

**City of Portland, Maine - Building or Use Permit Application**

389 Congress Street, 04101 Tel: (207) 874-8703, Fax: (207) 874-8716

Permit No: 07-0205	Issue Date:	CBL: 142 F004001
-----------------------	-------------	---------------------

Location of Construction: 915 FOREST AVE	Owner Name: BJFC LCC	Owner Address: 915 FOREST AVE	Phone:
Business Name:	Contractor Name: Benchmark	Contractor Address: 34 Thomas Dr. Westbrook	Phone: 2075917600
Lessee/Buyer's Name	Phone:	Permit Type: Foundation Only/Commercial	Zone: B-2

Past Use: Retail - Caravan Beads	Proposed Use: Retail - Caravan Beads- FOUNDATION & SLAB ONLY- for a 1 Story 12,308 sq ft addition to Caravan Beads	Permit Fee: \$970.00	Cost of Work: \$95,000.00	CEO District: 4
Proposed Project Description: FOUNDATION & SLAB ONLY- for a 1 Story 12,308 sq ft addition to Caravan Beads		FIRE DEPT: <input checked="" type="checkbox"/> Approved <input type="checkbox"/> Denied	INSPECTION: Use Group: <i>FOUNDATION ONLY</i> Type: <i>ONLY</i> 3/20/07 Signature: <i>[Signature]</i>	
		Signature: <i>Greg Cross</i>		

PEDESTRIAN ACTIVITIES DISTRICT (P.A.D.)	
Action: <input type="checkbox"/> Approved <input type="checkbox"/> Approved w/Conditions <input type="checkbox"/> Denied	Signature: _____ Date: _____

Permit Taken By: Idobson	Date Applied For: 02/26/2007	<b>Zoning Approval</b>
-----------------------------	---------------------------------	------------------------

<p>1. This permit application does not preclude the Applicant(s) from meeting applicable State and Federal Rules.</p> <p>2. Building permits do not include plumbing, septic or electrical work.</p> <p>3. Building permits are void if work is not started within six (6) months of the date of issuance. False information may invalidate a building permit and stop all work..</p>	<p>Special Zone or Reviews</p> <p><input type="checkbox"/> Shoreland <i>N/A</i></p> <p><input type="checkbox"/> Wetland</p> <p><input type="checkbox"/> Flood Zone <i>Panel 7 Zone X</i></p> <p><input type="checkbox"/> Subdivision</p> <p><input checked="" type="checkbox"/> Site Plan #2006-0160 Maj <input checked="" type="checkbox"/> Minor <input type="checkbox"/> MM <input type="checkbox"/></p> <p>Date: <i>2/27/07</i></p>	<p>Zoning Appeal</p> <p><input type="checkbox"/> Variance</p> <p><input type="checkbox"/> Miscellaneous</p> <p><input checked="" type="checkbox"/> Conditional Use</p> <p><input type="checkbox"/> Interpretation</p> <p><input checked="" type="checkbox"/> Approved</p> <p><input type="checkbox"/> Denied</p> <p>Date: <i>8/17/06</i></p>	<p>Historic Preservation</p> <p><input checked="" type="checkbox"/> Not in District or Landmark</p> <p><input type="checkbox"/> Does Not Require Review</p> <p><input type="checkbox"/> Requires Review</p> <p><input type="checkbox"/> Approved</p> <p><input type="checkbox"/> Approved w/Conditions</p> <p><input type="checkbox"/> Denied</p> <p>Date: <i>[Signature]</i></p>
---	---	--	---

**CERTIFICATION**

I hereby certify that I am the owner of record of the named property, or that the proposed work is authorized by the owner of record and that I have been authorized by the owner to make this application as his authorized agent and I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in the application is issued, I certify that the code official's authorized representative shall have the authority to enter all areas covered by such permit at any reasonable hour to enforce the provision of the code(s) applicable to such permit.

SIGNATURE OF APPLICANT \_\_\_\_\_ ADDRESS \_\_\_\_\_ DATE \_\_\_\_\_ PHONE \_\_\_\_\_

RESPONSIBLE PERSON IN CHARGE OF WORK, TITLE \_\_\_\_\_ DATE \_\_\_\_\_ PHONE \_\_\_\_\_

Applicant: Gravan Bexis

Date: 9/8/06

Address: 915 Forest Ave

C-B-L: 142-FO04

CHECK-LIST AGAINST ZONING ORDINANCE

Date - exist Deft.

# 2006-0168-2/27/07

Zone Location - B-2

Interior or corner lot -

Proposed Use/Work - 8/17/06 ZBA approved Conditional Use 5-0 (wholesale Distribution)

Sevage Disposal - City  
construct New Addition

Lot Street Frontage - 50' min - 201.93' along forest

Front Yard - None req - 4' or 5' at closest entry way

Rear Yard - 10' (doesn't abut a res. zone use) - 5' to closest entry

Side Yard - None req - (doesn't abut a res. zone use) - 6' or 10.5' scaled

Projections -

Width of Lot - N/A

Height - 45' max - 20.5'

Lot Area - 10,000<sup>sq</sup> max - 51,087<sup>sq</sup> given

Lot Coverage/Impervious Surface - 80% max - 78% given for existing  
73% given for New

Area per Family - N/A

Off-street Parking - removes majority of front yard parking

32 PKSP.  
Shown

Loading Bays - 1 - 14' x 50' OK

Site Plan - # 2006-0168

Shoreland Zoning/ Stream Protection - N/A

Flood Plains - panel 7 zone X

condition of use Granted  
8/17/06

retail	- 3,242 <sup>sq</sup>	7 SPCS	OK
wholesale	9,975 <sup>sq</sup>	10 SPCS	
office	4,587 <sup>sq</sup>	14 SPCS	
retail storage	(2,300 <sup>sq</sup> )		
business net retail	997 <sup>sq</sup>		
3,327		32 PKSP	

April 9, 2007  
06242

Francis O'Neill  
Benchmark Construction  
34 Thomas Drive  
Westbrook, Maine 04092

**Caravan of Beads, Building Corner Locations - 915 Forest Avenue, Portland, Maine**

Dear Francis:

As per your request, we have performed construction layout on the Caravan of Beads building expansion. I have attached an As-Built Sketch showing an overview of the construction which is reference herein.

- On March 30<sup>th</sup>, we established the proposed building corners and offset points at the existing ground level.
- On April 4<sup>th</sup>, we set nails in the partially excavated foundation hole (1-3) and set additional points for excavation (Point On Line [POL] nails between 3 & 4 and 4-8).
- On April 6<sup>th</sup>, we set nails (1-3 and POL between 3 & 4) on concrete footings and set nails in the excavated hole from there.
- On April 9<sup>th</sup>, we located foundation forms (1-3 and POL between 3 & 4) and set nails on the concrete footings (4-8)

The foundation forms located are within 1/2 inch of the proposed location and do not encroach into the 10 foot building setback.

I am faxing this letter to Amy Munson at the City of Portland (fax 874-8716) and to John Gerken at Benchmark's on-site office at 221-5104

If you have any questions or require additional services, please call.

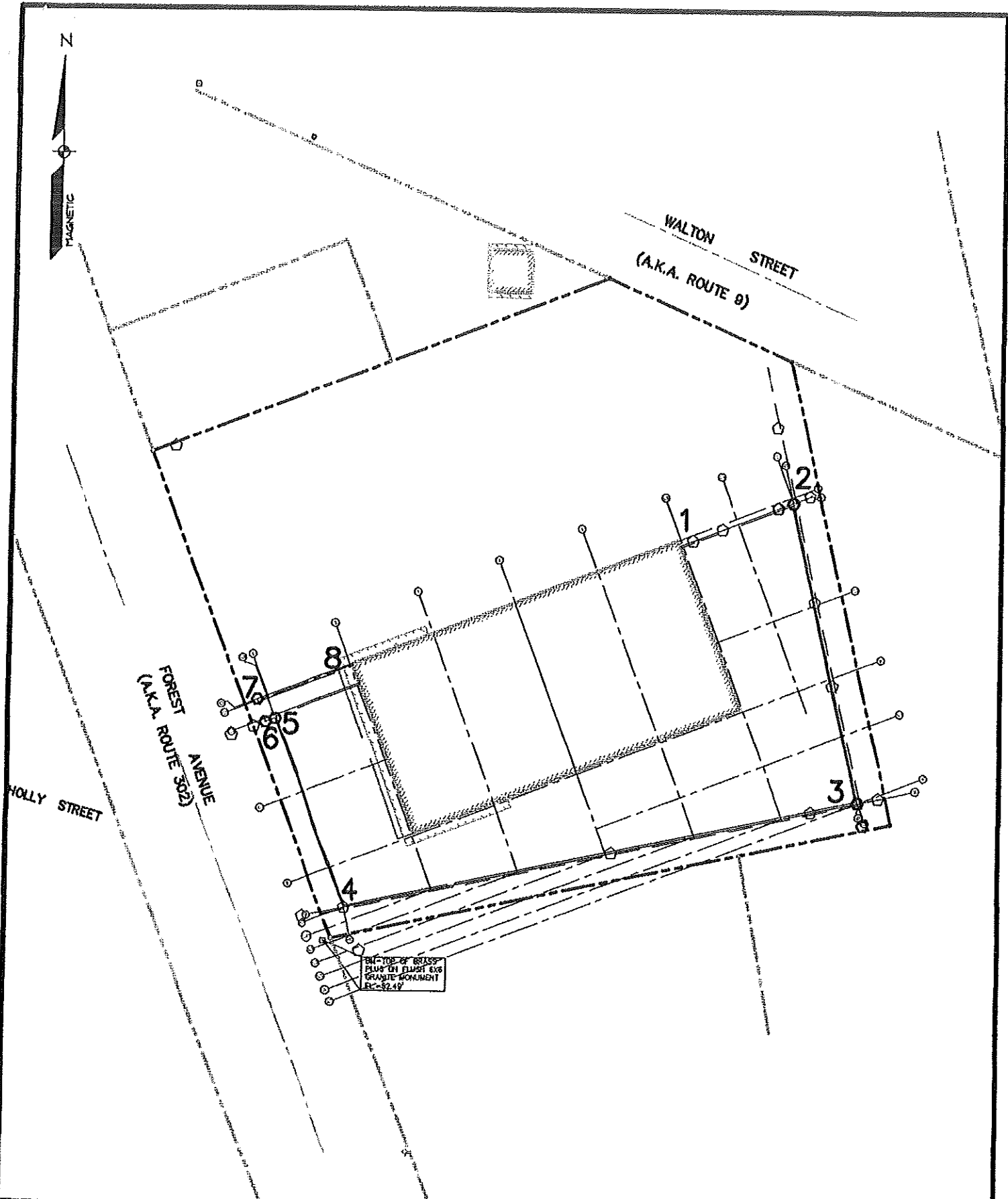
Sincerely,

SEBAGO TECHNICS, INC.

*Matthew W. Ek*  
Matthew W. Ek, PLS  
Project Manager

MWE:mwe/jc  
Enc.





**Sebago Technics**

Engineering Expertise You Can Build On  
 One Chobot Street  
 Westbrook, Me 04098-1339  
 Tel (207) 856-0277



**AS-BUILT SKETCH  
 OF CARAVAN OF BEADS**

LOCATION:  
 915 FOREST AVENUE  
 PORTLAND, MAINE

FOR:  
 BENCHMARK CONSTRUCTION  
 WESTBROOK, MAINE

SCALE: 1"=50'

DATE: 4/9/07

SHEET:  
 1 OF 1

FROM DESIGNER: Aaron C. Jones, P.E. / Structural Integrity, Inc.  
 DATE: 2/26/2007  
 Job Name: Caravan Beads Building Expansion Foundation  
 Address of Construction: 915 Forest Ave, Portland, ME 04103

**2003 International Building Code**

Construction project was designed to the building code criteria listed below:

Building Code & Year IBC 2003 Use Group Classification (s) IF Standard **SEFALL**  
 Type of Construction I/II **(2B) PER MURPHY CHANCE**  
 Will the Structure have a Fire suppression system in Accordance with Section 903.3.1 of the 2003 IRC yes  
 Is the Structure mixed use? No If yes, separated or non separated or non separated (section 302.3) \_\_\_\_\_  
 Supervisory alarm System? \_\_\_\_\_ Geotechnical/Soils report required? (See Section 1802.2) yes

*"B.O." INDICATES BY OTHERS*

**STRUCTURAL DESIGN CALCULATIONS**

B.O. Submitted for all structural members (106.1 - 106.11)

**DESIGN LOADS ON CONSTRUCTION DOCUMENTS**  
(1603)

Uniformly distributed floor live loads (7603.11, 1807)

Floor Area Use	Loads Shown
<u>Office</u>	<u>50 psf</u>
<u>public spaces</u>	<u>100 psf</u>
<u>Commercial</u>	<u>100 psf</u>
<u>Storage</u>	<u>125 psf</u>

**Wind loads (1603.1.4, 1609)**

B.O. Design option utilized (1609.1.1, 1609.6)  
100 Basic wind speed (1809.3)  
1.0 Building category and wind importance Factor,  $I_w$   
 (Table 1604.5, 1609.5)  
B Wind exposure category (1609.4)  
B.O. Internal pressure coefficient (ASCE 7)  
B.O. Component and cladding pressures (1609.1.1, 1609.6.2.2)  
B.O. Main force wind pressures (7603.1.1, 1609.6.2.1)

**Earth design data (1603.1.5, 1614-1623)**

B.O. Design option utilized (1614.1)  
1 Seismic use group ("Category") (Table 1604.5, 1616.2)  
.29 & .11 Spectral response coefficients,  $S_a$  &  $S_1$  (1615.1)  
C Site class (1615.1.5)

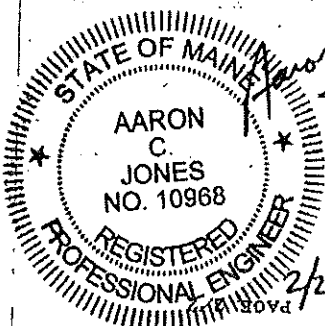
B.O. Live load reduction  
B.O. Roof live loads (1603.1.2, 1607.11)  
B.O. Roof snow loads (1603.7.3, 1608)  
50 Ground snow load,  $P_g$  (1608.2)  
43 If  $P_g > 10$  psf, flat-roof snow load  $p_f$   
1.0 If  $P_g > 10$  psf, snow exposure factor,  $C_e$   
1.0 If  $P_g > 10$  psf, snow load importance factor,  $I_s$   
1.0 Roof thermal factor,  $C_t$  (1608.4)  
N/A Sloped roof snowload,  $P_s$  (1608.4)  
B Seismic design category (1616.3)  
B.O. Basic seismic force resisting system  
 (Table 1617.6.2)  
B.O. Response modification coefficient,  $R$ , and  
 deflection amplification factor,  $C_d$  Table 1617.6.2  
B.O. Analysis procedure (1616.6, 1617.5)  
B.O. Design base shear (1617.4, 1617.5.1)

**Flood loads (1803.1.6, 1612) N/A**

\_\_\_\_\_ Flood Hazard area (1612.3)  
 \_\_\_\_\_ Elevation of structure

**Other loads N/A**

\_\_\_\_\_ Concentrated loads (1607.4)  
 \_\_\_\_\_ Partition loads (1607.5)  
 \_\_\_\_\_ Misc. loads (Table 1607.8, 1607.6.1, 1607.7,  
 1607.12, 1607.13, 1610, 1611, 2404)



*SEE PRE-ENGINEERED BUILDING DRAWINGS AND CALLS BY MURPHY FOR ADDITIONAL INFO.*

I have commence the review of the foundation permit for the above project and need the folowing info:

- 1) Please identify the use group of the structure (please include mixed uses and incidental uses)
- 2) Please identify the type of construction of the structure, keeping in mind that the existing structure impacts this determination.

The City Certification form was filled out with terms that do not match the 2003 IBC, I need this basic information to determine compliance withe area limitations, fire separation distances etc.

# CITY OF PORTLAND, MAINE

---

## ZONING BOARD OF APPEALS

Peter Coyne  
Philip Saucier  
Peter Thornton  
Kate Knox  
Catherine Alexander  
David Dore Secretary  
William Hall, Chair

August 28, 2006

David A. Kamila, P.E.  
Land Use Consultants Inc.  
966 Riverside St.  
Portland, ME 04103

### REVISED

RE: 915 Forest Avenue.  
CBL: 142 F004  
ZONE: B2

Dear Mr. Kamila:

As you know, at its August 17, 2006, meeting, the Board of Appeals voted 5-0 and granted your Conditional Use.

Enclosed please find the Board's decision, your invoice for the legal ad and abutters notices.

Your next step would be to submit a Site Plan Review (submitted to the Planning Department @ 874-8721) and a Building permit application, which I have enclosed.

Should you have any questions please feel free to contact me at 207-874-8701.

Sincerely,

Gayle Guertin  
Office Assistant

# CITY OF PORTLAND, MAINE

---

## ZONING BOARD OF APPEALS

### APPEAL AGENDA

The Board of Appeals will hold a public hearing on Thursday, August 17, 2006 at 6:30 p.m. on the second floor, Council Chambers, City Hall, 389 Congress Street, Portland, Maine to hear the following appeals:

To: City Clerk  
From: Marge Schmuckal, Zoning Administrator  
Date: August 21, 2006  
RE: Action taken by the Zoning Board of Appeals on August 17, 2006

The meeting was called to order at 6:38pm

**Roll call as follows:**

**Members Present:** Catherine Alexander (acting chair), Peter Thornton, David Dore (secretary), Peter Coyne and Philip Saucier.

**Members Absent:** Bill Hall and Kate Knox.

**1. New Business:**

**A. Conditional Use Appeal:**

915 Forest Avenue, BJFC, LLC owner, Tax Map #142 Block F Lot #004 in the B2 Community Business Zone is seeking a Conditional Use Appeal under section 14-183 (b) 3 of the City of Portland Zoning Ordinance. Appellant is requesting a Conditional Use to maximize the wholesale distribution area. The current building is 9,622 sq. ft. of which 3,061 sq. ft. is wholesale. Appellant would like to increase the size of the building to 12,009 sq. ft. and the wholesale distribution will be 9,975 sq. ft. The total area of the expanded building will be 21,631 sq. ft. Representing the Appeal is David A. Kamila, P.E. (Land Use Consultants, Inc). **Board voted 5-0 and granted the Conditional Use Appeal.**

**B. Practical Difficulty Variance Appeal:**

Shamrock Lane, Cushings Island, John H. & Livezey H. More, owners, Tax Map #106A Block A Lot #019 in the IR1 Island Residential Zone is seeking a Variance Appeal under section 14-145.5 (c) 2 of the City of Portland Zoning Ordinance. Appellants are requesting a Variance from the required 30 foot rear set back to a 13 foot 4 inches rear set back for the new one story structure. Representing the Appeal are the owners. **Board voted 5-0 and granted the Practical difficulty Appeal.**

**2. Other Business: None**

**3. Adjournment: 8:15pm**

**Enclosure:**

Agenda of August 3, 2006

Copy of Board's Decision

CC: Joseph Gray, City Manager

Alex Jaegerman, Planning Department

Lee Urban, Planning & Development Director

Aaron Shapiro, Housing & Neighborhood Services



**CITY OF PORTLAND, MAINE**  
**ZONING BOARD OF APPEALS**

B-2 Community Business Zone Wholesale Distribution Area Expansion:

**Conditional Use Appeal**

**DECISION**

Date of public hearing: 8/17/06

Name and address of applicant: BJFC, LLC  
915 Forest Ave  
Portland, ME

Location of property under appeal:  
915 Forest Ave.  
Portland, ME

**For the Record:**

Names and addresses of witnesses (proponents, opponents and others):

① David Canella  
of Land Use Consultants

Michael Charr  
Architect

② Barry Kersh - owner

DeLore Williams  
-Resident

Exhibits admitted (e.g. renderings, reports, etc.):

5. Landscaping and screening: The site shall be suitably landscaped for parking, surrounding uses and accessory site elements, including storage and solid waste receptacles where required by article IV (subdivisions) and article V (site plan).

Satisfied  Not Satisfied

SO

Reason:

6. Curbs and sidewalks: Curbs and sidewalks will be provided as specified in article VI of chapter 25.

Satisfied  Not Satisfied

SO

Reason:

7. Offstreet parking and loading: Offstreet parking and loading will be provided as required by division 20 and division 21 of this article: *For that part of every business, manufacturing, and industrial building not catering to retail trade and with floor area over three thousand (3,000) square feet: One (1) parking space for each one thousand (1,000) square feet of floor area, or major fraction thereof. (§14-332(l).)*

Satisfied  Not Satisfied

SO

Reason:

33 spaces

8. Front yard parking: There shall be no off-street parking in the front yard between the street line and the required minimum setback line. Where existing buildings exceed the minimum front yard setback, a maximum of ten (10) percent of the total parking provided on the site may be located between the principal structure and the street.

Satisfied  Not Satisfied

Reason:

13. Noise: Except as provided in 14-183(1)(iii)(2) (relating to Drive-throughs), the volume of sound, measured by a sound level meter with frequency weighting network (manufactured according to standards prescribed by the American Standards Association), generated shall not exceed sixty (60) decibels on the A scale between 7:00 a.m. and 9:00 p.m. and fifty-five (55) decibels on the A scale between 9:00 p.m. and 7:00 a.m., on impulse (less than one (1) second), at lot boundaries, excepting air raid sirens and similar warning devices.

SO

Satisfied  Not Satisfied

Reason: Wholesale  
9-5 M-F  
Retail 10-7 M-Sat

14. Vibration and heat: Vibration inherently and recurrently generated and heat shall be imperceptible without instruments at lot boundaries.

Satisfied  Not Satisfied

SO Reason:

15. Glare, radiation or fumes: Glare, radiation or fumes shall not be emitted to an obnoxious or dangerous degree beyond lot boundaries.

SO

Satisfied  Not Satisfied

Reason:

16. Smoke: Smoke shall not be emitted at a density in excess of twenty (20) percent opacity level as classified in Method 9 (Visible Emissions) of the Opacity Evaluation System of the U.S. Environmental Protection Agency.

SO

3. Such impact differs substantially from the impact which would normally occur from such a use in that zone.

5-0

Yes

No

Reason:

Conclusion: (check one)

Option 1: The Board finds that all of the relevant standards (1 through 17) described in section A above have been satisfied and that not all of the conditions (1 through 3) described in section B above are present, and therefore GRANTS the application.

Option 2: The Board finds that all of the relevant standards (1 through 17) described in section A above have been satisfied, and that while not all of the conditions (1 through 3) described in section B above are present, certain additional conditions must be imposed to minimize adverse effects on other property in the neighborhood, and therefore GRANTS the application SUBJECT TO THE FOLLOWING CONDITIONS:

Option 3: The Board finds that not all of the relevant standards (1 through 17) described in section A above have been satisfied and/or that all of the conditions (1 through 3) described in section B above are present, and therefore DENIES the application.

Dated:

8/17/06

Cathy D. Allen

Board Chair

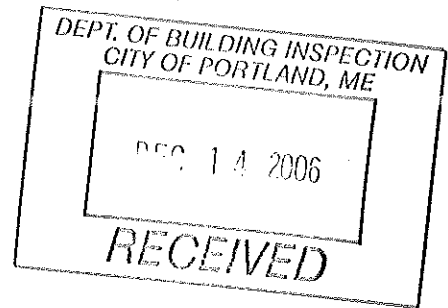
CITY OF PORTLAND, MAINE  
PLANNING BOARD

---

Kevin Beal, Chair  
Michael Patterson, Vice Chair  
Bill Hall  
Lee Lowry III  
Shalom Odokara  
David Silk  
Janice E. Tevanian

November 29, 2006

Barry Kahn  
BJFC, LLC  
c/o Caravan Beads, Inc.  
915 Forest Ave  
Portland, ME 04103



RE: **915 Forest Ave; Caravan Beads**  
**Application ID Number 2006-0168; CBL 142 F004001**

To Whom It May Concern:

On November 28, 2006 the Portland Planning Board considered the proposal to construct an addition to an existing building at 915 Forest Avenue. Approval was granted for the project by the following motions:

On the basis of plans submitted by the applicant and on the basis of information contained in Planning Report #65-06 relevant to standards for site plan regulations, and other findings as follows:

1. Upon the recommendation of the City's Transportation Engineer, the Planning Board **does** waive the City's Technical Standards for parking lots to allow compact parking spaces of 7½ ft by 16 ft for eleven (11) of the thirty-two parking (32) spaces, specifically those 11 spaces located in front of the trees adjacent to the property owned by Darlene A. Williams. The waiver is granted in order to ensure the presentation of the tree depicted on the site plan.
2. The Planning Board voted 6-0 (Lowry absent) that the plan is in conformance with the site plan standards of the land use code, subject to the following conditions of approval:
  - i. The applicant shall install the full cutoff visor for the exterior wall pack lights as proposed.
  - ii. The front window facing Forest Ave shall be of a shade no greater than that shown at the public hearing, namely "Solargray" by PPG Industries, which has visible light transmittance of 40%.

Please note the following provisions and requirements for all site plan approvals:

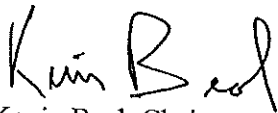
1. Where submission drawings are available in electronic form, the applicant shall submit any available electronic Autocad files (\*.dwg), release 14 or greater, with seven (7) sets of the final plans.

2. A performance guarantee covering the site improvements as well as an inspection fee payment of 2.0% of the guarantee amount and 7 final sets of plans must be submitted to and approved by the Planning Division and Public Works prior to the release of the building permit. If you need to make any modifications to the approved site plan, you must submit a revised site plan for staff review and approval.
3. The site plan approval will be deemed to have expired unless work in the development has commenced within one (1) year of the approval or within a time period agreed upon in writing by the City and the applicant. Requests to extend approvals must be received before the expiration date.
4. A defect guarantee, consisting of 10% of the performance guarantee, must be posted before the performance guarantee will be released.
5. Prior to construction, a pre-construction meeting shall be held at the project site with the contractor, development review coordinator, Public Work's representative and owner to review the construction schedule and critical aspects of the site work. At that time, the site/building contractor shall provide three (3) copies of a detailed construction schedule to the attending City representatives. It shall be the contractor's responsibility to arrange a mutually agreeable time for the pre-construction meeting.
6. If work will occur within the public right-of-way such as utilities, curb, sidewalk and driveway construction, a street opening permit(s) is required for your site. Please contact Carol Merritt at 874-8300, ext. 8828. (Only excavators licensed by the City of Portland are eligible.)

The Development Review Coordinator must be notified five (5) working days prior to date required for final site inspection. The Development Review Coordinator can be reached at the Planning Division at 874-8632. Please make allowances for completion of site plan requirements determined to be incomplete or defective during the inspection. This is essential as all site plan requirements must be completed and approved by the Development Review Coordinator prior to issuance of a Certificate of Occupancy.

If there are any questions, please contact Shukria Wiar at 756-8083 or [shukriaw@portlandmaine.gov](mailto:shukriaw@portlandmaine.gov).

Sincerely,

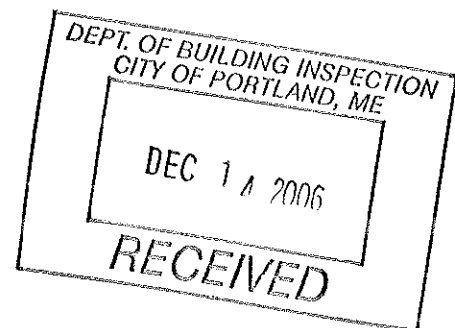


Kevin Beal, Chair  
Portland Planning Board

cc: Lee D. Urban, Planning and Development Department Director  
Alexander Jaegerman, Planning Division Director  
Barbara Barhydt, Acting Development Review Services Manager  
, Planner/Senior Planner

Jay Reynolds, Development Review Coordinator  
Marge Schmuckal, Zoning Administrator  
Michael Bobinsky, Public Works Director  
Jeanie Bourke, Inspections Division  
Eric Labelle, City Engineer  
Bill Clark, Public works  
Jim Carmody, Transportation Manager  
Jeff Tarling, City Arborist  
Penny Littell, Associate Corporation Counsel  
Captain Greg Cass, Fire Prevention  
Assessor's Office  
Approval Letter File

Attachment: Planning Board Report #65-06





# PORTLAND MAINE

*Strengthening a Remarkable City, Building a Community for Life* <sup>3</sup> [www.portlandmaine.gov](http://www.portlandmaine.gov)

Planning and Development Department  
Lee D. Urban, Director

Planning Division  
Alexander Jaegerman, Director

February 27, 2007

Mr. Francis O'Neill  
Benchmark Construction  
34 Thomas Drive  
Westbrook, ME 04092

RE: 915 Forest Ave., Caravan Beads, (Project # 2006-0168), (CBL 142 F004001)

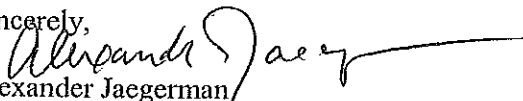
Dear Mr. O'Neill:

On November 28, 2006, the Portland Planning Authority granted approval for site plan at 915 Forest Ave. This letter shall serve as permission to start preliminary site work associated with this approved plan. The extent of work shall include only the following until the issuance of the foundation and/or building permits.

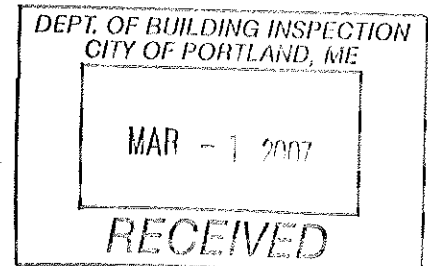
1. Removal of existing pavement.
2. Site grubbing and grading.
3. Trenching for the installation of drainage utilities.
4. Erosion and Sedimentation control shall be established prior to soil disturbance, and shall be done in accordance with Best Management Practices, Maine Department of Environmental Protection Technical and Design Standards and Guidelines.
5. Existing vegetation shall be conserved in areas shown on this site. Fencing or other protective barriers shall be erected outside the drip-line of individual trees designated for preservation prior to the onset of construction.

The approval is based on the submitted site plan. If you need to make any modifications to the approved site plan, you must submit a revised site plan for staff review and approval. If there are any questions, please contact the Planning Staff.

Sincerely,

  
Alexander Jaegerman  
Chief Planner

cc: Inspections Department  
Barbara Barhydt, Development Review Services Manager  
— Marge Schmuckal, Zoning Administrator  
Phil DiPierro, Development Review Coordinator  
Penny Littell, Corporation Counsel  
Approval Letter File



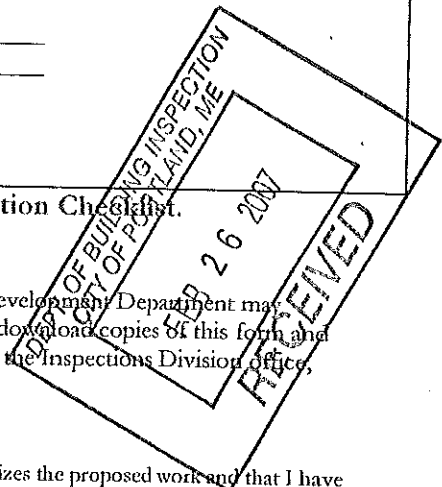




# General Building Permit Application

If you or the property owner owes real estate or personal property taxes or user charges on any property within the City, payment arrangements must be made before permits of any kind are accepted.

Location/Address of Construction: <u>915 Forest Avenue, Portland, Maine</u>		
Total Square Footage of Proposed Structure <u>12,308</u>	Square Footage of Lot <u>51,087</u>	
Tax Assessor's Chart, Block & Lot Chart# <u>142</u> Block# <u>F</u> Lot# <u>4</u>	Owner: <u>BJFL, LLC</u>	Telephone: <u>800-230-8941</u> <u>415-9562</u>
Lessee/Buyer's Name (If Applicable)	Applicant name, address & telephone: <u>Benchmark</u> <u>34 Thomas Dr</u> <u>Westbrook, ME 04092</u> <u>591-7600</u>	Cost Of Work: <del>\$628,784.00</del> Fee: <u>\$95,000</u> C of O Fee: <u>\$970</u>
Current legal use (i.e. single family) <u>business - Retail Caravan Beads</u>		
If vacant, what was the previous use? _____		
Proposed Specific use: _____		
Is property part of a subdivision? <u>NO</u> If yes, please name _____		
Project description: <u>Foundation &amp; Slab only for addition to Caravan Beads</u> <u>1 Story 12,308 sq Ft.</u>		
Contractor's name, address & telephone: <u>Benchmark</u> <u>34 Thomas Dr</u> <u>Westbrook, ME 04092</u>		
Who should we contact when the permit is ready: <u>Francis O'Neil</u>		
Mailing address: _____ Phone: <u>591-7600</u>		



Please submit all of the information outlined in the Commercial Application Checklist. Failure to do so will result in the automatic denial of your permit.

In order to be sure the City fully understands the full scope of the project, the Planning and Development Department may request additional information prior to the issuance of a permit. For further information or to download copies of this form and other applications visit the Inspections Division on-line at [www.portlandmaine.gov](http://www.portlandmaine.gov), or stop by the Inspections Division office, room 315 City Hall or call 874-8703.

I hereby certify that I am the Owner of record of the named property, or that the owner of record authorizes the proposed work and that I have been authorized by the owner to make this application as his/her authorized agent. I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in this application is issued, I certify that the Code Official's authorized representative shall have the authority to enter all areas covered by this permit at any reasonable hour to enforce the provisions of the codes applicable to this permit.

Signature of applicant: <u>[Signature]</u>	Date: <u>February 26, 2007</u>
--	--------------------------------

This is not a permit; you may not commence ANY work until the permit is issued.



# Certificate of Design

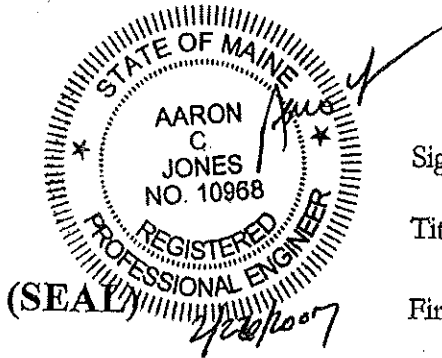
Date: 2/26/2007

From: Aaron C. Jones, P.E.

These plans and / or specifications covering construction work on:

Foundation for pre-engineered Building By Others, Murox,  
at 915 Forest Ave, Portland, ME 04103

Have been designed and drawn up by the undersigned, a Maine registered ~~Architect~~ / Engineer according to the *2003 International Building Code* and local amendments.



Signature: Aaron C. Jones

Title: President

Firm: Structural Integrity, Inc.

Address: 77 Oak Street  
Portland, ME 04101

Phone: 207-774-4614

For more information or to download this form and other permit applications visit the Inspections Division on our website at [www.portlandmaine.gov](http://www.portlandmaine.gov)

Project: Caran Beads  
Date Prepared: 2/26/2007

## Structural Statement of Special Inspections

Project: *Caravan Beads Building Expansion*

Location: *915 Forest Ave, Portland, Maine*

Owner: *Barry Kahn*

This *Statement of Special Inspections* encompass the following discipline: Structural, Foundation Only!

This *Statement of Special Inspections* is submitted as a condition for permit issuance in accordance with the Special Inspection and Structural Testing requirements of the Building Code. It includes a schedule of Special Inspection services applicable to this project as well as the name of the Structural Special Inspection Coordinator (SSIC) and the identity of other approved agencies to be retained for conducting these inspections and tests.

The Structural Special Inspection Coordinator shall keep records of all Structural inspections and shall furnish inspection reports to the Building Code Official (BCO) and the Structural Registered Design Professional in Responsible Charge (SRDP). Discovered discrepancies shall be brought to the immediate attention of the Contractor for correction. If such discrepancies are not corrected, the discrepancies shall be brought to the attention of the Building Official and the Structural Registered Design Professional in Responsible Charge. The Special Inspection program does not relieve the Contractor of his or her responsibilities.

Interim reports shall be submitted to the Building Official and the Structural Registered Design Professional in Responsible Charge at an interval determined by the SSIC and the BCO.

A *Final Report of Special Inspections* documenting completion of all required Special Inspections, testing and correction of any discrepancies noted in the inspections shall be submitted to the BCO prior to issuance of a Certificate of Use and Occupancy.

Job site safety and means and methods of construction are solely the responsibility of the Contractor.

Interim Report Frequency:  Upon request of Building Official \_\_\_\_\_ or  per attached schedule.

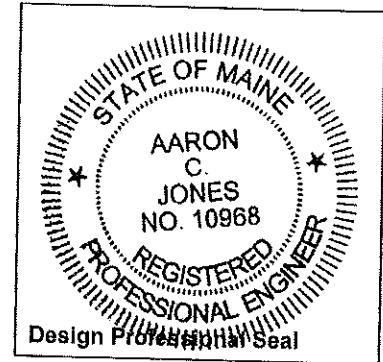
Prepared by:

*Aaron C. Jones, P.E.*

(type or print name of the Structural Registered Design Professional in Responsible Charge)

*Aaron C. Jones*  
Signature

*2/26/2007*  
Date



Owner's Authorization:

Building Code Official's Acceptance:

Signature

Date

Signature

Date

Project: Caran Beads  
 Date Prepared: 2/26/2007

## Structural Statement of Special Inspections (Continued)

### List of Agents

Project: *Caravan Beads Building Expansion*

Location: *915 Forest Ave, Portland, Maine*

Owner: *Barry Kahn*

This Statement of Special Inspections encompass the following discipline: Structural, **(FOUNDATION ONLY!)**

(Note: Statement of Special Inspections for other disciplines may be included under a separate cover)

This Statement of Special Inspections / Quality Assurance Plan includes the following building systems:

- Soils and Foundations
- Cast-in-Place Concrete
- Precast Concrete System
- Masonry Systems
- Structural Steel
- Wood Construction
- Special Cases

Special Inspection Agencies	Firm	Address, Telephone, e-mail
1. STRUCTURAL Special Inspections Coordinator (SSIC) To Be Determined		
2. Special Inspector (SI 1) Aaron C. Jones	<i>Structural Integrity, Inc</i>	<i>77 Oak Street Portland, ME 04101 207-774-4614 aaron@structuralinteg.com</i>
3. Special Inspector (SI 2) To Be Determined		
4. Testing Agency (TA 1) To Be Determined		
5. Testing Agency (TA 2)		
6. Other (O1)		

Note: The inspectors and testing agencies shall be engaged by the Owner or the Owner's Agent, and not by the Contractor or Subcontractor whose work is to be inspected or tested. Any conflict of interest must be disclosed to the Building Official, prior to commencing work.

Project: Caran Beads  
Date Prepared: 2/26/2007

## Structural Statement of Special Inspections (Continued)

### Final Report of Special Inspections (SSIC/SI 1)

[To be completed by the Structural Special Inspections Coordinator (SSIC/SI 1). Note that all Agent's Final Reports must be received prior to issuance.]

Project:  
Location:  
Owner:  
Owner's Address:

Architect of Record:

(name)

(firm)

Structural Registered Design  
Professional in Responsible Charge:

(name)

(firm)

To the best of my information, knowledge and belief, the Special Inspections required for this project, and itemized in the *Statement of Special Inspections* submitted for permit, have been performed and all discovered discrepancies have been reported and resolved.

Interim reports submitted prior to this final report form a basis for and are to be considered an integral part of this final report.

Respectfully submitted,  
Structural Special Inspection Coordinator

\_\_\_\_\_  
(Type or print name)

\_\_\_\_\_  
(Firm Name)

\_\_\_\_\_  
Signature

\_\_\_\_\_  
Date

*Licensed Professional Seal*

Project: Caran Beads  
Date Prepared: 2/26/2007

**Structural Statement of Special Inspections (Continued)**  
**Special Inspector's/Agent's Final Report**

Project:  
Special Inspector or  
Agent:

Designation:

(name)

(firm)

To the best of my information, knowledge and belief, the Special Inspections or testing required for this project, and designated for this Inspector/Agent in the *Statement of Special Inspections* submitted for permit, have been performed and all discovered discrepancies have been reported and resolved.

Interim reports submitted prior to this final report form a basis for and are to be considered an integral part of this final report.

Respectfully submitted,  
Special Inspector or Agent:

\_\_\_\_\_  
(Type or print name)

Signature

\_\_\_\_\_  
Date

\_\_\_\_\_  
*Licensed Professional Seal or  
Certification Number*

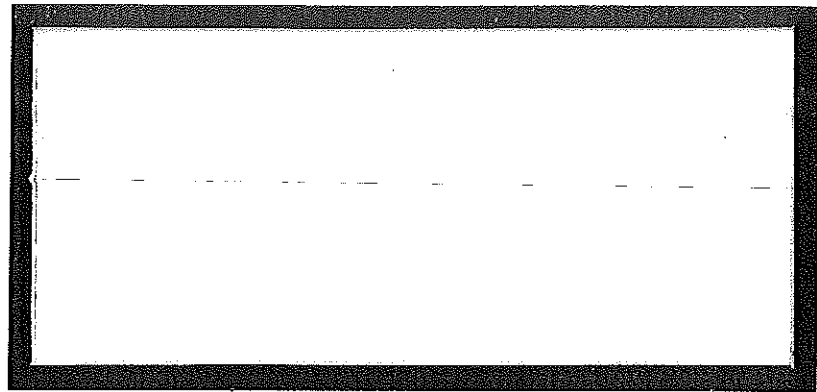
Project: Caran Beads  
 Date Prepared: 2/26/2007

**Structural Schedule of Special Inspections**  
**CONCRETE CONSTRUCTION**

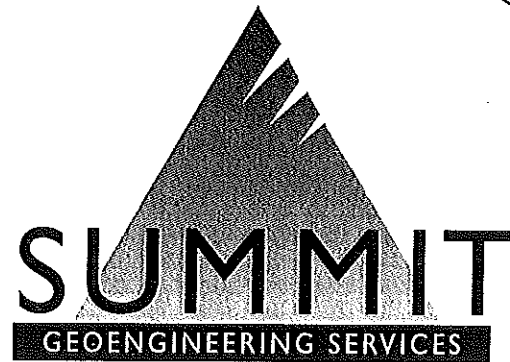
VERIFICATION AND INSPECTION	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS	AGEN T	AGENT QUALIFICATION	TASK COMPLETED
IBC Section 1704.4						
1. Inspection of reinforcing steel, including prestressing tendons, and placement	Y	P	ACI 318: 3.5, 7.1-7.7	2	PE/SE or EIT	
2. Inspection of reinforcing steel welding in accordance with Table 1704.3, Item 5B	N/A	N	Welding of Reinf Not Allowed		AWS-CWI	
3. Inspect bolts to be installed in concrete prior to and during placement of concrete where allowable loads have been increased	N	C	IBC 1912.5		PE/SE or EIT	
4. Verifying use of required design mix	Y	P	ACI 318: Ch 4, 5.2-5.4	2	PE/SE or EIT	
5. At time fresh concrete is sampled to fabricate specimens for strength test, perform slump and air content test and temperature	Y	C	ASTM C 172 ASTM C 31 ACI 318: 5.6, 5.8	4	ACI-CFTT or ACI-STT	
6. Inspection of concrete and shotcrete placement for proper application techniques	Y	C	ACI 318: 5.9, 5.10	2	PE/SE or EIT	
7. Inspection for maintenance of specified curing temperature and techniques	Y	P	ACI 318: 5.11-5.13	2	PE/SE or EIT	
8. Inspection of Prestressed Concrete	N/A					
a. Application of prestressing force	N	C	ACI 318: 18.20		PE/SE or EIT	
b. Grouting of bonded prestressing tendons in seismic force resisting system	N	C	ACI 318: 18.18.4		PE/SE or EIT	
9. Erection of precast concrete members	N	P	ACI 318: Ch 16		PE/SE or EIT	
10. Verification of in-situ concrete strength, prior to stressing of tendons in post-tensioned concrete and prior to removal of shores and forms beams and structural slabs	Y	P	ACI 318: 6.2		ACI-STT	

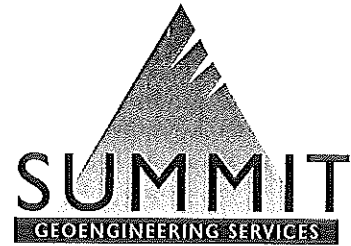






DEPT. OF BUILDING INSPECTION  
CITY OF PORTLAND, ME  
FEB 27 2007  
RECEIVED





## Geotechnical Report

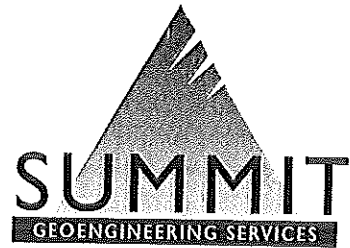
Proposed Building Addition  
Caravan Beads  
Portland, Maine

Prepared for:

Caravan Beads

Prepared by:

Summit Geoengineering Services  
Project #17088  
August 2006



August 10, 2006  
Summit #17088

Barry Kahn  
Caravan Beads  
915 Forest Avenue  
Portland, Maine 04103

Reference: Geotechnical Investigation  
Building Addition, 915 Forest Avenue, Portland, Maine

Dear Barry;

We have completed the geotechnical investigation in connection with the construction of a new building addition to the existing Caravan Beads in Portland, Maine. Our scope of services included observing the drilling of 6 test borings and 12 probes at the proposed site and preparing this report summarizing our findings and geotechnical recommendations.

### 1.0 Project and Site

The site is located near the corner of Walton Street and Forest Avenue in Portland, Maine. Currently the existing facility and surrounding pavement areas reside at the site. The project consists of constructing a 11,464 square foot addition to the existing building. The addition will be a steel-framed single story structure with a conventional spread footing with slab-on-grade foundation.

Existing grades at the site around the perimeter of the existing building range 83 to 87 feet in elevation. Existing grades rise from elevation 87 feet to 90 feet at a small knoll located northeast of the existing building and rise from elevation 87 feet to 92 feet within an existing parking lot area located north of the existing building. We understand the proposed building addition will match the existing building finish floor elevation of 87.71 feet. Based on this and the existing site grading, minimal cut up to 4 feet of fill will be required to construction to proposed addition.

### 2.0 Exploration

Summit observed the subsurface conditions at the site with the drilling of 6 test borings and 12 probes on July 20, 2006. The test borings and probes were drilled to a depth of 0.5 to 7.2 feet using a Diedrich D50 ATV provided and operated by Northern Test Boring under contract to Summit. The test boring and probe locations were paced from existing site features by Summit.

Ground surface elevations at the boring and probe locations were interpolated from a topographic site plan prepared by Sebago Technics. Locations of the explorations shown Figure 1, a subsurface conditions summary table, and logs of the borings and probes are all attached at the end of this report. Due to some uncertainty of exact locations for onsite underground utilities, Dig Smart of Maine was contracted to mark existing utilities and verify that boring locations were free from subsurface obstructions.

One sample, #17087-1, of the existing fill collected from a depth of 0 to 2 feet at boring B-3, was tested for grain size analysis in accordance with ASTM D422. A copy of the lab results is attached at the end of this report.

### 3.0 Subsurface Conditions

The soil at the site generally consists of 1.75 to 2.5 inches of bituminous *pavement* (where encountered) overlying *fill*, over relatively shallow *bedrock*. *Weathered bedrock* was encountered at boring B-1, and probes P-1, P-4, P-5, and P-9 with a thickness range of 4 to 18 inches. At boring B-6, silty clay *glacial marine deposits* were encountered beneath the pavement layer to a depth of 5.5 feet.

The *fill* at the site consisted of both imported sandy soil and reworked native soil. In general, the fill at the site consisted of orange brown, brown, and/or dark brown sand with some to trace silt and gravel and is visually classified as SP, SM, and SP-SM soil in accordance with the Unified Soil Classification System (USCS). The fill ranged from 6 inches to 6 feet in thickness and was generally damp. SPT N-values for the fill ranged from 8 to 28 blows per foot (bpf) and averaged 16 psf indicating compact soil conditions. At borings B-2 and probe P-12, occasional brick and concrete debris was encountered. Gradation results from a sample collected from boring B-3 at a depth of 0 to 2 feet contained approximately 7.9 percent fine material (percent weight finer than #200, 0.075 mm particle size).

*Glacial marine deposits* encountered at boring B-6, underlying pavement to a depth of 5.5 feet, consisted of olive and mottled silty clay and is visually classified as CL soil in accordance with the USCS. An SPT-N value for the glacial marine deposits was 12 bpf indicating stiff soil conditions. The glacial marine deposits were generally damp.

*Bedrock* was encountered at all boring and probe locations. Bedrock was observed at depths ranging from 0.5 to 7.2 feet, elevations 87.5 to 78.3 feet, respectively. Weathered bedrock was encountered at boring B-1, and probes P-1, P-4, P-5, and P-9 with a thickness range of 4 to 18 inches. The Bedrock Geologic Map by the Maine Geologic Survey of Portland indicates that the bedrock within the site location is part of the Vassalboro Formation (OSv) consisting of calcareous sandstone, interbedded sandstone and impure limestone. Samples of the bedrock were not obtained for verification.

*Groundwater* seepage was not observed in any of the boring or probe locations. Weathered bedrock was encountered at B-1, P-1, P-4, P-5, and P-9 indicating seasonal run-off along the bedrock surface during wet periods likely occurs.

#### 4.0 Foundation Recommendations

##### *A. General*

In general, the soils at the site are suitable to support the proposed building addition using conventional spread footings. Based on the anticipated finish floor elevation, the footings will be constructed on imported fill, existing fill, glacial till, and/or bedrock. In general, bedrock removal is not anticipated, except at the northeastern corner of the site where a ledge knoll is present. General blasting recommendations are attached at the end of this report.

##### *B. Allowable Bearing Pressure*

The interior isolated and exterior continuous footings for the building addition can be proportioned using the following allowable bearing pressures for the various soil conditions:

Soil Type	Allowable Bearing Pressure	Estimated Settlement
Imported Granular Fill	4,000 PSF	Less than 1 inch
Glacial Marine/Existing Fill	4,000 PSF	Less than 1 inch
Intact Bedrock or Crushed Stone Cushion over Intact Bedrock	20,000 PSF	Negligible

The bearing pressures and associated settlements are based on the following conditions:

- The existing pavement is removed, where applicable, and the ground surface is proof rolled prior to placing fill or excavation for footings for sandy subgrade soil conditions. Proof rolling should consist of a minimum of three passes in a north-south direction and then three passes in an east-west direction using a large (5 ton at drum static weight) vibratory roller.
- The base of excavations in glacial marine silty clay soils (encountered at boring B-6) is protected from disturbance, especially when wet. We recommend that any surface water during wet periods be diverted away from the footing excavations. Should the glacial marine silty clay subgrade soil become soft from water and disturbance, we recommend over-excavation to remove soft soils and replacement with filter fabric and/or crushed stone for stabilization.
- Fill required beneath the building consists of a minimum of 12 inches of Structural Backfill over Granular Borrow (where required), both compacted to a minimum of 95 percent of its maximum dry density, determined in accordance with ASTM D1557, or crushed stone.
- Intact bedrock assumes all weathered or loose material is removed

The depth of the footing subgrade may vary depending on the locations of the bedrock. Low spots can be leveled with crushed stone, sand, or flowable fill. We recommend that a transition zone be designed where the footing subgrade changes from bedrock to soil. The transition zone should consist of a minimum of 12 inches of structural fill beneath the footing at the bedrock/soil interface extending to 0 inches 10 feet from the interface. The footing can be constructed directly on the bedrock at 10 from the interface. We also recommend that a construction or control joint be installed at or near the bedrock/soil interface.

*C. Frost Protection*

Based on the required frost protection depth, the frost walls for the apartment building should be constructed at a minimum depth of 4 feet. This frost penetration depth is based on a design air-freezing index of 1,250 degree days for the Portland area. No minimum frost depth is required for footings constructed on hard bedrock, as long as surface water is drained from the footing base or diverted to prevent ponding adjacent to the footing. We recommend that the exterior of the foundation walls be backfilled with soil meeting the following gradation specification:

FOUNDATION BACKFILL	
Sieve Size	Percent finer
3 inch	100
No. 40	0 to 70
No. 200	0 to 5

The maximum particle size should be limited to 6 inches. The Foundation Backfill should be compacted to a minimum of 95 percent of its maximum dry density, determined in accordance with ASTM D1557.

*D. Building Slab*

We recommend the addition slab be constructed on a minimum 12-inch thick layer of Structural Backfill soil. If portions of the slab are on bedrock, the Structural Fill thickness can be reduced to 6 inches. The maximum particle size should be limited to 6 inches and meet the following gradation specifications passing the 3-inch sieve:

STRUCTURAL BACKFILL	
Sieve Size	Percent finer
3 inch	100
1/4 inch	0 to 70
No. 200	0 to 10

**Reference:** MDOT Specification 703.20, Gravel Borrow

The Structural Backfill should be placed in 8 to 12-inch lifts and should be compacted to 95 percent of its maximum dry density determined in accordance with ASTM D1557.

Backfill required beneath the Structural Fill should consist of Granular Borrow (MDOT 703.19). The portion of Granular Borrow soil passing the 3-inch sieve should meet the following:

GRANULAR BORROW	
Sieve Size	Percent finer
3"	100
No. 40	0 to 70
No. 200	0 to 20

Reference: MDOT Specification 703.19, Granular Borrow

The maximum particle size should be limited to 6 inches. Granular Borrow should be placed in a maximum of 12-inch lifts, and should be compacted to 95 percent, in accordance with ASTM D1557. Sandy soil should be proof rolled prior to placing Granular Borrow

For the conditions described above, the slabs can be designed using a subgrade modulus of 250 pci. Where slabs are constructed on 6 inches of soil over bedrock, the slab subgrade can be considered rigid for design purposes.

#### *E. Groundwater Control*

Groundwater is anticipated to be below exterior footing elevations. Based on this, perimeter underdrains are not strictly necessary. However, groundwater could be present within seams in any bedrock cut areas. Should groundwater become present within these cut areas, we recommend that perimeter underdrains be installed. It is also generally good practice to install underdrains to account for unobserved conditions or future changes in local hydrogeology.

Underdrains, if used, should consist of 4 inch rigid perforated PVC surrounded by 6 inches of ¾ inch crushed stone wrapped in geotextile filter fabric. We recommend that the ground surface slope away from the building and the subsurface be sealed with pavement or a low permeability soil (existing silty clay) to reduce infiltration of roof and building wall water runoff into the Foundation Backfill.

#### *F. Seismic Design*

The soils at the site are categorized as site classification C for foundations constructed on soil and as site classification B for foundations constructed on bedrock in accordance with the 2003 International Building Code.

### 5.0 Pavement Section Recommendations

The subgrade soils beneath pavement areas will consist of the existing sandy fill and reworked native soils. The mean annual freezing index for the Portland area is estimated to be 850-degree days with an annual frost penetration depth estimated to be 36 inches. The site will primarily receive passenger cars and trucks with some light delivery trucks. Based on this, we recommend a minimum total pavement section thickness of 50% of the mean annual frost depth (18 inches).

We further recommend that the pavement sections consist of the following materials:

MATERIAL	THICKNESS (in)	SPECIFICATION
Asphalt Surface Coarse	1	MDOT 703.09 Grading D, (or Superpave 9.5mm)
Asphalt Binder Coarse	2	MDOT 703.09 Grading B, (or Superpave 12mm)
Base Soil	3	MDOT 703.06 Type A
Subbase Soil	12	MDOT 703.06 Type D

The material specifications are referenced to the 1995 Maine Department of Transportation Standard Specifications for Highways and Bridges and Maine Department of Transportation Standard Specifications, Revision of 2002.

All base and subbase soil should be placed in 6 to 12 inch lifts and be compacted to a minimum of 95 percent of its maximum dry density, determined in accordance with ASTM D1557, Modified Proctor Density. Backfill beneath the Type D subbase soil, if required, should consist of Granular Borrow, compacted to a minimum of 95 of its maximum dry density.

We recommend that all existing sandy and silty glacial marine soil be proof-rolled prior to placing granular borrow or subbase soil. Proof rolling should consist of a minimum of three passes in a north-south direction and three passes in an east-west direction using a large (10 ton operating weight) vibratory roller.

Depending on final pavement area finish grades, bedrock removal may be required at various new parking lot areas. If required, we recommend that the following blasted bedrock subgrade preparation be performed.

- Proofroll the blasted bedrock subgrade with a large vibratory roller to densify inter-particle voids and provide a smooth surface.
- Install a layer of woven geotextile fabric (minimum weight 6 oz./square yard) directly over the fractured bedrock.
- Place and compact the Subbase and Base soil as described above.

Where the subgrade consists of soil, the geotextile fabric is not necessary. In general, groundwater is expected to be below pavement section elevations. Based on this, pavement section underdrains are not necessary. If groundwater becomes present within blasted bedrock areas, we recommend cut-off drainage at the toe of pavement areas be provide or pavement underdrains be installed.



7.0 Earthwork Consideration

Some bedrock removal may be required within the northeastern corner of the proposed building addition, likely requiring blasting. Additional bedrock removal may be required for pavement areas depending on final site grading. The depth of the footing subgrade may vary depending on the locations of the bedrock. Low spots can be leveled with crushed stone, Structural Backfill, or flowable fill. Control joints or construction joints should be placed where the footing subgrade transitions from bedrock to soil.

Based on our field observations and the gradation results from sample 17087-1, the sandy fill encountered at the site likely not meet Structural Backfill or Foundation Backfill specifications. The existing sandy fill will likely meet specifications for Granular Borrow. We recommend that prior to use as Granular Borrow, an additional sample should be obtained to verify conformance with the specification prior use.

Excavations below 4 feet and above the groundwater table should be sloped no greater than 1H to 1V for silty and sandy soil. Excavations below the groundwater table (if encountered) should be reduced to a slope no greater than 1.5H to 1V. These slopes are based on the current OSHA Excavation Guidelines.

We recommend that a qualified geotechnical consultant be retained to monitor and test soil materials used during construction. Summit would welcome the opportunity to provide this service.

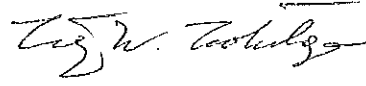
8.0 Closure

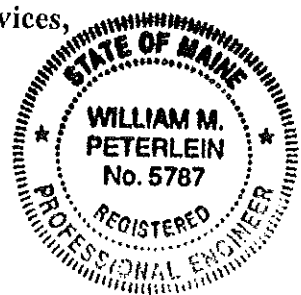
Our recommendations are based on professional judgment and generally accepted principles of geotechnical engineering. Some changes in subsurface conditions from those presented in this report may occur. Should these conditions differ materially from those described in this report, Summit should be notified so that we can re-evaluate our recommendations.

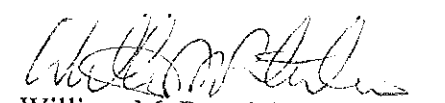
We recommend that Summit be retained to review our recommendations once the final building layout and grades have been established. Although it is unlikely, additional explorations may be necessary at that time.

We appreciate the opportunity to serve you during this phase of your project. If there are any questions or additional information is required, please do not hesitate to call.

Sincerely yours,  
Summit Geoenvironmental Services,

  
Craig W. Coolidge, E.I.T.  
Geotechnical Engineer



  
William M. Peterlein, P.E.  
Principal Geotechnical Engineer

LOCATION PLAN

**BORING/PROBE LOGS**

## EXPLORATION REPORT COVER SHEET

The exploration report has been prepared by the geotechnical engineer from both field and laboratory data. Differences between field logs and exploration reports may exist.

It is common practice in the soil and foundation engineering profession that field logs and laboratory data sheets not be included in engineering reports, because they do not represent the engineer's final opinion as to appropriate descriptions for conditions encountered in the exploration and testing work. The field logs will be retained in our office for review. Results of laboratory tests are generally shown on the borings logs or are described in the text of the report as appropriate.

### Drilling and Sampling Symbols:

SS = Split Spoon	Hyd = Hydraulic advance of probes
ST = Shelby Tube – 2" OD, disturbed	WOH = Weight of Hammer
UT = Shelby Tube – 3" OD, undisturbed	WOR = Weight of Rod
HSA = Hollow Stem Auger	GS = Grain Size Data
CS = Casing – size as noted	PI = Plasticity Index
Sv = Vane Shear	LL = Liquid Limit
PP = Pocket Penetrometer	w = Natural Water Content
RX = Rock Core – size as noted	USCS = unified Soil Classification System

### Water Level Measurements:

Water levels indicated on the boring logs are the levels measured in the boring at the times indicated. In pervious soils, the indicated elevations are considered reliable groundwater levels. In impervious soils, the accurate determination of groundwater elevations may not be possible, even after several days of observations; additional evidence of groundwater elevations via observation or monitoring wells must be sought.

### Gradation Description and Terminology:

Boulders:	Over 8 inches	Trace:	Less than 5%
Cobbles:	8 inches to 3 inches	Little:	5% to 15%
Gravel:	3 inches to No.4 sieve	Some:	15% to 25%
Sand:	No.4 to No. 200 sieve	Silty, Sandy, etc.:	Greater than 25%
Silt:	No. 200 sieve to 0.005 mm		
Clay:	less than 0.005 mm		

### Density of Granular Soils and Consistency of Cohesive Soils:

CONSISTENCY OF COHESIVE SOILS		DENSITY OF GRANULAR SOILS	
SPT N-value blows/ft	Consistency	SPT N-value blows/ft	Relative Density
0 to 2	Very Soft	0 to 3	Very Loose
3 to 4	Soft	4 to 9	Loose
5 to 8	Firm	10 to 29	Compact
9 to 16	Stiff	30 to 49	Dense
17 to 32	Very Stiff	50 to 80	Very Dense
>32	Hard		

**SUBSURFACE CONDITIONS SUMMARY**

Project Name: Caravan Beds Building Addition  
 Location: Forest Avenue, Portland, Maine

Project Number: 17087  
 Exploration Date: 7/20/2006

Boring/Probe No.	Ground Surface Elev (ft.)	SOIL			Weathered Bedrock	Depth	Bedrock Elev (ft.)
		Pavement	Fill/Reworked				
B-1	89.0	0" - 2"	2" - 11"		11" - 2.4'	2.4	86.6
B-2	88.5	0" - 2.5"	2.5" - 2.7'		NE	2.7	85.8
B-3	86.0	0" - 2.5"	2.5" - 12"		NE	3.5	82.5
B-4	84.5	0" - 2.5"	2.5" - 6.5'		NE	6.5	78.0
B-5	85.5	0" - 2.5"	2.5" - 7.2'		NE	7.2	78.3
B-6	84.0	0" - 2"	2" - 5.5'		NE	5.5	78.5
P-1	89.0	0" - 2.25"	2.25" - 12"		12" - 1.4'	1.4	87.6
P-2	90.0	0" - 2.5"	2.5" - 4.7'		NE	4.7	85.3
P-3	91.0	0" - 2.25"	2.25" - 2.5'		NE	2.5	88.5
P-4	86.5	0" - 2"	2" - 3'		3' - 3.3'	3.3	83.2
P-5	87.5	0" - 2.25"	2.25" - 2.5'		2.5' - 2.8'	2.8	84.7
P-6	85.5	0" - 2.25"	2.25" - 4'		NE	4.0	81.5
P-7	85.0	0" - 2.5"	2.5" - 6.2'		NE	6.2	78.8
P-8	85.5	0" - 2"	2" - 5.4'		NE	5.4	80.1
P-9	86.0	0" - 2"	2" - 5.1'		5.1' - 5.4'	5.4	80.6
P-10	87.0	NE	0" - 2.3'		NE	2.3	84.7
P-11	88.0	NE	0" - 6"		NE	0.5	87.5
P-12	89.5	0" - 1.75"	1.75" - 1.4'		NE	1.4	88.1

**NOTES:**

- 1.) Fill at the site generally consisted of orange/brown/dark brown sand with silt and gravel.
- 2.) P-10 had 1 inch of topsoil as ground cover.
- 3.) Groundwater seepage was not encountered at any boring or probe locations.
- 4.) NE = Not Encountered

SUMMIT GEOENGINEERING SERVICES 640 Main Street Lewiston, Maine 04240				SOIL BORING LOG			Boring #: B-1	
Drilling Co: Northern Test Boring				Project: Proposed Building Addition Caravan Beads Portland, Maine			Project #: 17087 Sheet: 1 of 1 Prep by: ARH	
Foreman: Mike Nadeau				Ground Elevation: Approximately 89 ft.				
Summit: Craig Coolidge, E.I.T.				Reference: Interpolated from site plan provided by Sebago Technics				
Date started: 7/20/2006				Date Comp: 7/20/2006				
DRILLING METHOD		SAMPLER		GROUND WATER DEPTH				
Vehicle: Trailer		Type: 24" SS		Date	Depth	Elevation	Comments	
Model: Deidrich D50		Hammer: 140 lb		7/20/2006	None Observed			
Method: 2-1/2" HSA		Fall: 30"						
Depth (ft.)	SAMPLE DATA				ENGINEERING DESCRIPTION	GEOLOGIC DESCRIPTION		
	No.	Pen/Rec (in.)	Depth (ft)	Blows				
1	S-1	11/7	0 - 0.9	4	Compact, dark brown/black SAND, some Silt, trace Gravel, damp, SM	Pavement = 2" FILL		
2				50/5"	Weathered bedrock from 11' to 2.4'	11" WEATHERED BEDROCK		
3					End boring at 2.4', auger Refusal	2.4' BEDROCK		
4								
5								
6								
7								
8								
9								
10								
11								
12								
13								
14								
15								
16								
17								
18								
19								
20								
21								
22								

SUMMIT GEOENGINEERING SERVICES 640 Main Street Lewiston, Maine 04240				SOIL BORING LOG			Boring #: B-2	
Drilling Co: Northern Test Boring				Project: Proposed Building Addition Caravan Beads Portland, Maine			Project #: 17087 Sheet: 1 of 1 Prep by: ARH	
Foreman: Mike Nadeau				Ground Elevation: Approximately 88.5 ft.			Reference: Interpolated from site plan provided by Sebago Technics	
Summit: Craig Coolidge, E.I.T.				Date started: 7/20/2006 Date Comp: 7/20/2006				
DRILLING METHOD		SAMPLER		GROUND WATER DEPTH				
Vehicle: Trailer		Type: 24" SS		Date	Depth	Elevation	Comments	
Model: Deidrich D50		Hammer: 140 lb		7/20/2006	None Observed			
Method: 2-1/2" HSA		Fall: 30"						
Depth (ft.)	SAMPLE DATA				ENGINEERING DESCRIPTION	GEOLOGIC DESCRIPTION		
	No.	Pen/Rec (in.)	Depth (ft)	Blows				
1	S-1	24/9	0 - 2	4	Compact, brown SAND, little Gravel and Silt, damp, SP	Pavement = 2.5"  FILL		
				5				
				5				
2				12	Brick debris at 6"			
3					End boring at 2.7', auger refusal	2.7'  BEDROCK		
4								
5								
6								
7								
8								
9								
10								
11								
12								
13								
14								
15								
16								
17								
18								
19								
20								
21								
22								

SUMMIT GEOENGINEERING SERVICES 640 Main Street Lewiston, Maine 04240				SOIL BORING LOG			Boring #: B-3	
Drilling Co: Northern Test Boring				Project: Proposed Building Addition Caravan Beads Portland, Maine			Project #: 17087 Sheet: 1 of 1 Prep by: ARH	
Foreman: Mike Nadeau				Ground Elevation: Approximately 86 ft.				
Summit: Craig Coolidge, E.I.T.				Reference: Interpolated from site plan provided by Sebago Technics				
Date started: 7/20/2006				Date Comp: 7/20/2006				
DRILLING METHOD		SAMPLER		GROUND WATER DEPTH				
Vehicle: Trailer		Type: 24" SS		Date	Depth	Elevation	Comments	
Model: Deidrich D50		Hammer: 140 lb		7/20/2006	None Observed			
Method: 2-1/2" HSA		Fall: 30"						
Depth (ft.)	SAMPLE DATA				ENGINEERING DESCRIPTION	GEOLOGIC DESCRIPTION		
	No.	Pen/Rec (in.)	Depth (ft)	Blows				
1	S-1	24/18	0 - 2	4	Loose, brown SAND, trace Silt and Gravel, damp, SM	Pavement = 2.5" FILL		
2				4	Compact, dark brown Silty fine SAND, damp, trace organics, SM	12"		
3				7	Compact, orange brown fine SAND, little Silt, damp, SM	18"		
4					End boring at 3.5', auger refusal	3.5' BEDROCK		
5								
6								
7								
8								
9								
10								
11								
12								
13								
14								
15								
16								
17								
18								
19								
20								
21								
22								



SUMMIT GEOENGINEERING SERVICES 640 Main Street Lewiston, Maine 04240				SOIL BORING LOG			Boring #: B-4	
Drilling Co: Northern Test Boring				Ground Elevation: Approximately 84.5 ft.			Project #: 17087	
Foreman: Mike Nadeau				Reference: Interpolated from site plan provided by Sebago Technics			Sheet: 1 of 1	
Summit: Craig Coolidge, E.I.T.				Date started: 7/20/2006 Date Comp: 7/20/2006			Prep by: ARH	
DRILLING METHOD		SAMPLER		GROUND WATER DEPTH				
Vehicle: Trailer		Type: 24" SS		Date	Depth	Elevation	Comments	
Model: Deidrich D50		Hammer: 140 lb		7/20/2006	None Observed			
Method: 2-1/2" HSA		Fall: 30"						
Depth (ft.)	SAMPLE DATA				ENGINEERING DESCRIPTION	GEOLOGIC DESCRIPTION		
	No.	Pen/Rec (in.)	Depth (ft)	Blows				
1	S-1	24/16	0 - 2	6	Compact, brown SAND, little Gravel and Silt, damp, SP-SM	Pavement = 2.5" FILL		
				14				
				12				
2				9	Compact, dark brown SAND, some Silt, little to trace Gravel, damp, SM	1.5'		
3								
4								
5								
6	S-2	24/12	5 - 6.2	10	Same as above	5.5'		
				16				
7				50/2"	Dense, olive brown SAND, some Silt, little Gravel, trace Clay, damp to moist, SM			
8					End Boring at 6.5', refusal	6.5'  BEDROCK		
9								
10								
11								
12								
13								
14								
15								
16								
17								
18								
19								
20								
21								
22								

SUMMIT GEOENGINEERING SERVICES 640 Main Street Lewiston, Maine 04240				SOIL BORING LOG			Boring #: B-5	
Drilling Co: Northern Test Boring				Project: Proposed Building Addition Caravan Beads Portland, Maine			Project #: 17087 Sheet: 1 of 1 Prep by: ARH	
Foreman: Mike Nadeau				Ground Elevation: Approximately 85.5 ft.				
Summit: Craig Coolidge, E.I.T.				Reference: Interpolated from site plan provided by Sebago Technics				
DRILLING METHOD				Date started: 7/20/2006 Date Comp: 7/20/2006				
Vehicle: Trailer		SAMPLER		GROUND WATER DEPTH				
Model: Deidrich D50		Type: 24" SS Hammer: 140 lb		Date	Depth	Elevation	Comments	
Method: 2-1/2" HSA		Fall: 30"		7/20/2006	None Observed			
Depth (ft.)	SAMPLE DATA				ENGINEERING DESCRIPTION	GEOLOGIC DESCRIPTION		
	No.	Pen/Rec (in.)	Depth (ft)	Blows				
1	S-1	24/10	0 - 2	5	Compact, brown/dark brown SAND, little Silt, little to trace Gravel, damp, SM-SP to SM	Pavement = 2.5"  FILL		
				5				
2				3				
3								
4								
5								
6	S-2	24/20	5 - 7	11	Compact, dark brown Silty SAND	5.5'		
				14	Compact, olive SILT, some fine SAND, trace			
				14	Clay, damp, ML-SM			
7				36	Dense, brown SAND, little Silt, moist to wet, SP	6.5'		
8					End Boring at 7.2', refusal	7.2'		
9						BEDROCK		
10								
11								
12								
13								
14								
15								
16								
17								
18								
19								
20								
21								
22								

SUMMIT GEOENGINEERING SERVICES 640 Main Street Lewiston, Maine 04240				SOIL BORING LOG			Boring #: B-6	
Drilling Co: Northern Test Boring				Project: Proposed Building Addition			Project #: 17087	
Foreman: Mike Nadeau				Caravan Beads			Sheet: 1 of 1	
Summit: Craig Coolidge, E.I.T.				Portland, Maine			Prep by: ARH	
Ground Elevation: Approximately 84 ft.				Reference: Interpolated from site plan provided by Sebago Technics				
Date started: 7/20/2006				Date Comp: 7/20/2006				
DRILLING METHOD		SAMPLER		GROUND WATER DEPTH				
Vehicle: Trailer		Type: 24" SS		Date	Depth	Elevation	Comments	
Model: Deidrich D50		Hammer: 140 lb		7/20/2006	None Observed			
Method: 2-1/2" HSA		Fall: 30"						
Depth (ft.)	SAMPLE DATA				ENGINEERING DESCRIPTION	GEOLOGIC DESCRIPTION		
	No.	Pen/Rec (in.)	Depth (ft)	Blows				
1	S-1	24/23	0 - 2	5	Stiff, olive and mottled Silty CLAY, damp, CL	Pavement = 2" Sandy = 3" GLACIAL MARINE		
				6				
2				6				
				8				
3								
4								
5								
6	S-2	7-Jul	5 - 5.5	15	Dense, olive and gray(layers) fine SAND, some			
				50/1"	Silt, little Clay, damp to moist, SM			
7					End Boring at 5.5', refusal	5.5'		
8						BEDROCK		
9								
10								
11								
12								
13								
14								
15								
16								
17								
18								
19								
20								
21								
22								

SUMMIT GEOENGINEERING SERVICES 640 Main Street Lewiston, Maine 04240				SOIL BORING LOG			Boring #: P-1	
Drilling Co: Northern Test Boring				Project: Proposed Building Addition Caravan Beads Portland, Maine			Project #: 17087 Sheet: 1 of 1 Prep by: ARH	
Foreman: Mike Nadeau				Ground Elevation: Approximately 89 ft.				
Summit: Craig Coolidge, E.I.T.				Reference: Interpolated from site plan provided by Sebago Technics				
				Date started: 7/20/2006 Date Comp: 7/20/2006				
DRILLING METHOD		SAMPLER		GROUND WATER DEPTH				
Vehicle: Trailer		Type: 24" SS		Date	Depth	Elevation	Comments	
Model: Deidrich D50		Hammer: 140 lb		7/20/2006	None Observed			
Method: 2-1/2" HSA		Fall: 30"						
Depth (ft.)	SAMPLE DATA				ENGINEERING DESCRIPTION	GEOLOGIC DESCRIPTION		
	No.	Pen/Rec (in.)	Depth (ft)	Blows				
1					Brown SAND, little Gravel and Silt, damp, SP	Pavement = 2.25" FILL		
2					Weathered Rock	12" Weathered Rock		
3					End Probe at 1.4', refusal	1.4' BEDROCK		
4								
5								
6								
7								
8								
9								
10								
11								
12								
13								
14								
15								
16								
17								
18								
19								
20								
21								
22								

SUMMIT GEOENGINEERING SERVICES 640 Main Street Lewiston, Maine 04240				SOIL BORING LOG			Boring #: P-2	
Drilling Co: Northern Test Boring				Project: Proposed Building Addition Caravan Beads Portland, Maine			Project #: 17087 Sheet: 1 of 1 Prep by: ARII	
Foreman: Mike Nadeau				Ground Elevation: Approximately 90 ft.				
Summit: Craig Coolidge, E.I.T.				Reference: Interpolated from site plan provided by Sebago Technics				
				Date started: 7/20/2006 Date Comp: 7/20/2006				
DRILLING METHOD		SAMPLER		GROUND WATER DEPTH				
Vehicle: Trailer		Type: 24" SS		Date	Depth	Elevation	Comments	
Model: Deidrich D50		Hammer: 140 lb		7/20/2006	None Observed			
Method: 2-1/2" HSA		Fall: 30"						
Depth (ft.)	SAMPLE DATA				ENGINEERING DESCRIPTION	GEOLOGIC DESCRIPTION		
	No.	Pen/Rec (in.)	Depth (ft)	Blows				
1					Brown SAND, little Gravel, little to trace Silt, damp, SP	Pavement = 2.5"  FILL		
2								
3								
4								
5					End Probe at 4.7', refusal	4.7'  BEDROCK		
6								
7								
8								
9								
10								
11								
12								
13								
14								
15								
16								
17								
18								
19								
20								
21								
22								

SUMMIT GEOENGINEERING SERVICES 640 Main Street Lewiston, Maine 04240				SOIL BORING LOG			Boring #: P-3	
Drilling Co: Northern Test Boring				Project: Proposed Building Addition Caravan Beads Portland, Maine			Project #: 17087 Sheet: 1 of 1 Prep by: ARH	
Foreman: Mike Nadeau				Ground Elevation: Approximately 91 ft.				
Summit: Craig Coolidge, E.I.T.				Reference: Interpolated from site plan provided by Sebago Technics				
Date started: 7/20/2006				Date Comp: 7/20/2006				
DRILLING METHOD		SAMPLER		GROUND WATER DEPTH				
Vehicle: Trailer		Type: 24" SS		Date	Depth	Elevation	Comments	
Model: Deidrich D50		Hammer: 140 lb		7/20/2006	None Observed			
Method: 2-1/2" HSA		Fall: 30"						
Depth (ft.)	SAMPLE DATA				ENGINEERING DESCRIPTION	GEOLOGIC DESCRIPTION		
	No.	Pen/Rec (in.)	Depth (ft)	Blows				
1					Dark brown SAND, some Silt, trace Gravel, damp, SM	Pavement = 2.25"		
2						FILL		
3					End Probe at 2.5', refusal	2.5'		
4						BEDROCK		
5								
6								
7								
8								
9								
10								
11								
12								
13								
14								
15								
16								
17								
18								
19								
20								
21								
22								

SUMMIT GEOENGINEERING SERVICES 640 Main Street Lewiston, Maine 04240				SOIL BORING LOG			Boring #: P-4	
Drilling Co: Northern Test Boring				Ground Elevation: Approximately 86.5 ft.			Project #: 17087	
Foreman: Mike Nadeau				Reference: Interpolated from site plan provided by Sebago Technics			Sheet: 1 of 1	
Summit: Craig Coolidge, E.I.T.				Date started: 7/20/2006			Date Comp: 7/20/2006	
DRILLING METHOD		SAMPLER		GROUND WATER DEPTH				
Vehicle: Trailer		Type: 24" SS		Date	Depth	Elevation	Comments	
Model: Deidrich D50		Hammer: 140 lb		7/20/2006	None Observed			
Method: 2-1/2" HSA		Fall: 30"						
Depth (ft.)	SAMPLE DATA				ENGINEERING DESCRIPTION	GEOLOGIC DESCRIPTION		
	No.	Pen/Rec (in.)	Depth (ft)	Blows			Pavement = 2"	
1					Brown SAND, little Gravel and Silt, damp, SP	FILL		
2								
3								
4					End Probe at 3.3', refusal	Weathered Rock		
5						3.3'		
6						BEDROCK		
7								
8								
9								
10								
11								
12								
13								
14								
15								
16								
17								
18								
19								
20								
21								
22								

SUMMIT GEOENGINEERING SERVICES 640 Main Street Lewiston, Maine 04240				SOIL BORING LOG			Boring #: P-5	
Drilling Co: Northern Test Boring				Ground Elevation: Approximately 87.5 ft.			Project #: 17087	
Foreman: Mike Nadeau				Reference: Interpolated from site plan provided by Sebago Technics			Sheet: 1 of 1	
Summit: Craig Coolidge, E.I.T.				Date started: 7/20/2006 Date Comp: 7/20/2006			Prep by: ARH	
DRILLING METHOD		SAMPLER		GROUND WATER DEPTH				
Vehicle: Trailer		Type: 24" SS		Date	Depth	Elevation	Comments	
Model: Deidrich D50		Hammer: 140 lb		7/20/2006	None Observed			
Method: 2-1/2" HSA		Fall: 30"						
Depth (ft.)	SAMPLE DATA				ENGINEERING DESCRIPTION	GEOLOGIC DESCRIPTION		
	No.	Pen/Rec (in.)	Depth (ft)	Blows				
1					Orange brown SAND, little Silt, damp, SP-SM	Pavement = 2.25" FILL		
2								
3						2.5' Weathered Rock		
4					End Probe at 2.8', refusal	2.8' BEDROCK		
5								
6								
7								
8								
9								
10								
11								
12								
13								
14								
15								
16								
17								
18								
19								
20								
21								
22								



SUMMIT GEOENGINEERING SERVICES 640 Main Street Lewiston, Maine 04240				SOIL BORING LOG			Boring #: P-6	
Drilling Co: Northern Test Boring				Project: Proposed Building Addition Caravan Beads Portland, Maine			Project #: 17087 Sheet: 1 of 1 Prep by: ARH	
Foreman: Mike Nadeau				Ground Elevation: Approximately 85.5 ft.				
Summit: Craig Coolidge, E.I.T.				Reference: Interpolated from site plan provided by Sebago Technics				
				Date started: 7/20/2006 Date Comp: 7/20/2006				
DRILLING METHOD		SAMPLER		GROUND WATER DEPTH				
Vehicle: Trailer		Type: 24" SS		Date	Depth	Elevation	Comments	
Model: Deidrich D50		Hammer: 140 lb		7/20/2006	None Observed			
Method: 2-1/2" HSA		Fall: 30"						
Depth (ft.)	SAMPLE DATA				ENGINEERING DESCRIPTION	GEOLOGIC DESCRIPTION		
	No.	Pen/Rec (in.)	Depth (ft)	Blows				
1					Orange brown - dark brown SAND, little Silt, little to trace Gravel, damp, SM-SP	Pavement = 2.25"  FILL		
2								
3								
4								
5					End Probe at 4', refusal	4.0'  BEDROCK		
6								
7								
8								
9								
10								
11								
12								
13								
14								
15								
16								
17								
18								
19								
20								
21								
22								

SUMMIT GEOENGINEERING SERVICES 640 Main Street Lewiston, Maine 04240				SOIL BORING LOG			Boring #: P-7	
Drilling Co: Northern Test Boring				Project: Proposed Building Addition Caravan Beads Portland, Maine			Project #: 17087 Sheet: 1 of 1 Prep by: ARH	
Foreman: Mike Nadeau				Ground Elevation: Approximately 85 ft.				
Summit: Craig Coolidge, E.I.T.				Reference: Interpolated from site plan provided by Sebago Technics				
DRILLING METHOD				Date started: 7/20/2006 Date Comp: 7/20/2006				
SAMPLER				GROUND WATER DEPTH				
Vehicle: Trailer		Type: 24" SS		Date	Depth	Elevation	Comments	
Model: Deidrich D50		Hammer: 140 lb		7/20/2006	None Observed			
Method: 2-1/2" HSA		Fall: 30"						
Depth (ft.)	SAMPLE DATA				ENGINEERING DESCRIPTION	GEOLOGIC DESCRIPTION		
	No.	Pen/Rec (in.)	Depth (ft)	Blows				
1					Orange brown - dark brown SAND, little Silt, trace Gravel, damp, SM-SP	Pavement = 2.5"  FILL		
2								
3								
4								
5								
6								
7					End Probe at 6.2', refusal	6.2'  BEDROCK		
8								
9								
10								
11								
12								
13								
14								
15								
16								
17								
18								
19								
20								
21								
22								

SUMMIT GEOENGINEERING SERVICES 640 Main Street Lewiston, Maine 04240				SOIL BORING LOG			Boring #: P-8		
Drilling Co: Northern Test Boring				Project: Proposed Building Addition Caravan Beads Portland, Maine			Project #: 17087 Sheet: 1 of 1 Prep by: ARH		
Foreman: Mike Nadeau				Ground Elevation: Approximately 85.5 ft.					
Summit: Craig Coolidge, E.I.T.				Reference: Interpolated from site plan provided by Sebago Technics					
DRILLING METHOD				SAMPLER		GROUND WATER DEPTH			
Vehicle: Trailer				Type: 24" SS		Date	Depth	Elevation	Comments
Model: Deidrich D50				Hammer: 140 lb		7/20/2006	None Observed		
Method: 2-1/2" HSA				Fall: 30"					
Depth (ft.)	SAMPLE DATA				ENGINEERING DESCRIPTION	GEOLOGIC DESCRIPTION			
	No.	Pen/Rec (in.)	Depth (ft)	Blows					
1					Dark brown SAND, little Silt, trace Gravel, damp SM	Pavement = 2"  FILL			
2									
3									
4									
5									
6									
6					End Probe at 5.4', refusal	5.4'  BEDROCK			
7									
8									
9									
10									
11									
12									
13									
14									
15									
16									
17									
18									
19									
20									
21									
22									





SUMMIT GEOENGINEERING SERVICES 640 Main Street Lewiston, Maine 04240				SOIL BORING LOG				Boring #: P-11	
Drilling Co: Northern Test Boring				Project: Proposed Building Addition Caravan Beads Portland, Maine				Project #: 17087 Sheet: 1 of 1 Prep by: ARH	
Foreman: Mike Nadeau				Ground Elevation: Approximately 88 ft.					
Summit: Craig Coolidge, E.I.T.				Reference: Interpolated from site plan provided by Sebago Technics					
				Date started: 7/20/2006 Date Comp: 7/20/2006					
DRILLING METHOD		SAMPLER		GROUND WATER DEPTH					
Vehicle: Trailer		Type: 24" SS		Date	Depth	Elevation	Comments		
Model: Deidrich D50		Hammer: 140 lb		7/20/2006	None Observed				
Method: 2-1/2" HSA		Fall: 30"							
Depth (ft.)	SAMPLE DATA				ENGINEERING DESCRIPTION			GEOLOGIC DESCRIPTION	
	No.	Pen/Rec (in.)	Depth (ft)	Blows					
1					Brown SAND, little Gravel and Silt, damp, SP			FILL	
					End Probe at 6", refusal			6"	
2								BEDROCK	
3									
4									
5									
6									
7									
8									
9									
10									
11									
12									
13									
14									
15									
16									
17									
18									
19									
20									
21									
22									



**LABORATORY RESULTS**



SUMMIT GEOENGINEERING SERVICES

P.O. Box 4698 Augusta, Maine 04330-4698

Phone:(207) 621-8334 Fax:(207) 626-9094

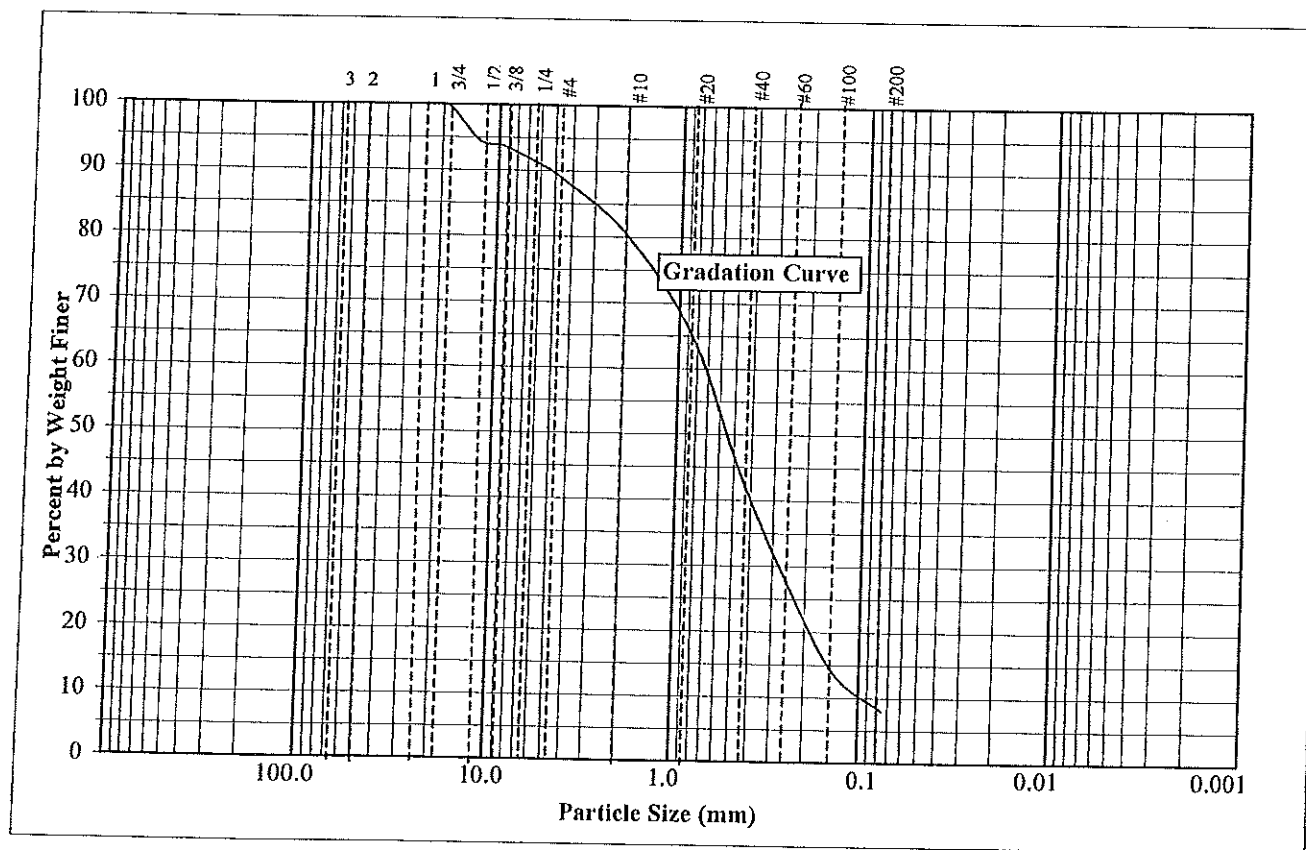
GRAIN SIZE ANALYSIS - ASTM D422

PROJECT NAME: Building Addition  
CLIENT: Caravan Beads  
SOIL DESCRIP: Granular Fill  
INTENDED USE: Existing Subgrade Fill

PROJ #: 17087  
SAMPLE: 17087-1  
DATE: 8/11/2006  
SOURCE: Boring B-3, S-1, 0' to 2'  
TECH: Austin Harrell

DATA

PARTICLE SIZE mm	% BY WT FINER
76.20 (3 in)	100.0
50.80 (2 in)	100.0
38.10 (1-1/2 in)	100.0
25.40 (1 in)	100.0
19.05 (3/4 in)	100.0
12.70 (1/2 in)	94.5
9.53 (3/8 in)	93.7
6.35 (1/4 in)	91.3
4.75 (No. 4)	89.2
2.00 (No. 10)	80.4
0.85 (No. 20)	64.9
0.43 (No. 40)	41.8
0.15 (No. 100)	15.1
0.08 (No. 200)	7.9



REMARKS:

Reviewed: Craig Coolidge, E.I.T.

**GENERAL BLASTING RECOMMENDATIONS**

## GENERAL BLASTING RECOMMENDATIONS

### Introduction

Blasting operations will be performed in general accordance with the applicable U.S. Department of the Interior Rules, the recommendations provided below, and a normal standard of care.

### Blast Design

The blasting contractor shall submit a blasting plan to the Owner for approval prior to blasting operations. The blasting plan shall include a schedule, sketches of the drill patterns (hole spacing and depth), type and amount of explosives, number and sequence of delays, methods for minimizing flyrock, and any other information pertinent to demonstrating compliance with the applicable U.S. Department of the Interior Rules and the recommendations provided below.

### Notification

Oral notification to the abutters within one-half mile of the blast area shall be provided prior to blasting. Warning and all clear signals of different character or pattern that are audible within one-half mile from the point of the blast shall be given. The meaning of the signals shall be conveyed to the abutters at the time they are notified.

### Pre-blast Surveys

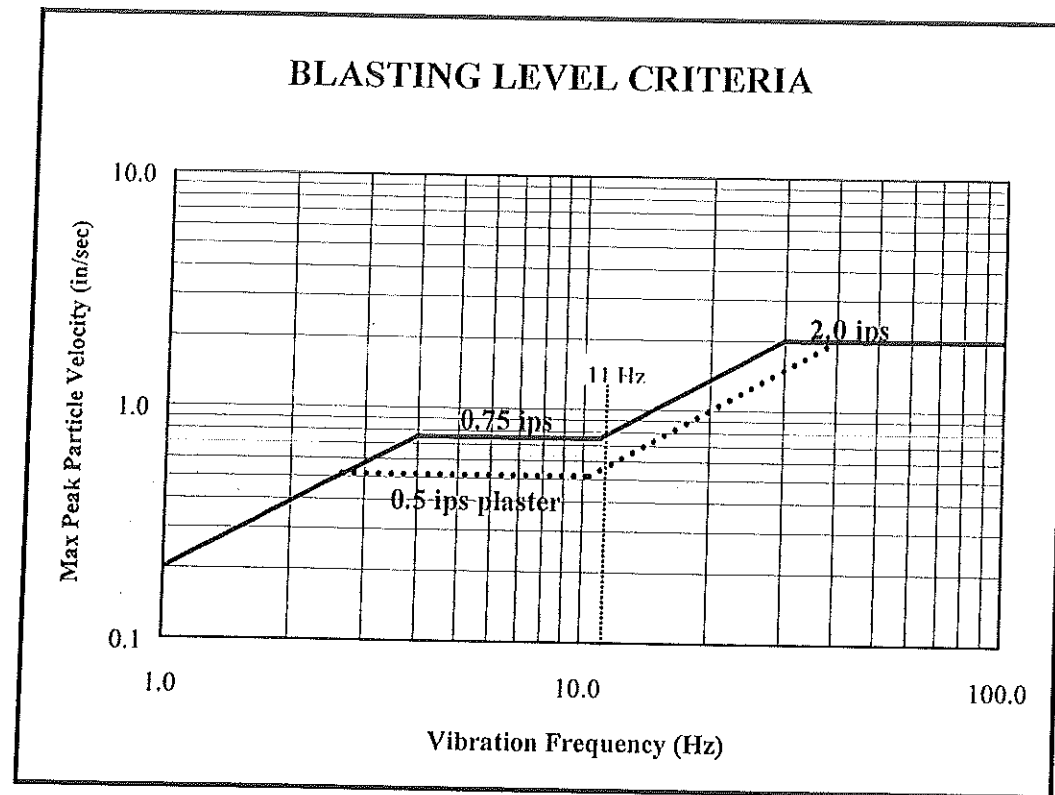
All blasting operations are the direct responsibility of the Blasting Contractor. Reports of damage to structures caused by blasting operations are the sole responsibility of the Blasting Contractor. Therefore, it is incumbent upon the Blasting Contractor to perform pre-blast surveys as they deem necessary.

### Airblast Limits

Airblast overpressure shall not exceed 136 dB (0.018 psi) at the nearest structure.

### Ground Vibration Limits

The maximum ground vibration at any structure shall not exceed the limits presented in the following chart:

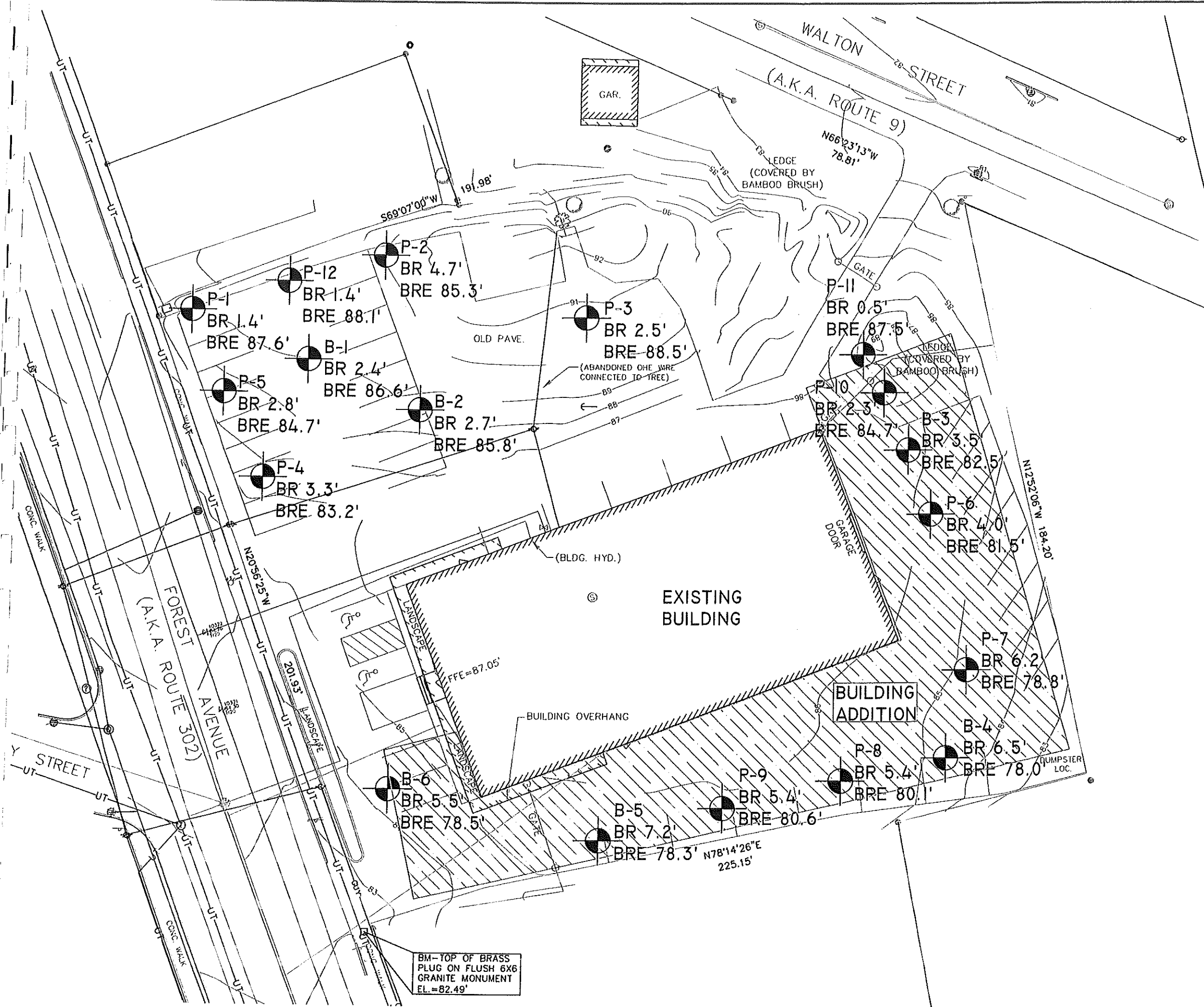


REFERENCE: OSM alternative blasting criteria (Modified from figure B-1, Bureau of Mines, RI 8507)

The Blasting Contractor shall provide a seismographic record to the Owner for each blast event at the nearest off-site structure. The record shall include the date and time of the blast, peak and resultant particle velocities and associated frequencies, and the airblast overpressure.


**Flyrock**

Blasting mats shall be used to cover the area which will be blasted, such that flyrock traveling along the ground or in the air shall not be cast more than one-half the distance to the nearest structure or beyond the property line, whichever is less.



BM-TOP OF BRASS  
PLUG ON FLUSH 6X6  
GRANITE MONUMENT  
EL.=82.49'


**KEY**

-  BORING/PROBE
- BR - BEDROCK DEPTH
- BRE - APPROX. BEDROCK ELEVATION

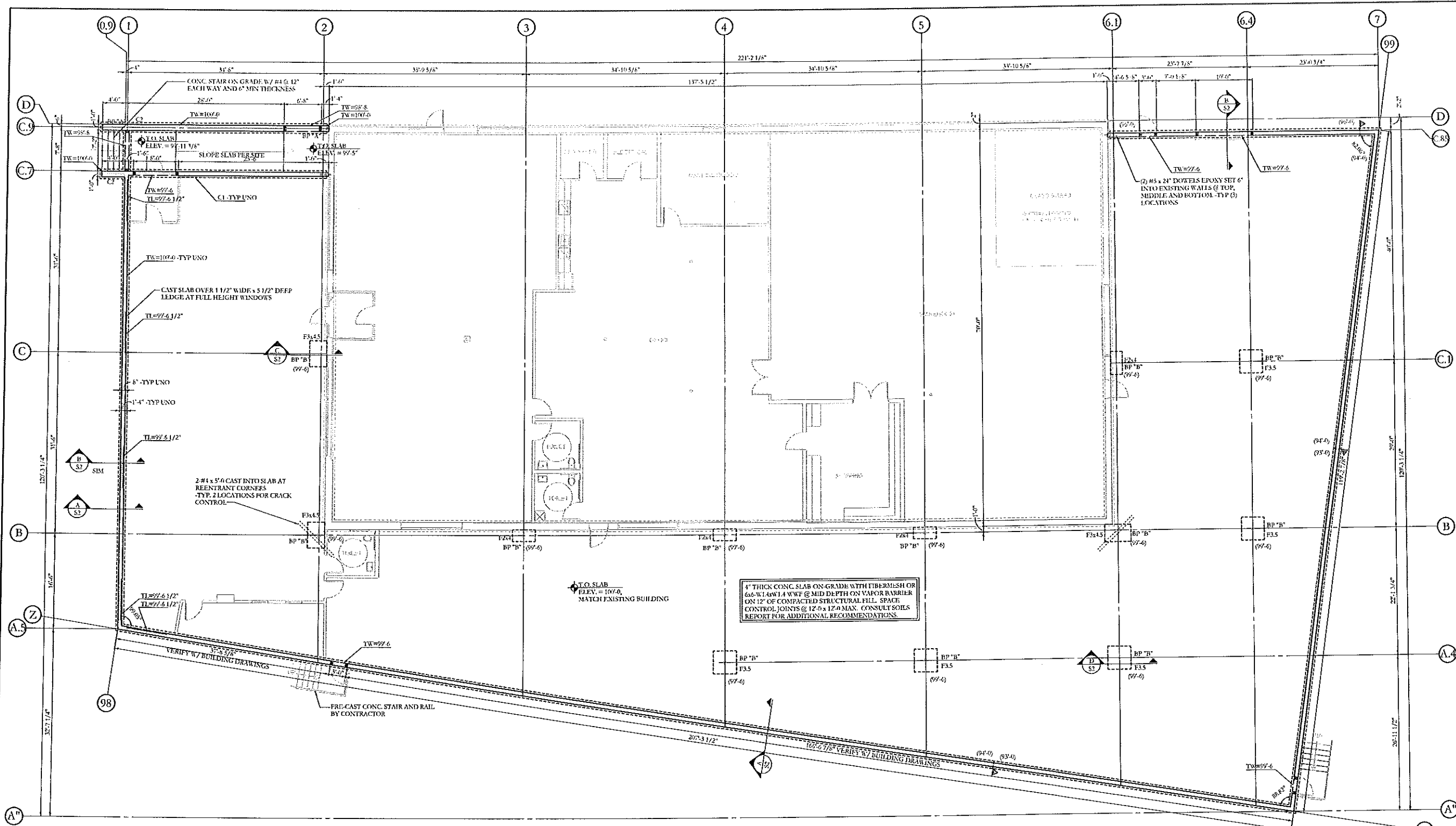


**NOTES:**  
BORING/PROBE LOCATIONS AND ELEVATIONS ARE ESTIMATED FROM SITEPLAN TOPOGRAPHY. BORING/PROBE LOCATIONS ARE APPROXIMATELY LOCATED BY SUMMIT FROM PACING AND TAPING FROM EXISTING SITE FEATURES.

**REFERENCE PLANS:**  
SITEPLAN BY SEBAGO TECHNICS,  
DATED JULY 18, 2006

CLIENT: CARAVAN BEADS		PROJECT: BUILDING ADDITION PORTLAND, MAINE	
 <b>SUMMIT</b> GEOENGINEERING SERVICES 640 Main Street Lewiston, Maine 04240		TITLE: BORING/PROBE LOCATION PLAN	
		DRAWN: CWC	SCALE: 1" = 30'
DESIGN: ---	DATE: 8/10/06	FIGURE: 1	
PROJECT NO.: 17087	APPROVED: WMP	FILE NO.:	

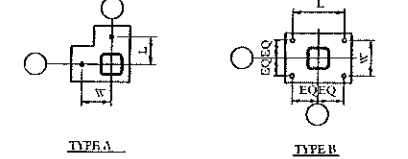




**ANCHOR BOLT SCHEDULE**

ANCHOR BOLT SPACING		ANCHOR BOLTS				
TYPE	DIM L	DIM W	NUMBER	SIZE	LENGTH	PROJ.
A	41/2"	41/2"	2	1/2"	12"	4"
B	12"	6"	4	1/2"	12"	4"

\* ALL ANCHOR BOLTS ARE TO BE 1" DIA. A36 UNO  
 \* VERIFY BASE PLATE ORIENTATION W/ LATEST MUROX DRAWINGS  
 \* SET ALL ANCHORS BOLTS WITH TEMPLATES FOR ACCURACY



**FOUNDATION PLAN**

- NOTES: SCALE 1/8"=1'-0"
- ALL CONTINUOUS FOOTINGS ARE 10" DEEP x 1'-6" WIDE x CONT. WITH (2) #5 BARS LONGITUDINAL, C1, UNO, ALSO SEE SCHEDULE
  - STEP IN TOP OF FOUNDATION WALL IS INDICATED THUS: [Symbol] AND SHOWS LOWER SIDE OF WALL
  - STEP IN TOP OF FOOTING IS INDICATED THUS: [Symbol] AND SHOWS LOWER FOOTING
  - TOP OF FOOTING ELEV. = 99'-0", UNLESS INDICATED THUS: (XXX)XX
  - TOP OF WALL ELEV. = 100'-0", UNLESS INDICATED THUS: TW=XXX'XX
  - FOOTINGS MAY BE RAISED TO BEAR 2'-0" MIN BELOW GRADE AT SOLID BEDROCK. COORDINATE W/ STRUCTURAL AND GEOTECHNICAL ENGINEER
  - COORDINATE ALL ANCHOR BOLT DIMENSIONS W/ LATEST MUROX DRAWINGS
  - SEE SO FOR STRUCTURAL GENERAL NOTES, ETC.

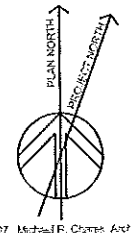
**FOOTING SCHEDULE**

MARK	SIZE	REINFORCING
C1	10'D x 1'-6" W x CONT.	(2) #5 CONT. LONG.
C2	10'D x 1'-8" W x CONT.	(2) #5 CONT. LONG.
F24	2'-0" x 4'-0" x 0'-12"	#5 @ 12" EACH WAY
F34.5	3'-0" x 4'-6" x 0'-12"	#5 @ 12" EACH WAY
F35	3'-6" x 5'-6" x 0'-12"	#5 @ 12" EACH WAY

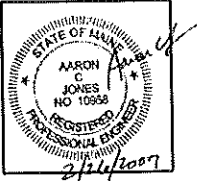
**Structural Integrity**  
 Consulting Engineers, Inc.

77 Oak Street  
 Portland, ME, 04101  
 p. 207-774-4614  
 f. 866-793-7835  
 www.structuralintegrity.com

**BUILD WITH CONFIDENCE**  
 © 2007 Structural Integrity Consulting Engineers, Inc.



**Michael R. Charek**  
 Architect  
 25 Hartley Street  
 Portland, Maine 04103  
 (207) 761-0566



**Caravan Beads**  
 Building Expansion  
 915 Forest Avenue  
 Portland, Maine 04103

Title  
 FOUNDATION PLAN

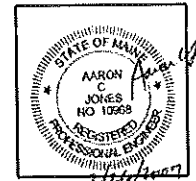
Scale: 1/8" = 1'-0"  
 Date: 2/23/2007

Revisions  
 FOR CONST.

Sheet  
**S1**  
 SI, INC. JOB# 08-0041-1

Michael R. Charek  
Architect

25 Hartley Street  
Portland, Maine 04103  
(207) 761-0556



Caravan Beads  
Building Expansion

915 Forest Avenue  
Portland, Maine 04103

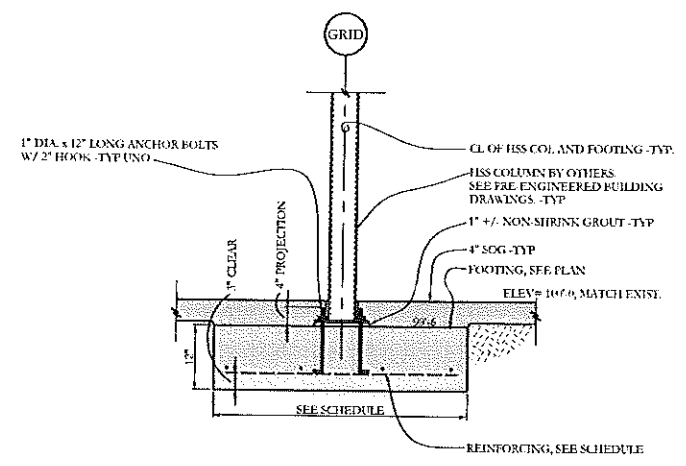
Title	SECTIONS
-------	----------

Scale: 3/4" = 1'-0"  
Date: 2/28/2007

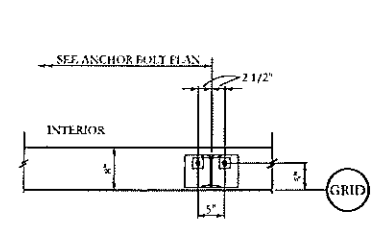
Revisions  
FOR CONST.

Sheet  
**S2**

SI, INC. JOB# 09-0041.1

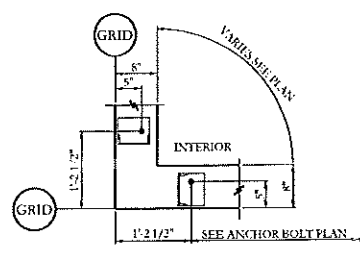


SECTION **D** 3/4"=1'-0

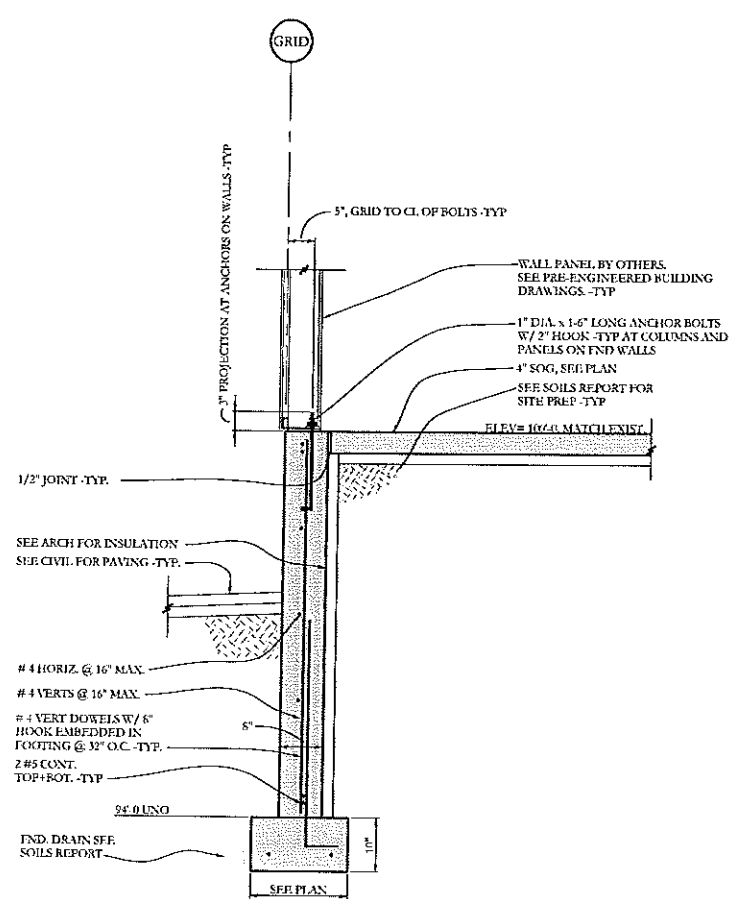


TYPICAL PANEL JOINT UNO

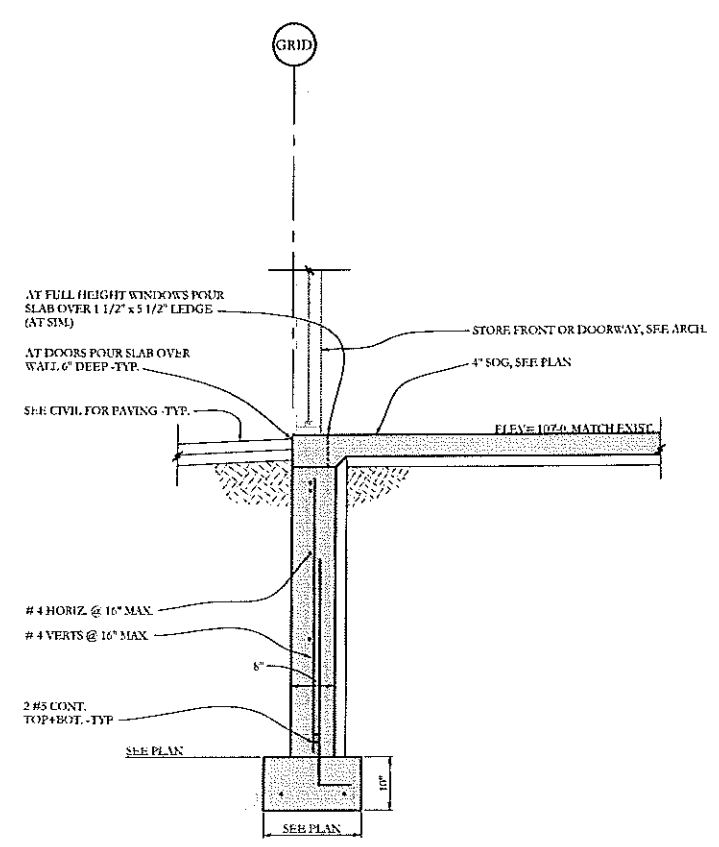
SECTION **E** 3/4"=1'-0



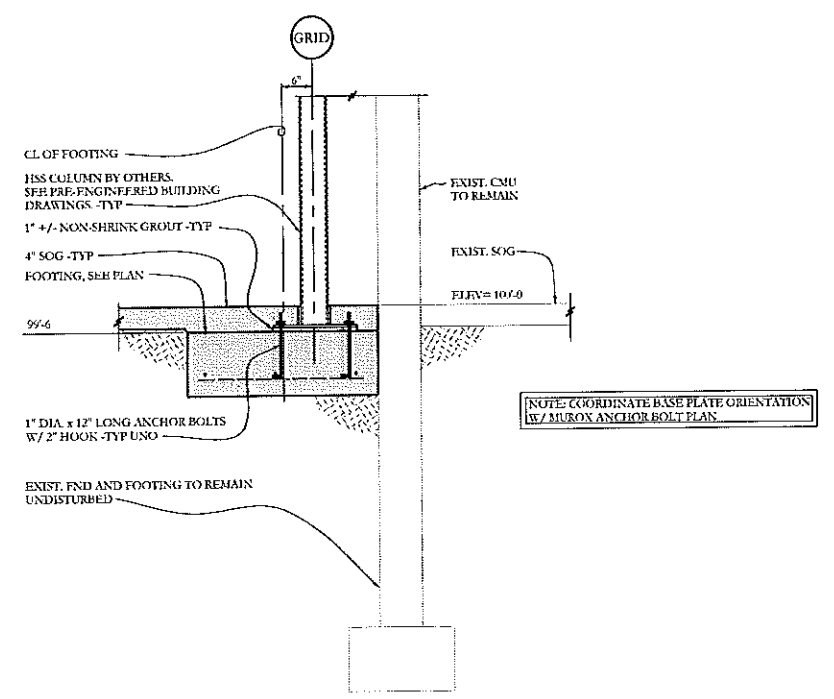
TYPICAL CORNER PANEL JOINT UNO



SECTION **A** 3/4"=1'-0



SECTION **B** 3/4"=1'-0

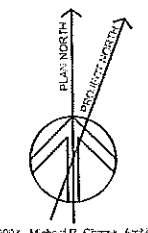


SECTION **C** 3/4"=1'-0

**Structural Integrity**  
Consulting Engineers, Inc.

77 Oak Street  
Portland, ME, 04101  
p. 207-774-4614  
f. 866-793-7835  
www.structuralinteg.com

BUILD WITH CONFIDENCE  
© 2007 Structural Integrity Consulting Engineers, Inc.



© 2007 Michael R. Charek Architect



# Site Plan Review

## GENERAL NOTES:

- PROJECT IS LOCATED WITHIN THE CITY OF PORTLAND.
- TOPOGRAPHIC INFORMATION FROM A GROUND SURVEY BY SEBAGO TECHNIQS DATED JULY, 2006.
- THE PROJECT RECEIVED SITE PLAN REVIEW APPROVAL FROM THE CITY OF PORTLAND ON NOVEMBER 28, 2006.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR NOTIFYING "DIG-SAFE" AND LOCAL UTILITY COMPANIES AT LEAST 3 BUSINESS DAYS, BUT NOT MORE THAN 30 CALENDAR DAYS, PRIOR TO THE COMMENCEMENT OF ANY EXCAVATION, OR AS OTHERWISE REQUIRED BY MAINE STATE LAW.
- THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ALL MEANS, METHODS AND TECHNIQUES EMPLOYED TO PERFORM THE WORK SHOWN ON THESE PLANS.
- ALL WORK SHALL COMPLY WITH ALL LOCAL, STATE AND FEDERAL SAFETY REGULATIONS.
- ALL WORK SHALL BE IN CONFORMANCE WITH THE CITY OF PORTLAND AND ALL UTILITY COMPANIES STANDARDS.
- CONTRACTOR SHALL VERIFY LOCATIONS AND DEPTHS OF ALL UTILITIES WITH THE RESPECTIVE COMPANY PRIOR TO THE START OF CONSTRUCTION. IF ANY DISCREPANCIES OR CONFLICTS ARE FOUND, THE CONTRACTOR SHALL NOTIFY THE ENGINEER AND OWNER PRIOR TO PROCEEDING.
- THE CONTRACTOR SHALL SECURE ALL NECESSARY CONSTRUCTION ACTIVITY PERMITS FOR THE WORK SHOWN ON THESE PLANS PRIOR TO CONSTRUCTION.
- NO BLASTING WILL BE ALLOWED WITHIN 500 FT OF ANY UTILITY WITHOUT THE NOTIFICATION AND APPROVAL OF THE APPROPRIATE UTILITY COMPANY. NO LEDGE BLASTING WILL BE PERMITTED WITHIN A UTILITY COMPANY EASEMENT UNTIL WRITTEN APPROVAL FROM THE UTILITY IS GIVEN. BLASTING OPERATIONS SHALL BE IN ACCORDANCE WITH MAINE DEPT OF ENVIRONMENTAL PROTECTION BLASTING REQUIREMENTS AND CITY OF PORTLAND REQUIREMENTS.
- ALL PAVEMENT CUTS TO EXISTING PAVEMENT SHALL BE SAW CUT TO RESULT IN CLEAN EDGES. A TACK COAT SHALL BE APPLIED ALONG THE CUT EDGES AND NEW PAVEMENT BUTTED TO IT, UNLESS OTHERWISE DIRECTED BY THE ENGINEER.
- THE CONTRACTOR SHALL BE RESPONSIBLE TO RECLAIM OR PROPERLY DISPOSE OF ALL REMOVED BITUMINOUS MATERIALS.
- CONDUIT SHALL BE USED FOR ELECTRIC, TELEPHONE AND TV IN ACCORDANCE WITH THE RESPECTIVE COMPANIES REQUIREMENTS.
- ALL EROSION & SEDIMENT CONTROL MEASURES SHALL BE INSTALLED & MAINTAINED IN ACCORDANCE WITH THE "MAINE EROSION AND SEDIMENT CONTROL HANDBOOK" AND AS OTHERWISE SPECIFIED OR INDICATED ON DRAWINGS.
- REFER TO ARCHITECTURAL PLANS FOR BUILDING DIMENSIONS.
- ALL SIDEWALKS SHALL INCLUDE NO ACCESSIBLE RAMPS AT ALL INTERSECTIONS & DRIVEWAYS.

## CITY OF PORTLAND SITE PLAN & SUBDIVISION NOTES:

- LANDSCAPING SHALL MEET THE "ARBORICULTURAL SPECIFICATIONS AND STANDARDS OF PRACTICE AND LANDSCAPE GUIDELINES" OF THE CITY OF PORTLAND TECHNICAL AND DESIGN STANDARDS AND GUIDELINES (LATEST REVISION).
- THE ENTIRE SITE SHALL BE DEVELOPED AND/OR MAINTAINED AS DEPICTED ON THE SITE PLAN. APPROVAL OF THE PLANNING AUTHORITY OR PLANNING BOARD SHALL BE REQUIRED FOR ANY ALTERATION TO OR DEVIATION FROM THE APPROVED SITE PLAN, INCLUDING, WITHOUT LIMITATION: TOPOGRAPHY; DRAINAGE; LANDSCAPING; RETENTION OF WOODED OR LAWN AREAS; ACCESS; SITE, LOCATION, AND SURFACING OF PARKING AREAS; AND LOCATION AND SIZE OF BUILDINGS.
- SIDEWALKS AND CURBS SHALL BE DESIGNED AND BUILT WITH TYPED RAMP AT ALL STREET CORNERS, CROSSWALKS AND DRIVEWAYS IN CONFORMANCE WITH THE CITY OF PORTLAND TECHNICAL AND DESIGN STANDARDS AND GUIDELINES.
- ALL EROSION AND SEDIMENT CONTROL MEASURES SHALL BE DESIGNED IN ACCORDANCE WITH MAINE EROSION AND SEDIMENT CONTROL HANDBOOK FOR CONSTRUCTION BEST MANAGEMENT PRACTICES PUBLISHED BY THE CUMBERLAND COUNTY SOIL AND WATER CONSERVATION DISTRICT AND MAINE DEPARTMENT OF ENVIRONMENTAL PROTECTION, MARCH 1991 OR LATEST EDITION. (NOTE: THE SITE PLAN SHOULD SPECIFY THE EROSION CONTROL DEVICE TO BE EMPLOYED (SILT FENCE, HAY BALE, ETC.) AS WELL AS THEIR LOCATION).
- ALL EROSION CONTROL MEASURES SHALL BE INSTALLED PRIOR TO ANY SITE EXCAVATION OR REGRADING.
- ALL DISTURBED AREAS ON THE SITE NOT COVERED BY BUILDINGS OR PAVED AREAS SHALL BE STABILIZED WITH LOAM AND SEED OR OTHER METHODS AS REQUIRED BY BEST MANAGEMENT PRACTICES (SEE ABOVE).
- PRIOR TO CONSTRUCTION, A PRECONSTRUCTION MEETING SHALL BE HELD AT THE PROJECT SITE WITH THE CONTRACTOR, DEVELOPMENT REVIEW COORDINATOR, PUBLIC WORKS REPRESENTATIVE AND OWNER TO REVIEW THE CONSTRUCTION SCHEDULE AND CRITICAL ASPECTS OF THE SITE WORK. AT THAT TIME, THE SITE BUILDING CONTRACTOR SHALL PROVIDE THREE (3) COPIES OF A DETAILED CONSTRUCTION SCHEDULE TO THE ATTENDING CITY REPRESENTATIVE. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO ARRANGE A MUTUALLY AGREEABLE TIME FOR THE PRECONSTRUCTION MEETING.
- EXISTING VEGETATION SHALL BE CONSERVED IN AREAS SHOWN ON THIS SITE. FENCING OR OTHER PROTECTIVE BARRIERS SHALL BE ERRECTED OUTSIDE THE DRP-LINE OF INDIVIDUAL GROUPINGS OF TREES DESIGNATED FOR PRESERVATION PRIOR TO THE ONSET OF CONSTRUCTION. REGRADING SHALL NOT TAKE PLACE WITHIN THE DRP-LINE OF TREES DESIGNATED FOR PRESERVATION. NO STORAGE OR CONSTRUCTION MATERIALS SHALL BE PERMITTED WITHIN THE DRP-LINE OF TREES TO BE PRESERVED.

## Project Team

### Engineering, Permitting & Landscape Architecture:

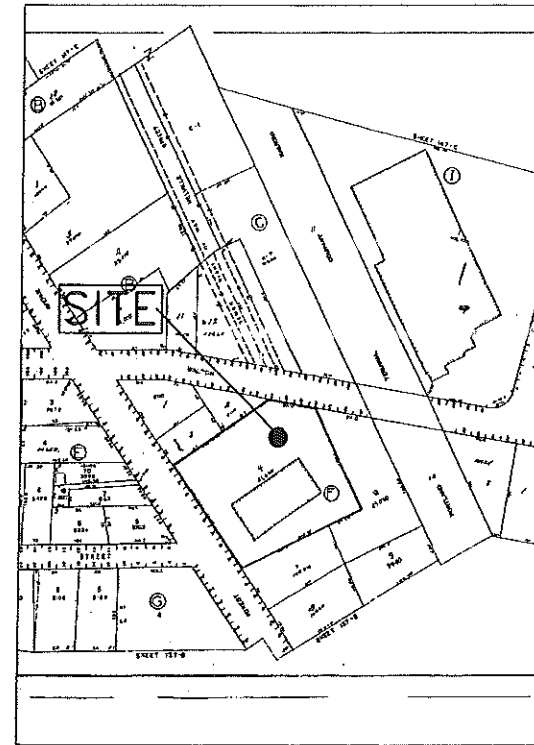
LAND USE CONSULTANTS, INC.  
956 Riverside Street  
Portland, ME 04103  
(207) 878-3313

### Survey:

Sebago Technics  
One Chabot Street  
Westbrook, ME 04098

### Architect

Michael R. Charek Architect  
25 Hartley Street  
Portland, ME 04103



Locus

# Caravan Beads Expansion

915 Forest Avenue  
Portland, Maine 04103

Prepared For:  
BJFC, LLC  
915 Forest Avenue, Portland, Maine 04103

## Legend

EXISTING		PROPOSED
---	CONTOUR	---
---	SPOT GRADE	---
---	PROPERTY LINE	---
---	ROADWAY CENTERLINE	---
---	BUILDING SETBACK	---
---	EDGE OF PAVEMENT	---
---	BIT CURB	---
---	BUILDING	---
---	SANITARY SEWER	---
---	STORM DRAIN	---
---	GAS	---
---	WATER MAIN	---
---	OVERHEAD WIRE	---
---	UTILITY POLE	---
---	CATCH BASIN	---
---	MANHOLE	---
---	HYDRANT	---
---	GATE VALVE	---

## Utilities:

- \* WATER: PORTLAND WATER DISTRICT
- \* SEWER: CITY OF PORTLAND DPW
- \* ELECTRIC: CMP - TERRY S. BRADISH
- \* TELEPHONE: VERIZON - GEORGE HILLMAN 207-797-1798
- \* GAS - NORTHERN UTILITIES

## Index Of Sheets

SHEET #	
0	COVER SHEET
(1 of 1)	EXISTING CONDITIONS PLAN
C1	EXISTING CONDITIONS & REMOVALS
C2	SITE LAYOUT, GRADING & EROSION CONTROL
C3	LANDSCAPING
C4	DETAILS
C5	DETAILS
A1	PRELIMINARY FLOOR PLAN
A2	PRELIMINARY ELEVATIONS
A3	STREETSCAPE ELEVATIONS

## Approvals:

CITY OF PORTLAND  
-Zoning Board of Appeals - Conditional Use Approval  
-Planning Board - Site Plan Review Approval

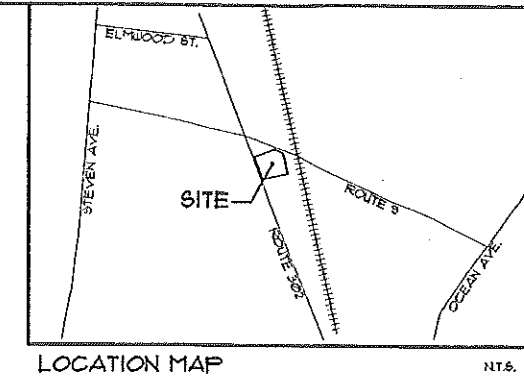
Received

August 17, 2006  
November 28, 2006

12/01/06 Revised per Conditions of Approval  
11/10/06 Revised Per Planning Board/Staff Comments  
10/06/06 Revised Per 09/25/06 Planning Department Comments  
Issue: 08/25/06 Site Plan Review - City of Portland



Drawing 0  
LUC PROJ #4516



LOCATION MAP NT.S.

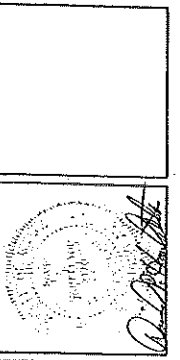
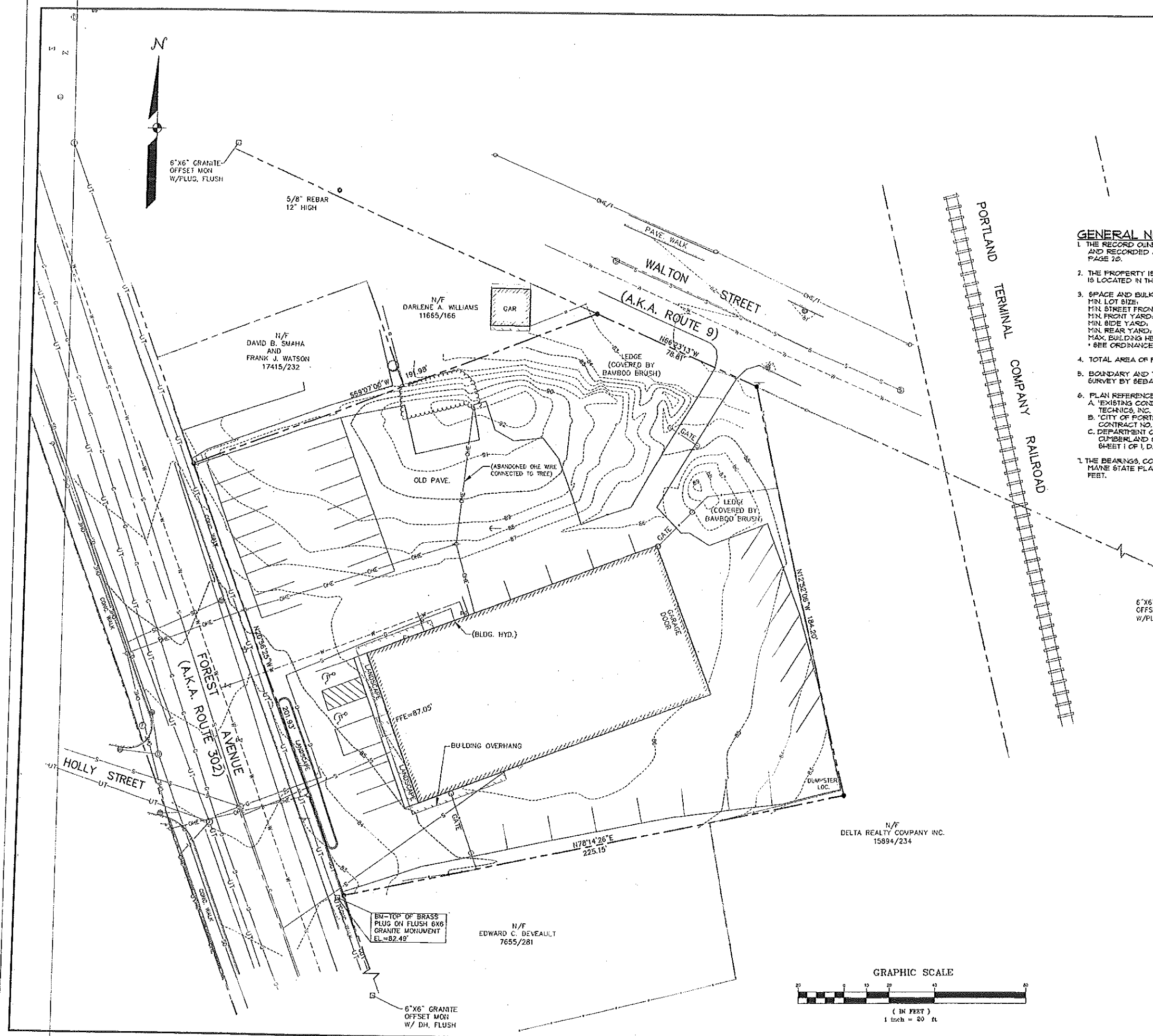
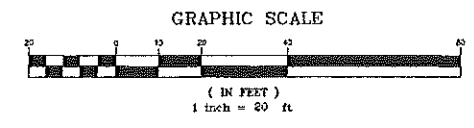
**GENERAL NOTES**

1. THE RECORD OWNER OF THE PARCEL IS B.J.C. LLC BY DEED DATED OCTOBER 25, 2002 AND RECORDED AT THE CUMBERLAND COUNTY REGISTRY OF DEEDS (CCRD) IN BOOK 18310 PAGE 70.
2. THE PROPERTY IS SHOWN ON THE CITY OF PORTLAND TAX MAP W2, BLOCK F AS LOT 4 AND IS LOCATED IN THE B2, BUSINESS ZONE.
3. SPACE AND BULK CRITERIA FOR THE B2 ZONE ARE AS FOLLOWS:  
 MIN. LOT SIZE: 10,000 SQ. FT.  
 MIN. STREET FRONTAGE: 30 FEET  
 MIN. FRONT YARD: SEE ORDINANCE  
 MIN. SIDE YARD: 10 FEET  
 MIN. REAR YARD: NONE  
 MAX. BUILDING HEIGHT: 25 FEET  
 \*SEE ORDINANCE FOR MORE PARTICULAR INFORMATION
4. TOTAL AREA OF PARCEL IS APPROXIMATELY 21,000 SQ. FT. OR 0.48 ACRES.
5. BOUNDARY AND TOPOGRAPHIC INFORMATION SHOWN HEREON IS BASED UPON A FIELD SURVEY BY SEBAGO TECHNICS, INC. PERFORMED IN JULY 2006.
6. PLAN REFERENCES:  
 A. "EXISTING CONDITIONS PLAN" OF 93 FOREST AVE. SITE FOR ZAJAC INC. BY SEBAGO TECHNICS, INC. DATED AUGUST 8, 1991.  
 B. "CITY OF PORTLAND, MAINE DEPARTMENT OF PUBLIC WORKS, FOREST AVE. RECONSTR. CONTRACT NO. 4" ALONG WITH CITY OF PORTLAND - UTILITY PLANS.  
 C. DEPARTMENT OF TRANSPORTATION RIGHT OF WAY MAP-STATE HIGHWAY 14, PORTLAND, CUMBERLAND COUNTY, FEDERAL AID PROJECT NO. F-NH-CHP(11), DATED JULY 1994, SHEET 1 OF 1, D.O.T. FILE NO. 3-41.
7. THE BEARINGS, COORDINATES, AND ELEVATIONS SHOWN HEREON ARE BASED UPON THE MAINE STATE PLANE COORDINATE GRID, WEST ZONE 1807 ON NAD83 AND NAVD83 IN US FEET.

6"X6" GRANITE OFFSET MON W/PLUG, FLUSH

**LEGEND**

EXISTING	DESCRIPTION
---	BOUNDARY LINE/ROLL
---	ABUTTER LINE/ROLL
---	EASEMENT
---	CENTERLINE
⊕	MONUMENT
—○—	IRON PIPE/ROD
⊙	DRILLHOLE
⊕	REBAR TO BE SET
---	EDGE PAVEMENT
---	TRAVEL WAY LINE
---	CURBLINE
---	BUILDING OVERHANG
⊕	BENCHMARK
---	TRIANGLE
---	CONTOURS
---	CHAIN LINK FENCE
---	STOCKADE FENCE
---	RAILROAD
⊕	GAS
⊕	WATER
⊕	GATE VALVE
⊕	HYDRANT
⊕	SEWER
⊕	SEWER MH
⊕	STORM DRAIN
⊕	UNDERDRAIN
⊕	CATCH BASIN
⊕	DRAINAGE MH
⊕	OVERHEAD ELEC. T. TEL.
⊕	UNDERGROUND ELEC. T. TEL.
⊕	TELEPHONE MANHOLE
⊕	UTILITY POLE
⊕	8" W
⊕	WATER GALT OFF



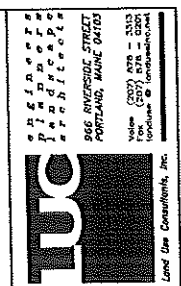
REV.	BY:	DATE:	STATUS:

**Sebago Technics**  
 Engineering Excellence You Can Build On  
 One Orchard Street  
 Westbrook, ME 04090-1139  
 Tel (207) 666-6277

PROJECT NO: 06242EC  
 FIELD BOOK: 824  
 DESIGN: MVE  
 CHECK: MVE  
 DRAWN: DPH

**EXISTING CONDITIONS PLAN**  
 OF:  
**CARAVAN OF BEADS**  
 FOREST AVE AND WALTON STREET  
 PORTLAND, MAINE  
 FOR:  
**MICHAEL CHAREK ARCHITECTS**  
 25 HARTLEY STREET  
 PORTLAND, MAINE 04103





STATE OF MAINE  
 PROFESSIONAL ENGINEER  
 License No. 1197

CARAVAN BEADS  
 FOREST AVENUE, PORTLAND, MAINE  
 PREPARED FOR:  
 MICHAEL CHAREK ARCHITECTS  
 25 HARTLEY STREET  
 PORTLAND, MAINE 04103

Revision	Date
DESIGNED PER PLANING DEPT. REVIEW 10/26/06	
CHECKED PER PLANING DEPT. REVIEW 10/26/06	
REVISED - CONDITIONS OF APPROVAL 10/26/06	

Designed PLC  
 Drawn PLC  
 Checked PLC  
 Scale 1" = 20'  
 Date 8/25/06

Job No. 4516  
 Drawing  
 C-2

**PROJECT DATA:**

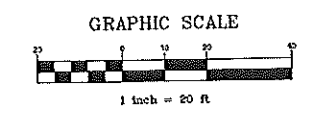
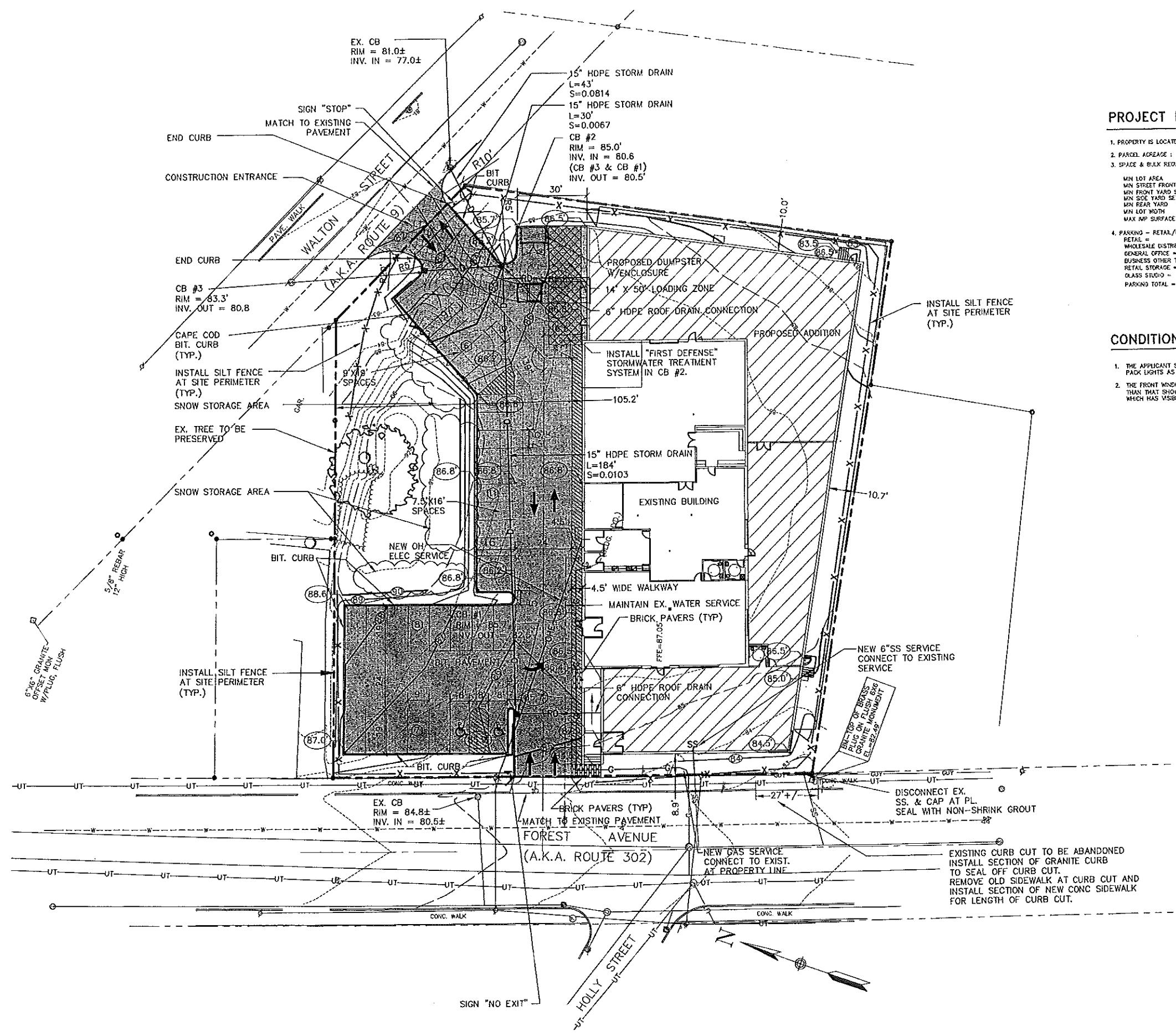
- PROPERTY IS LOCATED IN THE COMMUNITY BUSINESS (C2) DISTRICT.
- PARCEL ACREAGE : 1.17 AC (51,087 SF)
- SPACE & BULK REGULATIONS:
 

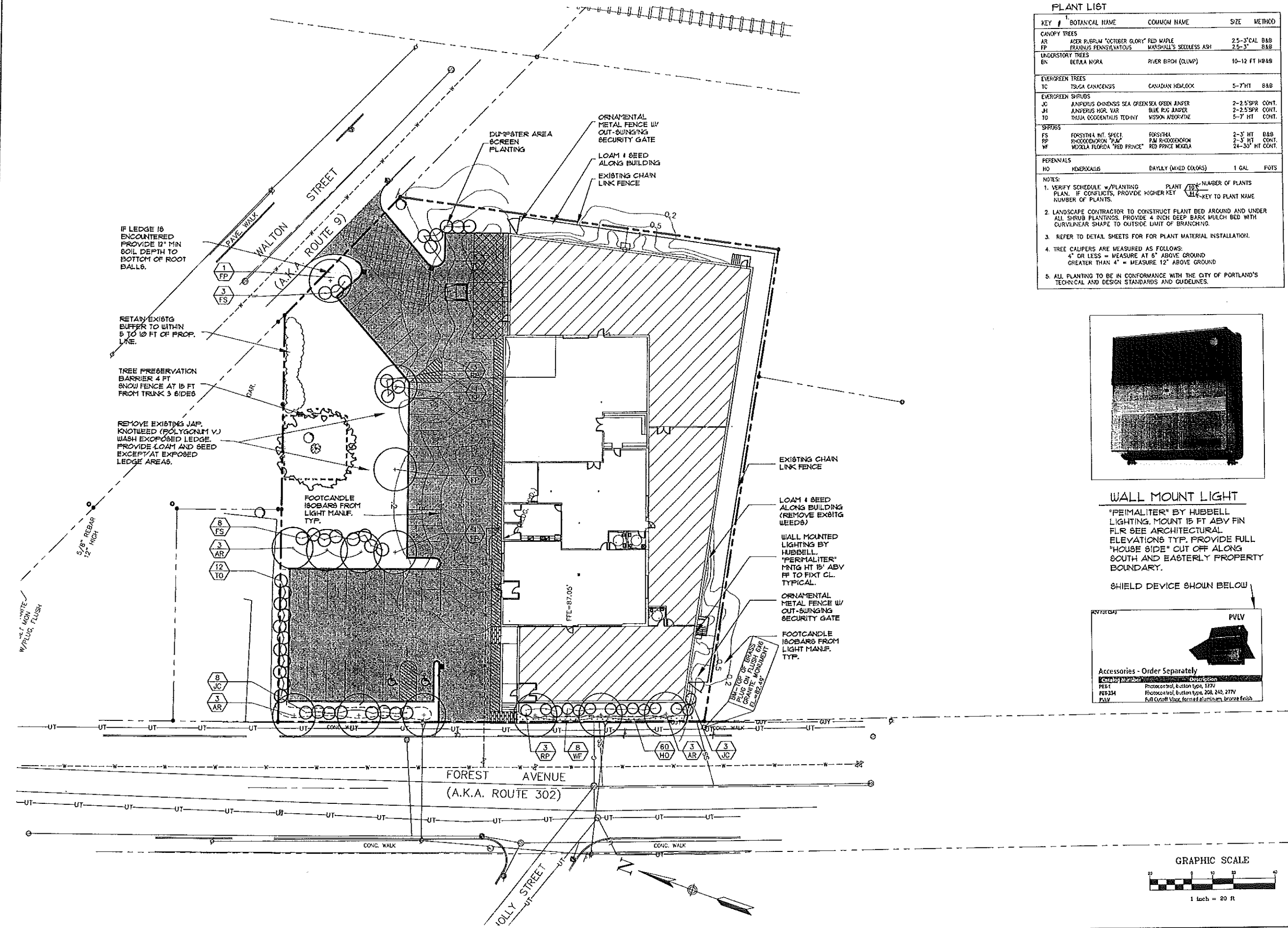
	REQUIRED/ALLOWED	PROVIDED
MIN LOT AREA	10,000 SF	51,087 SF
MIN STREET FRONTAGE	50'	201' - FOREST 78' + WALTON
MIN FRONT YARD SETBACK	NONE	8' +
MIN SIDE YARD SETBACK	NONE	10' + & 105' +
MIN REAR YARD	10'	10'
MIN LOT WIDTH	NONE	201' - FRONT & 184' - REAR
MAX NP SURFACE RATIO	80%	73%
- PARKING - RETAIL/WAREHOUSE
 

	AREA SF	REQUIRED	PROVIDED
RETAIL =	3,036 SF	6 SP	
WHOLESALE DISTRIBUTION WAREHOUSE =	9,991 SF	10 SP	
GENERAL OFFICE =	4,587 SF	14 SP	
BUSINESS OTHER THAN RETAIL =	997 SF	1 SP	
RETAIL STORAGE =	2,619 SF	0 SP	
CLASS STUDIO =	500 SF	1 SP	
PARKING TOTAL =		32 SP	32 SP

**CONDITIONS OF APPROVAL:**

- THE APPLICANT SHALL INSTALL THE FULL CUTOFF VISOR FOR THE EXTERIOR WALL PACK LIGHTS AS PROPOSED.
- THE FRONT WINDOW FACING FOREST AVENUE SHALL BE OF A SHADE NO GREATER THAN THAT SHOWN AT THE PUBLIC HEARING, NAMELY "SOLARGRAY" BY PPG INDUSTRIES, WHICH HAS VISIBLE LIGHT TRANSMITTANCE OF 40%.



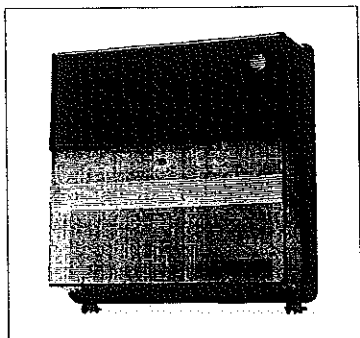


### PLANT LIST

KEY #	BOTANICAL NAME	COMMON NAME	SIZE	METHOD
<b>CANOPY TREES</b>				
AR	ACER RUBRUM 'OCTOBER GLORY'	RED MAPLE	2.5-3' CAL	B&B
FP	FRAXINUS PENNSYLVANICA	MARSHALL'S SEEDLESS ASH	2.5-3'	B&B
<b>LONGSTORY TREES</b>				
EN	BETULA NORA	RIVER BIRCH (CLUMP)	10-12 FT HR&B	
<b>EVERGREEN TREES</b>				
TC	TSUGA CANADENSIS	CANADIAN HEMLOCK	5-7' HT	B&B
<b>EVERGREEN SHRUBS</b>				
JC	JUNIPERUS OMOHENSIS SEA GREEN	SEA GREEN JUNPER	2-2.5' SPR	CONT.
JH	JUNIPERUS HUR. VAR.	BLUE ICE JUNPER	2-2.5' SPR	CONT.
TD	TRIALA OCCIDENTALIS TEDDY	WISSOH ASSOCIATE	5-7' HT	CONT.
<b>SHRUBS</b>				
FS	FOSYTHIA HT. SPECT.	FOSYTHIA	2-3' HT	B&B
RP	ROSEODORON 'PAM'	PAM ROSEODORON	2-3' HT	CONT.
WF	WEIGELA FLORIDA 'RED PRINCE'	RED PRINCE WEIGELA	24-30' HT	CONT.
<b>PERENNIALS</b>				
HO	HENROCALIS	DAYLILY (MIXED COLORS)	1 GAL	POTS

**NOTES:**

- VERIFY SCHEDULE w/ PLANTING PLAN. IF CONFLICTS, PROVIDE HIGHER KEY # - KEY TO PLANT NAME NUMBER OF PLANTS.
- LANDSCAPE CONTRACTOR TO CONSTRUCT PLANT BED AROUND AND UNDER ALL SHRUB PLANTINGS. PROVIDE 4 INCH DEEP BARK MULCH BED WITH CURVILINEAR SHAPE TO OUTSIDE LIMIT OF BRANCHES.
- REFER TO DETAIL SHEETS FOR PLANT MATERIAL INSTALLATION.
- TREE CALIPERS ARE MEASURED AS FOLLOWS:  
4" OR LESS = MEASURE AT 6" ABOVE GROUND  
GREATER THAN 4" = MEASURE 12" ABOVE GROUND
- ALL PLANTING TO BE IN CONFORMANCE WITH THE CITY OF PORTLAND'S TECHNICAL AND DESIGN STANDARDS AND GUIDELINES.



**WALL MOUNT LIGHT**

"PERMALITER" BY HUBBELL LIGHTING. MOUNT 15 FT ABV FIN FLR SEE ARCHITECTURAL ELEVATIONS TYP. PROVIDE FULL "HOUSE SIDE" CUT OFF ALONG SOUTH AND EASTERLY PROPERTY BOUNDARY.

SHIELD DEVICE SHOWN BELOW

Accessories - Order Separately

PERM Photocast, E-Clon type, 122V  
 PER34 Photocast, Button type, 20A, 240, 277V  
 PVLV Full Graft Vase, forced aluminum, bronze finish

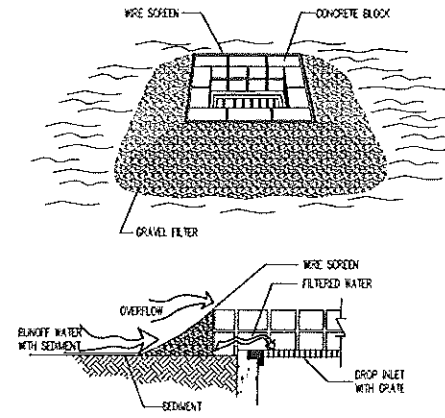
DATE: 8/25/06  
 REVISIONS: REVISED PER PLANNING DEPT. REVIEW 8/25/06  
 REVISED PER PLANNING DEPT. REVIEW 8/25/06  
 NO REVISIONS

DESIGNED: PLO/TME  
 DRAWN: PLO/TME  
 CHECKED: TME  
 SCALE: 1" = 20'  
 DATE: 8/25/06

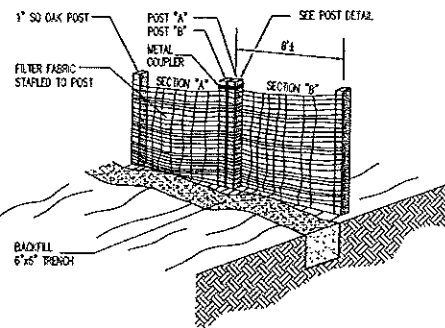
**CARAVAN BEADS**  
 FOREST AVENUE, PORTLAND, MAINE  
 PREPARED FOR:  
**MICHAEL CHAREK ARCHITECTS**  
 25 HARTLEY STREET  
 PORTLAND, MAINE 04103

**LANDSCAPE PLAN**

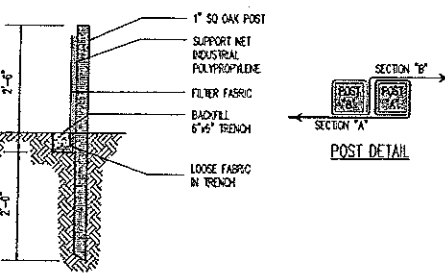
Job No. 4516  
 Drawing  
 C-3



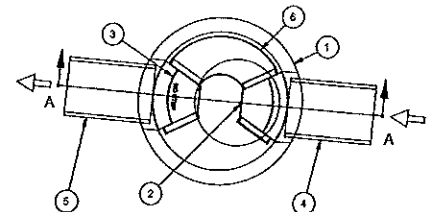
10 BLOCK & GRAVEL DROP INLET SEDIMENT FILTER  
NOT TO SCALE EC-34983-2101-01/21-02



11 PREFABRICATED SILT FENCE  
NOT TO SCALE EC-34983-2101/21-02



12 FIRST DEFENSE STORMWATER DETAIL  
NOT TO SCALE SS-44-FLD001-01/21-02



9 CONCRETE WALK  
NOT TO SCALE EUC-1042/21-05

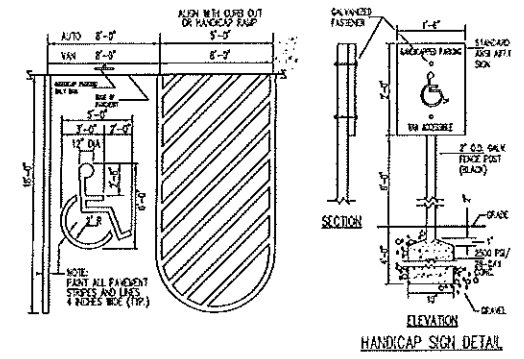
EQUIPMENT PERFORMANCE

THE STORMWATER TREATMENT UNIT SHALL ADHERE TO THE HYDRAULIC PARAMETERS GIVEN IN THE CHART BELOW AND PROVIDE THE REMOVAL EFFICIENCIES AND STORAGE CAPACITIES AS FOLLOWS:

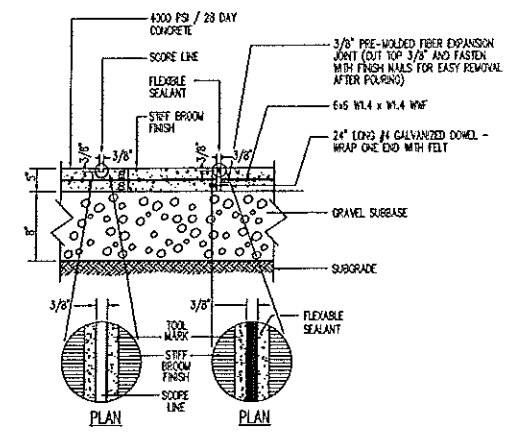
PEAK HYDRAULIC FLOW: 6.0 cfs  
ESTIMATED HEAD LOSS AT 6.0 cfs: 7 in.  
SEDIMENT STORAGE CAPACITY: 1 Cu. yd.  
GIL STORAGE CAPACITY: 180 Gal.

HEADLOSS IS DEFINED AS THE DIFFERENCE BETWEEN STATIC WATER LEVEL AT THE INLET OF THE FIRST DEFENSE TO THE FREE WATER SURFACE IN THE OUTLET PIPE, ASSUMING FREE DISCHARGE.

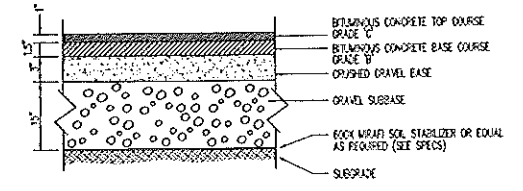
ITEM	SIZE (in)	DESCRIPTION
1	48	I.D. CONCRETE MANHOLE
2		INLET CHUTE (W/ FLOATABLES TRAP)
3		OUTLET CHUTE
4	18	INLET PIPE (BY OTHERS)
5	18	OUTLET PIPE (BY OTHERS)
6		HIGH FLOW BYPASS
7	30	FRAME AND GRATE



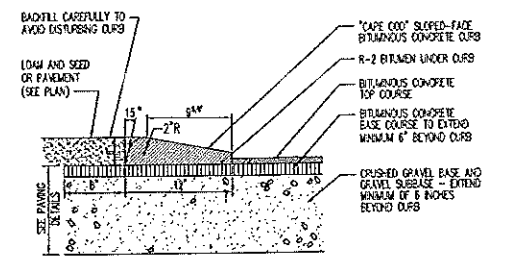
8 ACCESSIBLE PARKING STALL  
NOT TO SCALE NC-3511-8-001/3-17



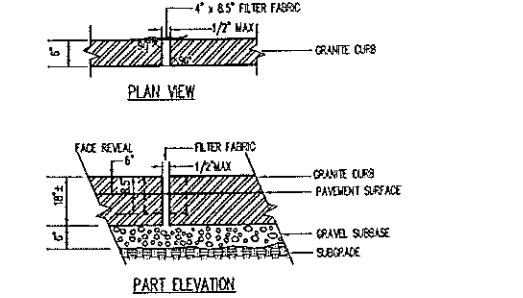
1 TYPICAL SEWER SERVICE LEAD CONNECTION  
NOT TO SCALE SS-SERVICE-CONNECTION/21-05



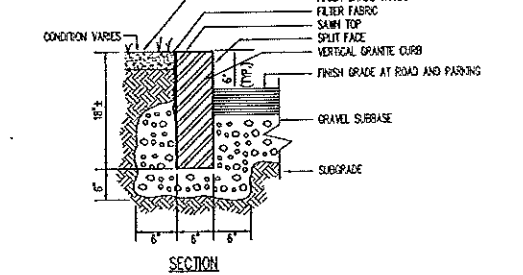
4 BITUMINOUS CONCRETE DRIVE & PARKING  
NOT TO SCALE EC-31-3616-7101/24-15



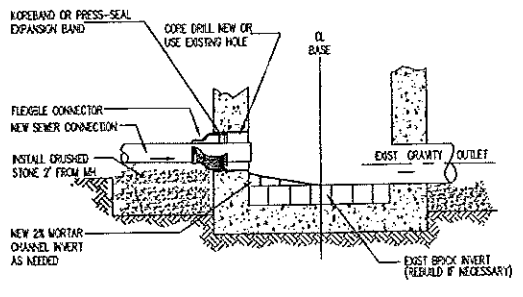
5 "CAPE COD" BITUMINOUS CONCRETE CURB  
NOT TO SCALE CUB-31-3616-01/24-17



2 TYPICAL UNDERGROUND CABLE TRENCH  
NOT TO SCALE UTL-4111-1000/1/21-05

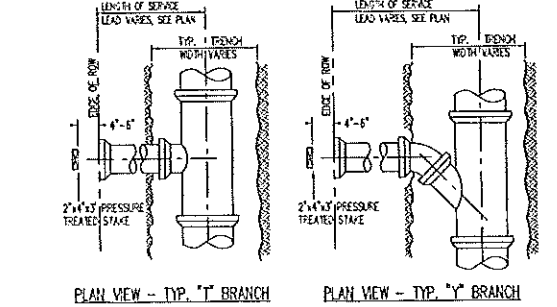


6 VERTICAL GRANITE CURB (MDOT TYPE "1")  
NOT TO SCALE CUB-31-3616-01/24-17

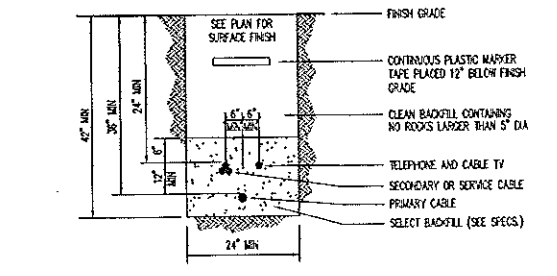


7 FLEXIBLE CONNECTION TO EXISTING MANHOLE  
NOT TO SCALE SS-44-FLD001-01/21-02

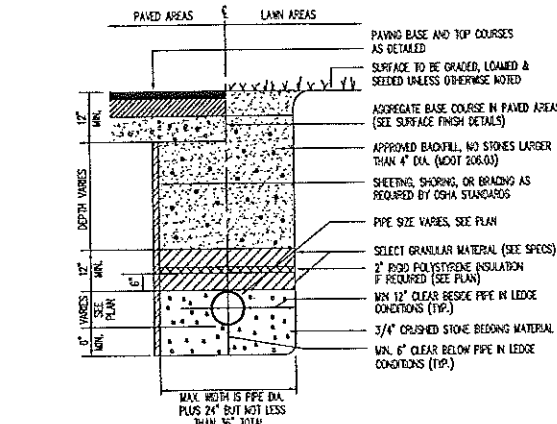
ALL UTILITY SERVICE LEADS SHALL BE INSTALLED TO THE RIGHT-OF-WAY LINE. SANITARY SEWER SERVICE LEADS SHALL BE 6-INCH DIAMETER, STORM GRAB SERVICE LEADS SHALL BE 4-INCH DIAMETER, WATER SERVICE LEADS SHALL BE 1/4-INCH DIAMETER. A PRESSURE-TREATED WOODEN STAKE 2"x4"x5' SHALL BE INSTALLED TO MARK THE D.O. OF EACH SERVICE LEAD.



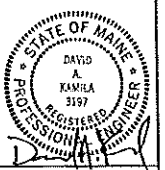
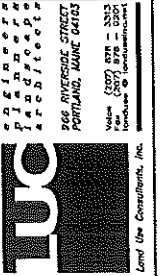
3 TYPICAL PIPE BEDDING DETAIL  
NOT TO SCALE SS-7000/1/21-04



NOTES:  
1. INSTALLATION SHOULD NOT ALLOW THE INTER-TWING OF CABLES.  
2. BEDDING AND BACKFILL SHALL BE FREE OF ROOTS, STAMPS AND OTHER DEBRIS.  
3. INSTALL SCHEDULE 40 OR LIGHT LARGER ALL PAVED AREAS PER APPROPRIATE UTILITY COMPANY SPECIFICATIONS.



3 TYPICAL PIPE BEDDING DETAIL  
NOT TO SCALE SS-7000/1/21-04



CARAVAN BEADS  
FOREST AVENUE, PORTLAND, MAINE  
PREPARED FOR:  
MICHAEL CHARREK ARCHITECTS  
25 HARTLEY STREET  
PORTLAND, MAINE 04103

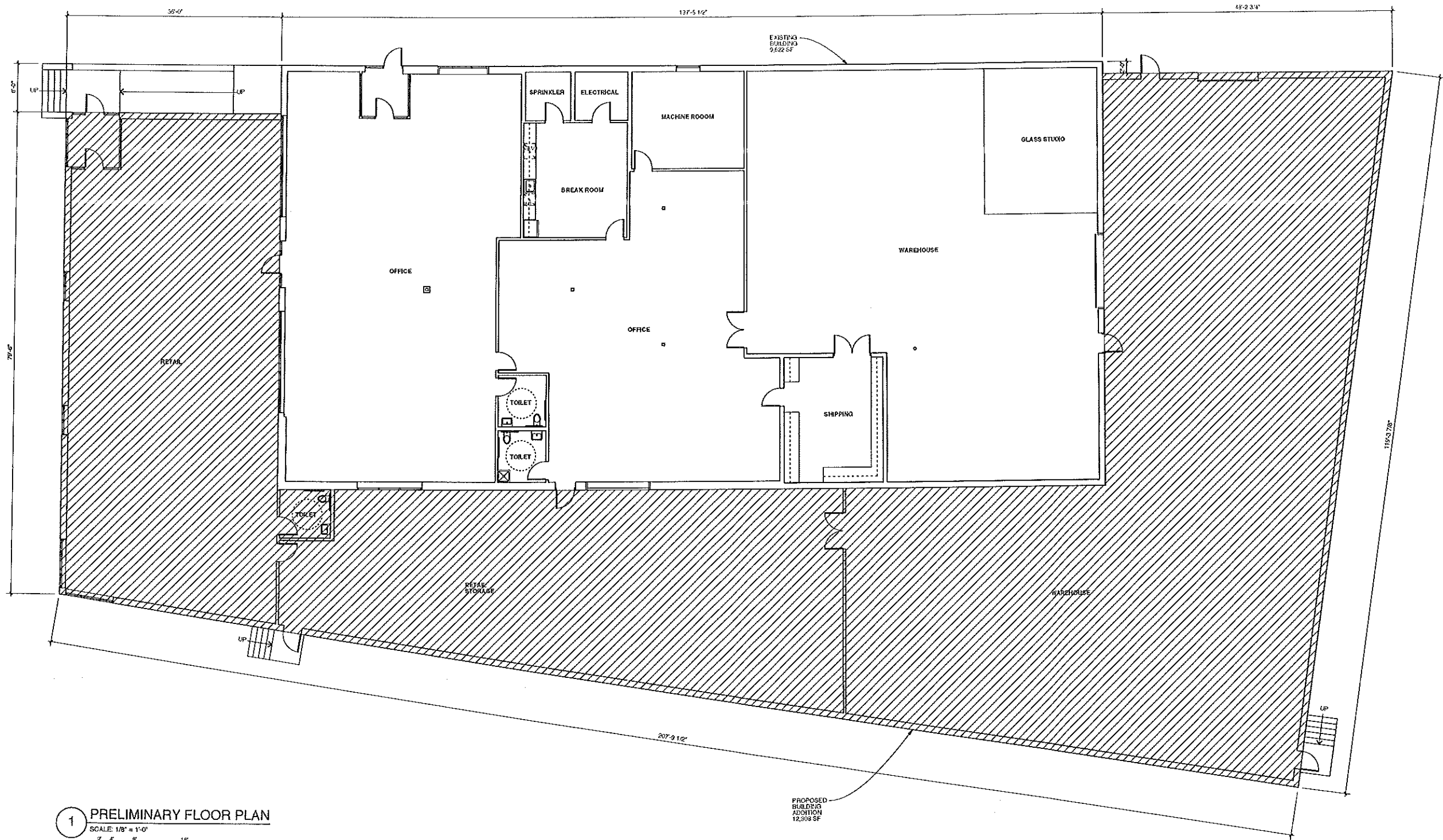
Date	Revision	Revised Per	Planning Dept.	Reviewed	Reviewed	Reviewed

Designed	FLC	Checked	FLC	Scale	Date
				NO SCALE	8/25/06

DETAILS

Job No. 4516  
Sheet  
C-4

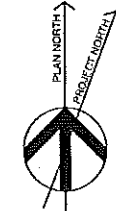




1 PRELIMINARY FLOOR PLAN  
 SCALE: 1/8" = 1'-0"  
 0 2 4 6 8 10

PROPOSED BUILDING ADDITION 12,308 SF

EXISTING BUILDING 9,622 SF



Michael R. Charek  
 Architect

25 Hartley Street  
 Portland, Maine 04103  
 (207) 761-0556

Caravan Beads  
 Building Expansion

915 Forest Avenue  
 Portland, ME 04103

Title  
 PRELIMINARY FLOOR PLAN

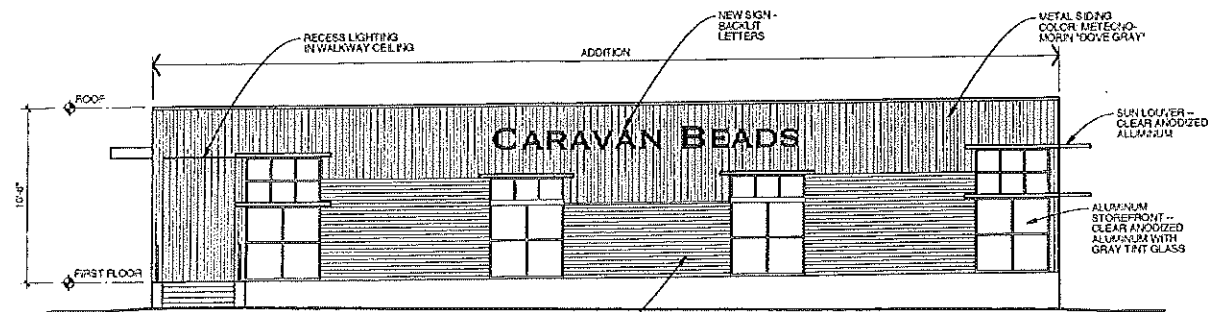
Scale: 1/8" = 1'-0"

Date: 8/25/06

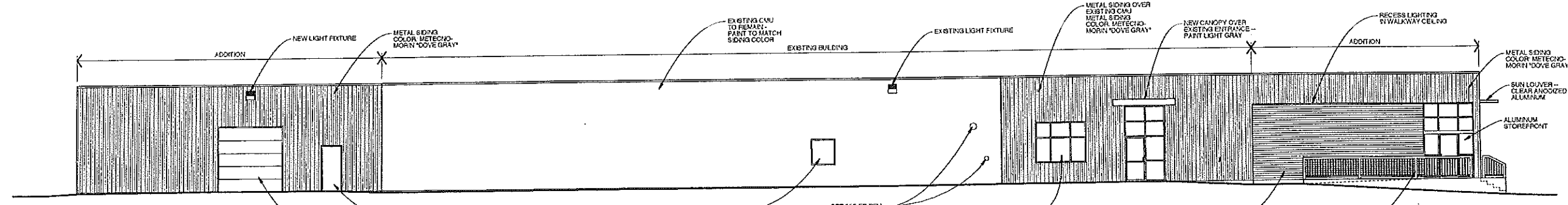
Revisions

Sheet  
**A1**

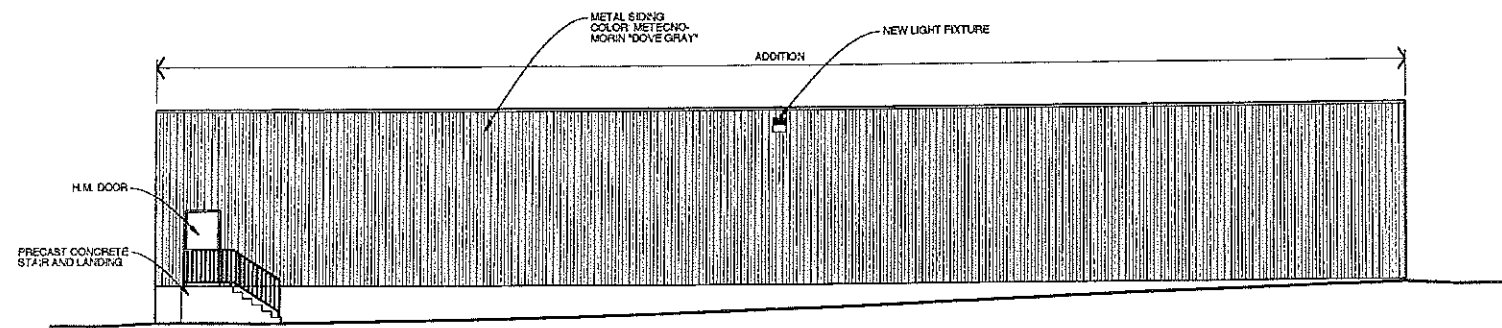




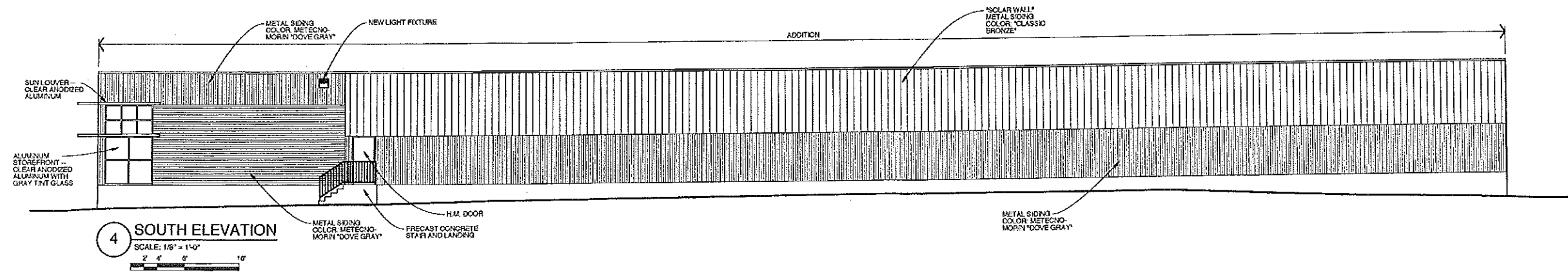
1 WEST ELEVATION  
SCALE: 1/8" = 1'-0"  
2' 4' 8' 16'



2 NORTH ELEVATION  
SCALE: 1/8" = 1'-0"  
2' 4' 8' 16'



3 EAST ELEVATION  
SCALE: 1/8" = 1'-0"  
2' 4' 8' 16'



4 SOUTH ELEVATION  
SCALE: 1/8" = 1'-0"  
2' 4' 8' 16'

NOTE: FINAL COLOR SELECTION TO BE DETERMINED BY OWNER

© 2008 Michael R. Charek, Architect

Michael R. Charek  
Architect

25 Hartley Street  
Portland, Maine 04103  
(207) 761-0556

Caravan Beads  
Building Expansion

915 Forest Avenue  
Portland, ME 04103

Title  
PRELIMINARY ELEVATIONS

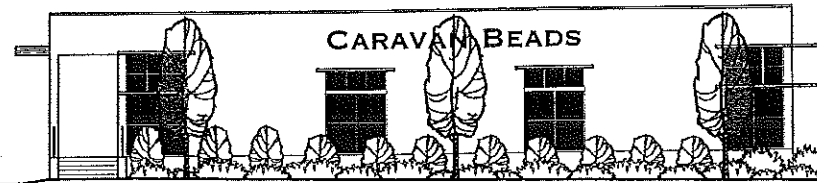
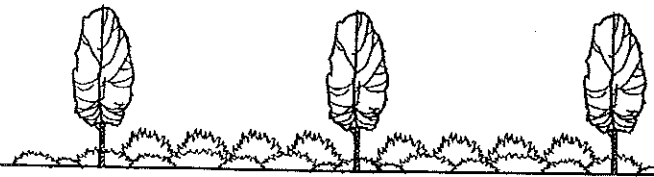
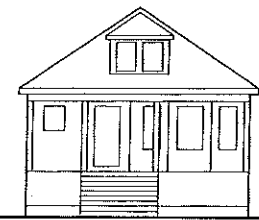
Scale: 1/8" = 1'-0"

Date: 11/8/06

Revisions

Sheet

A2



Michael R. Charek  
Architect

25 Hertley Street  
Portland, Maine 04103  
(207) 761-0556

Caravan Beads  
Building Expansion

915 Forest Avenue  
Portland, ME 04103

**A3**

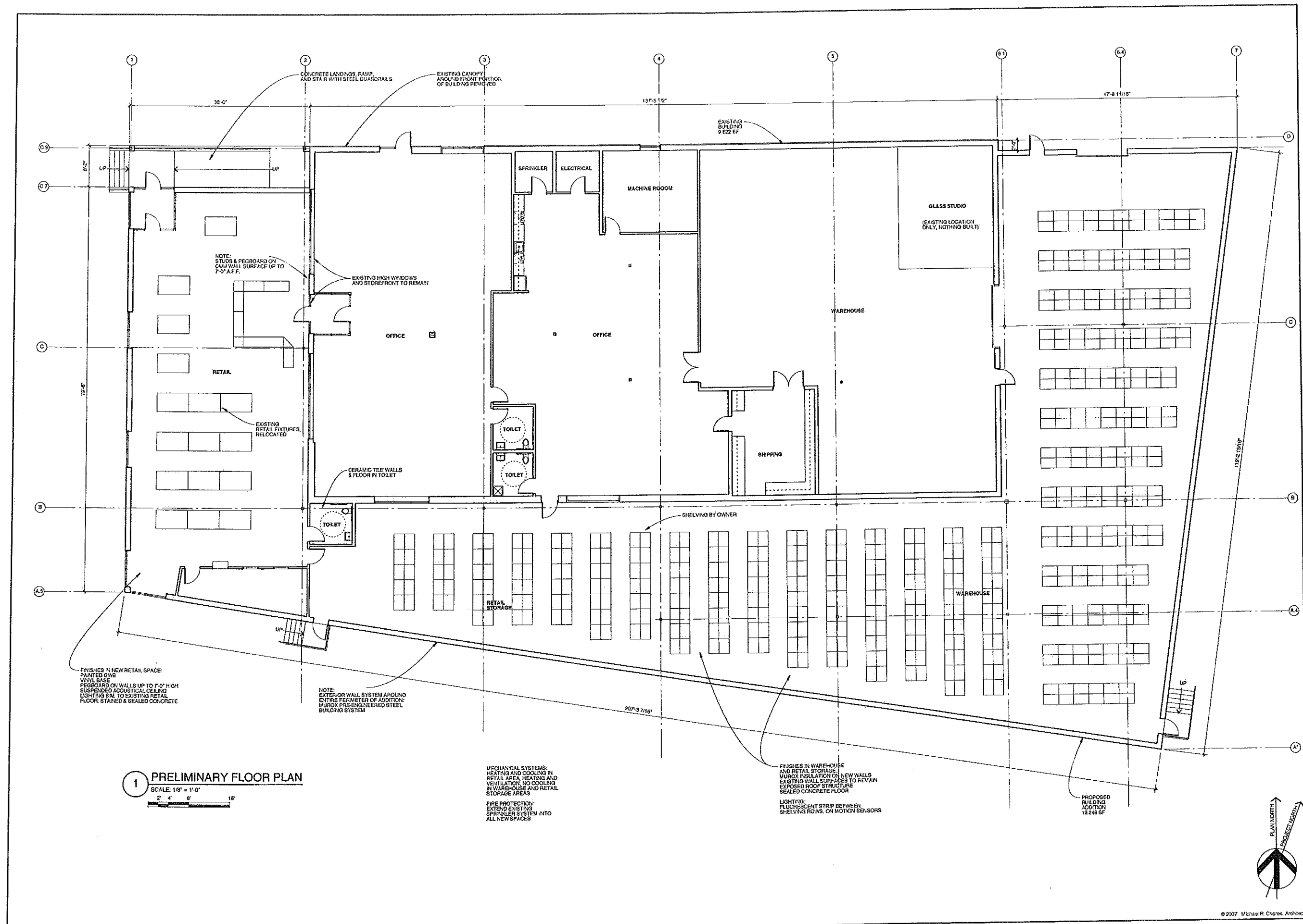
**Michael R. Charek  
Architect**  
25 Hartley Street  
Portland, Maine 04103  
(207) 761-0556

**Caravan Beads  
Building Expansion**  
915 Forest Avenue  
Portland, ME 04103

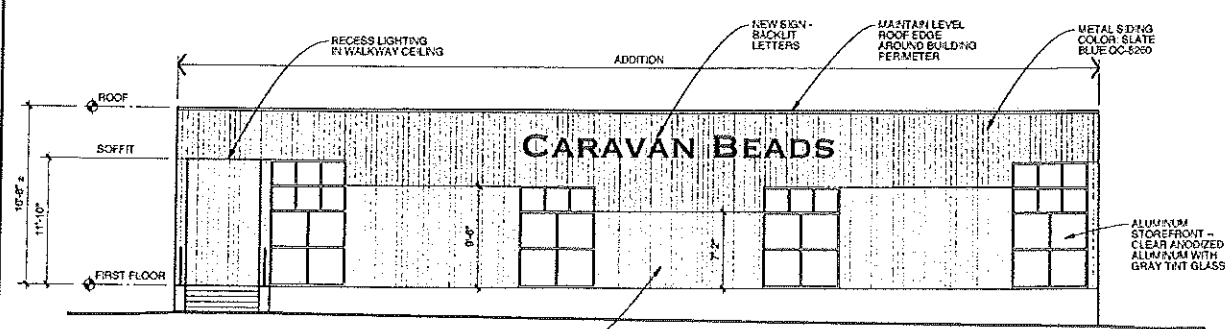
Title  
**PRELIMINARY  
FLOOR PLAN**  
Scale: 1/8" = 1'-0"  
Date: 2/22/07

Revisions

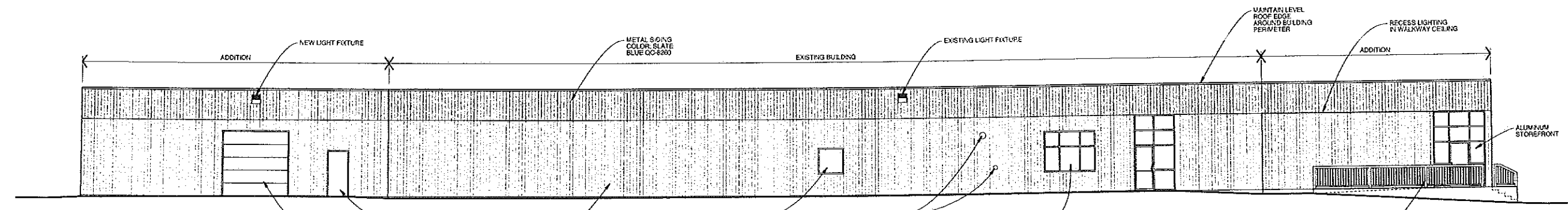
Sheet  
**A1**



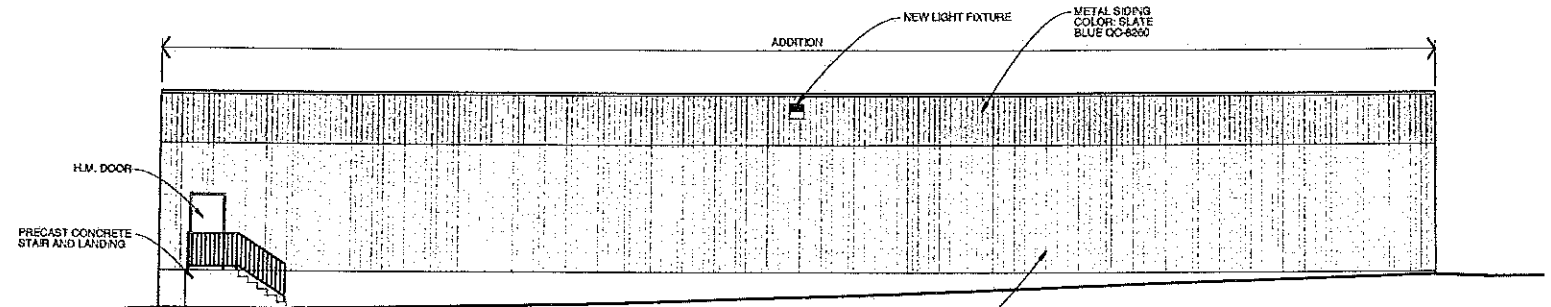
**1 PRELIMINARY FLOOR PLAN**  
SCALE: 1/8" = 1'-0"  
0' 4' 8' 16'



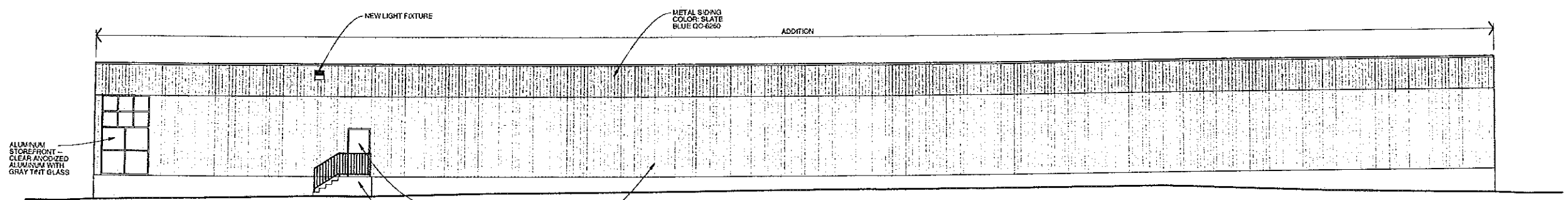
1 WEST ELEVATION  
SCALE: 1/8" = 1'-0"



2 NORTH ELEVATION  
SCALE: 1/8" = 1'-0"



3 EAST ELEVATION  
SCALE: 1/8" = 1'-0"



4 SOUTH ELEVATION  
SCALE: 1/8" = 1'-0"

NOTE: FINAL COLOR SELECTION TO BE DETERMINED BY OWNER

© 2007 Michael R. Charek, Architect

Michael R. Charek  
Architect

25 Hartley Street  
Portland, Maine 04103  
(207) 761-0556

Caravan Beads  
Building Expansion

915 Forest Avenue  
Portland, ME 04103

Title  
PRELIMINARY  
ELEVATIONS

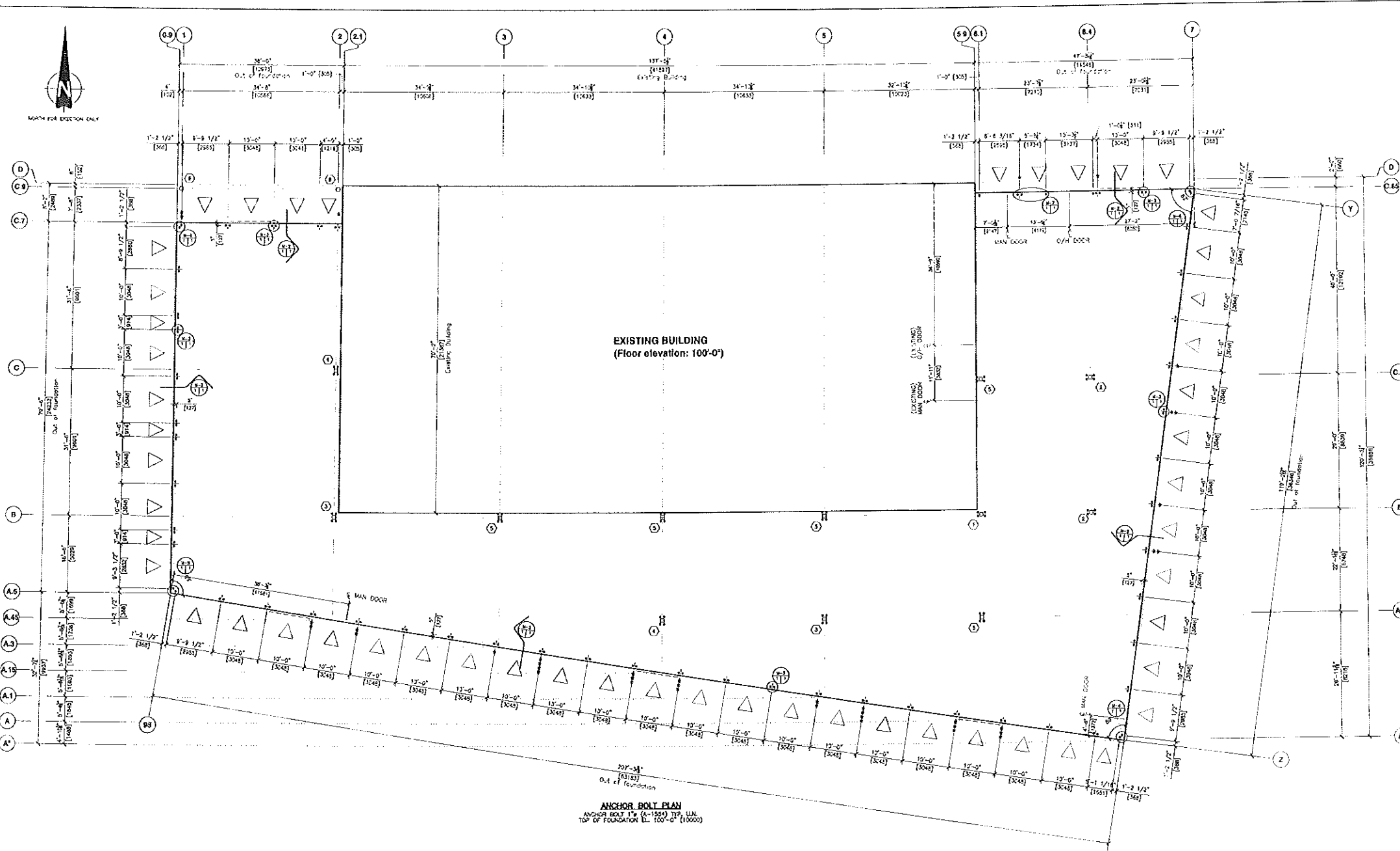
Scale: 1/8" = 1'-0"

Date: 2/22/07

Revisions

Sheet

A2

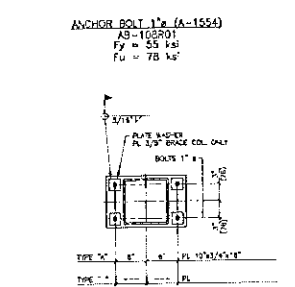
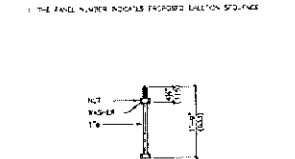


- SECTION LEGEND**
- 1. WALL LOAD (CONCRETE, BRICK, GYP. BOARD)
  - 2. ROOF LOAD (DEAD, LIVE, WIND, SEISMIC)
  - 3. FLOOR LOAD (DEAD, LIVE, WIND, SEISMIC)
  - 4. WIND LOAD (SIDE WALL, END WALL)
  - 5. WIND LOAD (ROOF)
  - 6. WIND LOAD (CORNER)
  - 7. WIND LOAD (GABLE)
  - 8. WIND LOAD (TRUSS)
  - 9. WIND LOAD (DOME)
  - 10. WIND LOAD (TOWER)
  - 11. WIND LOAD (SLOPE)
  - 12. WIND LOAD (CURVE)
  - 13. WIND LOAD (SPECIAL)
  - 14. WIND LOAD (OTHER)

- SECTION NOTES**
1. THE DESIGN OF THE FOUNDATION SHALL BE IN ACCORDANCE WITH THE 2012 INTERNATIONAL BUILDING CODE (IBC) AND THE 2012 INTERNATIONAL GEOTECHNICAL AND SOIL FOUNDATION CODE (IGBC).
  2. THE DESIGN OF THE FOUNDATION SHALL BE IN ACCORDANCE WITH THE 2012 INTERNATIONAL BUILDING CODE (IBC) AND THE 2012 INTERNATIONAL GEOTECHNICAL AND SOIL FOUNDATION CODE (IGBC).
  3. THE DESIGN OF THE FOUNDATION SHALL BE IN ACCORDANCE WITH THE 2012 INTERNATIONAL BUILDING CODE (IBC) AND THE 2012 INTERNATIONAL GEOTECHNICAL AND SOIL FOUNDATION CODE (IGBC).
  4. THE DESIGN OF THE FOUNDATION SHALL BE IN ACCORDANCE WITH THE 2012 INTERNATIONAL BUILDING CODE (IBC) AND THE 2012 INTERNATIONAL GEOTECHNICAL AND SOIL FOUNDATION CODE (IGBC).
  5. THE DESIGN OF THE FOUNDATION SHALL BE IN ACCORDANCE WITH THE 2012 INTERNATIONAL BUILDING CODE (IBC) AND THE 2012 INTERNATIONAL GEOTECHNICAL AND SOIL FOUNDATION CODE (IGBC).

- FOUNDATION NOTES**
1. FOUNDATION SHALL BE DESIGNED FOR ALL APPLIED LOADS.
  2. FOUNDATION SHALL BE DESIGNED FOR ALL APPLIED LOADS.
  3. FOUNDATION SHALL BE DESIGNED FOR ALL APPLIED LOADS.
  4. FOUNDATION SHALL BE DESIGNED FOR ALL APPLIED LOADS.
  5. FOUNDATION SHALL BE DESIGNED FOR ALL APPLIED LOADS.
  6. FOUNDATION SHALL BE DESIGNED FOR ALL APPLIED LOADS.
  7. FOUNDATION SHALL BE DESIGNED FOR ALL APPLIED LOADS.
  8. FOUNDATION SHALL BE DESIGNED FOR ALL APPLIED LOADS.
  9. FOUNDATION SHALL BE DESIGNED FOR ALL APPLIED LOADS.
  10. FOUNDATION SHALL BE DESIGNED FOR ALL APPLIED LOADS.

- ANCHOR BOLT PLAN**
- ANCHOR BOLT 1/2" (A-1554) 210 L IN. TOP OF FOUNDATION EL. 100'-0" (10000)



**INTERIOR COLUMN SCHEDULE**

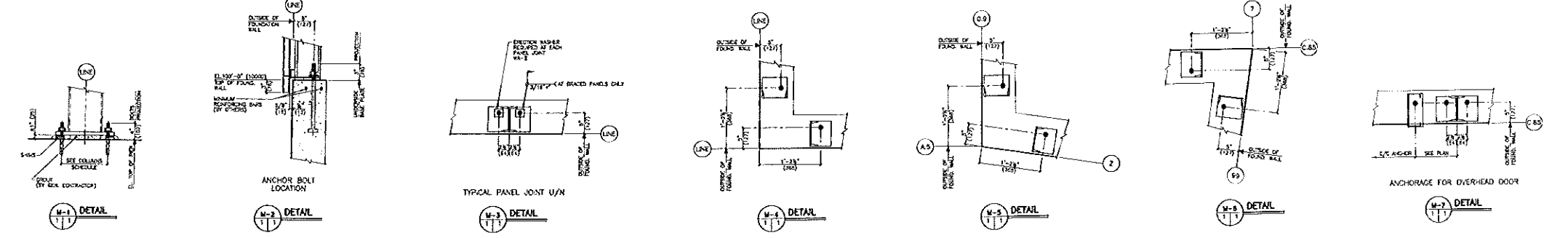
NOTE: ALL SERVICE LOADS ARE EXPRESSED IN KIP.

NO.	DIMENSIONS	REF. DETAIL	BASE PLATE TYPE	VERTICAL LOAD			WIND			SEISMIC		
				EL.	LL	UPLIFT	H	V	H	V	H	V
1	10'-0" x 10'-0"	M-1	10'-0"	14.0	33.5	6.2	---	---	---	---	---	---
2	10'-0" x 10'-0"	M-2	10'-0"	11.8	25.1	3.2	---	---	---	---	---	---
3	10'-0" x 10'-0"	M-3	10'-0"	12.3	28.4	4.5	---	---	---	---	---	---
4	10'-0" x 10'-0"	M-4	10'-0"	8.2	18.7	3.7	---	---	---	---	---	---
5	10'-0" x 10'-0"	M-5	10'-0"	8.2	18.7	3.7	---	---	---	---	---	---
6	10'-0" x 10'-0"	M-6	10'-0"	10.0	22.4	4.5	---	---	---	---	---	---
7	10'-0" x 10'-0"	M-7	10'-0"	3.8	8.0	1.7	---	---	---	---	---	---

**REACTIONS ON FOUNDATION FOR "MURX" PANEL COLUMN @ 10'-0" C/C**

NOTE: ALL SERVICE LOADS ARE EXPRESSED IN KIP.

NO.	DIMENSIONS	REF. DETAIL	SERVICED LOAD			REACTION AT THE BASE			BRIDGE REACTION			
			EL.	LL	UPLIFT	H	V	H	V	H	V	
1	10'-0" x 10'-0"	M-1	10'-0"	14.0	33.5	6.2	---	---	---	---	---	---
2	10'-0" x 10'-0"	M-2	10'-0"	11.8	25.1	3.2	---	---	---	---	---	---
3	10'-0" x 10'-0"	M-3	10'-0"	12.3	28.4	4.5	---	---	---	---	---	---
4	10'-0" x 10'-0"	M-4	10'-0"	8.2	18.7	3.7	---	---	---	---	---	---
5	10'-0" x 10'-0"	M-5	10'-0"	8.2	18.7	3.7	---	---	---	---	---	---
6	10'-0" x 10'-0"	M-6	10'-0"	10.0	22.4	4.5	---	---	---	---	---	---
7	10'-0" x 10'-0"	M-7	10'-0"	3.8	8.0	1.7	---	---	---	---	---	---



**ACCEPTED**

**FOR APPROVAL**  
NOT TO BE USED FOR CONSTRUCTION

**Legend anchor bolt plan**

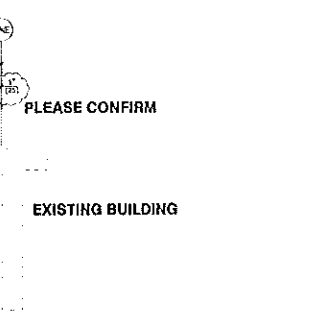
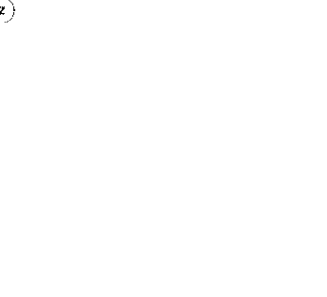
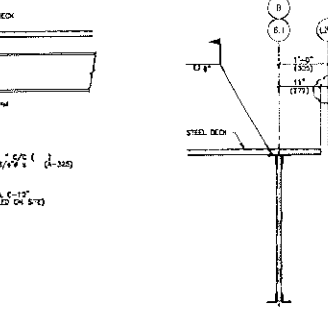
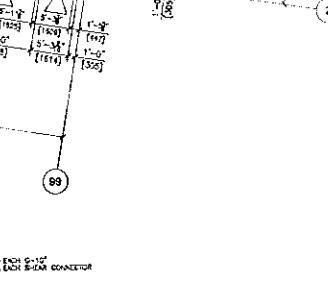
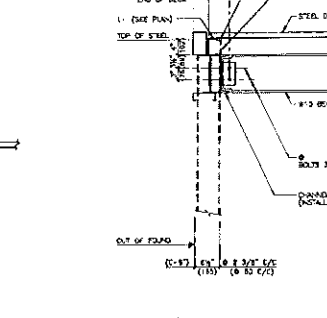
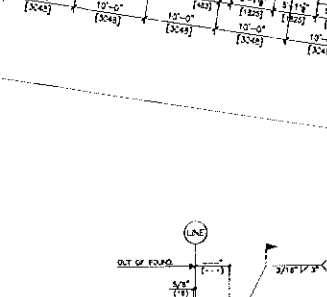
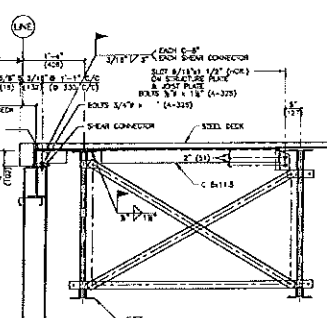
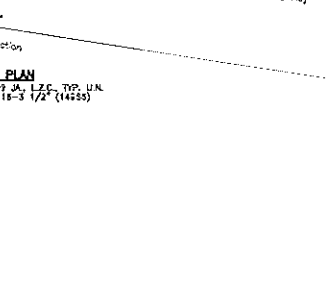
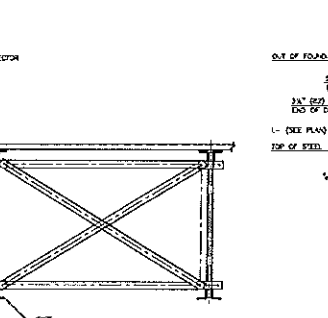
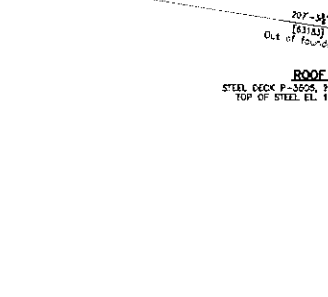
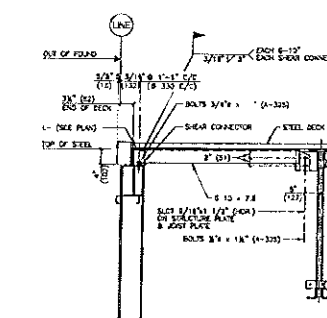
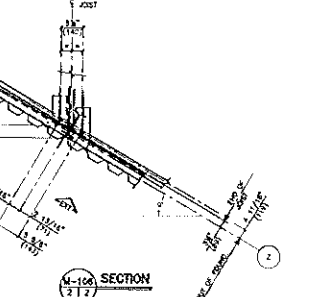
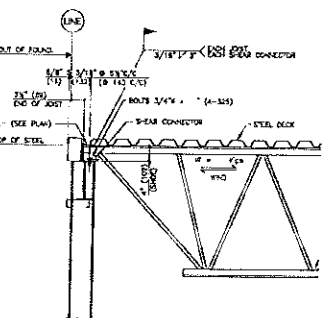
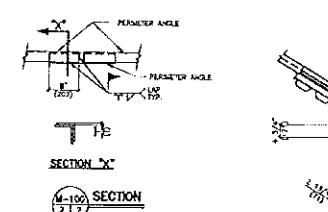
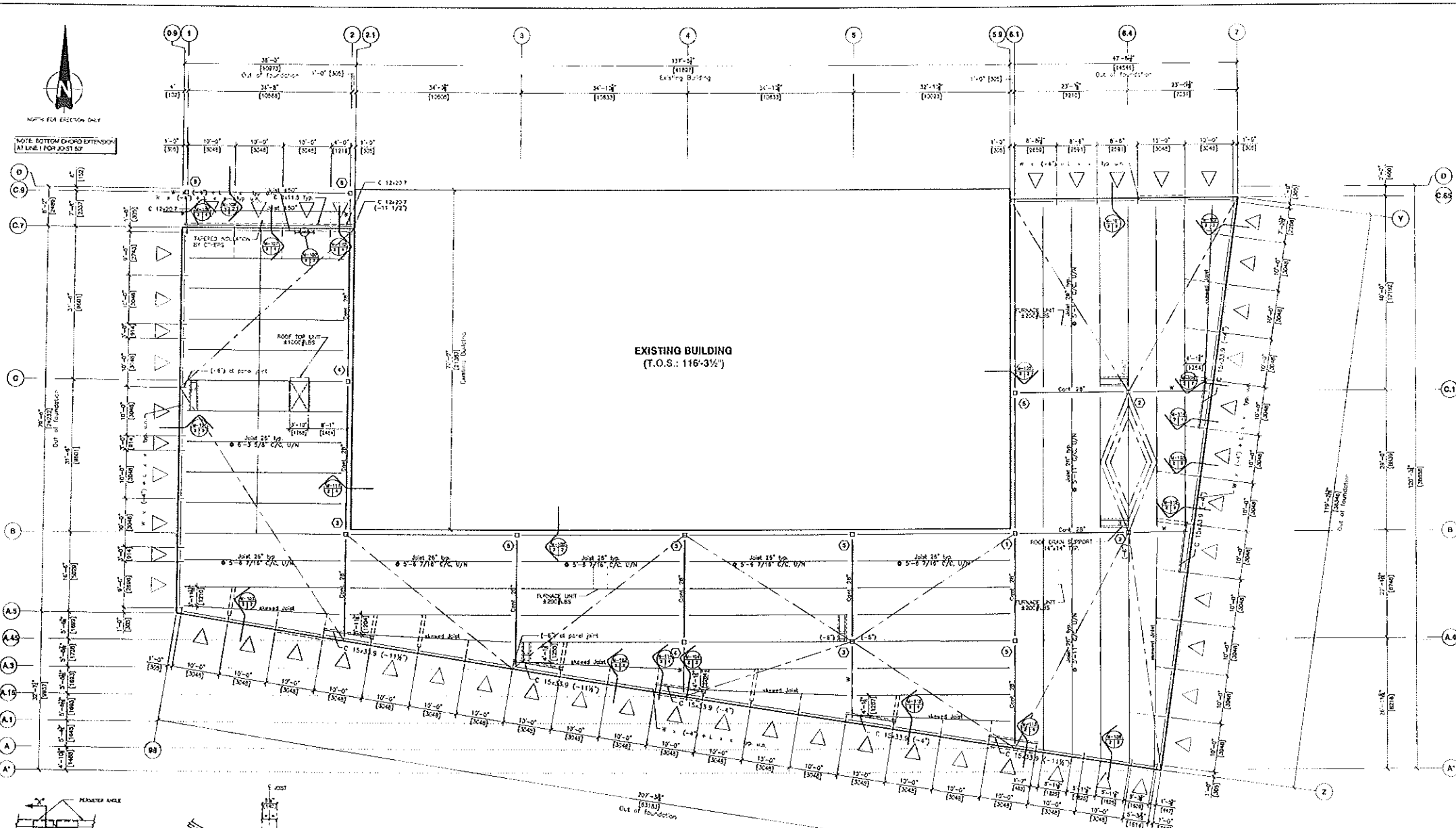
NO = Typical  
U/W = Unbraced wall  
A/B = Full to edge  
C = Center  
D = Corner  
E = End  
F = Face  
G = Gable  
H = Half  
I = Interior  
J = Joint  
K = Key  
L = Line  
M = Member  
N = Note  
O = Other  
P = Panel  
Q = Quarter  
R = Ridge  
S = Side  
T = Top  
U = Under  
V = Vertical  
W = Wall  
X = X-axis  
Y = Y-axis  
Z = Zone

**MURX**  
High Performance Buildings  
2001 12th Ave. South  
Portland, ME 04106  
Tel: (413) 225-1400

**CARAVAN BEADS**  
PORTLAND, MAINE  
U.S.A.

**Anchor bolts plan**  
4 details

H00727 1 of 4



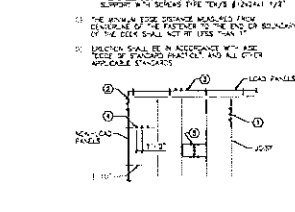
- DESIGN LOADS**
- 1. DEAD LOAD (INCLUDING DECK, STRUCTURE) 15 psf
  - 2. ROOF LIVE LOAD (AS PER LOCAL CODE) 20 psf
  - 3. WIND LOAD (AS PER LOCAL CODE) 15 psf
  - 4. SNOW LOAD (AS PER LOCAL CODE) 15 psf
  - 5. SEISMIC LOAD (AS PER LOCAL CODE) 0.15

- DESIGN NOTES**
1. ALL STEEL SHALL BE A36 UNLESS OTHERWISE SPECIFIED.
  2. ALL WELDS SHALL BE E70XX UNLESS OTHERWISE SPECIFIED.
  3. ALL BOLTS SHALL BE A325 UNLESS OTHERWISE SPECIFIED.
  4. ALL DIMENSIONS SHALL BE IN FEET AND INCHES UNLESS OTHERWISE SPECIFIED.
  5. ALL DIMENSIONS SHALL BE TO THE CENTERLINE UNLESS OTHERWISE SPECIFIED.
  6. ALL DIMENSIONS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.
  7. ALL DIMENSIONS SHALL BE TO THE CENTERLINE UNLESS OTHERWISE SPECIFIED.
  8. ALL DIMENSIONS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.
  9. ALL DIMENSIONS SHALL BE TO THE CENTERLINE UNLESS OTHERWISE SPECIFIED.
  10. ALL DIMENSIONS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.

- STEEL DECK**
- 1. THE STEEL DECK SHALL BE TYPE 3025, 22 GA, L-20, TYP. U.S.A.
  - 2. THE STEEL DECK SHALL BE TYPE 112-3 1/2, 14 GA, TYP. U.S.A.
  - 3. THE STEEL DECK SHALL BE TYPE 112-3 1/2, 14 GA, TYP. U.S.A.
  - 4. THE STEEL DECK SHALL BE TYPE 112-3 1/2, 14 GA, TYP. U.S.A.
  - 5. THE STEEL DECK SHALL BE TYPE 112-3 1/2, 14 GA, TYP. U.S.A.

- ROOF DECK EXPANSION**
1. THE ROOF DECK IS REQUIRED TO ACT AS A LATERAL FORCE RESISTING SYSTEM.
  2. THE ROOF DECK SHALL BE WELDED TO THE STRUCTURAL FRAMING MEMBER WITH E70XX ELECTRODES.
  3. ALL WELDS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.
  4. USE A MINIMUM WELD SIZE OF 1/4" UNLESS OTHERWISE SPECIFIED.
  5. WELDS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.
  6. WELDS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.
  7. WELDS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.
  8. WELDS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.
  9. WELDS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.
  10. WELDS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.

- GENERAL NOTES FOR DECK**
1. THE ROOF DECK IS REQUIRED TO ACT AS A LATERAL FORCE RESISTING SYSTEM.
  2. THE ROOF DECK SHALL BE WELDED TO THE STRUCTURAL FRAMING MEMBER WITH E70XX ELECTRODES.
  3. ALL WELDS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.
  4. USE A MINIMUM WELD SIZE OF 1/4" UNLESS OTHERWISE SPECIFIED.
  5. WELDS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.
  6. WELDS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.
  7. WELDS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.
  8. WELDS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.
  9. WELDS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.
  10. WELDS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.



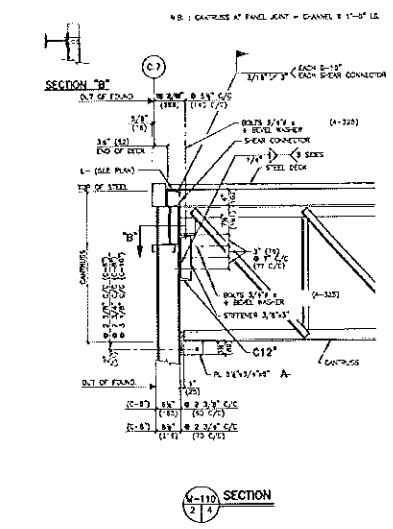
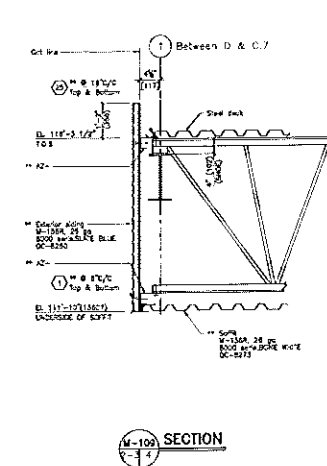
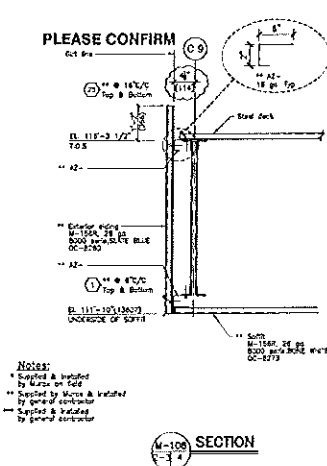
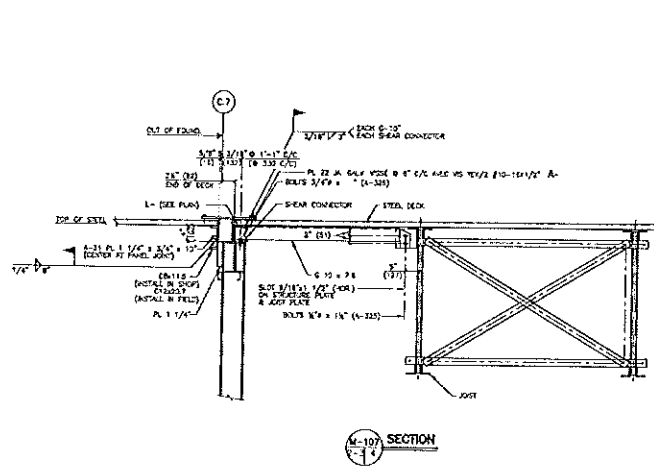
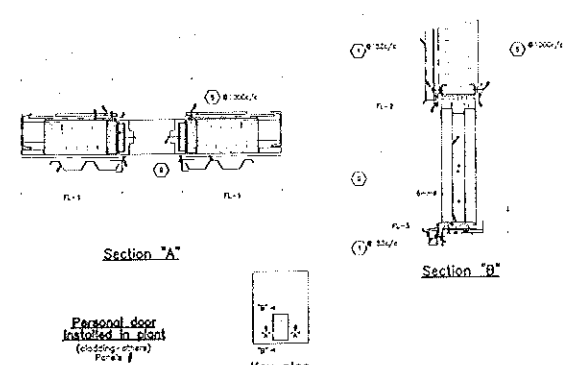
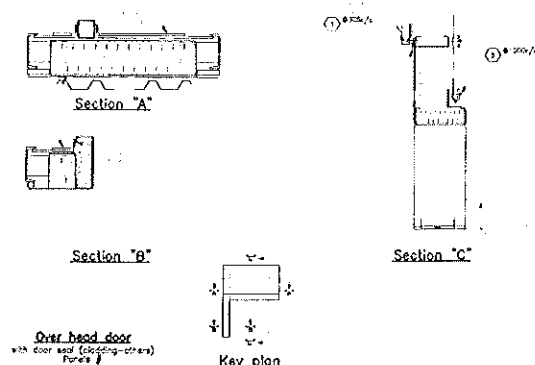
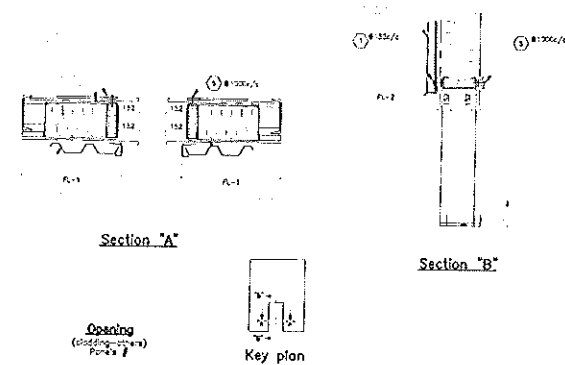
- TRANSVERSE FRAMING**
1. FRAMING MEMBER SHALL BE WELDED TO THE STRUCTURAL FRAMING MEMBER WITH E70XX ELECTRODES.
  2. ALL WELDS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.
  3. USE A MINIMUM WELD SIZE OF 1/4" UNLESS OTHERWISE SPECIFIED.
  4. WELDS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.
  5. WELDS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.
  6. WELDS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.
  7. WELDS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.
  8. WELDS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.
  9. WELDS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.
  10. WELDS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.

- SEAM LAPPING**
- 1. SEAM LAPPING SHALL BE WELDED WITH E70XX ELECTRODES.
  - 2. ALL WELDS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.
  - 3. USE A MINIMUM WELD SIZE OF 1/4" UNLESS OTHERWISE SPECIFIED.
  - 4. WELDS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.
  - 5. WELDS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.
  - 6. WELDS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.
  - 7. WELDS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.
  - 8. WELDS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.
  - 9. WELDS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.
  - 10. WELDS SHALL BE TO THE FACE UNLESS OTHERWISE SPECIFIED.

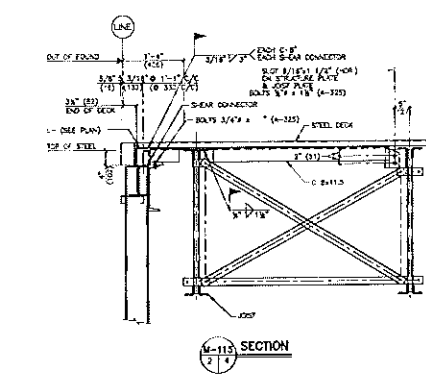
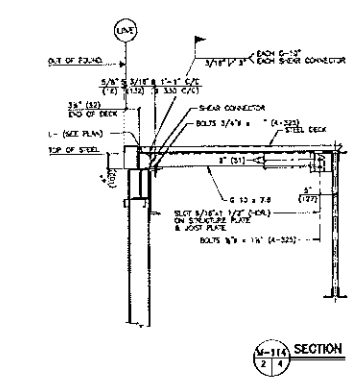
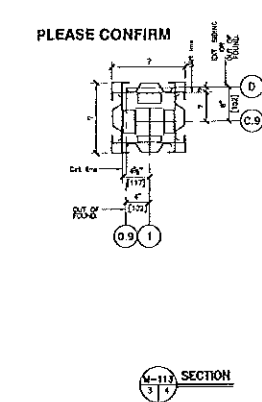
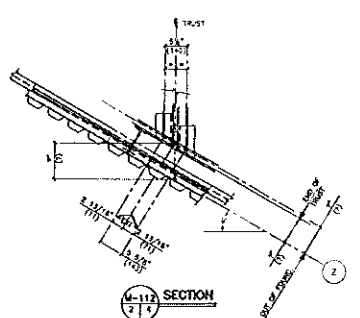
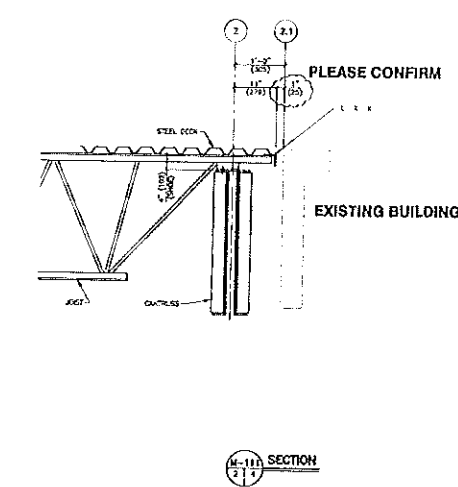
- PERMITTING**
1. ATTACH DECK TO FRAMING MEMBER AT EACH RB.
  2. ATTACH DECK TO FRAMING MEMBER AT EACH RB.
  3. ATTACH DECK TO FRAMING MEMBER AT EACH RB.
  4. ATTACH DECK TO FRAMING MEMBER AT EACH RB.
- SEAM CONNECTIONS**
- 1. PROVIDE CONNECTION BETWEEN DECK AND EACH SEAM CONNECTION WITH:
  - 2. SPOT WELD 3/16"
  - 3. SPOT WELD 3/16"

PLEASE CONFIRM  
EXISTING BUILDING

ACCEPTED	FOR APPROVAL NOT TO BE USED FOR CONSTRUCTION	Legend and notes	M.B.M.C.	MUROX High Performance Buildings 2200 12th Ave. Portland, ME 04106 Tel: (617) 826-4328	CARAVAN BEADS PORTLAND, MAINE U.S.A.	Roof plan & details
Notes: - The present drawings must be returned to Murox's office before 03/22/02 in order to meet the delivery schedule. - Client to make Murox aware if he needs any documents other than wet stamped drawings for his building permit.			02-09-07			H00727 2 of 4 A



PLEASE CONFIRM THE LENGTH OF CLADDING  
NOTE: SEND IN FIELD (50) M-156R, 26 ga. SLATE BLUE QC-8260 OF 5'-7 1/2" LONG.  
(50) M-156R, 26 ga. BONE WHITE QC-8273 OF 12'-4" LONG.  
+ FLASHING TO COVER THE EXISTING PART.



ACCEPTED DATE: _____ BY: _____	FOR APPROVAL NOT TO BE USED FOR CONSTRUCTION	U.S.N.C. G.L. DATE: _____ BY: _____	<b>MUROX</b> High Performance Buildings 228 1/2 P.A. South Street Portland, Maine 04101 Tel: (617) 236-6231 Fax: (617) 236-4821	<b>CARAVAN BEADS</b> PORTLAND, MAINE U.S.A.	Shop-fishing & details H00727 4 of 4 A
	Notes: - The present drawings must be referred to Murox's office before 02/27/92 in order to meet the delivery schedule. - Client to make Murox aware if he needs any documents other than set stamped drawings for his building permit.				