Client:

Project:

Address: 199 Woodford Street

Portland Maine

Date: 10/16/2017

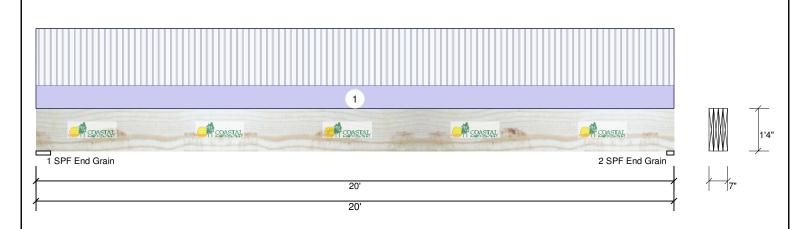
Designer: PD

Job Name: Jones, Rich and Barnes

Project #:

4-Ply - PASSED 2.0E CP-LAM 1.750" X 16.000"

Level: Level



Member Info	rmation			Reaction	ns lb (Up	lift)				
Type:	Girder	Application:	Floor	Brg	Live	Dead	Snow	٧	Vind	Const
Plies:	4	Design Method:	ASD	1	6135	2748	0		0	0
Moisture Condition	on: Dry	Building Code:	IBC/IRC 2015	2	6015	2694	0		0	0
Deflection LL:	360	Load Sharing:	Yes							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal									
Temperature:	Temp <= 100°F									
				Bearings	S					
				Bearing	Length	Cap. Rea	ct D/L lb	Total	Ld. Case	Ld. Co
				1 - SPF End	5.500"	27% 27	48 / 6135	8883	L	D+L
				1 ~ .						

Analysis Results

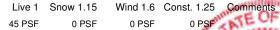
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	42121 ft-lb	10'1 3/16"	75755 ft-lb	0.556 (56%)	D+L	L
Unbraced	42121 ft-lb	10'1 3/16"	42220 ft-lb	0.998 (100%)	D+L	L
Shear	7435 lb	1'7 3/4"	21280 lb	0.349 (35%)	D+L	L
LL Defl inch	0.420 (L/559)	10'1 1/4"	0.652 (L/360)	0.640 (64%)	L	L
TL Defl inch	0.608 (L/386)	10'1 1/4"	0.979 (L/240)	0.620 (62%)	D+L	L

Design Notes

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- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Compression edge bracing required at 5'4" o.c. or less.
- 5 Lateral slenderness ratio based on single ply width

ID	Load Type	Location	Trib Width	Side	Dead 0.9
1	Uniform		13-6-0	Тор	18 PSF
	Self Weight				29 PLF

.d. Comb.)+L Grain 2 - SPF 2.750" 2694 / 6015 8709 L D+L End Grain



10/20/2017

Page 1 of 6

Notes

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Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Job# PER171747 P. E. Robbins, P.E. 1777 State Rt 167 Victoria IL 61485 ph#309-879-3258

Manufacturer Info

Pacific Woodtech Corp 1850 Park Lane Burlington, WA 98233 (888) 707-2285 www.pacificwoodtech.com APA: PR-L233, ICC-ES: ESR-2909





Client:

Project:

Address: 199 Woodford Street

Portland Maine

Date: 10/16/2017

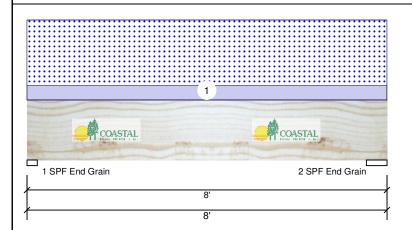
Designer: PD

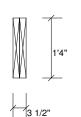
Job Name: Jones, Rich and Barnes

Project #:

2-Ply - PASSED 2.0E CP-LAM 1.750" X 16.000"

Level: Level





Const

0

0

Wind

0

0

Page 2 of 6

Member Information

Type: Girder Plies: 2 Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance: Normal Temperature: Temp <= 100°F

Application: Floor Design Method: ASD **Building Code: IBC/IRC 2015** Load Sharing: Nο Deck: Not Checked

Reactions Ib (Uplift) Live Dead Brg Snow 720 2984 0 1

756

0

Bearings

2

Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 2.750" 720 / 2984 3704 L D+S End Grain 2 - SPF 5.500" 24% 756 / 3136 3892 L D+S End Grain

3136

Analysis Results

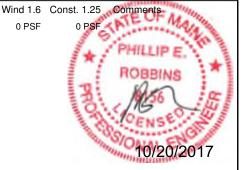
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	6807 ft-lb	3'10 13/16"	41884 ft-lb	0.163 (16%)	D+S	L
Unbraced	6807 ft-lb	3'10 13/16"	15387 ft-lb	0.442 (44%)	D+S	L
Shear	2329 lb	6'4 1/4"	12236 lb	0.190 (19%)	D+S	L
LL Defl inch	0.024 (L/3836)	3'10 7/8"	0.252 (L/360)	0.090 (9%)	S	L
TL Defl inch	0.029 (L/3090)	3'10 7/8"	0.379 (L/240)	0.080 (8%)	D+S	L

Design Notes

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- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9
1	Uniform		17-0-0	Тор	10 PSF
	Self Weight				15 PLF

Snow 1.15 0 PSF 0 PSF 45 PSF



Notes

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Client:

Project: Address: 199 Woodford Street

Portland Maine

Date: 10/16/2017

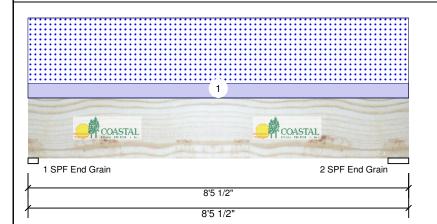
Designer: PD

Job Name: Jones, Rich and Barnes

Project #:

2-Ply - PASSED 2.0E CP-LAM 1.750" X 16.000"

Level: Level



Application:

Design Method:

Building Code:

Load Sharing:

Deck:

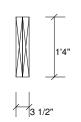
Floor

ASD

Nο

IBC/IRC 2015

Not Checked



Page 3 of 6

Member Information

Type: Girder Plies: 2 Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance: Normal

Temperature: Temp <= 100°F

Reactions Ib (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	0	969	4089	0	0
2	0	1015	4285	0	0

Bearings

Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 2.750" 969 / 4089 5058 L D+S End Grain 2 - SPF 5.500" 1015 / 4285 5300 L D+S End Grain

Analysis Results

•						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	9873 ft-lb	4'1 9/16"	41884 ft-lb	0.236 (24%)	D+S	L
Unbraced	9873 ft-lb	4'1 9/16"	14543 ft-lb	0.679 (68%)	D+S	L
Shear	3285 lb	1'5 3/8"	12236 lb	0.268 (27%)	D+S	L
LL Defl inch	0.039 (L/2485)	4'1 5/8"	0.268 (L/360)	0.140 (14%)	S	L
TL Defl inch	0.048 (L/2009)	4'1 5/8"	0.402 (L/240)	0.120 (12%)	D+S	L

Design Notes

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- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9
1	Uniform		22-0-0	Тор	10 PSF
	Self Weight				15 PLF

Snow 1.15 Wind 1.6 Const. 1.25 0 PSF 0 PSF 45 PSF

0 PSI 10/20/2017

Notes

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- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

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Client:

Project:

Address: 199 Woodford Street

Portland Maine

Date: Designer:

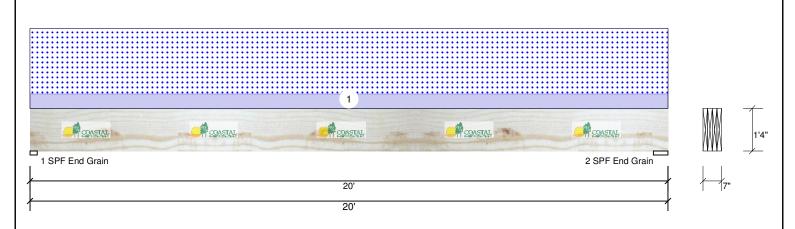
10/16/2017

PD Job Name: Jones, Rich and Barnes

Project #:

4-Ply - PASSED 2.0E CP-LAM 1.750" X 16.000"

Level: Level



Member Information Reactions Ib (Uplift) Type: Girder Application: Floor Plies: 4 Design Method: ASD Moisture Condition: Dry **Building Code:** IBC/IRC 2015 Deflection LL: 360 Load Sharing: Yes Deflection TL: 240 Deck: Not Checked Importance: Normal Temperature: Temp <= 100°F **Bearings**

Analysis Resul	suits
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•						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	36949 ft-lb	9'10 13/16"	87119 ft-lb	0.424 (42%)	D+S	L
Unbraced	36949 ft-lb	9'10 13/16"	37004 ft-lb	0.999 (100%)	D+S	L
Shear	6522 lb	1'5 3/8"	24472 lb	0.267 (27%)	D+S	L
LL Defl inch	0.420 (L/559)	9'10 7/8"	0.652 (L/360)	0.640 (64%)	S	L
TL Defl inch	0.533 (L/441)	9'10 7/8"	0.979 (L/240)	0.540 (54%)	D+S	L

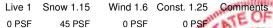
Design Notes

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- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Compression edge bracing required at 6'3" o.c. or less.
- 5 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9
1	Uniform		13-6-0	Тор	10 PSF
	Self Weight				29 PI F

Brg	Live	Dead	Snow	Wind	Const
1	0	1625	6015	0	0
2	0	1657	6135	0	0

Bearing Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF 2.750" End Grain	47%	1625 / 6015	7640	L	D+S
2 - SPF 5.500" End Grain	24%	1657 / 6135	7792	L	D+S



10/20/2017

Page 4 of 6

Notes

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Dry service conditions, unless noted otherwise
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Handling & Installation

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- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Job# PER171747 P. E. Robbins, P.E. 1777 State Rt 167 Victoria IL 61485 ph#309-879-3258

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Client:

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Portland Maine

Date: 10/16/2017

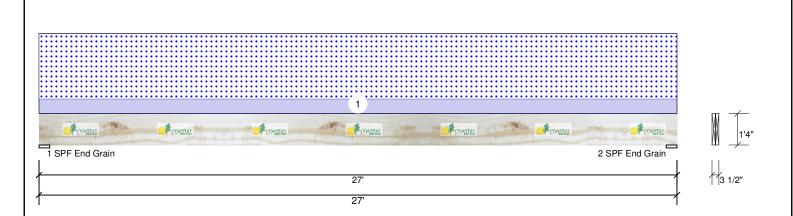
Designer: PD

Job Name: Jones, Rich and Barnes

Project #:

2-Ply - PASSED 2.0E CP-LAM 1.750" X 16.000"

Level: Level



					· · · · · · · · · · · · · · · · · · ·					
Type:	Girder	Application:	Floor	Brg	Live	Dead	Snow	Wind	Const	
Plies:	2	Design Method:	ASD	1	0	736	2430	0	0	
Moisture Conditio	n: Dry	Building Code:	IBC/IRC 2015	2	0	736	2430	0	0	
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal									
Temperature:	Temp <= 100°F									_
				Bearing	gs					
				Bearing	g Length	Cap. Rea	ct D/L lb	Total Ld. Case	Ld. Comb	٠.
		1		I 4 ODE		100/ 70	00 / 0400	0100	D . C	

Analysis Results

Member Information

L							
	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	20395 ft-lb	13'6"	41884 ft-lb	0.487 (49%)	D+S	L
	Unbraced	20395 ft-lb	13'6"	20459 ft-lb	0.997 (100%)	D+S	L
	Shear	2780 lb	25'4 1/4"	12236 lb	0.227 (23%)	D+S	L
	LL Defl inch	0.820 (L/386)	13'6 1/16"	0.879 (L/360)	0.930 (93%)	S	L
	TL Defl inch	1.069 (L/296)	13'6 1/16"	1.319 (L/240)	0.810 (81%)	D+S	L

Design Notes

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- 3 Top loads must be supported equally by all plies.

4 Compre	ssion eage bracing requirea	at 5'7" o.c. or less	i.						
5 Lateral slenderness ratio based on single ply width.									
ID	Load Type	Location	Trib Width	Side	Dead 0.9				
1	Uniform		4-0-0	Тор	10 PSF				
	Self Weight				15 PLF				

Reactions Ib (Uplift)

Bearing	Length	Cap. R	eact D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	5.500"	19%	736 / 2430	3166	L	D+S
2 - SPF End Grain	5.500"	19%	736 / 2430	3166	L	D+S

Live 1 Snow 1.15 Wind 1.6 Const. 1.25 0 PSF 0 PSF 45 PSF 0 PSF

10/20/2017

Page 5 of 6

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Portland Maine

Date: 10/16/2017

Designer: PD

Job Name: Jones, Rich and Barnes

Project #:

2-Ply - PASSED 2.0E CP-LAM 1.750" X 16.000"

Application:

Design Method:

Building Code:

Load Sharing:

Deck:

Floor

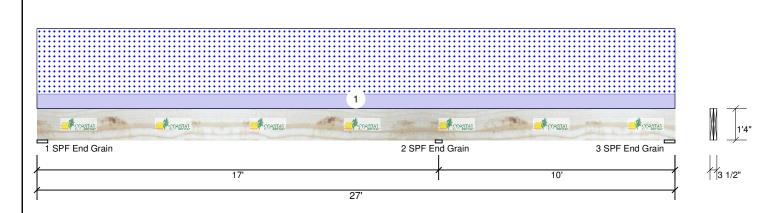
ASD

Nο

IBC/IRC 2015

Not Checked

Level: Level



Member	Information
Type:	Girder
DI:	•

Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240

Importance: Normal Temperature: Temp <= 100°F

Reactions Ib (Uplift)

Bearings Bearing Length

End Grain

End Grain 3 - SPF 5.500"

End Grain

1 - SPF 5.500"

2 - SPF 3.500"

Brg	Live	Dead	Snow	Wind	Const
1	0	386	1274	0	0
2	0	954	3147	0	0
3	0	133	439	0	0

Cap. React D/L lb

10%

39%

386 / 1297

954 / 3147

133 / 643

Total Ld. Case

1683 L

4101 LL

776 _L

Ld. Comb.

D+S

D+S

D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Neg Moment	-6176 ft-lb	17'	41884 ft-lb	0.147 (15%)	D+S	LL
Unbraced	-6176 ft-lb	17'	7827 ft-lb	0.789 (79%)	D+S	LL
Pos Moment	5527 ft-lb	7'2 1/8"	41884 ft-lb	0.132 (13%)	D+S	L_
Unbraced	5527 ft-lb	7'2 1/8"	7827 ft-lb	0.706 (71%)	D+S	L_
Shear	2014 lb	15'8"	12236 lb	0.165 (16%)	D+S	LL
LL Defl inch	0.077 (L/2585)	7'11 1/8"	0.556 (L/360)	0.140 (14%)	S	L_
TL Defl inch	0.100 (L/2012)	7'10 15/16"	0.834 (L/240)	0.120 (12%)	D+S	L_

Anaiysis	Actual	Location	Allowed	Capacity	Comb.	Case
Neg Moment	-6176 ft-lb	17'	41884 ft-lb	0.147 (15%)	D+S	LL
Unbraced	-6176 ft-lb	17'	7827 ft-lb	0.789 (79%)	D+S	LL
Pos Moment	5527 ft-lb	7'2 1/8"	41884 ft-lb	0.132 (13%)	D+S	L_
Unbraced	5527 ft-lb	7'2 1/8"	7827 ft-lb	0.706 (71%)	D+S	L_
Shear	2014 lb	15'8"	12236 lb	0.165 (16%)	D+S	LL
LL Defl inch	0.077 (L/2585)	7'11 1/8"	0.556 (L/360)	0.140 (14%)	S	L_
TL Defl inch	0.100 (L/2012)	7'10 15/16"	0.834 (L/240)	0.120 (12%)	D+S	L_

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- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9
1	Uniform		4-0-0	Тор	10 PSF
	Self Weight				15 PLF

Live 1 Snow 1.15 0 PSF 45 PSF

Wind 1.6 Const. 1.25 Comments 0 PSF

10/20/2017

Page 6 of 6

Notes

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