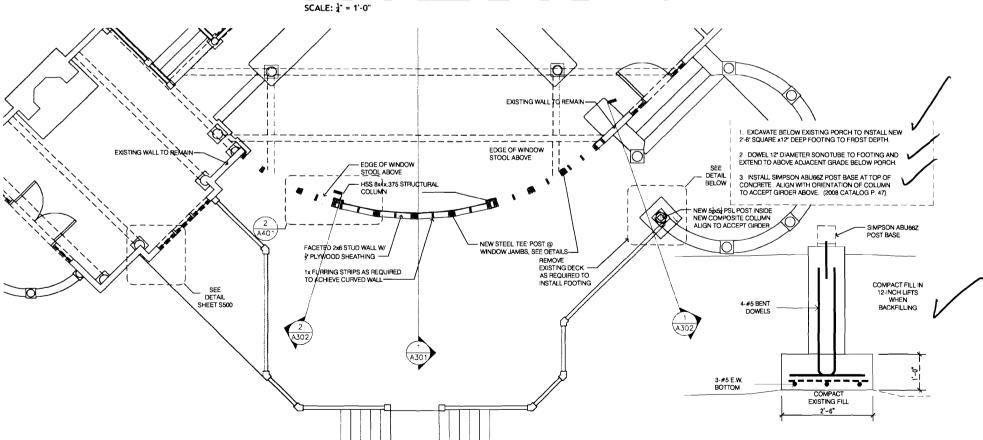


MAIN LEVEL REFLECTED CEILING PLAN - LVL STRENGTHENING



MAIN LEVEL WALL FRAMING PLAN AND FOOTING DETAIL

SCALE: 4" = 1'-0"

STRUCTURAL GENERAL NOTES

1. THE CONTRACTOR SHALL COMPLY WITH ALL FEDERAL, STATE, AND LOCAL SAFETY REQUIREMENTS.

 NO PROVISIONS HAVE BEEN MADE FOR ANY TEMPORARY CONDITIONS THAT MAY ARISE DURING CONSTRUCTION PROOR TO THE COMPLETION OF THE PROJECT. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ACCOUNTE DESIGN AND CONSTRUCTION OF ALL FORMS, SHORMS, AND TEMPORARY BRACING DURING THE PROJECT.

 NOTEY ENGINEER OF ANY DISCREPANCES BETWEEN THE DRAWINGS AND BUSTING CONDITIONS THAT MAY AFFECT THE WORK. BECAUSE THIS PROJECT IMPOLYES RETROFITTING AND MODIFICATIONS OF EDISTING STRUCTURES, THE CONTRACTOR SHALL TARE THE NECESSARY MEASURE TO FIELD VERBY EXISTING CONDITIONS AS SHOWN ON THE DRAWINGS.

4. ANY MODIFICATION OR ALTERATION OF THESE CONSTRUCTION DOCUMENTS OR CHANGES IN CONSTRUCTION FROM THE INTERT OF THESE DOCUMENTS BY THE CONTRUCTOR WITHOUT PRITTEN APPROVAL OF THE CHANGEES PAIL REBOYK ALL PROPERSONAL AND LIABLAY RESPONSIBILITY ON THE PART OF THE ENGINEES. ALTERNATE CONNECTION DETAILS MAY BY USED IF SUBMITTED TO THE ENGINEER FOR REVIEW AND ACCEPTANCE GRAVITED.

5 DO NOT SCALE FROM THE DRAWINGS.

DESIGN CRITERIA

1. INTERNATIONAL BUILDING CODE, 2003 EDITION, INCLUDING CONSIDERATION OF CHAPTER 34, EXISTING BUILDINGS.

ADDRESS: 420 SEASHORE AVE, PEAKS ISLAND, PORTLAND, MAINE

SNOW LOAD: UNHEATED CANTILEVER PORCH ROOF STRUCTURE 50 POUNDS PER SQUARE FOOT, UNIFORM (INCLUDES DRIFT)

LIVE LOADS: 40 PSF, (30 PSF IN SLEEPING ROOMS)

WIND LOAD: PER IBC SECTION 1609,0/ASCE 7-02 CHAPTER 6

BASIC WIND SPEED, (3 SEC GUST)	100 m
IMPORTANCE FACTOR No	1.00
EXPOSURE CATEGORY	C
BUILDING CLASSIFICATION	
BASIC WIND PRESSURE	20 ps
COMPONENTICS ADDING PRESSURE	30 ~

SEISMIC LOAD: PER IBC SECTION 1615.0; EARTHQUAKE DESIGN DATA PER SECTION 1616.3:

SEISMIC IMPORTANCE FACTOR, In	1.0
SEISMIC USE GROUP	1
SHORT-PERIOD RESPONSE ACCELLARATION	0.37
1-SECOND RESPONSE ACCELLARATION	0.10
SEISMIC DESIGN CATEGORY	С
BASIC SEISMIC FORCE-RESISTING SYSTEM	SHEAR
RESPONSE MODIFICATION FACTOR	1.5

LUMBER AND JOIST HANGERS

1. ALL COMPOSITE LUMBER USED ON THIS JOB SHALL BE VERSALAM MANUFACTURED BY BOISE ENGINEERED WOOD PRODUCTS. BENDING STRESS FO = 3100 psi. ELASTIC MODULUS E = 2,000 tsi. FRAMING LUMBER USED FOR OTHER WALL AND ROUGH CAMPENTA PAPICATIONS SHALL BE SPRUCE PINE-RIR NO. 2 OR BETTER KILU DRIED TO A MOISTURE CONTENT OF LESS THAN 19 PERCENT, FASTENERS I, OT ATTACH VERSALAM MEMBERS ARE TO BE 3th TRUSSLOK FASTENERS, MANUFACTURED BY FASTENMASTER INC. AGAMPAM, MA (1-800-516-3508, www.fastenmastic.com)

 ALL JOIST HANGERS, HURRICANE TIES, AND ATTACHMENT HARDWARE ARE TO BE AS SPECIFIED, MANUFACTURED BY SIMPSON STRONG-TH. CONNECT ALL JOIST HANGERS PER SIMPSON STRONG-TE REQUIREMENTS. ALL SIMPSON HARDWARE SHALL BE TREATED WITH A Z-MAX" GALVANIZING TREATEMENT, OR BE MANUFACTURED FROM STAINLESS STEEL.

3. SPECIAL ATTENTION SHOULD BE TAKEN TO INSTALL FASTENERS AS INDICATED ON THE DRAWINGS.

4. WHEN FASTENING INTO EXISTING LUMBER, IT MAY BE NECESSARY TO PREDRILL HOLES SO THAT MALE OR SCREWS DO NOT CRACK EXISTING FRAMING. TO ACHIEVE PROPER FASTENER ALIGNMENT: TEMPORARILY PIN 10ST HANGER IN PLACE WHILE DRILLING HOLES.

NEW STRUCTURAL FRAMING INSTALLATION SHOULD OCCUR WHILE TEMPORARY SHORING. IS IN PLACE. FINISH CAPPENTRY AND FINISH PAINTING SHOULD OCCUR AFTER TEMPORARY. SHORING IS REMOVED.

8. KEEP LUMBER STORED ON SITE OFF OF THE GROUND BY USING SPACER BLOCKS STORE OUT OF DIRECT SUNLIGHT TO DIMINISH UNEVEN DRYING EFFECTS.

7. DO NOT NOTCH JOISTS IN THE MIDDLE-THIRD OF THEIR SPAN.
REPAIR EXISTING, NOTCHED JOISTS AND RAFTERS BY "SISTERING" THEM
WITH NEW MATERIAL OF THE SAME CEPTH.
QLUE AND SCREW NEW MATERIAL TO THE EXISTING MATERIAL.
PREDRILL HOLES INTO ADDED PIECES TO PROVIDE PRE-DRILLING TEMPLATE
FOR EXISTING MATERIAL. REFER TO PLANS AND DETRILS FOR OVERLAY LENGTHS.

STRUCTURAL STEEL

UNLESS NOTED, ALL STRUCTURAL STEEL ON THIS PROJECT SHOULD CONFORM TO THE FOLLOWING ASTM STANDARDS:
PLATES, BARS ASTM AS Fy = 50 (as)
THESS AND LOW STRUCTURAL SECTIONS). ACTU A CO.

LTS ASTM A325

 STRUCTURAL STEEL FABRICATION, ERECTION, AND CONNECTION DESIGN SHALL CONFORM TO T AISC "SPECIFICATION FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL" LATEST EDITION.

3. WELDED CONSTRUCTION SHALL COMPLY WITH THE LATEST EDITION OF AWS D1.1, STRUCTURAL WELDING CODE FOR PROCEDURES, APPEARANCE, QUALITY OF WELDS, AND METHODS USED IN CONNECTING WELDING WORK. WELDING ELECTRODES SHALL BE ETXX.

4. FIELD BOLT CONNECTIONS USING ASTM A325N HIGH STRENGTH BOLTS, GALVANIZED PER ASTM A122, EXCEPT WHERE FIELD WELDING IS NOTED ON THE DRAWINGS. BOLT HOLES SHALL BE DESIGNED FOR REACTIONS SHOWN ON DRAWINGS.

5. PRIME STEEL PER SSPC-SP3 REQUIREMENTS, USING TWEMEC 10-99 ALAYD RUST INHIBITIVE PRIMER. FOR BOTTOMS OF POSTS PLACED IN CONTACT WITH CONCRETE, PROVIDE TWO LAYERS OF PRIMER TO A HEIGHT UP TO 1-0" ABOVE POST BASES, PRIME ALL TUBE SHAPES INSIDE AND OUT

SPECIAL NOTES TO BUILDER

 THE SCOPE FOR THIS PROJECT ENTAILS MODIFYING THE EXISTING FRAMING TO PROVIDE A LEVEL SURFACE FOR THE NEW FIRST INDOR CELLING, WHILE SUPPORTING CODE REQUIRED LIVE LOADS ON THE SECOND FLOOR AND ROOF, AS NECESSARY.

2. IN ORDER TO NOT SIGNIFICANTLY PRESTRESS THE NEW FRAMING AND DAMAGE EXISTING SECOND FLOOR PRINSHES, THE EXISTING SECOND FLOOR JOISTS SHOULD NOT BE SIGNIFICANTLY JACKED, DESPITE THE VARYING LEVEL OF THE FLOOR FRAMING. ANY JACKING THAT OCCURS SHOULD OCCUR SLOWLY, SO AS NOT TO CRACK EXISTING FINISHES. MINIMAL JACKING, UP TO }; MAY OCCUR BEACHT SUGGESTED TEMPORARY SHORING, PROVIDED THAT VERY LITTLE RESISTANCE IS ENCOUNTERED WHEN JACKING.

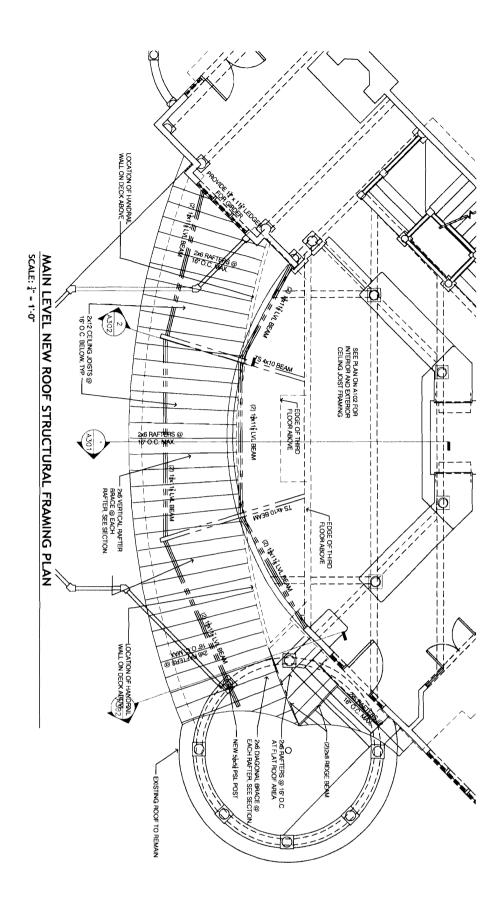
3. NOTIFY THE ENGINEER BEFORE SUBCONTRACTORS DRILL HOLES INTO THE FRAMING OR NOTCH FRAMING



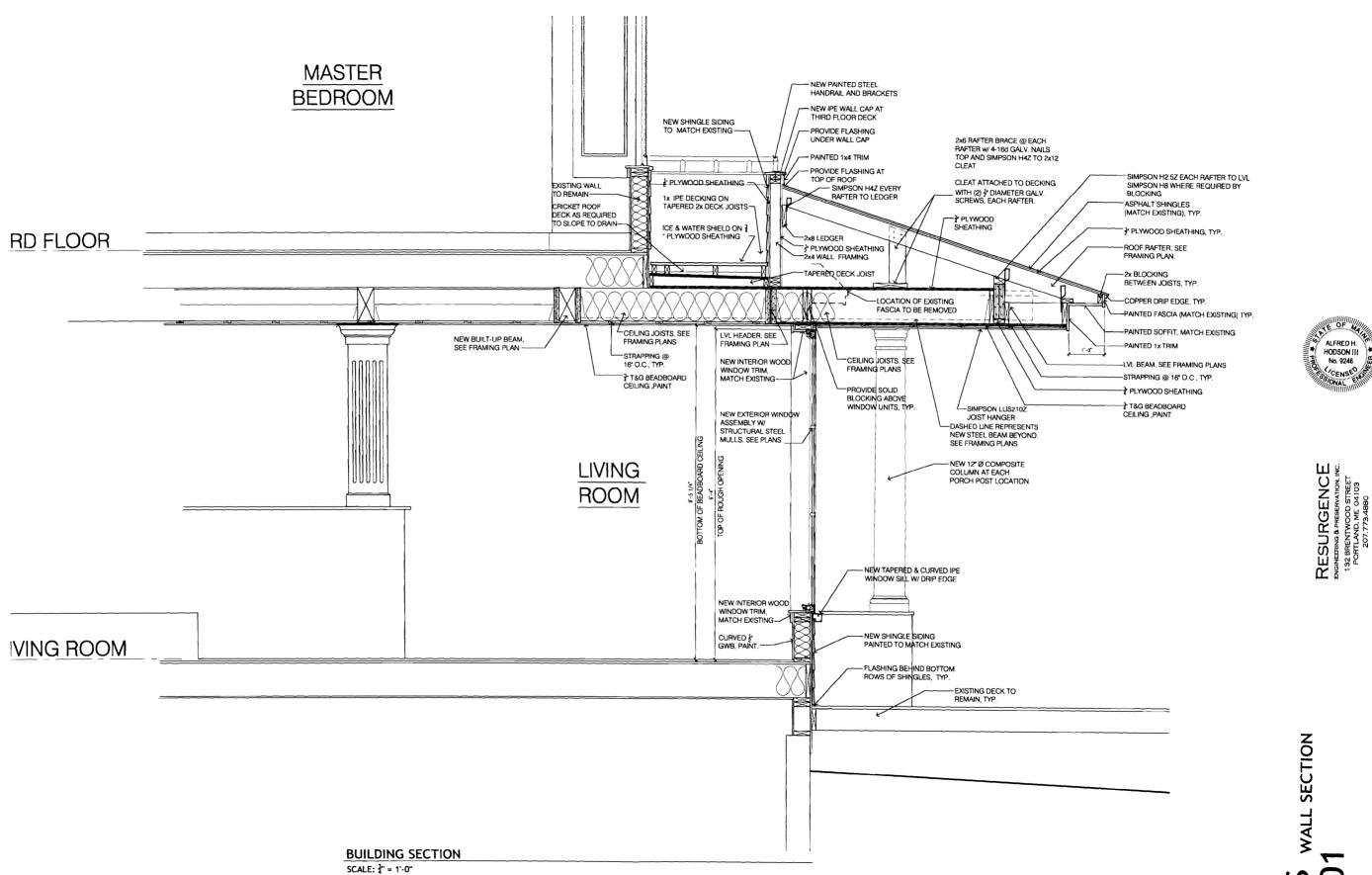
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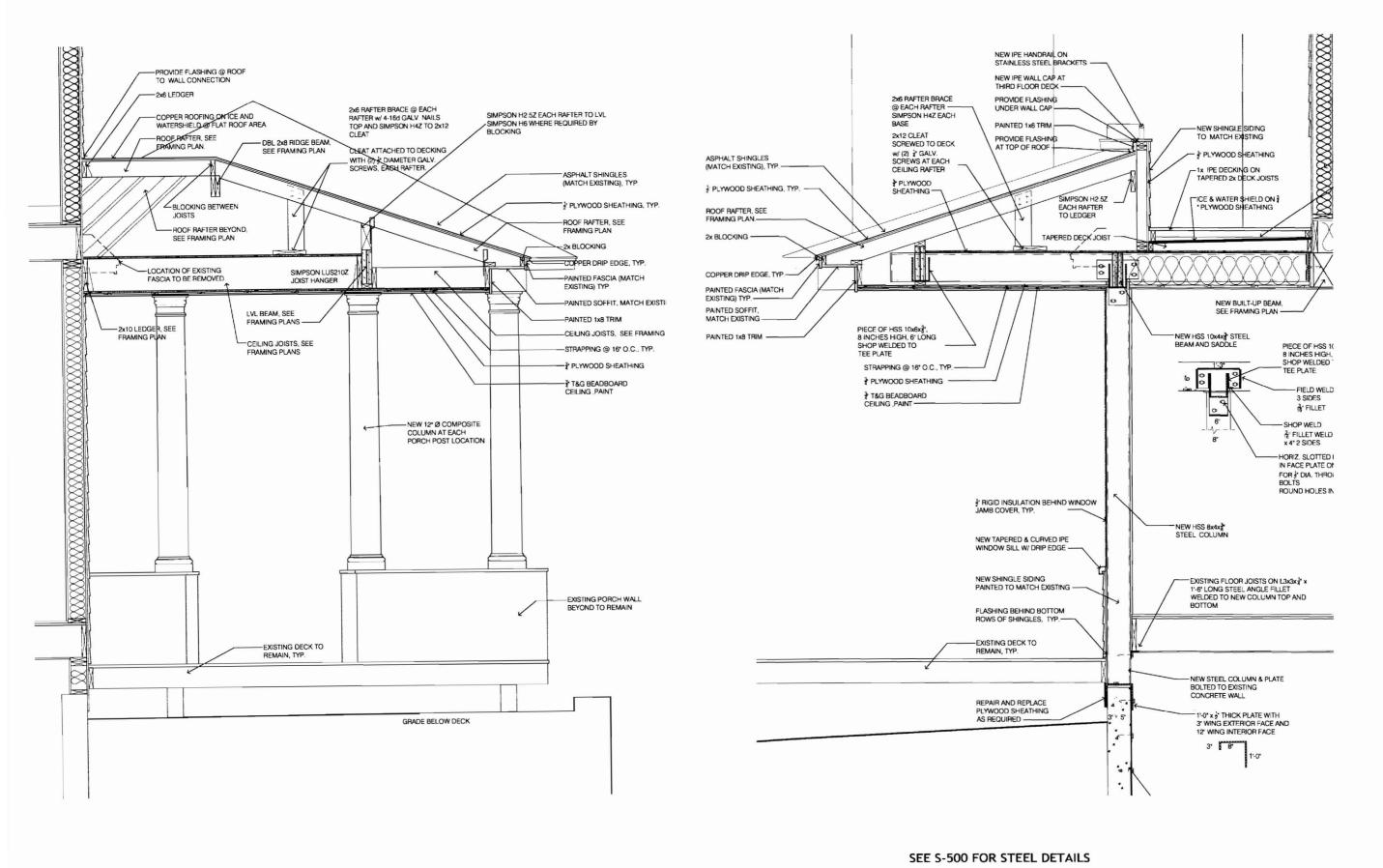
STRUCTURAL PL AND CONCRETE











ALFRED H.
HODSON III **
No. 9246
CENSE

RESURGENCE
122 BENTHAUD STREET
PORTLAND, IN CARGO

ILL SECTIONS

302

BUILDING SECTION

SCALE: $\frac{3}{4}$ " = 1'-0"

BUILDING SECTION

ALE: 3" = 1'-0"

