SF35



SITE PLAN GENERATED FROM DIGITAL FILE. SCALE IS APPROXIMATE AND USER IS CAUTIONED AGAINST SCALING ITEMS FROM PLAN.

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