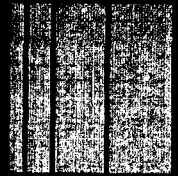


Foundation Investigation
Proposed Portland Assisted Living Facility
Phase II
Portland, Maine

for

Curry Brandaw Architects
2260 McGilchrist Street SE, Suite 100
Salem, OR 97302

February 9, 2004



February 9, 2004
96592

Mr. Garth Brandaw
Curry Brandaw Architects
2260 McGilchrist Street SE, Suite 100
Salem, OR 97302

Foundation Investigation
Proposed Portland Assisted Living Facility, Phase II, Portland, Maine

Dear Garth:

This report presents the results of our evaluation of the subsurface conditions and foundation requirements for the proposed Portland Assisted Living Facility - Phase II in Portland, Maine. As you may recall, Haley & Aldrich, Inc. performed a subsurface investigation for the facility. Results of their evaluation were presented in their report dated October 3, 1997. Kenneth Recker, our Geotechnical Engineering Manager, completed the original evaluation while at Haley & Aldrich.

In summary, it is our opinion that the buildings may be supported on spread and continuous footings bearing on undisturbed naturally-deposited soils, sound intact bedrock, or on compacted structural fill placed after removal of unsuitable soil. In addition, earth-supported slabs-on-grade may be used for the lowest floors. Specific recommendations regarding foundation design and construction considerations are presented below.

Introduction

The approximately 11.3-acre site of the Phase II construction is immediately south and east of the Phase I portion of the facility. The entire site is an approximately 20-acre parcel on the east side of Canco Road, between Torrey Road and Murray Street. Several bedrock exposures are located in the Phase II area. Ground surface elevations within the proposed building areas vary from approximately El. 70 to El. 80. Elevations in this report are in feet and referenced to National Geodetic Vertical Datum.

Phase II will consist of a two-story building containing 80 Assisted Living Suites and four one-story Cottage Suites in two buildings. In addition, there will be roadways and parking for 65 vehicles.

Proposed Construction

The buildings will be wood frame structures. The two-story building will have maximum column loads of 80 kips and wall loads of 3.5 kips per foot. The one-story buildings will have nominal loads. Ground floor elevation of the two-story building is El. 83.0 and the one-story buildings are El. 77.5.

Subsurface Explorations

On September 17, 1997, nineteen test pits, TP29 and TP31 through TP49, except TP44, were excavated by W. H. Lavigne (WHL) of Standish, Maine at locations shown on Sheet 1, Site and Subsurface Exploration Plan. The proposed locations of test pit TP44 was on a bedrock exposure and TP44 was not excavated. WHL excavated test pits to depths below ground surface varying from 0.5 foot to **6.0** feet. Haley & Aldrich prepared the logs included as Appendix A. Exploration results are summarized in Table I, Summary of Test Pits.

Sebago Technics determined the locations and ground surface elevations at test pits.

The test pits and related information depict subsurface conditions and water levels only at their specific locations at the time of excavation. Soil conditions at other locations may differ from conditions at these locations. Also, the passage of time may result in a change in groundwater conditions at exploration locations.

Subsurface liti

The test pits encountered four principal soil units overlying bedrock at the site: forest mat, topsoil, marine deposit, and glacial till. Encountered thickness and generalized descriptions of the soil units are presented below in order of increasing depth below ground surface.

Forest Mat - Forest mat consisting of leaves, pine needles, twigs and other organic debris was encountered at the surface of the test pits. Encountered thickness varies from 0.2 foot to 0.5 foot.

Topsoil - Topsoil consists of dark brown to rust-brown, loamy SILT; to loamy fine SAND with roots. Encountered thickness ranges from 0.3 foot to 1.4 feet.

Marine Deposit - The marine deposit consists of interbedded gray-brown to rust brown, silty fine SAND and sandy SILT; to gray-brown silty fine SAND; to clayey SILT; to gray-brown, mottled, silty CLAY. Encountered thickness of the marine deposit varies from 0.3 foot to 6.9 feet.

Glacial Till - Glacial till consisting of brown, silty, medium to fine SAND, with fractured rock fragments was encountered in several test pits. Encountered thickness ranges from 0.4 foot to 0.8 foot.

Bedrock was encountered in most of the test pits.

Groundwater was not encountered in the test pits. However, test pits were made over a short period of time and may not represent the stabilized groundwater level. It is likely that groundwater flows along the soil/bedrock interface, in the weathered portions of bedrock and upper bedrock, following the trend of the top of rock. In addition, water levels at the site will vary with precipitation, season, temperature and construction activity in the area. Therefore, groundwater levels during and following construction may differ from that observed in the explorations.

Recommendations for Foundation Design

Recommended Foundation Type and Design Criteria

The forest mat and topsoil are not considered suitable for support of the buildings. All forest mat and topsoil should be removed from within the limits of foundations. It is our opinion that the buildings may be supported on spread footings bearing on the undisturbed, naturally-deposited marine deposit and glacial till or bedrock or on compacted structural fill placed after removal of unsuitable soil (forest mat and topsoil).

We recommend that, for uniformity, the footings be proportioned for an allowable bearing stress, in pounds per square foot (psf), equal to 1,500 multiplied by the least lateral dimension of the footing in feet, up to a maximum of 4,500 psf. All footings should be at least 1.5 feet wide. In some areas, bedrock will likely be at or near the proposed bottom of footing. For footings bearing on bedrock, the maximum slope of the bedrock surface should not be steeper than **4** horizontal to 1 vertical. Steeper slopes should be benched or tapered to the above criteria.

Individual footings should be founded either on soil or bedrock. Continuous footings may span both soil and rock provided a transition from soil to rock is provided. Tapering the bedrock surface to a slope of **4** horizontal to 1 vertical and backfilling with structural fill to a minimum depth of 1 foot would be acceptable.

Exterior footings bearing on soil should be founded at least **4.5** feet below the lowest adjacent ground surface exposed to freezing. Interior footings, if required, should be founded a minimum of 1.5 feet below the ground floor slab. Exterior footings bearing on sound bedrock may be founded at least **2** feet below the lowest adjacent ground surface exposed to freezing.

Bedrock may be encountered above the proposed floor level and bearing level for foundations. Therefore, rock cuts may be required for foundation construction in some areas. Figure **3** of the Haley & Aldrich report may be used to estimate the required volume of rock excavation. The contours shown on Figure **3** are based on information from the explorations referenced therein. Actual top of rock between exploration locations will vary from the indicated contours.

Rock should be defined as "any material that is geologically classified as rock and requires drilling and blasting to excavate." Boulders and cobbles should not be classified as bedrock. Provisions should be made in the contract plans and payment items for adjusting bearing levels in the field to accommodate actual bedrock surface grades.

Compacted structural fill supporting footings should extend laterally from the footings to at least the limits defined by 1 horizontal to 1 vertical lines sloped outward and downward from points located at least 2 feet horizontally beyond the bottom edges of the footings.

At the recommended bearing stress, we anticipate that foundation settlement will be on the order of one inch, or less. We anticipate that more than 50 percent of this settlement will occur during the construction period. We anticipate that settlement of this magnitude is acceptable. However, the structural engineer should determine the final acceptability of settlement.

Ground Floor Slabs

We recommend that the lowest level floor slabs be designed as earth-supported slabs-on-grade bearing on a minimum 6-inch thickness of compacted structural fill. All forest mat and topsoil should be removed from within the building limits prior to placing fill. All fill placed below the floor slabs for raises-in-grade should consist of compacted structural fill. Normal dampproofing and vapor barriers should be used below floor slabs.

Seismic Design Considerations

We recommend that the buildings be designed according to the seismic requirements of the latest edition of the International Building Code. The site classification is Class C; the site response coefficient F_a , is 1.2 for short period spectral response acceleration S_s of 0.37g; the site response coefficient F_v , is 1.7 for the one-second period spectral response acceleration S_1 of 0.10g. The soils are not considered liquefaction susceptible.

Lateral Foundation Loads

We recommend that lateral loads be resisted by bottom friction on footings. We recommend that a coefficient of friction equal to 0.40 be used for footings bearing on soil and a coefficient of friction equal to 0.70 for footings bearing on sound bedrock.

Lateral Soil Pressure

We recommend that foundation walls which are restrained at the top and backfilled be designed to resist a lateral earth pressure calculated on the basis of an equivalent fluid unit weight of 55 pounds per cubic foot. This fluid unit weight assumes an at rest earth pressure coefficient of 0.45 and a free-draining granular backfill. If any buildings will have below grade space, we recommend that a perimeter foundation drain consisting of a perforated pipe surrounded by crushed stone and filter fabric be constructed at the exterior base of the wall. Gravity drainage should be provided.

Retaining walls, if required, should be designed for an equivalent fluid unit weight of 40 pounds per cubic foot. Walls should be backfilled with free-draining structural fill, and gravity drainage should be provided.

Backfill Materials

Structural fill used below foundations and floor slabs and for backfill adjacent to walls should consist of sandy gravel to gravelly sand. It should be free of organic material, loam, trash, snow, ice, frozen soil and other objectionable material, and should conform to the following gradation:

<u>Sieve Size</u>	<u>Percent Finer by Weight</u>
6 inch	100
No. 4	30 to 90
No. 40	10 to 50
No. 200	0 to 8

Compacted structural fill should be placed in layers not exceeding eight inches in loose measure and compacted by self propelled vibratory equipment at the approximate optimum moisture content to a dry density of at least 95 percent of the maximum dry density, as determined in accordance with ASTM Test Designation D1557. In confined areas, the maximum particle size should be reduced to **3** inches and the loose layer thickness should be reduced to **6** inches and compaction performed by hand-guided equipment.

Compacted structural fill on the outside of the foundation walls and behind retaining walls should extend laterally a minimum of two feet from the wall. Backfill beyond this limit may consist of common fill. The top 12 inches of fill on the exterior of the buildings should consist of low permeability material to minimize water infiltration next to the building. Grading should provide for runoff away from the building.

Common fill may consist of inorganic mineral soil that can be placed in layers and compacted. Common fill should be placed and spread in layers not exceeding 12 inches in thickness and compacted with a minimum of two systematic passes of the equipment placing the fill.

In our opinion, the soil encountered in the test pits is not suitable for structural fill but would be suitable for common fill and for raises-in-grade in landscaped areas. The fine-grained soil (clayey silt and silty clay) will be moisture sensitive and may require moisture conditioning (drying) before being placed in layers and compacted.

Pavement on

The recommended pavement sections for roads and parking areas are as follows:

Roads

- 3 in. bituminous concrete, placed in two layers
- 3 in. screened or crushed gravel base course
- 18 in. sand or gravel subbase course

Automobile Parking Areas

- 3 in. bituminous concrete, placed in two layers
- 18 in. sand or gravel subbase course

Base and subbase course materials should conform to the following gradations:

Base Course

Screened or Crushed Gravel (Maine DOT Standard Specification, Highways and Bridges; Section 703.06a, Type A).

<u>Sieve Size</u>	<u>Percent Finer by Weight</u>
2 inch	100
112 inch.	45 to 70
114 inch	30 to 55
No. 40	0 to 20
No. 200	0 to 5

Subbase Course

Sand or Gravel (Maine DOT, Section 703.06b, Type D)

<u>Sieve Size</u>	<u>Percent Finer by Weight</u>
4 inch.	100
114 inch.	25 to 70
No. 40	0 to 30
No. 200	0 to 7

(Note: Type D aggregate should be modified to a maximum **4** inch size. Compacted structural fill may be substituted for gravel subbase course, but the maximum particle size should be reduced to **4** inches.)

All forest mat and topsoil should be removed from within the limits of pavement.

Subbase course material should be placed in maximum 8-inch thick loose lifts and compacted at approximately optimum moisture content to a dry density of at least 95 percent of maximum dry density, as determined in accordance with ASTM Test Designation D1557. Base course material should be placed in one lift and compacted with a minimum of two coverages with self-propelled vibratory compaction equipment.

Some portions of the access roads and parking areas may have relatively shallow depth of soil overlying bedrock. If proposed final pavement grades require the excavation of bedrock, we anticipate that rock excavation will require systematic drilling and blasting for removal. In pavement areas where subbase directly overlies bedrock, it is our opinion that the subbase thickness may be reduced to 6 inches.

Permanent slopes in bedrock may be as steep as 1 horizontal to **4** vertical. Permanent slopes in soil should be no steeper than 2 horizontal to 1 vertical. In areas where soil slopes overlie excavated bedrock slopes, we recommend that a 5-foot bench be provided at the top of the rock slope; that is, the toe of the soil slope be set back a minimum of 5 feet from the top of the bedrock slope.

It should be noted that the subgrade soils are considered frost-susceptible. Therefore, pavement roughness due to non-uniform frost movement may occur. To eliminate such non-uniform frost movement would require approximately 4.5 feet of structural fill subbase, except where bedrock is within the depth of freezing. However, it is common practice to tolerate seasonal movement to avoid the high cost of the added thickness of subbase.

Construction Considerations

General

The primary purpose of this section of the report is to comment on items related to excavation, earthwork, and related geotechnical aspects of proposed construction. It is written primarily for the engineer having responsibility for preparation of plans and specifications. Since it identifies potential construction problems related to foundations and earthwork, it will also aid personnel who monitor the construction activity. Prospective contractors for this project must evaluate the construction problems on the basis of their own knowledge and experience in the Portland, Maine area, and on the basis of similar projects in other localities, taking into account their proposed construction methods, procedures, equipment and personnel.

Excavation, Lateral Support and Control of Water

As discussed above, rock excavation may be required for foundation construction and roadway and parking areas. Based on observations of the character of bedrock revealed in the test pits and bedrock exposures on the site, we anticipate that excavation will require systematic drilling and blasting.

Normally, specifications require that the contractor is completely responsible for all damages resulting from blasting operations. In addition, specifications should require the contractor to control blasting vibrations measured at the locations of the nearest structures to not exceed the "safe limits" recommended by the U.S. Bureau of Mines in Appendix B of BUMINES RI 8507, attached; peak air over-pressure to less than 0,014 psi, measured at the location of the nearest structure, considering wind direction, and to provide protection against fly rock.

We anticipate that foundation excavation can be accomplished with sloped open excavation through the overburden soils provided safe side slopes can be maintained. Some sloughing and raveling should be anticipated in temporary slopes. Temporary excavations should be made in accordance with all OSHA and other applicable regulatory agency requirements.

We anticipate that groundwater may be encountered at proposed subgrade level or bearing level of footings. If encountered, groundwater can likely be controlled by open pumping from sumps. In general, the contractor should control groundwater and water from runoff and other sources by methods which prevent disturbance of bearing surfaces or adjacent soils and allow construction in-the-dry.

Construction Monitoring

The foundation recommendations contained herein are based on the known and predictable behavior of a properly engineered and constructed foundation. Monitoring of the foundation construction is required to enable the geotechnical engineer to keep in contact with procedures and techniques used in construction. Therefore, we recommend that a person qualified by training and experience be present to provide monitoring at the site during preparation of foundation bearing surfaces and placement of compacted structural fill.

Limitations of Recommendations

This report has been prepared for specific application to the subject project in accordance with generally accepted geotechnical engineering practices. In the event that any changes in the nature, design or location of the buildings are planned, the conclusions and recommendations contained in this report should not be considered valid, unless the changes are reviewed and the conclusions of this report modified or verified in writing.


The recommendations presented herein are based in part on the data obtained from the referenced test pits. The nature and extent of variations between the explorations may not become evident until construction. If variations then appear evident, it will be necessary to re-evaluate the recommendations of this report.

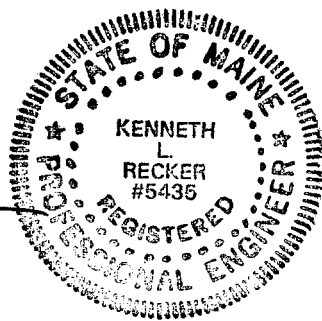
We request that we be provided the opportunity for a general review of final design and specifications in order to determine that our earthwork and foundation recommendations have been interpreted and implemented in the design and specifications as they were intended.

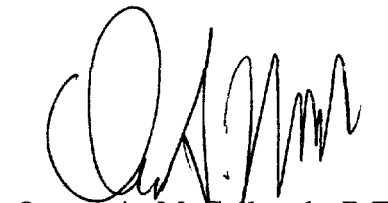
It has been a pleasure to work with you on this project. Please do not hesitate to contact us if you have any questions or require additional information.

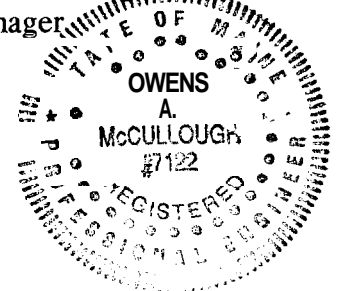
Sincerely,

SEBAGO TECHNICS, INC.


Kenneth L. Recker, P.E.
Geotechnical Engineering Manager




Owens A. McCullough, P.E.
Project Manager



KLR/OAM :klr/jc

Enclosures:

- Table I - Summary of Test Pits
- Sheet 1 - Site and Subsurface Exploration Plan
- Appendix A - Logs of Test Pits

TABLE I
SUMMARY OF TEST PITS
PORTLAND ASSISTED LIVING FACILITY – PHASE II
PORTLAND, MAINE

Test Pit No.	Ground Surface El. (Ft.)	Depth (Ft.)	Depth to Water (Ft.)	Strata Thickness (Ft.)							Approx. El. Top of Rock (Ft.)
				Forest Mat	Topsoil	Sand	Silt	Clay	Glacial Till	Bedrock	
TP29	83.6	2.5	NE	0.2	0.8	1.5	--	--	--	0.0*	81.1
TP31	82.4	4.0	NE	0.3	1.0	2.0	0.7	--	--	0.0*	78.4
TP32	76.1	2.0	NE	0.3	0.9	--	--	--	0.8	0.0*	74.1
TP33	75.0	0.5	NE	0.5	--	--	--	--	--	0.0*	74.5
TP34	73.1	3.5	NE	0.3	0.8	--	1.9	--	--	0.0*	69.6
TP35	70.8	6.0	NE	0.2	0.4	--	1.9	2.5/1.0*(5)	--	--	--
TP36	74.7	2.5	NE	0.2	1.4	0.9	--	--	--	0.0*	72.2
TP37	81.6	1.5	NE	0.2	0.6	--	0.7	--	--	0.0*	80.1
TP38	79.6	4.0	NE	0.3	0.9	0.8	2.0	--	--	0.0*	75.6
TP39	69.7	5.0	NE	0.2	1.3	--	3.5*	--	--	--	--
TP40	67.6	1.0	NE	0.2	0.4	0.4	--	--	--	0.0*	66.6
TP41	78.0	1.4	NE	0.	0.8	--	--	--	0.4	0.0*	76.6
TP42	86.9	3.7	NE	0.3	0.9	2.5	--	--	--	0.0*	83.2
TP43	77.5	7.5	NE	0.3	0.3	2.9	4.0	--	--	0.0*	70.0
TP44	83.0	0.0	NE	--	--	--	--	--	--	0.0*	83.0
TP45	69.7	5.5	NE	0.5	1.0	3.0	1.0	--	--	0.0*	64.2
TP46	71.8	1.3	NE	0.3	0.5	0.3	--	--	--	0.0*	70.5
TP47	64.7	4.0	NE	0.5	0.6	--	2.9	--	--	0.0*	60.7
TP48	65.2	3.0	NE	0.3	0.7	2.0	--	--	--	0.0*	62.2
TP49	85.2	2.5	NE	0.2	1.3	1.0	--	--	--	0.0*	82.7

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NOTES:

1. Elevations referenced to National Geodetic Vertical Datum.
2. NE Indicates water not encountered within depth of test pit.
3. -- Indicates stratum not encountered within depth of exploration.
4. * Indicates depth of penetration into stratum.
5. Sand encountered below the clay in TP35.
6. Test Pit TP44 was on a bedrock exposure and no excavation was required.

Appendix A

Logs of Test Pits

TEST PIT REPORT

TEST PIT NO. **TP29**

PROJECT PROPOSED PORTLAND RETIREMENT COMMUNITY
 LOCATION CANCO ROAD, PORTLAND, MAINE
 CLIENT CURRY BRANDAU ARCHITECTS
 CONTRACTOR W.H. LAVIGNE, INC.
 EQUIPMENT USED FORD 555B RUBBER TIRE BACKHOE

FILE NO. **80635000**
 LOCATION SEE PLAN
 ELEVATION 83.6
 DATE 17 September 1997
 H & A REP K. RECKER

DEPTH (FT)	SAMPLE NUMBER	SAMPLE DEPTH RANGE	ELEV./DEPTH (FT)	VISUAL DESCRIPTION	REMARKS
0			83.4	-FOREST MAT-	
			0.2	Dark brown, loamy SILT with roots	
			82.6	-TOPSOIL-	
			1.0	Brown silty fine SAND with fractured rock fragments	
2			81.1	-MARINE DEPOSIT-	
			2.5	Refusal on bedrock at 2.5 ft. Bottom of exploration at 2.5 ft.	

WATER LEVEL FROM GROUND SURFACE			APPROXIMATE PIT DIMENSIONS AT SURFACE		SUMMARY
DATE	TIME* (hours)	DEPTH (feet)	LENGTH: 9.0 FEET	WIDTH: 3.0 FEET	DEPTH (ft) 25
			BOULDERS		JAR SAMPLES
			8" TO 18" DIAMETER: = VOLUME:		BAG SAMPLES
			OVER 18" DIAMETER: (number) = VOLUME: (cu.ft.)		WATER LEVEL (ft) NE
* AFTER COMPLETED					



HALEY & ALDRICH, INC.
SOUTH PORTLAND,
MAINE

TEST FIT REPORT

TEST PIT NO. **TP31**

PROJECT PROPOSED PORTLAND RETIREMENT COMMUNITY
LOCATION CANCO ROAD, PORTLAND, MAINE
CLIENT CURRY BRANDAW ARCHITECTS
CONTRACTOR WH LAVIGNE, INC.
EQUIPMENT USED FORD 555B RUBBER TIRE BACKHOE

FILE No. 80635000
LOCATION SEE PLAN
ELEVATION 82.4
DATE 17 September 1997
H & A REP K. RECKER

DEPTH (FT)	SAMPLE NUMBER	SAMPLE DEPTH RANGE	ELEV./DEPTH (FT)	VISUAL DESCRIPTION	REMARKS
0			82.1	-FOREST MAT-	
			0.3	Brown, loamy SAND with roots	
				-TOPSOIL-	
			81.1	-----	
			1.3	Gray-brown sandy SILT	
2			80.4	-----	
			2.0	Brown medium to fine SAND, trace coarse sand and fractured rock fragments	
				-MARINE DEPOSIT-	
4			78.4		
			4.0	Refusal on weathered rock at 4.0 ft. Bottom of exploration at 4.0 ft.	
WATER LEVEL FROM GROUND SURFACE			APPROXIMATE PIT DIMENSIONS AT SURFACE		SUMMARY
DATE	TIME (hours)	DEPTH (feet)	LENGTH: 9.0 FEET	WIDTH: 3.0 FEET	DEPTH (ft) 4.0
			BOULDERS		JAR SAMPLES
			8" TO 18" DIAMETER: = VOLUME:		BAG SAMPLES
			OVER 18" DIAMETER: = VOLUME:		WATER LEVEL (ft) NE
* AFTER COMPLETED			(number)	(cu. ft.)	



HALEY & ALDRICH, INC.
SOUTH PORTLAND,
MAINE

TEST PIT REPORT

TEST PIT NO. **TP32**

PROJECT PROPOSED PORTLAND RETIREMENT COMMUNITY
 LOCATION CANCO ROAD, PORTLAND, MAINE
 CLIENT CURRY BRANDAW ARCHITECTS
 CONTRACTOR W.H. LAVIGNE, INC.
 EQUIPMENT USED FORD 555B RUBBER TIRE BACKHOE

FILE No. 80635000
 LOCATION SEE PLAN
 ELEVATION 76.1
 DATE 17 September 1997
 H & A REP K RECKER

DEPTH (FT)	SAMPLE NUMBER	SAMPLE DEPTH RANGE	ELEV./ DEPTH (FT)	VISUAL DESCRIPTION	REMARKS
0			75.8	-FOREST MAT-	
			0.3	Brown loamy SILT with roots	
			74.9	-TOPSOIL-	
			12	Brown silty medium to fine SAND with fractured	
2			74.1		
			20		

WATER LEVEL FROM GROUND SURFACE			APPROXIMATE PIT DIMENSIONS AT SURFACE		SUMMARY
DATE	TIME* (hours)	DEPTH (feet)	LENGTH: 6.0 FEET	WIDTH: 3.0 FEET	DEPTH (ft) 2.0
			BWLERS		JAR SAMPLES
			8" TO 18" DIAMETER: =	VOLUME:	BAG SAMPLES
			OVER 18" DIAMETER: =	VOLUME:	WATER LEVEL (ft) NE
			(number)	(cu. ft.)	



HALEY & ALDRICH, INC.
SOUTH PORTLAND,
MAINE

TEST PIT REPORT

TEST PIT NO. **TP33**

PROJECT PROPOSED PORTLAND RETIREMENT COMMUNITY
 LOCATION CANCO ROAD, PORTLAND, MAINE
 CLIENT CURRY BRANDAW ARCHITECTS
 CONTRACTOR UH LAVIGNE, INC.
 EQUIPMENT USED FORD 555B RUBBER TIRE BACKHOE

FILE No. 80635000
 LOCATION SEE PLAN
 ELEVATION **75.0**
 DATE 17 September 1997
 H & A REP K. RECKER

DEPTH (FT)	SAMPLE NUMBER	SAMPLE DEPTH RANGE	ELEV. / DEPTH (FT)	VISUAL DESCRIPTION	REMARKS
- 0			74.5 0.5	-FOREST MAT- Refusal on bedrock at 0.5 ft. Bottom of exploration at 0.5 ft.	

WATER LEVEL FROM GROUND SURFACE			APPROXIMATE PIT DIMENSIONS AT SURFACE		SUMMARY
DATE	TIME* (hours)	DEPTH (feet)	LENGTH: 6.0 FEET	WIDTH: 3.0 FEET	DEPTH (ft) 0.5
			BOULDERS		JAR SAMPLES
			8" TO 18" DIAMETER: <input type="checkbox"/>	VOLUME:	BAG SAMPLES
			OVER 18" DIAMETER: =	VOLUME:	WATER LEVEL (ft) NE
* AFTER COMPLETED			(number)	(cu.ft.)	



HALEY & ALDRICH, INC.
SOUTH PORTLAND,
MAINE

TEST PIT REPORT

TEST PIT NO. **TP34**

PROJECT PROPOSED PORTLAND RETIREMENT COMMUNITY
 LOCATION CANCO ROAD, PORTLAND, MAINE
 CLIENT CURRY BRANDAU ARCHITECTS
 CONTRACTOR U.H. LAVIGNE, INC.
 EQUIPMENT USED FORD 555B RUBBER TIRE BACKHOE

FILE No. 80635000
 LOCATION SEE PLAN
 ELEVATION 73.1
 DATE 17 September 1997
 H & A REP K. RECKER

DEPTH (FT)	SAMPLE NUMBER	SAMPLE DEPTH RANGE	ELEV./ DEPTH (FT)	VISUAL DESCRIPTION	REMARKS
0			72.8 0.3	-FOREST MAT- Brown loamy SILT with roots	
			72.0 1.1	-TOPSOIL- Gray-brom mottled sandy SILT	
2				-MARINE DEPOSIT-	
			70.1 3.0 69.6 3.5	Brown silty coarse to fine SAND with fractured rock fragments -GLACIAL TILL- Refusal on weathered bedrock at 3.5 ft. Bottom of exploration at 3.5 ft.	

WATER LEVEL FROM GROUND SURFACE			APPROXIMATE PIT DIMENSIONS AT SURFACE		SUMMARY
DATE	TIME* (hours)	DEPTH (feet)	LENGTH: 9.0 FEET	WIDTH: 3.0 FEET	DEPTH (ft) 3.5
			BWLERS		JAR SAMPLES
			8" TO 18" DIAMETER: = VOLUME:		BAG SAMPLES
			OVER 18" DIAMETER: = VOLUME:		WATER LEVEL (ft) NE
* AFTER COMPLETED			(number)	(cu.ft.)	



HALEY & ALDRICH, INC.
SOUTH PORTLAND,
MAINE

TEST PIT REPORT

TEST PIT NO. TP35

PROJECT PROPOSED PORTLAND RETIREMENT COMMUNITY
 LOCATION CANCO ROAD, PORTLAND, MAINE
 CLIENT CURRY BRANDAU ARCHITECTS
 CONTRACTOR W.H. LAVIGNE, INC.
 EQUIPMENT USED FORD 555B RUBBER TIRE BACKHOE

FILE No. 80635000
 LOCATION SEE PLAN
 ELEVATION 70.8
 DATE 17 September 1997
 H & A REP K. RECKER

DEPTH (FT)	SAMPLE NUMBER	SAMPLE DEPTH RANGE	ELEV./DEPTH (FT)	VISUAL DESCRIPTION	REMARKS
0			70.6	-FOREST MAT-	
			0.2	Gray-brown loamy SILT with roots	
			70.2	-TOPSOIL-	
			0.6	Gray-brown mottled clayey SILT	
				-MARINE DEPOSIT-	
2			68.3	Gray-brom silty CLAY	
			2.5		
4			65.8	Brown silty fine SAND	
			5.0		
				-MARINE DEPOSIT-	
6			64.8	Bottom of exploration at 6.0 ft.	
			6.0	No refusal	

WATER LEVEL FROM GROUND SURFACE			APPROXIMATE PIT DIMENSIONS AT SURFACE		SUMMARY
DATE	TIME* (hours)	DEPTH (feet)	LENGTH: 9.0 FEET	WIDTH: 3.0 FEET	DEPTH (ft) 6.0
			BOULDERS		JAR SAMPLES
			8" TO 18" DIAMETER: <input type="checkbox"/>	VOLUME:	BAG SAMPLES
			OVER 18" DIAMETER: (number) <input type="checkbox"/>	VOLUME: (cu.ft.)	WATER LEVEL (ft) NE
* AFTER COMPLETE0					



HALEY & ALDRICH, INC.
 150 SWTH PORTLAND,
 MAINE

TEST PIT REPORT

TEST PIT NO. **TP36**

PROJECT	PROPOSED PORTLAND RETIREMENT COMMUNITY	FILE No.	80635000
LOCATION	CANCO ROAD, PORTLAND, MAINE	LOCATION	SEE PLAN
CLIENT	CURRY BRANDAW ARCHITECTS	ELEVATION	74.7
CONTRACTOR	U.H. LAVICNE, INC.	DATE	17 September 1997
EQUIPMENT USED	FORD 555B RUBBER TIRE BACKHOE	H & A REP	K. RECKER

DEPTH (FT)	SAMPLE NUMBER	SAMPLE DEPTH RANGE	ELEV./DEPTH (FT)	VISUAL DESCRIPTION	REMARKS
0			74.5	-FOREST MAT-	
			0.2	Dark brown loamy SILT with roots	
				-TOPSOIL-	
			73.1		
			1.6	Rust-brown silty fine SAND with fractured rock fragments and cobbles	
2			72.2	-MARINE DEPOSIT-	
			2.5	Refusal on bedrock at 2.5 ft. Bottom of exploration at 2.5 ft.	

WATER LEVEL FROM CRWIND SURFACE			APPROXIMATE PIT DIMENSIONS AT SURFACE			SUMMARY	
DATE	TIME* (hours)	DEPTH (feet)	LENGTH: 9.0 FEET	WIDTH: 3.0 FEET	DEPTH (ft)	2.5	
			BOULDERS			JAR SAMPLES	
			8" TO 18" DIAMETER: = VOLUME:			BAG SAMPLES	
			OVER 18" DIAMETER: (number) □ VOLUME: (cu. ft.)			WATER LEVEL (ft) NE	
* AFTER COMPLETED							



HALEY & ALDRICH, INC.
SOUTH PORTLAND,
MAINE

TEST PIT REPORT

TEST PIT NO. **TP37**

PROJECT	PROPOSED PORTLAND RETIREMENT COMMUNITY	FILE NO.	80635000
LOCATION	CANCO ROAD, PORTLAND, MAINE	LOCATION	SEE PLAN
CLIENT	CURRY BRANDAU ARCHITECTS	ELEVATION	81.6
CONTRACTOR	U.H. LAVIGNE, INC.	DATE	17 September 1997
EQUIPMENT USED	FORD 555B RUBBER TIRE BACKHOE	H & A REP	K. RECKER

DEPTH (FT)	SAMPLE NUMBER	SAMPLE DEPTH RANGE	ELEV./DEPTH (FT)	VISUAL DESCRIPTION	REMARKS
			81.4	-FOREST MAT-	
			0.2	Dark brown , loamy SILT with roots	
			80.8	-TOPSOIL-	
			0.8	Brown sandy SILT	
			80.1	-MARINE DEPOSIT-	
			1.5	Refusal on bedrock at 15 ft. Bottom of exploration at 15 ft.	

WATER LEVEL FROM GROUND SURFACE			APPROXIMATE PIT DIMENSIONS AT SURFACE			SUMMARY	
DATE	TIME* (hours)	DEPTH (feet)	LENGTH: 9.0 FEET	WIDTH: 3.0 FEET	DEPTH (ft)	1.5	
			BOULDERS			JAR SAMPLES	
			8" TO 18" DIAMETER: =	VOLUME:	BAG SAMPLES		
			OVER 18" DIAMETER: □	VOLUME: (number)	WATER LEVEL (ft)	NE	
* AFTER COMPLETED				(cu.ft.)			



HALEY & ALDRICH, INC.
SOUTH PORTLAND,
MAINE

TEST PIT REPORT

TEST PIT NO. **TP38**
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PROJECT PROPOSED PORTLAND RETIREMENT COMMUNITY
LOCATION CANCO ROAD, PORTLAND, MAINE
CLIENT CURRY BRANDAU ARCHITECTS
CONTRACTOR U.H. LAVIGNE, INC.
EQUIPMENT USED FORD 555B RUBBER TIRE BACKHOE

FILE No. 80635000
LOCATION SEE PLAN
ELEVATION **79.6**
DATE 17 September 1997
H & A REP K. RECKER

DEPTH (FT)	SAMPLE NUMBER	SAUPLE DEPTH RANGE	ELEV./ DEPTH (FT)	VISUAL DESCRIPTION	REMARKS
0			79.3	-FOREST MAT-	
			0.3	Dark brown loamy SILT with roots -TOPSOIL-	
			78.4		
			1.2	Brown silty fine SAND	
2			77.6		
			2.0	Gray-brown sandy SILT	
				-MARINE DEPOSIT-	
4			75.6		
			4.0	Refusal on bedrock at 4.0 ft. Bottom of exploration at 4.0 ft.	

WATER LEVEL FROM GROUND SURFACE			APPROXIMATE PIT DIMENSIONS AT SURFACE		SUMMARY
DATE	TIME* (hours)	DEPTH (feet)	LENGTH: 8.0 FEET	WIDTH: 3.0 FEET	DEPTH (ft) 4.0
			BOULDERS		JAR SAMPLES
			8" TO 18" DIAMETER: =	VOLUME:	BAG SAMPLES
			OVER 18" DIAMETER: =	VOLUME:	WATER LEVEL (ft) NE
* AFTER COMPLETED			(number)	(cu.ft.)	



HALEY & ALDRICH, INC.
SOUTH FLORENCE, MAINE

TEST PIT REPORT

TEST PIT NO. **TP39**

PROJECT PROPOSED PORTLAND RETIREMENT COMMUNITY
 LOCATION CANCO ROAD, PORTLAND, MAINE
 CLIENT CURRY BRANDAW ARCHITECTS
 CONTRACTOR UH LAVICNE, INC.
 EQUIPMENT USED FORD 555B RUBBER TIRE BACKHOE

FILE No. 80635000
 LOCATION SEE PLAN
 ELEVATION 69.7
 DATE 17 September 1997
 H & A REP K. RECKER

DEPTH (FT)	SAMPLE NUMBER	SAMPLE DEPTH RANGE	ELEV./DEPTH (FT)	VISUAL DESCRIPTION	REMARKS
0			69.5	-FOREST MAT-	
			0.2	Brown loamy SILT with roots	
			68.2	Gray-brown mottled clayey SILT	
2			1.5		
				-MARINE DEPOSIT-	
4					
			64.7	Bottom of exploration at 5.0 ft.	
			5.0	No refusal	

WATER LEVEL FROM GROUND SURFACE			APPROXIMATE PIT DIMENSIONS AT SURFACE		SUMMARY
DATE	TIME* (hours)	DEPTH (feet)	LENGTH: 8.0 FEET	WIDTH: 3.0 FEET	DEPTH (ft) 5.0
			BOULDERS		JAR SAMPLES
			8" TO 18" DIAMETER: = VOLUME:		BAG SAMPLES
			OVER 18" DIAMETER: = VOLUME:		WATER LEVEL (ft) NE
* AFTER COMPLETED			(number)	(cu. ft.)	-



HALEY & ALDRICH, INC.
SWTH PORTLAND,
MAINE

TEST PIT REPORT

TEST PIT NO. **TP40**

PROJECT PROPOSED PORTLAND RETIREMENT COMMUNITY
 LOCATION CANCO ROAD, PORTLAND, MAINE
 CLIENT CURRY BRANDAU ARCHITECTS
 CONTRACTOR W.H. LAVIGNE, INC.
 EQUIPMENT USED FORD 555B RUBBER TIRE BACKHOE

FILE NO. 80635000
 LOCATION SEE PLAN
 ELEVATION 67.6
 DATE 17 September 1997
 H & A REP K. RECKER

DEPTH (FT)	SAMPLE NUMBER	SAMPLE DEPTH RANGE	ELEV. / DEPTH (FT)	VISUAL DESCRIPTION	REMARKS
0			67.4	-FOREST MAT-	
			0.2	Brown loamy SILT with roots	
			67.0	-TOPSOIL-	
			0.6	Brown silty fine SAND	
			66.6	-MARINE DEPOSIT-	
			1.0	Refusal on bedrock at 1.0 ft. Bottom of exploration at 1.0 ft.	

WATER LEVEL FROM GROUND SURFACE			APPROXIMATE PIT DIMENSIONS AT SURFACE		SUMMARY
DATE	TIME* (hours)	DEPTH (feet)	LENGTH: 6.0 FEET	WIDTH: 3.0 FEET	DEPTH (ft) 1.0
			BOULDERS		JAR SAMPLES
			8" TO 18" DIAMETER: =	VOLUME:	BAG SAMPLES
			OVER 18" DIAMETER: =	VOLUME:	WATER LEVEL (ft) NE
• AFTER COMPLETED			(number)	(cu. ft.)	



HALEY & ALDRICH INC.
SOUTH PORTLAND, ME

TEST PIT REPORT

TEST PIT NO. **TP41**

PROJECT PROPOSED PORTLAND RETIREMENT COMMUNITY
 LOCATION CANCO ROAD, PORTLAND, MAINE
 CLIENT CURRY ERANAU ARCHITECTS
 CONTRACTOR U.H. LAVIGNE, INC.
 EQUIPMENT USED FORD 555B RUBBER TIRE BACKHOE

FILE No. 80635000
 LOCATION SEE PLAN
 ELEVATION **78.0**
 DATE 17 September 1997
 H & A REP K. RECKER

DEPTH (FT)	SAMPLE NUMBER	SAMPLE DEPTH RANGE	ELEV./DEPTH (FT)	VISUAL DESCRIPTION	REMARKS
0			77.8	-FOREST MAT-	
			0.2	Dark brom loamy SILT with roots	
			77.0	-TOPSOIL-	
			1.0	Brown silty fine SAND with fractured rock	
			76.6	-GLACIAL TILL-	
			1.4	Refusal on bedrock at 1.4 ft. Bottom of exploration at 1.4 ft.	

WATER LEVEL FROM GROUND SURFACE			APPROXIMATE PIT DIMENSIONS AT SURFACE		SUMMARY
DATE	TIME* (hours)	DEPTH (feet)	LENGTH: 6.0 FEET	WIDTH: 3.0 FEET	DEPTH (ft) 1.4
			BOULDERS		JAR SAMPLES
			8" TO 18" DIAMETER: = VOLUME:		BAG SAMPLES
			OVER 18" DIAMETER: = VOLUME: (number) (cu.ft.)		WATER LEVEL (ft) NE
• AFTER COMPLETED					



HALEY & ALDRICH, INC.
SOUTH PORTLAND,
MAINE

TEST PIT REPORT

TEST PIT NO. **TP42**

PROJECT	PROPOSED PORTLAND RETIREMENT COMMUNITY	FILE No.	80635000
LOCATION	CANCO ROAD, PORTLAND, MAINE	LOCATION	SEE PLAN
CLIENT	CURRY BRANDAW ARCHITECTS	ELEVATION	86.9
CONTRACTOR	WH LAVIGNE, INC.	DATE	18 September 1997
EQUIPMENT USED	FORD 555B RUBBER TIRE BACKHOE	H & A REP	M. SNOW

DEPTH (FT)	SAMPLE NUMBER	SAMPLE DEPTH RANGE	ELEV./DEPTH (FT)	VISUAL DESCRIPTION	REMARKS
0			86.6 0.3		
			85.7 1.2	Rust brown loamy fine SAND, little gravel, roots	
2			84.6 2.3	Light brown d i m to fine SAND, little silt, little gravel, occasional cobbles -MARINE DEPOSIT-	
			83.2 3.7	Refusal on bedrock at 3.7 ft. Bottom of exploration at 3.7 ft.	

WATER LEVEL FROM GROUND SURFACE			APPROXIMATE PIT DIMENSIONS AT SURFACE		SUMMARY
DATE	TIME (hours)	DEPTH (feet)	LENGTH: 11.0 FEET	WIDTH: 3.0 FEET	DEPTH (ft) 3.7
			BOULDERS		JAR SAMPLES
			8" TO 18" DIAMETER: =	VOLUME:	BAG SAMPLES
			OVER 18" DIAMETER: =	VOLUME:	WATER LEVEL (ft) NE
* AFTER COMPLETED			(number)	(cu.ft.)	



HALEY & ALDRICH, INC.
SOUTH PORTLAND,
MAINE

TEST PIT REPORT

TEST PIT NO. **TP43**

PROJECT PROPOSED PORTLAND RETIREMENT COMMUNITY
LOCATION CANCO ROAD, PORTLAND, MAINE
CLIENT CURRY BRANDAU ARCHITECTS
CONTRACTOR U.H. LAVIGNE, INC.

FILE No. **80635000**
LOCATION SEE PLAN
ELEVATION **77.5**
DATE **18 September 1997**

DEPTH (FT)	SAMPLE NUMBER	SAMPLE DEPTH RANGE	ELEV./DEPTH (FT)	VISUAL DESCRIPTION	REMARKS
0			77.2	-FOREST ((AT-	
			0.3	Dark brom loamy SILT, roots	
			76.9	Rust brom fine SAND, little silt, little gravel, roots	
			0.6	-MARINE DEPOSIT-	
2			75.3	Light brown fine SAND, little silt, little coarse to mediu sand and gravel, feu cobbles	
			2.2	-MARINE DEPOSIT-	
4			74.0	Stiff olive-brown to gray-brown mottled clayey SILT, Little fine sand	
			3.5	-MARINE DEPOSIT-	
6			70.0	Refusal on bedrock at 7.5 ft. Bottom of exploration at 7.5 ft.	
			7.5		

WATER LEVEL FROM GROUND SURFACE			APPROXIMATE PIT DIMENSIONS AT SURFACE		SUMMARY
DATE	TIME* (hours)	DEPTH (feet)	LENGTH: 13.0 FEET	WIDTH: 3.0 FEET	DEPTH (ft) 7.5
			BOULDERS		JAR SAMPLES
			8" TO 18" DIAMETER: =	VOLUME:	BAG SAMPLES
			OVER 18" DIAMETER: =	VOLUME:	WATER LEVEL (ft) NE
• AFTER COMPLETED			(number)	(cu.ft.)	



HALEY & ALDRICH, INC.
SOUTH PORTLAND, ME

TEST PIT REPORT

TEST PIT NO. **TP45**

PROJECT	PROPOSED PORTLAND RETIREMENT COMMUNITY	FILE No.	80635000
LOCATION	CANCO ROAD, PORTLAND, MAINE	LOCATION	SEE PLAN
CLIENT	CURRY BRANDAW ARCHITECTS	ELEVATION	69.7
CONTRACTOR	U.H. LAVIGNE, INC.	DATE	17 September 1997
EQUIPMENT USED	FORD 555B RUBBER TIRE BACKHOE	H & A REP	K. RECKER

DEPTH (FT)	SAMPLE NUMBER	SAMPLE DEPTH RANGE	ELEV./DEPTH (FT)	VISUAL DESCRIPTION	REMARKS
0			69.2	-FOREST MAT-	
			0.5	Dark brown <i>loamy</i> SILT with roots	
				-TOPSOIL-	
			68.2		
			1.5	Gray-brown sandy SILT	
2				-MARINE DEPOSIT-	
			67.2		
			2.5	Interbedded brown, silty fine SAND and gray-brown sandy SILT	
4				-MARINE DEPOSIT-	
			64.2		
			5.5	Refusal on bedrock at 5.5 ft. Bottom of exploration at 5.5 ft.	

WATER LEVEL FROM GROUND SURFACE			APPROXIMATE PIT DIMENSIONS AT SURFACE		SUMMARY
DATE	TIME* (hours)	DEPTH (feet)	LENGTH: 9.0 FEET	WIDTH: 3.0 FEET	DEPTH (ft) 5.5
			BOULDERS		JAR SAMPLES
			8" TO 18" DIAMETER: =	VOLUME:	BAG SAMPLES
			OVER 18" DIAMETER: =	VOLUME :	WATER LEVEL (ft) NE
* AFTER COMPLETED			(number)	(cu.ft.)	



HALEY & ALDRICH, INC.
SOUTH PORTLAND,
MAINE

TEST PIT REPORT

TEST PIT NO. **TP46**

PROJECT	PROPOSED PORTLAND RETIREMENT COMMUNITY	FILE No.	80635000
LOCATION	CANCO ROAD, PORTLAND, MAINE	LOCATION	SEE PLAN
CLIENT	CURRY BRANDAW ARCHITECTS	ELEVATION	71.8
CONTRACTOR	W.H. LAVICNE, INC.	DATE	17 September 1997
EQUIPMENT USED	FORD 5558 RUBBER TIRE BACKHOE	H & A REP	<input checked="" type="checkbox"/> RECKER

DEPTH (FT)	SAMPLE NUMBER	SAMPLE DEPTH RANGE	ELEV./DEPTH (FT)	VISUAL DESCRIPTION	REMARKS
0			71.5	-FOREST MAT-	
			0.3	Dark brown loamy SILT with roots -TOPSOIL-	
			70.8	Brown silty fine SAND	
			1.0	-MARINE DEPOSIT-	
			70.5		
			1.3	Refusal on bedrock at 1.3 ft. Bottom of exploration at 1.3 ft.	

WATER LEVEL FROM GRWND SURFACE			APPROXIMATE PIT DIMENSIONS AT SURFACE			SUMMARY	
DATE	TIME* (hours)	DEPTH (feet)	LENGTH: 6.0 FEET	WIDTH: 3.0 FEET	DEPTH (ft) 1.3		
			BOULDERS			JAR SAMPLES	
			8" TO 18" DIAMETER: = VOLUME:	BAG SAMPLES			
			OVER 18" DIAMETER: (number) = VOLUME: (cu.ft.)	WATER LEVEL (ft) NE			



HALEY & ALDRICH, INC.
SWTH PORTLAND,
MAINE

TEST PIT REPORT

TEST PIT NO. **TP47**

PROJECT PROPOSED PORTLAND RETIREMENT COMMUNITY
 LOCATION CANCO ROAD, PORTLAND, MAINE
 CLIENT CURRY BRANDAU ARCHITECTS
 CONTRACTOR U.H. LAVIGNE, INC.
 EQUIPMENT USED FORD 5558 RUBBER TIRE BACKHOE

FILE No. 80635000
 LOCATION SEE PLAN
 ELEVATION 64.7
 DATE 17 September 1997
 H & A REP RECKER

DEPTH (FT)	SAMPLE NUMBER	SAMPLE DEPTH RANGE	ELEV./DEPTH (FT)	VISUAL DESCRIPTION	REMARKS
0				-FOREST MAT-	
			64.2 0.5	Dark brown Loamy SILT with roots -TOPSOIL-	
			63.6 1.1	Gray-brown sandy SILT to silty fine SAND	
2				-MARINE DEPOSIT-	
	01	3.0 3.5			
4			60.7 4.0	Refusal on bedrock at 4.0 ft. Bottom of exploration at 4.0 ft.	

WATER LEVEL FROM GRWND SURFACE			APPROXIMATE PIT DIMENSIONS AT SURFACE		SUMMARY
DATE	TIME* (hours)	DEPTH (feet)	LENGTH: 8.0 FEET	WIDTH: 3.0 FEET	DEPTH (ft) 4.0
			BWLDRS		JAR SAMPLES
			8" TO 18" DIAMETER: = VOLUME:		BAG SAMPLES
			OVER 18" DIAMETER: = VOLUME:		WATER LEVEL (ft) NE
* AFTER COMPLETED			(number)	(cu.ft.)	



HALEY & ALDRICH, INC.
SOUTH PORTLAND,
MAINE

TEST PIT REPORT

TEST PIT NO. **TP48**

PROJECT PROPOSED PORTLAND RETIREMENT COMMUNITY
 LOCATION CANCO ROAD, PORTLAND, MAINE
 CLIENT CURRY BRANDAW ARCHITECTS
 CONTRACTOR WH LAVICNE, INC.
 EQUIPMENT USED FORD 5558 RUBBER TIRE BACKHOE

FILE No. 80635000
 LOCATION SEE PLAN
 ELEVATION **65.2**
 DATE 17 September 1997
 H & A REP K. RECKER

DEPTH (FT)	SAMPLE NUMBER	SAMPLE DEPTH RANGE	ELEV. / DEPTH (FT)	VISUAL DESCRIPTION	REMARKS
0			64.9	-FOREST MAT-	
			0.3	Dark brown loamy SILT with roots	
			64.2	-TOPSOIL-	
			1.0	Gray-brown silty fine SAND with pieces of fractured rock	
2				-MARINE DEPOSIT-	
			62.2		
			3.0	Refusal on apparent bedrock at 3.0 ft. Bottom of exploration at 3.0 ft.	

WATER LEVEL FROM GROUND SURFACE			APPROXIMATE PIT DIMENSIONS AT SURFACE		SUMMARY
DATE	TIME* (hours)	DEPTH (feet)	LENGTH: 9.0 FEET	WIDTH: 3.0 FEET	DEPTH (ft) 3.0
			EWLERS		JAR SAMPLES
			8" TO 18" DIAMETER: =	VOLUME:	BAG SAMPLES
			OVER 18" DIAMETER: =	VOLUME:	WATER LEVEL (ft) NE
* AFTER COMPLETED			(number)	(cu.ft.)	



HALEY & ALDRICH, INC.
SOUTH PORTLAND, ME

TEST PIT REPORT

TEST PIT NO. **TP49**

PROJECT PROPOSED PORTLAND RETIREMENT COMMUNITY
 LOCATION CANCO ROAD, PORTLAND, MAINE
 CLIENT CURRY BRANOAU ARCHITECTS
 CONTRACTOR UH LAVIGNE, INC.
 EQUIPMENT USED FORD 555B RUBBER TIRE BACKHOE

FILE No. 80635000
 LOCATION SEE PLAN
 ELEVATION 85.2
 DATE 17 September 1997
 H & A REP K. RECKER

DEPTH (FT)	SAMPLE NUMBER	SAMPLE DEPTH RANGE	ELEV./DEPTH (FT)	VISUAL DESCRIPTION	REMARKS
			85.0 0.2	-FOREST MAT- Brown Loamy fine SAND with roots	
			83.7 1.5	-TOPSOIL- Brown silty fine SAND, trace gravel	
			82.7 2.5	-MARINE DEPOSIT- Refusal on bedrock at 25 ft. Bottom of exploration at 25 ft.	

WATER LEVEL FROM GROUND SURFACE			APPROXIMATE PIT DIMENSIONS AT SURFACE		SUMMARY
DATE	TIME* (hours)	DEPTH (feet)	LENGTH: 8.0 FEET	WIDTH: 3.0 FEET	DEPTH (ft) 25
			BOULDERS		JAR SAMPLES
			8" TO 18" DIAMETER: <input type="checkbox"/>	VOLUME:	BAG SAMPLES
			OVER 18" DIAMETER: <input type="checkbox"/>	VOLUME:	WATER LEVEL (ft) NE
• AFTER COMPLETED			(number)	(cu.ft.)	

