



General Notes:

- A. General**
 1. Structural drawings shall be used in conjunction with the architectural, mechanical, electrical, site, shop drawings and specifications.
 2. Governing building code is the Maine State Building Code.
- B. Demolition**
 1. CONTRACTOR IS SOLELY RESPONSIBLE FOR ANY AND ALL TEMPORARY SUPPORT, SHORING, ETC., IN ORDER TO MAINTAIN THE INTEGRITY OF THE EXISTING STRUCTURE DURING DEMOLITION AND CONSTRUCTION.
 2. Once the finish work has been removed and before any demolition takes place within the limits of the existing building which is to remain, this office shall be given a minimum of 3 days notice to review existing conditions to ensure that initial assumptions of the structural system agree with the as-built conditions.
 3. During construction the contractor shall notify this office of any conditions that are shown on the structural drawings which do not agree with the as-built conditions.
- C. Shop Drawings**
 1. Shop drawings, reviewed by general contractor for reinforcing steel, concrete mix designs, joist deck and structural steel shall, be submitted to the architect and a stamped approval received before fabrication can proceed. Erection shall be made from approved shop drawings only.
 2. Shop drawings shall be original drawings prepared by the general contractor and/or his subcontractor for each discipline requiring approval. Reproduction of these contract drawings for use as a shop drawing submittal shall be unacceptable, and will be rejected as not complying with the contract documents.
 3. Changes, substitutions, or omissions made by the general contractor or their subcontractors to the contract documents shall be submitted to the architect for approval. The submission shall highlight and note the proposed change substitution, or omissions to the contract documents which do not follow this approval procedure shall be considered not approved.
- D. Soil Conditions and Structural Fill**
 1. All footings shall be carried to the depths shown and deeper, if required, and shall rest on undisturbed soil or compacted structural fill having a net allowable soil bearing pressure of 3,000 PSF. Subsurface soil bearing conditions and preparation shall be completed in accordance with the geotechnical report issued by S.W. Cole Engineering, Inc., dated March 3, 2008.
 2. No footing shall be placed on frozen soil or in standing water.
 3. Structural fill shall be well graded bank run, screened or crushed gravel, and shall be placed in 8" maximum lifts and compacted to 95% of maximum dry density as determined by ASTM D1557.
 4. Provide well compacted, clean, coarse sand and gravel under all slabs on grade after the top soil has been removed. Refer to geotechnical report for required depth.
- E. Concrete**
 1. All reinforcement to be billet steel deformed bars conforming to ASTM A615 grade 60.
 2. Welded wire fabric shall conform to "Standard specifications for Welded Steel wire Fabric for Concrete Reinforcement" (ASTM A185) and shall be supplied in flat sheets.
 3. Where continuous bars are called for indicates or required they shall be run continuously around corners, lapped at necessary splices, splices staggered and hooked at disconnect ends. Laps shall be 48 diameters of the larger bar.
 4. Provide at all four sides of openings in concrete walls, 1-#5 bar, each face, extending 2'-0" beyond openings or hooked if necessary.
 5. All reinforcing bars are to be cold bent.
 6. All concrete shall attain a minimum compressive strength of 3,000 psi at 28 days except where noted in the specifications.
 7. The following minimum cover shall be provided for reinforcement:
 Concrete against the earth: 3"
 Formed concrete exposed to earth or weather #5 or smaller 1 1/2" #6 or larger: 2"
 Concrete not exposed to earth or weather slabs or walls 3/4" beams and columns: 1 1/2"
 8. All concrete work and detailing shall comply with the latest specifications and recommendations of the ACI.
- F. Steel**
 1. All W sections shall conform to ASTM A992 Grade 50. All other sections shall conform to ASTM A36.
 HSS sections shall conform to ASTM A500 Grade B.
 2. All exposed steel shall be hot dipped galvanized.
 3. Steel headers for lintels not labeled shall consist of the following: Masonry openings to 4'-0" L4x3x1/8 (for each 4" width of block) Masonry openings 4'-0" to 6'-0" L6x3x1/8
 Shop and field connections shall be made by high strength bolts or welding. All beam/beam and beam/column connections shall be designed for a reaction Wc/2L as described by the AISC manual, amended to date, unless connection is shown on drawings.
 5. Structural bolts shall be high strength bolts conforming to ASTM A325, bearing type, conforming to the provisions of the "Specifications for Structural Joints Using ASTM A325 or A490 Bolts", latest edition.
 6. Welding shall be performed only by AWS certified welders and shall conform to provisions of the Structural Welding Code-Steel of the American Welding Society, latest edition.
- G. Wood**
 1. All framing members shall have the properties of Spruce-Pine-Fir #2 or better (Fb=875 psi, E=1,400,000 psi) except that wood posts shall have the properties of Spruce-Pine-Fir #1 or better (Fb=700 psi, E=1,300,000 psi).
 2. All engineered lumber shall be installed in accordance with the manufacturer's specifications and recommendations.
 3. Engineered lumber shall have the following minimum properties:
 Timberstrand Fb=1700 psi E=1,300,000 psi
 Microlam Fb=2600 psi E=1,900,000 psi
 Parallam Fb=2900 psi E=2,000,000 psi
 4. All flush framing shall use Simpson metal timber hangers (or equal).
 5. Use stainless steel or galvanized nails for all connections exposed to the weather.
- H. Concrete Block**
 1. Concrete block shall be normal or light weight, hollow load-bearing concrete masonry units conforming to ASTM C 90, Grade N with a minimum compressive strength of 3,000 psi.
 2. Mortar for masonry units shall comply with ASTM C 270 Type S.
 3. Grout for masonry units shall comply with ASTM C 476.
 4. Mortar Mix (Parts by Volume)
 Portland Cement: 1
 Hydrated lime: 1/4 to 1/2 Max
 Shovel Count (Sand): 21-25 @ 2 1/4 to 28-34 @ 3
 Damp loose Aggregate: 2 1/4 to 3 times the sum of the volumes of the cement and the lime used.
 5. Horizontal reinforcement shall be #9 wire, standard "dur-a-wall" truss type.
 6. All openings in cmu walls shall be reinforced at the jamba with (one) #6 vertical bar, extending 2'-0" above head height, fully grouted in the cells.
 7. Provide one vertical bar (#6) each side of all control joints, full height and doweled into foundation.
- I. Open Web Steel Joists**
 1. Joists and joist girders designated on the drawings shall conform to standard specifications for series K joists adopted by the Steel Joist Institute and the American Institute of Steel Construction.
 2. Design fabrication, erection and bracing shall be in accordance with S.J.I. and A.W.S.
 3. All joists to be painted one coat of primer. Touch up all welds.
 4. Provide bottom chord extension at all columns. All other joists to have standard bottom chord extensions.
 5. Submit shop drawings for approval.
 6. Joist manufacturer is to be a member of the Steel Joist Institute.
- K. Design Loads**
 1. Design Live Loads Roof 42 PSF + Drift where applicable (Ground Snow Load = 60 PSF)
 First Floor 100 psf
 2. Wind Load Basic wind speed = 90 MPH (3 Second Gust)
 Exposure B
 3. Seismic Design Data Seismic Hazard Exposure Group1
 Seismic Performance CategoryC (Ss = 0.35, S1 = 0.1)
 Soil Profile TypeD
 Basic Seismic Force Resisting System Load Bearing Wall System/
 Reinforced masonry shear walls
 Response Modification FactorR = 2 1/2
 Deflection Amplification FactorCd = 1 3/4
 Analysis Procedure Used.....Simplified Analysis Procedure
- L. Construction Site Visits**
 1. Site visits to review construction progress will be required by this office at the following intervals, at a minimum:
 A. Before footing placements.
 B. Before foundation wall placement.
 C. Before bond beams are poured at E1 8'-8", 17'-4" and 19'-4".
 D. After the structural steel is erected and roof deck attached.
 2. This office requires a minimum of 48 hours notification (excluding Saturdays, Sundays and Holidays) in order to view the construction.
 3. This office will not be responsible for delays in construction due to the failure to provide adequate notice of observation. In addition, should the contractor decide to continue work without the previous work being observed, this office will be unable to sign off that the previous work was done in conformance with the Design Drawings.
- Sheet Notes:**
 1. Coordinate roof drains and sewer line locations with A1.1, P1.1 and Civil Drawings.
 2. Vestibule and bathroom slabs slope to drain. Coordinate with the Architectural Drawings.
 3. See the Architectural Drawings for limits of recessed slab.
 4. S.F. denotes location of a stepped footing.
 5. C.J. denotes location of control joint.
 6. (E) indicates location of existing building component.

MOESER & ASSOCIATES
 206 AYER ROAD HARVARD, MA 978-456-6905
ARCHITECT

DRAWINGS/SPECIFICATIONS BY:

<input type="checkbox"/> WALGREENS'
<input checked="" type="checkbox"/> LANDLORD'S CONSULTANT

ALL CONSTRUCTION WORK, UNLESS NOTED OTHERWISE, BY:

<input type="checkbox"/> WALGREENS' CONTRACTOR
<input checked="" type="checkbox"/> LANDLORD'S CONTRACTOR (TURNKEY CONSTRUCTION)

STORE	BUILDING
NEW <input checked="" type="checkbox"/>	NEW <input checked="" type="checkbox"/>
REMODELING <input type="checkbox"/>	EXISTING <input type="checkbox"/>
RELOCATION <input type="checkbox"/>	NEW SHELL ONLY <input type="checkbox"/>
OTHERS <input type="checkbox"/>	

STRUCTURAL ENGINEER:
DWD ENGINEERING, INC.

5 Michael Road
 E. Bridgewater, MA
 TEL. 508.378.9602
 FAX. 508.378.2922

NO.	DATE	BY	DESCRIPTION	CONST

REVISIONS

CERTIFICATION AND SEAL

I HEREBY CERTIFY THAT THIS PLAN AND SPECIFICATION WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION AND THAT I AM A DULY REGISTERED ARCHITECT OR ENGINEER UNDER THE LAWS OF THE STATE OF MAINE AS SIGNIFIED BY MY HAND AND SEAL.

FISCAL 2007 CRITERIA - STORE # 12325

WALGREENS
 616 FOREST AVENUE
 PORTLAND, MAINE

DRAWING TITLE
FOUNDATION/FIRST FLOOR PLAN

CADD PLOT:	SCALE: AS NOTED	DRAWING NO.
VOID PLOT:	DRAWN BY: RGC	S1
RELEASED TO CONSTRUCTION	DATE: 12/10/08	
	REVIEWED BY: DWD	