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Site Information:

<i>Customer:</i> Aroostook Trusses, Inc.	<i>Job Number:</i> HLY30913
<i>Job Description:</i> Rainbow - Jones, Rich and Barnes	
<i>Address:</i>	<i>City, State, Zip:</i> Portland, ME

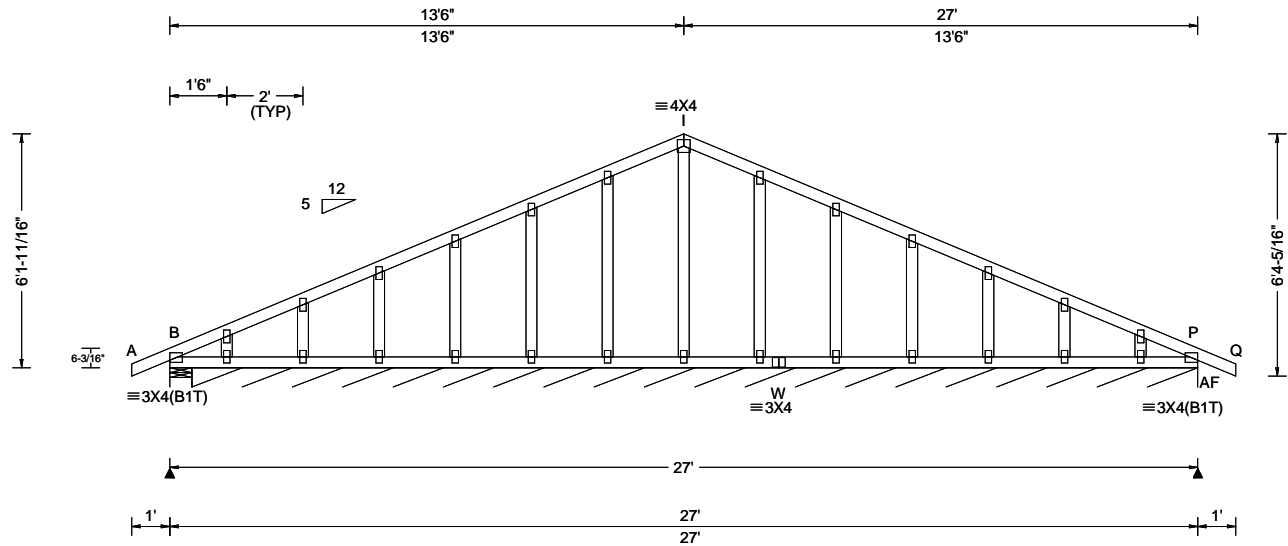
Job Engineering Criteria:

<i>Design Code:</i> IRC 2009	<i>View Version:</i> 16.02.01.0314.13	<i>JRef #:</i> 1W5575540002
<i>Wind Standard:</i> ASCE 7-05	<i>Wind Speed (mph):</i> 100	<i>Roof Load (psf):</i> 46.20-10.00- 0.00- <i>Floor Load (psf):</i> None 10.00

This package contains a job notes page, 6 truss drawings and 4 details.

Item	Seal #	Truss
1	290.17.1438.58997	1C1G
3	290.17.1440.47893	2S1
5	290.17.1441.51547	2S2

Item	Seal #	Truss
2	290.17.1439.49900	1S1
4	290.17.1441.35313	2S1G
6	290.17.1442.08980	2S3



Loading Criteria (psf) TCLL: 46.20 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 66.20 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 *	Wind Criteria Wind Std: ASCE 7-05 Speed: 100 mph Enclosure: Closed Category: II EXP: C Mean Height: 15.00 ft TCCL: 6.0 psf BCDL: 6.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any I: 1.0 GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 60.0 Ct: 1.1 CAT: II Pf: 46.2 Ce: 1.0 Lu: - Cs: not used Snow Duration: 1.15 Code / Misc Criteria Bldg Code: IRC 2009 TPI Std: 2007 Rep Factors Used: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.002 H 999 360 VERT(TL): 0.003 H 999 240 HORZ(LL): 0.002 N - - HORZ(TL): 0.003 M - - Creep Factor: 1.5 Max TC CSI: 0.117 Max BC CSI: 0.033 Max Web CSI: 0.160 VIEW Ver: 16.02.01D.0314.13	▲ Maximum Reactions (lbs), or *PLF <table border="1"> <thead> <tr> <th>Loc</th> <th>R</th> <th>/U</th> <th>/Rw</th> <th>/Rh</th> <th>/RL</th> <th>/W</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>269</td> <td>/30</td> <td>/49</td> <td>/-</td> <td>/144</td> <td>/7.0</td> </tr> <tr> <td>AF*138</td> <td>/11</td> <td>/26</td> <td>/-</td> <td>/-</td> <td>/317</td> <td></td> </tr> </tbody> </table> Wind reactions based on MWFRS B Min Brg Width Req = 1.5 AF Min Brg Width Req = - Bearings B & B are a rigid surface. Members not listed have forces less than 375#	Loc	R	/U	/Rw	/Rh	/RL	/W	B	269	/30	/49	/-	/144	/7.0	AF*138	/11	/26	/-	/-	/317	
Loc	R	/U	/Rw	/Rh	/RL	/W																			
B	269	/30	/49	/-	/144	/7.0																			
AF*138	/11	/26	/-	/-	/317																				

Lumber

Top chord 2x4 SPF #1/#2
 Bot chord 2x4 SPF #1/#2
 Webs 2x4 SPF #1/#2

Laterally brace end wall. Top wall plate is not braced by truss. Top of end wall shall be laterally braced by ceiling diaphragm or by other means as specified by the building designer.

Plating Notes

All plates are 2X4 except as noted.

Loading

Bottom chord checked for 10.00 psf non-concurrent bottom chord live load applied per IRC-09 section 301.5.

Truss designed for unbalanced snow loads.

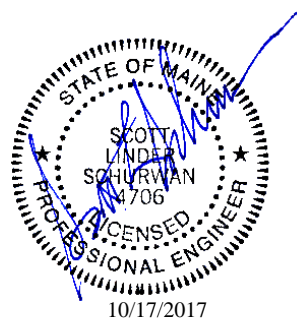
Wind

Wind loads based on MWFRS with additional C&C member design.

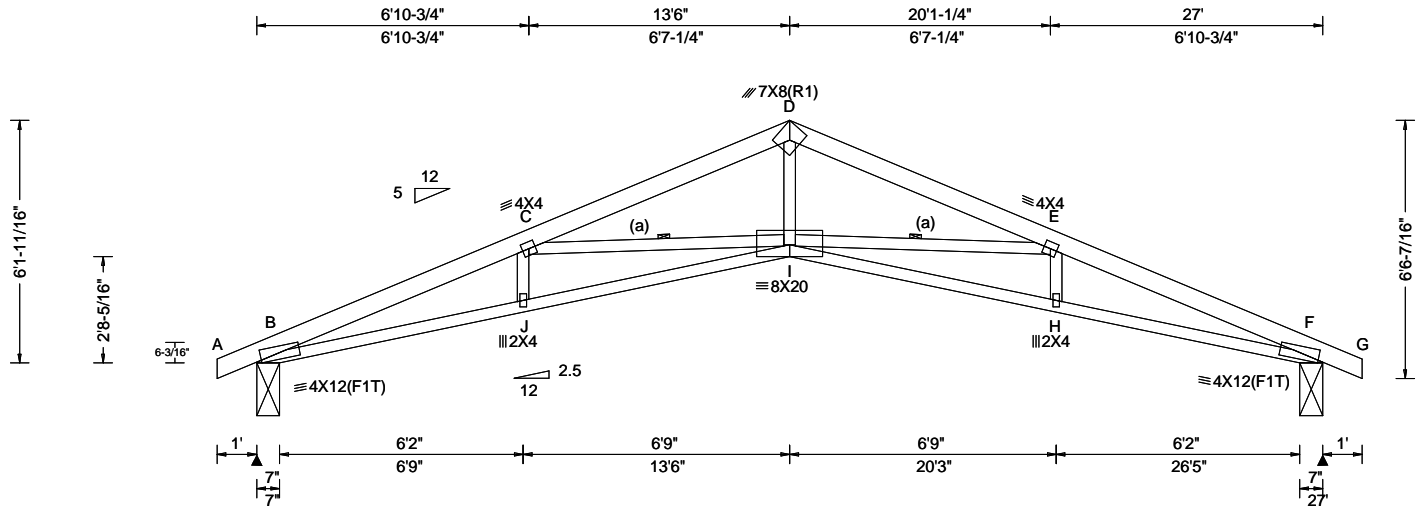
Additional Notes

See DWGS A10015051014, GBLLETIN1014, & GABRST051014 for gable wind bracing and other requirements.

Top Chord overhang(s) may be field trimmed.



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 For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinet.org; SBCA: www.sbcindustry.com; ICC: www.iccsafe.org



Loading Criteria (psf) TCLL: 46.20 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 66.20 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-05 Speed: 100 mph Enclosure: Closed Category: II EXP: C Mean Height: 15.00 ft TCCL: 6.0 psf BCCL: 6.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any I: 1.0 GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 60.0 Ct: 1.1 CAT: II Pf: 46.2 Ce: 1.0 Lu: - Cs: not used Snow Duration: 1.15 <hr/> Code / Misc Criteria Bldg Code: IRC 2009 TPI Std: 2007 Rep Factors Used: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.500 I 641 360 VERT(TL): 0.834 I 384 240 HORZ(LL): 0.332 H - - HORZ(TL): 0.556 H - - Creep Factor: 1.5 Max TC CSI: 0.602 Max BC CSI: 0.879 Max Web CSI: 0.826 <hr/> VIEW Ver: 16.02.01D.0314.13	▲ Maximum Reactions (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Loc</th> <th>R</th> <th>/U</th> <th>/Rw</th> <th>/Rh</th> <th>/RL</th> <th>/W</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>1933</td> <td>/147</td> <td>/419</td> <td>-</td> <td>/144</td> <td>/7.0</td> </tr> <tr> <td>F</td> <td>1933</td> <td>/147</td> <td>/419</td> <td>-</td> <td>-</td> <td>/7.0</td> </tr> </tbody> </table> Wind reactions based on MWFRS B Min Brg Width Req = 2.4 F Min Brg Width Req = 2.4 Bearings B & F are a rigid surface. <hr/> Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>1238 - 5854</td> <td>D - E</td> <td>993 - 4613</td> </tr> <tr> <td>C - D</td> <td>1002 - 4613</td> <td>E - F</td> <td>1245 - 5854</td> </tr> </tbody> </table>	Loc	R	/U	/Rw	/Rh	/RL	/W	B	1933	/147	/419	-	/144	/7.0	F	1933	/147	/419	-	-	/7.0	Chords	Tens.Comp.	Chords	Tens. Comp.	B - C	1238 - 5854	D - E	993 - 4613	C - D	1002 - 4613	E - F	1245 - 5854
Loc	R	/U	/Rw	/Rh	/RL	/W																															
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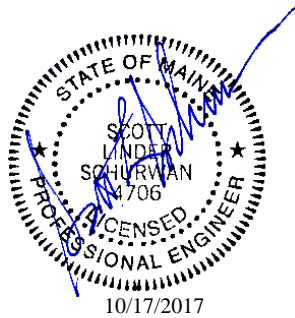
Lumber
 Top chord 2x6 SPF 2100f-1.8E
 Bot chord 2x4 SPF 2100f-1.8E
 Webs 2x4 SPF #1/#2

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Loading
 Bottom chord checked for 10.00 psf non-concurrent bottom chord live load applied per IRC-09 section 301.5.
 Truss designed for unbalanced snow loads.

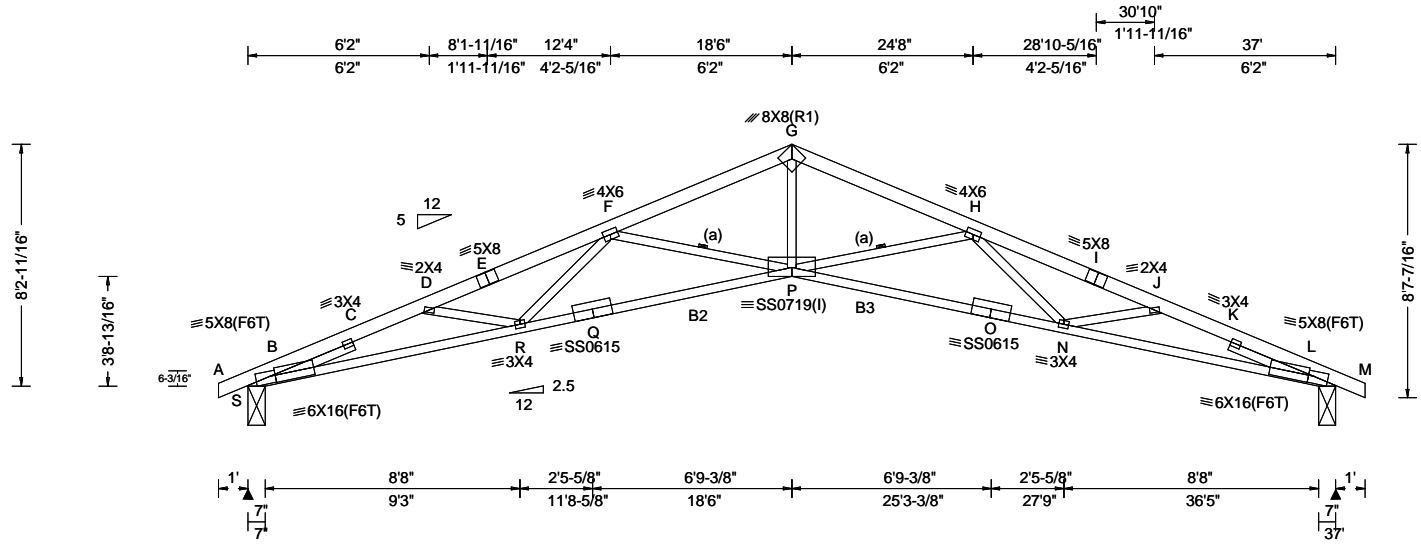
Wind
 Wind loads based on MWFRS with additional C&C member design.

Additional Notes
 Top Chord overhang(s) may be field trimmed.



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Loading Criteria (psf) TCLL: 46.20 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 66.20 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-05 Speed: 100 mph Enclosure: Closed Category: II EXP: C Mean Height: 15.00 ft TCCL: 6.0 psf BCDL: 6.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.70 ft Loc. from endwall: Any I: 1.0 GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 60.0 Ct: 1.1 CAT: II Pf: 46.2 Ce: 1.0 Lu: - Cs: not used Snow Duration: 1.15 Code / Misc Criteria Bldg Code: IRC 2009 TPI Std: 2007 Rep Factors Used: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, 18SS	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.883 P 494 360 VERT(TL): 1.484 P 294 240 HORZ(LL): 0.580 N - - HORZ(TL): 0.974 N - - Creep Factor: 1.5 Max TC CSI: 0.493 Max BC CSI: 0.910 Max Web CSI: 0.952 VIEW Ver: 16.02.01D.0314.13	▲ Maximum Reactions (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Loc</th> <th>R</th> <th>/ U</th> <th>/ Rw</th> <th>/ Rh</th> <th>/ RL</th> <th>/ W</th> </tr> </thead> <tbody> <tr> <td>S</td> <td>2605</td> <td>/ 195</td> <td>/ 588</td> <td>/ -</td> <td>/ 204</td> <td>/ 7.0</td> </tr> <tr> <td>L</td> <td>2605</td> <td>/ 195</td> <td>/ 588</td> <td>/ -</td> <td>/ -</td> <td>/ 7.0</td> </tr> </tbody> </table> Wind reactions based on MWFRS S Min Brg Width Req = 2.1 L Min Brg Width Req = 2.1 Bearings S & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>1972 - 8847</td> <td>G - H</td> <td>1327 - 6153</td> </tr> <tr> <td>C - D</td> <td>1907 - 8695</td> <td>H - I</td> <td>1730 - 8095</td> </tr> <tr> <td>D - E</td> <td>1717 - 8185</td> <td>I - J</td> <td>1709 - 8185</td> </tr> <tr> <td>E - F</td> <td>1737 - 8095</td> <td>J - K</td> <td>1900 - 8695</td> </tr> <tr> <td>F - G</td> <td>1335 - 6153</td> <td>K - L</td> <td>1964 - 8847</td> </tr> </tbody> </table>	Loc	R	/ U	/ Rw	/ Rh	/ RL	/ W	S	2605	/ 195	/ 588	/ -	/ 204	/ 7.0	L	2605	/ 195	/ 588	/ -	/ -	/ 7.0	Chords	Tens.Comp.	Chords	Tens. Comp.	B - C	1972 - 8847	G - H	1327 - 6153	C - D	1907 - 8695	H - I	1730 - 8095	D - E	1717 - 8185	I - J	1709 - 8185	E - F	1737 - 8095	J - K	1900 - 8695	F - G	1335 - 6153	K - L	1964 - 8847
Loc	R	/ U	/ Rw	/ Rh	/ RL	/ W																																											
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Lumber
 Top chord 2x6 SPF 2100f-1.8E
 Bot chord 2x4 SP 2400f-2.0E :B2, B3 2x4 SPF 2100f-1.8E:
 Webs 2x4 SPF #1/#2
 :Lt Slider 2x4 SPF #1/#2: BLOCK LENGTH = 3.043'
 :Rt Slider 2x4 SPF #1/#2: BLOCK LENGTH = 3.043'

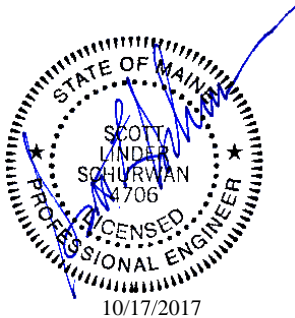
Bracing
 (a) Continuous lateral restraint equally spaced on member.

Plating Notes
 (I) - plates so marked were sized using 0% Fabrication Tolerance, 0 degrees Rotational Tolerance, and/or zero Positioning Tolerance.

Loading
 Bottom chord checked for 10.00 psf non-concurrent bottom chord live load applied per IRC-09 section 301.5.
 Truss designed for unbalanced snow loads.

Wind
 Wind loads based on MWFRS with additional C&C member design.

Additional Notes
 Top Chord overhang(s) may be field trimmed.



Maximum Bot Chord Forces Per Ply (lbs)

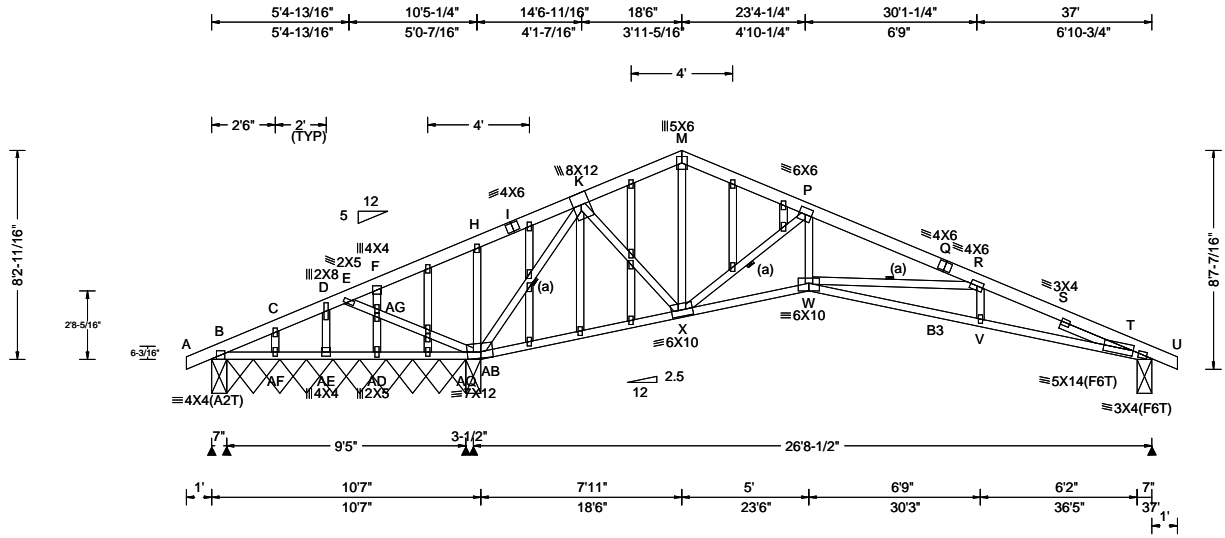
Chords	Tens.Comp.	Chords	Tens. Comp.
B - R	8102 - 1641	P - O	7270 - 1385
R - Q	7242 - 1374	O - N	7242 - 1391
Q - P	7270 - 1368	N - L	8102 - 1642

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
D - R	286 - 495	P - H	505 - 2011
R - F	539 - 85	H - N	539 - 86
F - P	506 - 2011	N - J	286 - 495
G - P	3881 - 718		

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Loading Criteria (psf) TCLL: 46.20 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 66.20 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 *	Wind Criteria Wind Std: ASCE 7-05 Speed: 100 mph Enclosure: Closed Category: II EXP: C Mean Height: 15.00 ft TCCL: 6.0 psf BCDL: 6.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.70 ft Loc. from endwall: Any I: 1.0 GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 60.0 Ct: 1.1 CAT: II Pf: 46.2 Ce: 1.0 Lu: - Cs: not used Snow Duration: 1.15 Code / Misc Criteria Bldg Code: IRC 2009 TPI Std: 2007 Rep Factors Used: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.261 R 999 360 VERT(TL): 0.482 R 650 240 HORZ(LL): 0.119 V - - HORZ(TL): 0.221 V - - Creep Factor: 1.5 Max TC CSI: 0.918 Max BC CSI: 0.602 Max Web CSI: 0.959 VIEW Ver: 16.02.01D.0314.13	▲ Maximum Reactions (lbs), or * = PLF <table border="1"> <thead> <tr> <th>Loc</th> <th>R</th> <th>/U</th> <th>/Rw</th> <th>/Rh</th> <th>/RL</th> <th>/W</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>-609</td> <td>/609</td> <td>/11</td> <td>/-</td> <td>/204</td> <td>/7.0</td> </tr> <tr> <td>B*</td> <td>28</td> <td>/73</td> <td>/14</td> <td>/-</td> <td>/-</td> <td>/113</td> </tr> <tr> <td>AQ</td> <td>4195</td> <td>/252</td> <td>/831</td> <td>/-</td> <td>/-</td> <td>/7.0</td> </tr> <tr> <td>T</td> <td>1528</td> <td>/122</td> <td>/383</td> <td>/-</td> <td>/-</td> <td>/7.0</td> </tr> <tr> <td>B</td> <td></td> <td>/609</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>AE</td> <td></td> <td>/654</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>AD</td> <td></td> <td>/466</td> <td></td> <td></td> <td></td> <td></td> </tr> </tbody> </table> Wind reactions based on MWFRS B Min Brg Width Req = 1.5 B Min Brg Width Req = - AQ Min Brg Width Req = 6.6 T Min Brg Width Req = 1.9 Bearings B, B, AQ, & T are a rigid surface.	Loc	R	/U	/Rw	/Rh	/RL	/W	B	-609	/609	/11	/-	/204	/7.0	B*	28	/73	/14	/-	/-	/113	AQ	4195	/252	/831	/-	/-	/7.0	T	1528	/122	/383	/-	/-	/7.0	B		/609					AE		/654					AD		/466				
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AE		/654																																																										
AD		/466																																																										

Lumber
 Top chord 2x6 SPF #1/#2
 Bot chord 2x4 SPF #1/#2 :B3 2x4 SPF 2100f-1.8E:
 Webs 2x4 SPF #1/#2
 :Rt Slider 2x4 SPF #1/#2: BLOCK LENGTH = 3.043'

Fasten rated sheathing to one face of the 24" o.c. or less vertical webs in this truss.

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Plating Notes
 All plates are 2X4 except as noted.

Loading
 Bottom chord checked for 10.00 psf non-concurrent bottom chord live load applied per IRC-09 section 301.5.
 Truss designed for unbalanced snow loads.

Wind
 Wind loads based on MWFRS with additional C&C member design.

Additional Notes
 Negative reaction(s) of -689# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.
 See DWGS A10015051014, GBLLETIN1014, & GABRST051014 for gable wind bracing and other requirements.
 Shim all supports to solid bearing.
 Top Chord overhang(s) may be field trimmed.

Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - C	1501 -256	K - M	272 -527
C - D	1448 -211	M - P	278 -462
D - E	1696 -230	P - Q	506 -2443
E - F	2825 -454	Q - R	479 -2501
F - H	3037 -461	R - S	879 -4188
H - I	2943 -376	S - T	905 -4338
I - K	2996 -368		

Maximum Bot Chord Forces Per Ply (lbs)

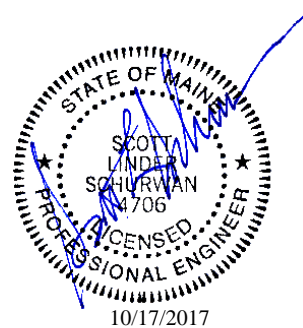
Chords	Tens.Comp.	Chords	Tens. Comp.
B-AF	294 -1334	AB-X	315 -1058
AF-AE	306 -1357	X-W	2203 -242
AE-AD	302 -1326	W-V	3925 -684
AD-AB	302 -1327	V-T	3917 -687

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
E-AB	337 -1569	X-P	488 -2534
H-AB	142 -549	P-W	1426 -168
AB-K	621 -3295	W-R	475 -1661
K-X	1850 -260		

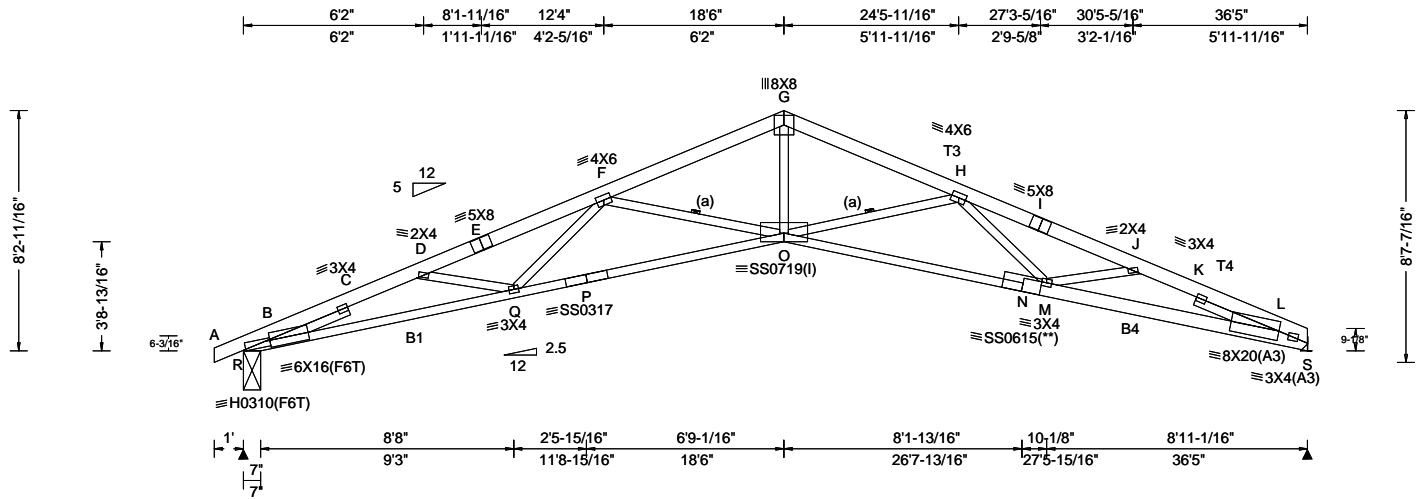
Maximum Gable Forces Per Ply (lbs)

Gables	Tens.Comp.	Gables	Tens. Comp.
C-AF	157 -431	F-AG	460 -44
D-AE	678 -90	AD-AG	517 -57



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Loading Criteria (psf) TCLL: 46.20 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 66.20 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 *	Wind Criteria Wind Std: ASCE 7-05 Speed: 100 mph Enclosure: Closed Category: II EXP: C Mean Height: 15.00 ft TCCL: 6.0 psf BCDL: 6.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.64 ft Loc. from endwall: Any I: 1.0 GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 60.0 Ct: 1.1 CAT: II Pf: 46.2 Ce: 1.0 Lu: - Cs: not used Snow Duration: 1.15 Code / Misc Criteria Bldg Code: IRC 2009 TPI Std: 2007 Rep Factors Used: Yes FT/RT:20(0)/10(0) Plate Type(s): HS, WAVE, 18SS	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.850 O 506 360 VERT(TL): 1.429 O 301 240 HORZ(LL): 0.534 M - - HORZ(TL): 0.899 M - - Creep Factor: 1.5 Max TC CSI: 0.920 Max BC CSI: 0.895 Max Web CSI: 0.923 VIEW Ver: 16.02.01D.0314.13	▲ Maximum Reactions (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Loc</th> <th>R</th> <th>/U</th> <th>/Rw</th> <th>/Rh</th> <th>/RL</th> <th>/W</th> </tr> </thead> <tbody> <tr> <td>R</td> <td>2568</td> <td>/193</td> <td>/581</td> <td>/-</td> <td>/191</td> <td>/7.0</td> </tr> <tr> <td>S</td> <td>2445</td> <td>/171</td> <td>/538</td> <td>/-</td> <td>/-</td> <td>/-</td> </tr> </tbody> </table> Wind reactions based on MWFRS R Min Brg Width Req = 2.1 S Min Brg Width Req = - Bearing R is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>1960 - 8693</td> <td>G - H</td> <td>1346 - 5967</td> </tr> <tr> <td>C - D</td> <td>1894 - 8541</td> <td>H - I</td> <td>1780 - 7825</td> </tr> <tr> <td>D - E</td> <td>1716 - 8024</td> <td>I - J</td> <td>1767 - 7968</td> </tr> <tr> <td>E - F</td> <td>1736 - 7933</td> <td>J - K</td> <td>1962 - 8379</td> </tr> <tr> <td>F - G</td> <td>1326 - 5971</td> <td>K - L</td> <td>1957 - 8552</td> </tr> </tbody> </table>	Loc	R	/U	/Rw	/Rh	/RL	/W	R	2568	/193	/581	/-	/191	/7.0	S	2445	/171	/538	/-	/-	/-	Chords	Tens.Comp.	Chords	Tens. Comp.	B - C	1960 - 8693	G - H	1346 - 5967	C - D	1894 - 8541	H - I	1780 - 7825	D - E	1716 - 8024	I - J	1767 - 7968	E - F	1736 - 7933	J - K	1962 - 8379	F - G	1326 - 5971	K - L	1957 - 8552
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Lumber
 Top chord 2x6 SPF 2100f-1.8E :T3, T4 2x6 SPF #1/#2:
 Bot chord 2x4 SPF 2100f-1.8E :B1 2x4 SP 2400f-2.0E:
 :B4 2x6 SPF 2100f-1.8E:
 Webs 2x4 SPF #1/#2
 :Lt Slider 2x4 SPF #1/#2: BLOCK LENGTH = 3.043'
 :Rt Slider 2x4 SPF #1/#2: BLOCK LENGTH = 3.043'

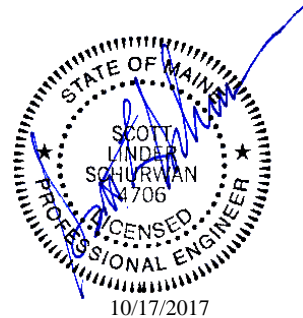
Additional Notes
 Top Chord overhang(s) may be field trimmed.

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Plating Notes
 (l) - plates so marked were sized using 0% Fabrication Tolerance, 0 degrees Rotational Tolerance, and/or zero Positioning Tolerance.
 (**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

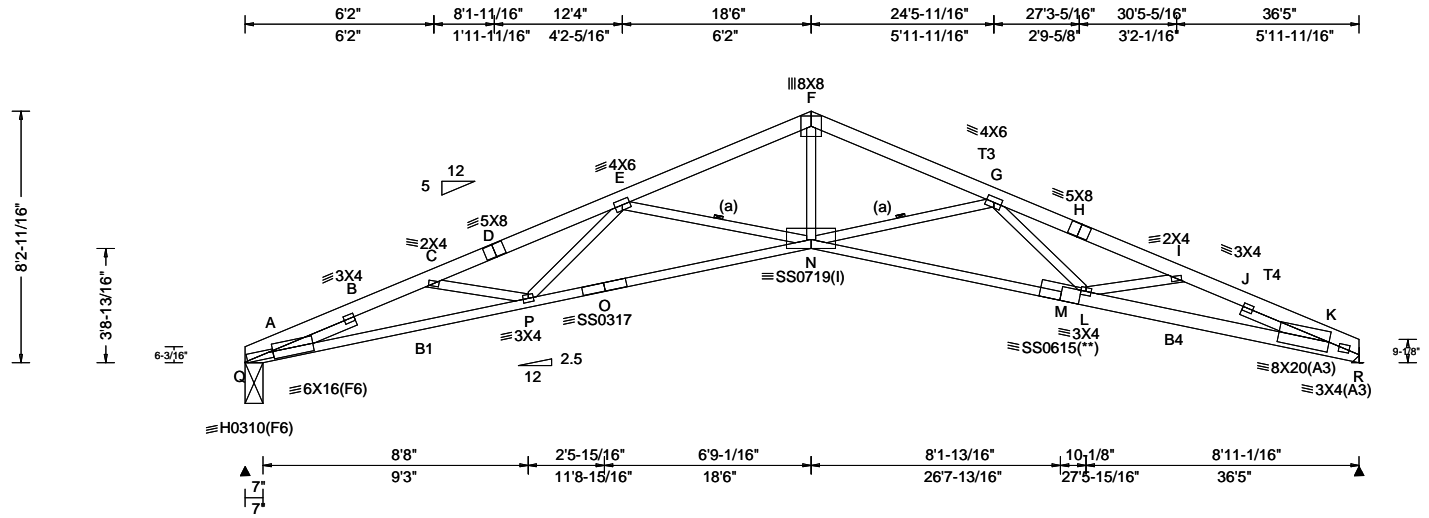
Loading
 Bottom chord checked for 10.00 psf non-concurrent bottom chord live load applied per IRC-09 section 301.5.
 Truss designed for unbalanced snow loads.

Wind
 Wind loads based on MWFRS with additional C&C member design.



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Loading Criteria (psf) TCLL: 46.20 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 66.20 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-05 Speed: 100 mph Enclosure: Closed Category: II EXP: C Mean Height: 15.00 ft TCCL: 6.0 psf BCDL: 6.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.64 ft Loc. from endwall: Any I: 1.0 GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 60.0 Ct: 1.1 CAT: II Pf: 46.2 Ce: 1.0 Lu: - Cs: not used Snow Duration: 1.15	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.852 N 505 360 VERT(TL): 1.429 N 301 240 HORZ(LL): 0.536 L - - HORZ(TL): 0.899 L - - Creep Factor: 1.5 Max TC CSI: 0.920 Max BC CSI: 0.895 Max Web CSI: 0.923	▲ Maximum Reactions (lbs) Loc R / U / Rw / Rh / RL / W Q 2451 / 177 / 537 / - / 182 / 7.0 R 2445 / 171 / 537 / - / - / - Wind reactions based on MWFRS Q Min Brg Width Req = 2.0 R Min Brg Width Req = - Bearing Q is a rigid surface.

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 Bot chord 2x4 SPF 2100f-1.8E :B1 2x4 SP 2400f-2.0E:
 :B4 2x6 SPF 2100f-1.8E:
 Webs 2x4 SPF #1/#2
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Loading
 Bottom chord checked for 10.00 psf non-concurrent bottom chord live load applied per IRC-09 section 301.5.
 Truss designed for unbalanced snow loads.

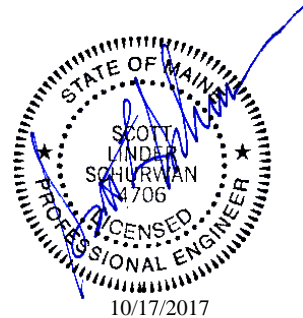
Wind
 Wind loads based on MWFRS with additional C&C member design.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - P	7960 - 1771	N - M	7031 - 1405
P - O	7084 - 1434	M - L	6999 - 1407
O - N	7112 - 1428	L - K	7803 - 1722

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - P	333 - 504	N - G	484 - 1954
P - E	547 - 119	G - L	575 - 127
E - N	508 - 1983	L - I	312 - 398
F - N	3761 - 734		



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GABLE STUD REINFORCEMENT DETAIL

ASCE 7-05: 100 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C, Kzt = 1.00

MAX GABLE VERTICAL LENGTH	2X4 GABLE VERTICAL		BRACE	NO BRACES	(1) 1X4 'L' BRACE *		(1) 2X4 'L' BRACE *		(2) 2X4 'L' BRACE **		(1) 2X6 'L' BRACE *		(2) 2X6 'L' BRACE **	
	SPACING	SPECIES			GRADE	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A
	24" O.C.	SPF	HF	#1 / #2	4' 2"	7' 3"	7' 5"	8' 7"	8' 9"	10' 2"	10' 6"	13' 5"	13' 10"	14' 0"
#3				4' 1"	6' 9"	6' 9"	8' 7"	8' 7"	10' 2"	10' 2"	13' 5"	13' 5"	14' 0"	14' 0"
STUD				4' 1"	6' 9"	6' 9"	8' 7"	8' 7"	10' 2"	10' 2"	13' 5"	13' 5"	14' 0"	14' 0"
SP		DFL	#1	4' 7"	7' 3"	7' 9"	8' 7"	9' 3"	10' 2"	11' 0"	13' 5"	14' 0"	14' 0"	14' 0"
			#2	4' 6"	7' 3"	7' 9"	8' 7"	9' 3"	10' 2"	11' 0"	13' 5"	14' 0"	14' 0"	14' 0"
			#3	4' 3"	6' 11"	6' 11"	8' 7"	9' 0"	10' 2"	10' 9"	13' 5"	14' 0"	14' 0"	14' 0"
16" O.C.	SPF	HF	#1 / #2	4' 10"	8' 3"	8' 6"	9' 9"	10' 1"	11' 8"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 8"	8' 3"	8' 3"	9' 9"	9' 9"	11' 8"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			STUD	4' 8"	8' 3"	8' 3"	9' 9"	9' 9"	11' 8"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	DFL	#1	5' 3"	8' 3"	8' 11"	9' 9"	10' 7"	11' 8"	12' 7"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 2"	8' 3"	8' 11"	9' 9"	10' 7"	11' 8"	12' 7"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 11"	8' 3"	8' 6"	9' 9"	10' 4"	11' 8"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"
12" O.C.	SPF	HF	#1 / #2	5' 3"	9' 1"	9' 4"	10' 9"	11' 1"	12' 10"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 2"	9' 1"	9' 1"	10' 9"	10' 9"	12' 10"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
			STUD	5' 2"	9' 1"	9' 1"	10' 9"	10' 9"	12' 10"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	DFL	#1	5' 9"	9' 1"	9' 10"	10' 9"	11' 7"	12' 10"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 8"	9' 1"	9' 10"	10' 9"	11' 7"	12' 10"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 5"	9' 1"	9' 7"	10' 9"	11' 4"	12' 10"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"
STUD	5' 5"	9' 1"	9' 7"	10' 9"	11' 4"	12' 10"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
STANDARD	5' 3"	8' 5"	8' 5"	10' 9"	11' 1"	12' 10"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		

BRACING GROUP SPECIES AND GRADES:

GROUP A:			
SPRUCE-PINE-FIR		HEM-FIR	
#1 / #2	STANDARD	#2	STUD
#3	STUD	#3	STANDARD
DOUGLAS FIR-LARCH		SOUTHERN PINE	
#3	STUD	#3	STUD
STANDARD	STANDARD	STANDARD	STANDARD
GROUP B:			
HEM-FIR			
#1 & BTR			
#1			
SOUTHERN PINE		DOUGLAS FIR-LARCH	
#1	#1	#2	#2

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 60 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH 'L' BRACE WITH 10d NAILS. (0.128"x3" min)

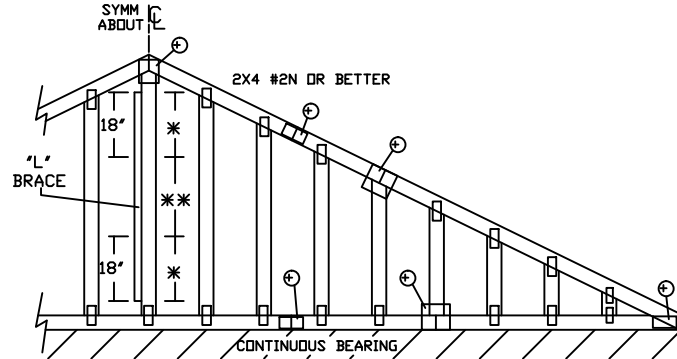
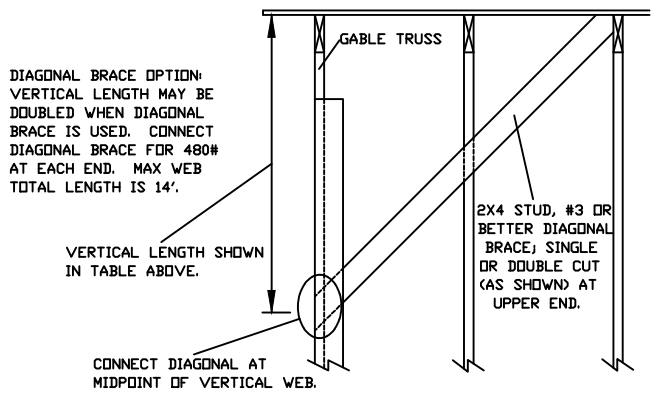
* FOR (1) 'L' BRACE: SPACE NAILS AT 2' O.C. IN 18" END ZONES AND 4' O.C. BETWEEN ZONES.

**FOR (2) 'L' BRACES: SPACE NAILS AT 3' O.C. IN 18" END ZONES AND 6' O.C. BETWEEN ZONES.

'L' BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.



REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.



13723 Riverport Drive
Suite 200
Maryland Heights, MO 63043

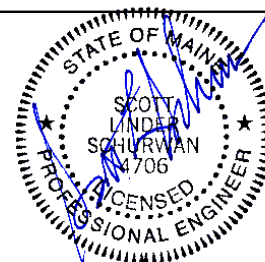
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MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF ASCE7-05-GAB10015
DATE 10/01/14
DRWG A10015051014

ASCE 7-05: EXPOSURE C COMMON RESIDENTIAL GABLE END WIND BRACING REQUIREMENTS - STIFFENERS

100 MPH, 30FT. MEAN HGT, ASCE 7-05, CLOSED
BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP C,
Kzt = 1.00, WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF.

LATERAL CHORD BRACING REQUIREMENTS
TOP: CONTINUOUS ROOF SHEATHING
BOT: CONTINUOUS CEILING DIAPHRAGM

SEE ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL
FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN
ON THIS DETAIL.

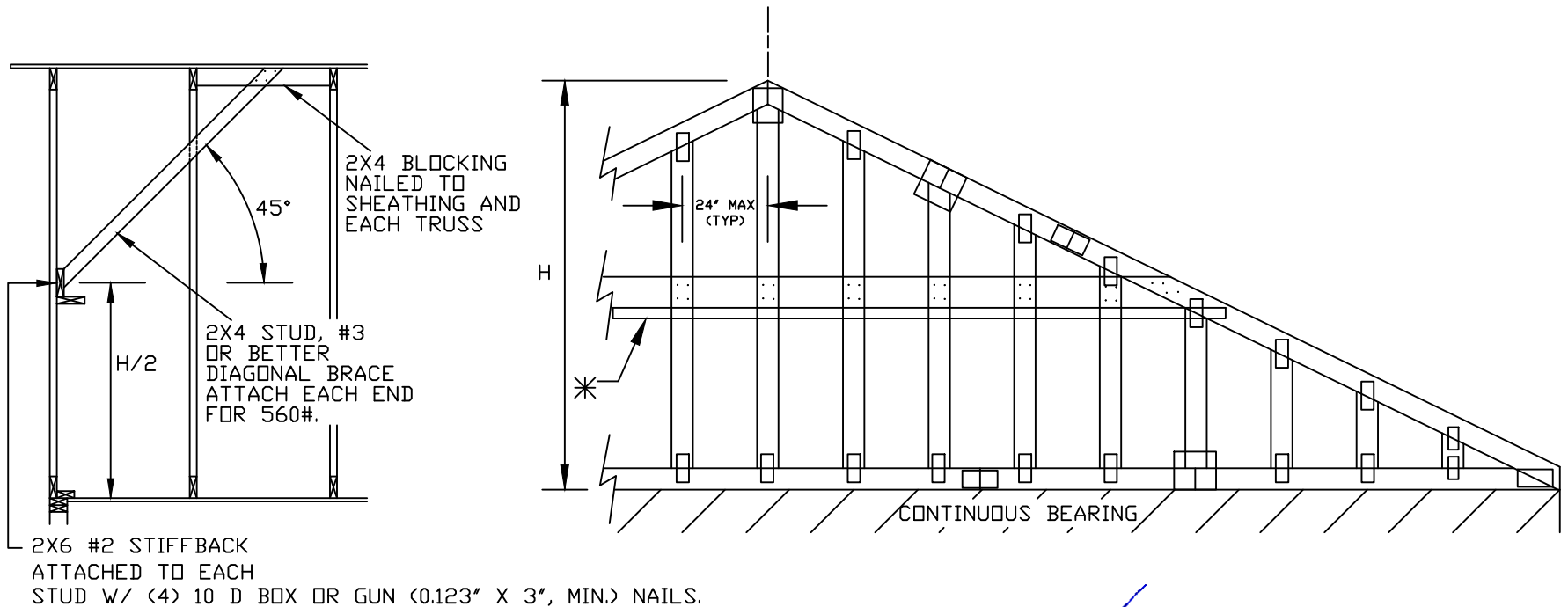
NAILS: 10d COMMON (0.148"x3") OR BOX (0.128"x3",MIN) NAILS
OR GUN (0.125" X 3",min) NAILS.

H LESS THAN 4'6" - NO STUD BRACING REQUIRED

H GREATER THAN 4'6" TO 7'6" IN LENGTH
PROVIDE A 2X6 STIFFBACK AT MID-HEIGHT AND BRACE STIFFBACK
TO ROOF DIAPHRAGM EVERY 6'0" (SEE DETAIL BELOW OR
REFER TO DRAWING A1003005).

H GREATER THAN 7'6" TO 12'0" MAX:
PROVIDE A 2X6 STIFFBACK AT MID-HEIGHT AND BRACE
TO ROOF DIAPHRAGM EVERY 4'0" (SEE DETAIL BELOW OR
REFER TO DRWG A1003005).

* OPTIONAL 2X L-REINFORCEMENT ATTACHED
TO STIFFBACK WITH 10D BOX OR GUN
(0.128" X 3", MIN.) NAILS @ 6" O.C.



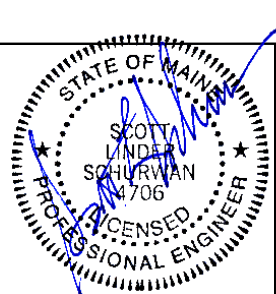
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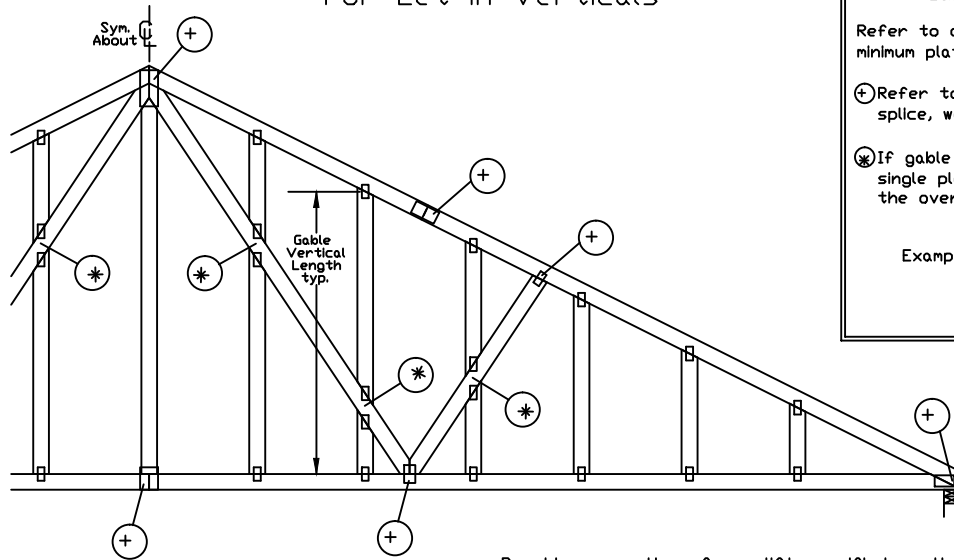
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TC LL	PSF	REF	GE WHALER
TC DL	PSF	DATE	10/01/14
BC DL	PSF	DRWG	GABRST051014
BC LL	PSF		
TOT. LD.	PSF		
DUR. FAC.			
MAX SPACING	24"		

Gable Detail For Let-in Verticals

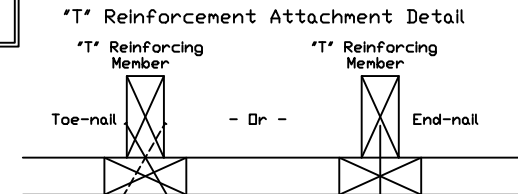


Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

- ⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.
- ⊗ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with
End Driven Nails:
10d Common (0.148"x 3",min) Nails at 4' o.c. plus
(4) nails in the top and bottom chords.

Toenailed Nails:
10d Common (0.148"x3",min) Toenails at 4' o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

- ASCE 7-05 Gable Detail Drawings
A13015051014, A12015051014, A11015051014, A10015051014, A14015051014,
A13030051014, A12030051014, A11030051014, A10030051014, A14030051014
- ASCE 7-10 Gable Detail Drawings
A11515ENC101014, A12015ENC101014, A14015ENC101014, A16015ENC101014,
A18015ENC101014, A20015ENC101014, A20015END101014, A20015PED101014,
A11530ENC101014, A12030ENC101014, A14030ENC101014, A16030ENC101014,
A18030ENC101014, A20030ENC101014, A20030END101014, A20030PED101014
S11515ENC100815, S12015ENC100815, S14015ENC100815, S16015ENC100815,
S18015ENC100815, S20015ENC100815, S20015END100815, S20015PED100815,
S11530ENC100815, S12030ENC100815, S14030ENC100815, S16030ENC100815,
S18030ENC100815, S20030ENC100815, S20030END100815, S20030PED100815

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.

To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate Alpine gable detail).

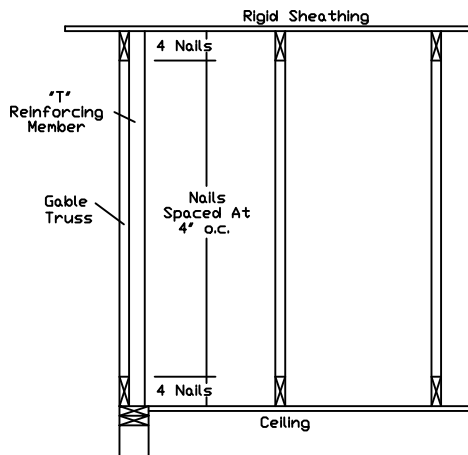
Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

Web Length Increase w/ "T" Brace

"T" Reinf. Mbr. Size	"T" Increase
2x4	30 %
2x6	20 %

Example:
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 1.00
Gable Vertical = 24' o.c. SP #3
"T" Reinforcing Member Size = 2x4
"T" Brace Increase (From Above) = 30% = 1.30
(1) 2x4 "L" Brace Length = 8' 7"
Maximum "T" Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"



WARNING: READ AND FOLLOW ALL NOTES ON THIS DRAWING
IMPORTANT: FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

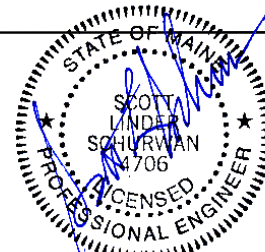
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this Job's general notes page and these web sites:
ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.org; ICC: www.iccsafe.org



13723 Riverport Drive
Suite 200
Maryland Heights, MO 63043



REF LET-IN VERT
DATE 10/01/14
DRWG GBLLETIN1014

MAX. TOT. LD. 60 PSF
DUR. FAC. ANY
MAX. SPACING 24.0"

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

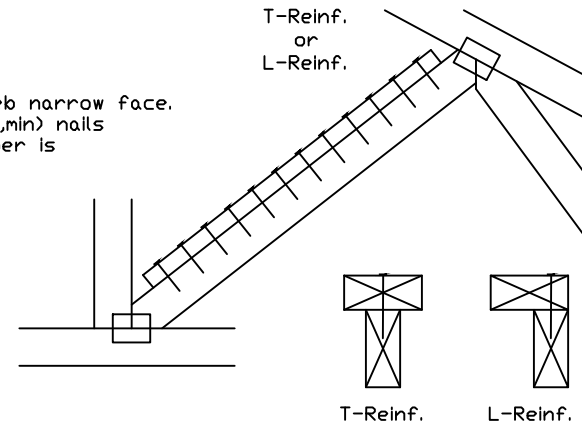
Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

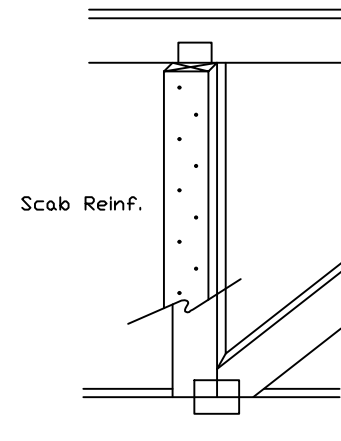
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



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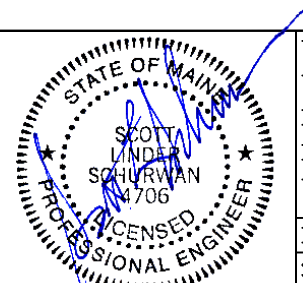
WARNING: READ AND FOLLOW ALL NOTES ON THIS DRAWING
IMPORTANT: FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

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For more information see this Job's general notes page and these web sites:
ALPINE: www.alpineitw.com TPI: www.tpinst.org SBCA: www.sbcindustry.org ICC: www.iccsafe.org



TC LL	PSF	REF CLR Subst.
TC DL	PSF	DATE 10/01/14
BC DL	PSF	DRWG BRCLBSUB1014
BC LL	PSF	
TOT. LD.	PSF	
DUR. FAC.		
SPACING		