GENERAL NOTES

1. THE NOTES ON THESE DRAWINGS ARE NOT INTENDED TO REPLACE SPECIFICATIONS. SEE SPECIFICATIONS FOR REQUIREMENTS IN ADDITION TO GENERAL NOTES. INCONSISTENCIES BETWEEN THESE DRAWINGS AND THE SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT PRIOR TO PROCEEDING WITH THE AFFECTED PORTION OF THE WORK.
2. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH JOB SPECIFICATIONS AND ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND SITE DRAWINGS. CONSULT THESE DRAWINGS FOR LOCATIONS AND DIMENSIONS OF OPENINGS, CHASES, INSERTS, REGLETS, SLEEVES, DEPRESSIONS, AND OTHER DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS.
3. ALL DIMENSIONS, EXISTING CONDITIONS, AND AS-BUILT CONDITIONS MUST BE VERIFIED IN THE FIELD. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK.
4. THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE ONLY AFTER THE STRUCTURAL WORK CONTAINED IN THE S- DRAWINGS IS COMPLETED. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO ENSURE THE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS OR TIEDOWNS. SUCH MATERIAL SHALL REMAIN THE PROPERTY OF THE CONTRACTOR AFTER COMPLETION OF THE PROJECT.
5. SECTIONS AND DETAILS SHOWN ON ANY STRUCTURAL DRAWINGS SHALL BE CONSIDERED TYPICAL FOR SIMILAR CONDITIONS AS DETERMINED BY THE STRUCTURAL ENGINEER. THE STRUCTURAL ENGINEER RESERVES THE RIGHT TO INTERPRET DETAILS TO ADDRESS OTHER PROJECT CONDITIONS.
6. PROVIDE AND INSTALL NECESSARY MATERIAL TO CONNECT ELEVATOR SUPPORT BEAMS AND GUIDE RAILS. LOCATION AND SIZE OF MEMBERS AND ANY INSERTS REQUIRED SHALL BE DETERMINED BY THE ELEVATOR MANUFACTURER.
7. THE CONTRACTOR SHALL SUBMIT COMPLETE SHOP DRAWINGS FOR ALL PARTS OF THE WORK, INCLUDING DESCRIPTION OF SHORING, AND CONSTRUCTION METHODS AND SEQUENCING WHERE APPLICABLE. NO PERFORMANCE OF THE WORK INCLUDING, BUT NOT LIMITED TO, DEMOLITION OF EXISTING STRUCTURE, OR FABRICATION OR ERECTION OF NEW STRUCTURAL ELEMENTS, SHALL COMMENCE WITHOUT REVIEW OF THE SHOP DRAWINGS BY THE ARCHITECT AND ENGINEER. SUBMIT ONE COPY AND ONE SEPIA. COPY WILL BE REVIEWED AND SEPIA WILL BE RETURNED. FOR SHOP DRAWINGS AND SUBMITTALS REQUIRED, REFERENCE THE PROJECT SPECIFICATION.
8. ALL APPLICABLE FEDERAL, STATE, AND MUNICIPAL REGULATIONS SHALL BE FOLLOWED, INCLUDING THE FEDERAL DEPARTMENT OF LABOR OCCUPATIONAL SAFETY AND HEALTH ACT.
9. IN ACCORDANCE WITH THE MAINE UNIFORM BUILDING AND ENERGY CODE/INTERNATIONAL BUILDING CODE (2009 EDITION, SECTION 1704.1), A STATEMENT OF SPECIAL INSPECTIONS IS REQUIRED AS A CONDITION FOR PERMIT ISSUANCE BY THE LOCAL CODE OFFICIAL. THIS STATEMENT SHALL INCLUDE A COMPLETE LIST OF MATERIALS AND WORK REQUIRING SPECIAL INSPECTIONS, THE INSPECTIONS TO BE PERFORMED AND A LIST OF THE INDIVIDUALS, APPROVED AGENCIES AND FIRMS INTENDED TO BE RETAINED FOR CONDUCTING SUCH INSPECTIONS.
10. REFERENCE THE PROJECT SPECIFICATIONS FOR ALL TESTING REQUIREMENTS.

DESIGN LOADS

1. BUILDING CODE:
   MAINE UNIFORM BUILDING AND ENERGY CODE
   INTERNATIONAL BUILDING CODE, 2009 EDITION
   INTERNATIONAL EXISTING BUILDING CODE, 2009 EDITION
   ASCE 7-05 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES
2. DESIGN FLOOR LIVE LOADS:
   OFFICES: 50 PSF + 15 PSF PARTITION ALLOWANCE
   PRIVATE ROOMS AND CORRIDORS SERVING: 40 PSF
   PUBLIC ROOMS AND CORRIDORS SERVING: 100 PSF
   PRIVATE BALCONIES: SAME AS OCCUPANCY SERVED: 40 PSF
   MEP ROOMS: 100 PSF
   STAIRS: 100 PSF
   ROOFTOP GARDEN: 100 PSF OCCUPANT + 100 PSF PLANTING ALLOWANCE
3. DESIGN ROOF SNOW LOAD:
   GROUND SNOW LOAD (Pg): 60 PSF
   SNOW EXPOSURE FACTOR (Ce): 1.0
   SNOW LOAD IMPORTANCE FACTOR (Is): 1.0
   SNOW LOAD THERMAL FACTOR (Ct): 1.1
   FLAT ROOF SNOW LOAD (Pf): 46 PSF + DRIFT
4. DESIGN WIND LOAD:
   BASIC WIND SPEED: 100 MPH
   WIND LOAD IMPORTANCE FACTOR (Iw): 1.0
   WIND EXPOSURE: B
   INTERNAL PRESSURE COEFFICIENT: ±0.18
   COMPONENTS & CLADDING PER ASCE 7-05
5. DESIGN SEISMIC LOADS (NEW BUILDING):
   EQUIVALENT LATERAL FORCE PROCEDURE
   SEISMIC OCCUPANCY CATEGORY: II
   SEISMIC IMPORTANCE FACTOR (Ie): 1.0
   MAPPED SPECTRAL RESPONSE ACCELERATIONS:
      Ss: 0.318
      S1: 0.078
   SEISMIC SITE CLASS: C
   SPECTRAL RESPONSE COEFFICIENTS:
      Sds: 0.254
      Sd1: 0.088
   SEISMIC DESIGN CATEGORY: B
   BASIC STRUCTURAL SYSTEM: BUILDING FRAME SYSTEM
                MOMENT RESISTING FRAME SYSTEM
   BASIC SEISMIC FORCE RESISTING SYSTEM:
      STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC
      RESISTANCE (R=3)
   RESPONSE MODIFICATION FACTOR (R): 3.0
   SEISMIC RESPONSE COEFFICIENT (Cs): 0.041

FOUNDATION NOTES (SOIL SUPPORTED)

1. FOUNDATIONS HAVE BEEN DESIGNED IN ACCORDANCE WITH A REPORT ENTITLED "GEOTECHNICAL ENGINEERING SERVICES, PROPOSED SENIOR LIVING FACILITY, THE PARK DANFORTH, ARBOR STREET & FOREST AVENUE, PORTLAND, MAINE, PREPARED BY S.W. COLE ENGINEERING, INC., DATED SEPTEMBER 30, 2014. THE RECOMMENDATIONS OF THE REPORT ARE PART OF THIS WORK. REFER TO THIS REPORT FOR SPECIFIC RECOMMENDATIONS.
2. FOUNDATION DESIGN IS BASED ON SHALLOW SPREAD FOOTINGS BEARING ON DENSIFIED NATIVE OUTWASH SANDS AND/OR NEW COMPACTED STRUCTURAL FILL EXTENDING TO DENSIFIED NATIVE OUTWASH SANDS PER THE REQUIREMENTS OF THE GEOTECHNICAL REPORT. REFER TO THIS REPORT FOR SPECIFIC BEARING RECOMMENDATIONS.
3. ALLOWABLE BEARING CAPACITY 4,000 PSF
4. EXTEND BOTTOM OF EXTERIOR FOOTINGS AT LEAST 4.5 FEET BELOW THE FINAL EXTERIOR GRADE FOR PROTECTION AGAINST FROST.
5. NO FILL FOR BUILDING SUPPORT SHALL BE PLACED UNTIL SUBGRADES HAVE BEEN OBSERVED AND APPROVED BY THE GEOTECHNICAL ENGINEER.
6. REFERENCE THE GEOTECHNICAL REPORT FOR ALL EXCAVATION, BACKFILL, COMPACTION, CONSTRUCTION DEWATERING AND PERMANENT DRAINAGE REQUIREMENTS.
7. SOILS EXPOSED AT THE BASE OF ALL SATISFACTORY FOUNDATION EXCAVATIONS SHOULD BE PROTECTED AGAINST ANY DETRIMENTAL CHANGE IN CONDITION, SUCH AS DISTURBANCE FROM RAIN OR FROST. SURFACE RUNOFF SHALL BE DRAINED AWAY FROM THE EXCAVATIONS AND NOT BE ALLOWED TO POND. FOUNDATION EXCAVATIONS SHALL BE ADEQUATELY PROTECTED FROM RAINFALL OR FREEZING CONDITIONS. GROUNDWATER SHOULD BE ANTICIPATED FOR EXCAVATIONS AND APPROPRIATE DEWATERING MEASURES SHALL BE EMPLOYED.
8. EXCAVATIONS FOR BUILDING CONSTRUCTION SHALL BE IN ACCORDANCE WITH OSHA REQUIREMENTS. BRACED EXCAVATIONS SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF MAINE. DO NOT UNDERMINE EXISTING FOUNDATIONS OF ANY ADJACENT STRUCTURES. REFER TO THE GEOTECHNICAL REPORT FOR ADDITIONAL AND/OR MORE SPECIFIC REQUIREMENTS.

CONCRETE NOTES

1. CONCRETE WORK SHALL CONFORM TO "ACI MANUAL OF CONCRETE PRACTICE", LATEST EDITION. THIS PUBLICATION IS AVALABLE THROUGH THE AMERICAN CONCRETE INSTITUTE (248) 848-3800.
2. ALL CONCRETE SHALL HAVE A 28-DAY COMPRESSIVE STRENGTH OF 3,500 PSI, U.N.O. EXTERIOR SLAB-ON-GRADE SHALL HAVE A 28-DAY COMPRESSIVE STRENGTH OF 5,000 PSI. ADDITIONAL CONCRETE MIX PERFORMANCE DATA INCLUDING AIR CONTENT, WATER-CEMENT RATIO, AGGREGATE SIZE, SLUMP, ETC. HAS BEEN INCLUDED IN THE PROJECT SPECIFICATIONS. SEE THE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.
3. CONCRETE SHALL NOT BE PLACED IN WATER OR ON FROZEN GROUND.
4. PROVIDE PVC SLEEVES WHERE PIPES PASS THROUGH EXTERIOR CONCRETE, OR SLABS.
5. REINFORCING BARS SHALL CONFORM TO ASTM A615 GRADE 60 DEFORMED BARS AND SHALL BE DETAILED, FABRICATED AND PLACED IN ACCORDANCE WITH ACI 315, LATEST EDITION.
6. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185 AND BE PROVIDED IN FLAT SHEETS.
7. FIBER REINFORCEMENT SHALL BE TYPE III SYNTHETIC VIRGIN HOMOPOLYMER POLYPROPYLENE FIBERS CONFORMING TO ASTM C1116.
8. MINIMUM CONCRETE PROTECTIVE COVERING FOR REINFORCEMENT, UNLESS NOTED OTHERWISE, SHALL BE AS FOLLOWS:
   A. SURFACES CAST AGAINST AND PERMANENTLY IN CONTACT WITH EARTH, 3.0"
   B. FORMED SURFACES IN CONTACT WITH EARTH OR EXPOSED TO WEATHER
      #5 BARS, 5/8" DIAMETER WIRE AND SMALLER, 1.5"
      #6 THROUGH #11 BARS, 2.0"
   C. SURFACES NOT IN CONTACT WITH EARTH OR EXPOSED TO WEATHER
      WALLS, SLABS, JOISTS #11 BARS AND SMALLER, 1.0"
      BEAMS, GIRDERS, AND COLUMNS; ALL REINFORCEMENT, 1.5"
9. REINFORCEMENT SHALL BE CONTINUOUS AROUND CORNERS AND AT INTERSECTIONS. PROVIDE LAPPED BARS AT NECESSARY SPLICES OR HOOKED BARS AT DISCONTINUOUS ENDS. PROVIDE TENSION LAP SPLICES S2.1, FOR ALL REINFORCING UNLESS OTHERWISE SHOWN ON PLAN.
10. WELDING OF REINFORCEMENT IS NOT PERMITTED
11. FOR ALL OPENINGS IN CONCRETE WALLS AND SLABS, PROVIDE SUPPLEMENTAL REINFORCING AROUND OPENING AS SHOWN ON THE CONTRACT DOCUMENTS TYPICAL DETAILS. NO PENETRATIONS SHALL BE MADE THROUGH FOOTINGS WITHOUT WRITTEN PERMISSION FROM ENGINEER.
12. CONSTRUCTION JOINTS SHOWN ON DRAWINGS ARE MANDATORY. OMISSIONS, ADDITIONS, OR CHANGES SHALL NOT BE MADE EXCEPT WITH THE SUBMITTAL OF A WRITTEN REQUEST TOGETHER WITH DRAWINGS OF THE PROPOSED JOINT LOCATIONS FOR APPROVAL OF THE STRUCTURAL ENGINEER. WHERE CONSTRUCTION JOINTS ARE NOT SHOWN, OR WHEN ALTERNATE LOCATIONS ARE PROPOSED, DRAWINGS SHOWING LOCATION OF CONSTRUCTION AND CONTROL JOINTS AND CONCRETE PLACEMENT SEQUENCE SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW PRIOR TO PREPARATION OF THE REINFORCEMENT SHOP DRAWINGS. CONCRETE SHALL BE PLACED WITHOUT HORIZONTAL CONSTRUCTION JOINTS EXCEPT WHERE SHOWN OR NOTED. VERTICAL CONSTRUCTION JOINTS AND STOPS IN CONCRETE BEAMS/ GRADE BEAMS SHALL BE MADE AT MIDSPAN OR AT POINTS OF MINIMUM SHEAR, UNLESS NOTED OTHERWISE.
13. SPACING OF CONSTRUCTION JOINTS, UNLESS NOTED OTHERWISE SHALL BE AS FOLLOWS:
   A. FOOTINGS AND WALLS
      MAX LENGTH 40'-0" OR 15'-0" FROM ANY CORNER**
   B. SLABS ON GRADE
      SEE FOUNDATION PLAN

** EXCEED ONLY WHERE INTERMEDIATE CONTRACTION JOINTS ARE PROVIDED. MINIMUM OF 72 HOURS SHALL ELAPSE BETWEEN ADJACENT CONCRETE PLACEMENTS.

14. ANCHOR RODS SHALL BE HEADED RODS CONFORMING TO ASTM F1554, GRADE 36 KSI WELDABLE STEEL, UNLESS NOTED OTHERWISE ON DRAWINGS. ANCHOR RODS THAT ARE TO BE IN CONTACT WITH PRESSURE TREATED LUMBER SHALL BE HOT-DIPPED GALVANIZED.
15. ALL GROUT BENEATH BASE PLATES & BEARING PLATES SHALL BE "5-STAR" 5000-PSI NON-SHRINK GROUT BY U.S. GROUT CORP.
16. SLAB THICKNESSES INDICATED ON THE DRAWINGS ARE MINIMUMS. PROVIDE SUFFICIENT CONCRETE TO ACCOUNT FOR STRUCTURE DEFLECTION, SUBGRADE FLUCTUATIONS, AND TO OBTAIN THE SPECIFIED SLAB ELEVATION AT THE FLATNESS AND LEVELNESS INDICATED.
17. INSTALLATION OF REINFORCEMENT SHALL BE COMPLETED AT LEAST 24 HOURS PRIOR TO THE SCHEDULED CONCRETE PLACEMENT. NOTIFY ARCHITECT AND STRUCTURAL ENGINEER OF COMPLETION AT LEAST 24 HOURS PRIOR TO THE SCHEDULED COMPLETION OF THE INSTALLATION OF REINFORCEMENT.
18. ALL ITEMS TO BE EMBEDDED INTO CONCRETE SHALL BE INSTALLED PRIOR TO PLACEMENT OF CONCRETE. PROVIDE ADDITIONAL REINFORCEMENT AND/OR TEMPLATES AS REQUIRED TO ENSURE THE CORRECT POSITIONS OF EMBEDMENTS. "WET SETTING" OF EMBEDMENTS INTO CONCRETE IS STRICTLY PROHIBITED. EMBEDMENTS INCLUDE, BUT NOT BY LIMITATION, REINFORCEMENT, REINFORCING DOWELS, EMBEDDED PLATES, ANCHOR RODS, ANCHOR INSERTS, SLEEVES, LOAD TRANSFER PLATES, DIAMOND DOWELS, AND SHELF BULK HEADS.

STRUCTURAL STEEL NOTES

1. STRUCTURAL STEEL FABRICATION, ERECTION, AND CONNECTION DESIGN SHALL CONFORM TO AISC "SPECIFICATION FOR THE DESIGN FABRICATIONS, AND ERECTION OF STRUCTURAL STEEL" LATEST EDITION, AND THE "CODE OF STANDARD PRACTICE", LATEST EDITION.
2. STRUCTURAL STEEL: STEEL PLATES, SHAPES, AND BARS, CONFORM TO ASTM A36 UNLESS NOTED OTHER WISE (U.N.O.). STRUCTURAL STEEL SHAPES DESIGNATED ON THE DRAWINGS FOR WIDE-FLANGE SECTIONS: ASTM A992 (ASTM A572 GRADE 50 WITH SPECIAL REQUIREMENTS PER AISC TECHNICAL BULLETIN #3 DATED MARCH, 1997)
3. STRUCTURAL TUBING: CONFORM TO ASTM A500 GRADE B46 KSI.
4. CONNECTION DESIGN FOR THIS PROJECT IS THE RESPONSIBILITY OF THE FABRICATOR. CONNECTION CALCULATIONS, SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF MAINE SHALL BE SUBMITTED WITH THE SHOP DRAWINGS FOR THIS PROJECT. SEE THE PROJECT SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.
5. FIELD CONNECTIONS SHALL BE BOLTED USING ASTM A325N HIGH STRENGTH BOLTS (U.N.O.) EXCEPT WHERE SLIP CRITICAL CONNECTIONS ARE REQUIRED AND NOTED BY A325 (SC) ON THE DRAWINGS. PROVIDE SLIP CRITICAL (SC) CONNECTIONS AT ALL MOMENT CONNECTIONS, BRACED FRAMES, RELIEVING ANGLES AND AS OTHERWISE NOTED. USE A490 BOLTS WHERE INDICATED.
6. WHERE WELDING IS INDICATED, ALL WELDING SHALL CONFORM TO AWS D1.1-LATEST EDITION. ELECTRODES SHALL CONFORM TO AWS A5.1 E70XX SERIES WITH PROPER ROD TO PRODUCE OPTIMUM WELD (LOW HYDROGEN)
7. SEE CONCRETE NOTES AND DRAWINGS FOR ANCHOR BOLT INFORMATION, TYP.
8. PROVIDE 3/8" MINIMUM STIFFENER PLATES EACH SIDE OF BEAM WEB AT BEAMS FRAMING OVER COLUMNS AND AT BEAMS SUPPORTING COLUMNS ABOVE.
9. PROVIDE 1/4" THICK LEVELING PLATE UNDER ALL COLUMN BASE PLATES UNLESS OTHERWISE NOTED. LEVELING PLATES SHALL BE SET AND GROUTED PRIOR TO ERECTING COLUMNS.
10. PROVIDE ALL MISCELLANEOUS ANGLES, PLATES, ANCHOR BLOTS ETC., SHOWN ON ARCHITECTURAL DRAWINGS FOR SUPPORT OF BLOCKING, PARAPETS, FINISHES, ETC. COORDINATE WITH MISCELLANEOUS METAL FABRICATOR TO ENSURE COMPLETE COVERAGE OF ALL ITEMS.
11. PROVIDE L3x3x1/4 SLAB SUPPORT ANGLE AS REQUIRED AT COLUMNS WHERE STRUCTURAL MEMBERS DO NOT FRAME IN AT ALL FOUR SIDES.

METAL DECK

1. THE METAL ROOF AND FLOOR DECK SHALL BE FORMED OF STEEL SHEETS CONFORMING TO THE FOLLOWING STANDARDS:
   A. FLOOR DECKING: ASTM A1008, GRADE C, D OR ASTM A653, STRUCTURAL QUALITY, GRADE 40 OR HIGHER
   B. ROOF DECKING: ASTM A1008, GRADE C, D OR ASTM A653, STRUCTURAL QUALITY, GRADE 33 OR HIGHER
2. FLOOR AND ROOF DECK SHALL BE AS NOTED ON THE DRAWINGS (OR EQUIVALENT).
3. FOR DECK ATTACHMENTS, PENETRATIONS AND ACCESSORIES REFER TO SPECIFICATIONS.

MASONRY NOTES

1. ALL MASONRY CONSTRUCTION SHALL CONFORM TO ACI 530.1-LATEST.
2. ALL CONCRETE MASONRY UNITS WALL BE ASTM C90 GRADE N, TYPE I STANDARD WEIGHT BLOCKS INCLUDING STRETCHERS AND CORNER BLOCKS. MINIMUM ASSEMBLY STRENGTH OF BLOCK SHALL BE F'M = 1500 PSI IN 28 DAYS.
3. MORTAR SHALL CONFORM TO ASTM SPECIFICATION C270, TYPE M OR S
4. GROUT SHALL CONFORM TO ASTM-C476
5. REINFORCING FOR BOND BEAMS, LINTEL BLOCKS AND VERTICAL WALL REINFORCING SHALL BE BILLET STEEL CONFORMING TO ASTM A615, GRADE 60
6. HORIZONTAL JOINT REINFORCING SHALL BE DUR-O-WAL TRUSS DESIGN, STANDARD CLASS MILL GALVANIZED WITH 3/16" DIAMETER SIDE RODS AND 9 GAUGE CROSS TIES, U.N.O. REINFORCING SHALL BE PLACED IN MASONRY WALLS AT EVERY SECOND BLOCK COURSE.
7. CONCRETE MASONRY UNITS SHALL BE LAID IN RUNNING BOND UNLESS OTHERWISE NOTED. PROVIDE FULL MORTAR COVERAGE ON ALL WEBS AND FACE SHELLS. PROVIDE CORNER BLOCKS AND END BLOCKS TO FINISH ALL 90 DEGREE CORNERS AND WALL OPENINGS.
8. PROVIDE LINTELS AT WALL PENETRATIONS AS SHOWN IN THE LINTEL SCHEDULE.
9. STANDARD LAP LENGTH OF GRADE 60 MASONRY REINFORCING BARS SHALL BE 48 BAR DIAMETERS FOR BARS #5 AND SMALLER. PROVIDE MECHANICAL SPLICES RATED FOR 125% THE BAR YIELD STRENGTH FOR BARS #6 AND LARGER. PROVIDE CORNER BARS TO MATCH HORIZONTAL REINFORCEMENT.
10. CELLS TO BE GROUTED SHALL BE 2-CELL BLOCK. ALIGN CELLS TO MAINTAIN A CLEAR UNOBSTRUCTED, CONTINUOUS VERTICAL CHASE. CELLS MUST BE KEPT CLEAN OF PROTRUSIONS OR FINS OF MORTAR. FILL CELLS OF MASONRY UNITS AND WALL CAVITIES WHERE INDICATED WITH 2500 PSI GROUT. MAXIMUM GROUT LIFT WITHOUT CLEAN-OUTS SHALL BE 4'-0". HIGH LIFT GROUTING SHALL CONFORM TO CODE REQUIREMENTS WITH A MINIMUM CEMENT CONTENT OF 8 SACKS PER CUBIC YARD. SUPPORT ALL VERTICAL BARS IN CENTER OF GROUTED CELLS WITH VERTICAL BAR POSITIONER.
11. FIELD PENETRATIONS THROUGH BLOCK WALLS SHALL NOT BE MADE THROUGH BOND BEAMS, LINTELS OR GROUTED CELLS.

PRECAST CONCRETE BALCONIES

1. ALL WORK SHALL CONFORM TO THE LATEST EDITION OF THE FOLLOWING: ACI 318 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE"; PCI MNL-116 "MANUAL FOR QUALITY CONTROL FOR PLANTS AND PRODUCTION OF PRECAST AND PRESTRESSED CONCRETE PRODUCTS" AND PCI "DESIGN HANDBOOK-PRECAST AND PRESTRESSED CONCRETE".
2. PRECAST BALCONIES SHALL BE DESIGNED FOR THE LOADS AS INDICATED WITHIN THESE DRAWINGS. DESIGN SHOULD INCLUDE FOR ALL DEAD LOADS DUE TO SELF WEIGHT AND RAILING ASSEMBLIES.
3. CONCRETE STRENGTH SHALL BE MINIMUM 5000 PSI AT 28 DAYS.
4. ALL CONCRETE SHALL BE AIR ENTRAINED. MEET AIR ENTRAINING REQUIREMENTS PER ACI "SPECIFICATIONS FOR STRUCTURAL CONCRETE (ACI 318)", FOR CLASS OF CONCRETE AND APPLICATION. AIR ENTRAINMENT ALL EXTERIOR CONCRETE 6% ± 1%.
5. PRESTRESSING TENDONS (IF USED) SHALL CONFORM WITH ASTM A416, GRADE 250.
6. COMPLETE SHOP DRAWINGS AND DESIGN CALCULATIONS STAMPED BY A LICENSED PROFESSIONAL ENGINEER LICENSED IN THE STATE OF MAINE SHALL BE SUBMITTED TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR REVIEW PRIOR TO COMMENCEMENT OF THAT PORTION OF THE WORK.

LINTELS

1. THE FOLLOWING LINTELS SHALL BE USED FOR MASONRY OPENINGS, U.N.O. ON DRAWINGS:

MASONRY OPENING	LINTEL SIZE
UP TO 3'-0"	L 3 1/2 x3 1/2 x 5/16
3'-1" TO 4'-6"	L 4 x 3 1/2 x 5/16 (LLV)
4'-7" TO 6'-0"	L 5 x 3 1/2 x 5/16 (LLV)
6'-1" TO 8'-0"	L 6 x 3 1/2 x 3/8 (LLV)

2. PROVIDE ONE ANGLE FOR EACH 4" WALL THICKNESS. FOR 6" WALL THICKNESS, PROVIDE WT OR BUILT-UP SECTION WITH PROPERTIES EQUAL TO OR GREATER THAN 1 ½ TIMES THE ANGLES PROPERTIES FOR A 4" WALL THICKNESS.
3. PROVIDE 8" OF BEARING AT EACH END OF ALL LINTELS.
4. ALL EXTERIOR LINTELS SHALL BE HOT-DIPPED GALVANIZED.

CMU LINTEL SCHEDULE

CLEAR SPAN	WIDTH	DEPTH	REINF
< 6'-0"	8"	8"	2#5 CONT
6'-0" - 8'-0"	8"	16"	2#5 CONT

ABBREVIATIONS

A.B.	ANCHOR BOLT
ABV	ABOVE
ACI	AMERICAN CONCRETE INSTITUTE
ACT	ACOUSTICAL CEILING TILE
ADDL	ADDITIONAL
AESS	ARCHITECTURAL EXPOSED STRUCTURAL STEEL
A.F.F.	ABOVE FINISH FLOOR
ALT	ALTERNATE
ALUM	ALUMINUM
APA	AMERICAN PLYWOOD ASSOCIATION
APPROX	APPROXIMATE
A.R.	ANCHOR ROD
ARCH	ARCHITECT OR ARCHITECTURAL
BAL	BALANCE
B.C.X.	BOTTOM CHORD EXTENSION
BD	BOARD
B.F.	BRACED FRAME
BLDG	BUILDING
BLKG	BLOCKING
BM	BEAM
BIT	BITUMINOUS
B.O.	BOTTOM OF/ BY OTHERS
BOT	BOTTOM
B.P.	BEAM POCKET
B.PL	BASE PLATE
BRG	BEARING
B.S.	BOTH SIDES
BSMT	BASEMENT
BTWN	BETWEEN
C/C	CENTER TO CENTER
C	CHANNEL
CFMF	COLD FORM METAL FRAMING
C.I.P.	CAST IN PLACE
C.J.	CONTRACTION/CONST. JOINT
℄	CENTER LINE
CLG	CEILING
CLR	CLEAR
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
CONC	CONCRETE
CONN	CONNECTION
CONST	CONSTRUCTION
CONT	CONTINUOUS
CONTR	CONTRACTOR
COORD	COORDINATE
CTR(D)	CENTER(ED)
d	PENNY
DBL	DOUBLE
DIA OR ø	DIAMETER
DIAG	DIAGONAL
DIM	DIMENSION
DL	DEAD LOAD
DN	DOWN
DO/do	DITTO/DO OVER
DP	DRILLED PIER OR DEEP
DTL(S)	DETAIL(S)
DWG(S)	DRAWING(S)
DWL(S)	DOWEL(S)
(E) OR EXIST	EXISTING
EA.	EACH
E.E.	EACH END
E.F.	EACH FACE
E.J.	EXPANSION JOINT
EL	ELEVATION
ELEV	ELEVATOR
ELEC	ELECTRICAL
EMBED	EMBEDMENT
ENGR	ENGINEER
E.O.P.	EDGE OF DECK
E.O.R.	ENGINEER OF RECORD
E.O.S.	EDGE OF SLAB
EQ	EQUAL
EQ SP	EQUALLY SPACED
EQUIP	EQUIPMENT
E.S.	EACH SIDE
E.W.	EACH WAY
E.W.B.	EACH WAY BOTTOM
EXIST/EX	EXISTING
EXP ANCHOR	EXPANSION ANCHOR
EXP	EXPANSION
EXT	EXTERIOR
FB	FLAT BAR
F.D.	FLOOR DRAIN
FDN	FOUNDATION
FIN. FL.	FINISH FLOOR
F.F.	FINISH FLOOR/ FAR FACE
FLG	FLANGE
FLR	FLOOR
F.F.E.	FINISH FLOOR ELEVATION
F.O.B.	FACE OF BRICK
F.O.	FACE OF
FRMG	FRAMING
F.S.	FAR SIDE
F.T.	FOOT OR FEET
FTG	FOOTING
GA	GAGE/GAUGE
GALV	GALVANIZED
GL	GLU-LAM
G.B.	GRADE BEAM
G.C.	GENERAL CONTRACTOR
GR	GRADE OR GRIND
GWB	GYPSUM WALL BOARD
H.D.	HOLDOWN
H.D. GALV	HOT DIPPED GALVANIZED
HK	HOOK
HORIZ	HORIZONTAL
HT	HEIGHT
HVAC	HEATING VENTILATION & COOLING
HSS	HOLLOW STRUCTURAL SHAPE
I.D.	INSIDE DIAMETER
INFO	INFORMATION
I.F.	INSIDE FACE
IN	INCH
INSUL	INSULATION
INT	INTERIOR
JST	JOIST
JT	JOINT
K	KIPS (1K=1000LBS)
L	ANGLE
LG	LENGTH
LB(S)	POUND(S)
LL	LIVE LOAD
LLBB	LONG LEGS BACK TO BACK
LLH	LONG LEG HORIZ
LLV	LONG LEG VERT.
LOC(S)	LOCATION(S) OR LOCATE
LONG	LONGITUDINAL
LSL	LAMINATED STRAND LUMBER
LT	LIGHT
LTS	TENSION LAP SPLICE LENGTH
LTWT	LIGHTWEIGHT
LVL	LEVEL OR LAMINATE VENEER LUMBER
MACH	MACHINE
MACH RM	MACHINE ROOM
MAS	MASONRY
MATL	MATERIAL
MAX	MAXIMUM
MECH	MECHANICAL
M.E.P.	MECHANICAL/ELECTRICAL/PLUMBING
MANUF	MANUFACTURER
MIN	MINIMUM
MISC	MISCELLANEOUS
ML	MICRO-LAM
M.O.	MASONRY OPENING
MTL	METAL
N	NORTH
N.I.C	NOT IN CONTRACT
NO OR #	NUMBER
NOM	NOMINAL
N-S	NORTH-SOUTH
N.S.	NEAR SIDE
N.T.S.	NOT TO SCALE
O.C.	ON CENTER
O.D.	OUTSIDE DIAMETER
O.F.	OUTSIDE FACE
O.H.	OPPOSITE HAND
OPNG	OPENING
OPP	OPPOSITE
P.A.F.	POWDER ACTUATED FASTENER
P.C.	PILE CAP
PCA	PORTLAND CONCRETE ASSOCIATION
PEN	PENETRATION
PERP	PERPENDICULAR
PL	PLATE
PLCS	PLACES
PLF	POUNDS PER LINEAR FOOT
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PREFAB	PREFABRICATION
PRELIM	PRELIMINARY
P.T.	PRESSURE TREATED
PVC	POLYVINYL CHLORIDE
QTY	QUANTITY
R	RADIUS
R.A.	RELIEVING ANGLE
RE OR REF	REFER TO REFERENCE
R.D.	ROOF DRAIN
REINF	REINFORCE(ING)(D)(MENT)
REQD	REQUIRED
REQMNTS	REQUIREMENT(S)
R.O.	ROUGH OPENING
RTU	ROOF TOP UNIT
S.C.	SLIP CRITICAL
SCHED	SCHEDULE
SECT	SECTION
S.F.	SQUARE FOOT
SHT	SHEET
SIM	SIMILAR
SLH	SHORT LEG HORIZONTAL
SLV	SHORT LEG VERTICAL
S	SOUTH
SP @	SPACE AT
SP	SPACE(S)
SPECS	SPECIFICATIONS
S.K.	SHEAR KEY
S.L.	SHEAR LUG
S.S.	STAINLESS STEEL
SSLT	SHORT SLOT
STD	STANDARD
STL	STEEL
STRUCT	STRUCTURAL
STIFF	STIFFENER
S.W.	SHEARWALL
SYM	SYMMETRICAL
T	TOP
T&B	TOP AND BOTTOM
T.C.F.	TOP CHORD EXTENSION
THK	THICK
TL	TOTAL LOAD
T.J.	TIE JOIST
T.O. OR T/	TOP OF
T.O.S. T/STL	TOP OF STEEL etc.
TRANS	TRANSVERSE
TYP	TYPICAL
U.N.O.	UNLESS NOTED OTHERWISE
VERT	VERTICAL
V.I.F.	VERIFY IN FIELD
W/	WITH
W/O	WITHOUT
WD	WIDTH OR WOOD
WF	WIDE FLANGE
W.P.	WORK POINT
WT	WEIGHT
W.W.F.	WELDED WIRE FABRIC

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STATE OF MAINE DANIEL W. BURNS No. 10910 LICENSED PROFESSIONAL ENGINEER

Owner

The Park Danforth

77 Stevens Ave, Portland, ME 03103

NO.	DESCRIPTION	DATE

N

CONTENT:
GENERAL NOTES

DRAWN BY:	A.P.P.
PROJECT NO: