THIS HOME HAS BEEN DESIGNED SPECIFICALLY FOR:

CARTER

19 MERRIAM STREET (ON PEAKS ISLAND)
CITY OF PORTLAND, ME 04108
CUMBERLAND COUNTY

■ EXCEL HOMES OF ME.
56 MECHANIC FALLS ROAD
OXFORD, ME 04270
PHONE: (888) 333-1748
FAX: (207) 539-0944

BUILDER:

HALLMARK HOMES

PFS CORPORATION
Approval Limited to Factory Built Portion Only

State:	0DLQH
Signature:	Renee Moist
Title:	Staff Plan Reviewer
Date:	12/12/14

Renee Moist
Digitally signed by Renee Moist
DN: cn=Renee Moist, o=PFS Corporation, ou=NorthEast Region, email=RMoist@PFSCorporation.Com, c=US
Date: 2014.12.12 11:24:33 -05'00'

THERE ARE NO LOT LINE FIRE SEPARATION REQUIREMENTS

FRONT "WEST" ELEVATION

SITE CONDITIONS:

GROUND SNOW LOAD:	50 PSF
WIND SPEED:	<100 MPH
EXPOSURE:	B
SEISMIC CATEGORY:	B
USE GROUP:	SINGLE FAMILY
CONSTRUCTION TYPE:	VB WOOD FRAME UNPROTECTED

SQUARE FOOTAGE:

FIRST FLOOR:	887 SQ. FT.
SECOND FLOOR:	559 SQ. FT.
BONUS ROOM:	- SQ. FT.
GARAGE:	- SQ. FT.
TOTAL:	1,446 SQ. FT.
OVERALL SIZE	27'-0 3/4" x 20'-8"/32'-9 3/8"
MODEL:	CUSTOM COLONIAL

NOTES:

1. ITEMS SHOWN ON THE EXTERIOR ELEVATION DRAWINGS ARE FOR ILLUSTRATIVE PURPOSES ONLY
2. GRILLS SHOWN ARE FOR ILLUSTRATIVE PURPOSES ONLY (SEE WINDOW MANUFACTURER CATALOG FOR ACTUAL GRILL PATTERN)

PTL#: KIM 4350
☐ PD ■ QN ☐ SN
Inspections Division
Date: 02/12/15

BUILDER: HALLMARK HOMES

CUSTOMER/PROJECT: CARTER (32307)

KEISER HOMES BRAND
BUILT BY EXCEL HOMES OF MAINE

THIS DRAWING WAS EXTRACTED FROM APPROVED PLANS AND/OR APPROVED SYSTEMS DRAWINGS

NAME DATE

THIS DRAWING AND DESIGN IS THE PROPERTY OF EXCEL HOMES GROUP, LLC. ANY REPRODUCTION OF THE DRAWING OR DESIGN IS PROHIBITED WITHOUT THE EXPRESS WRITTEN CONSENT OF EXCEL HOMES GROUP, LLC.

ADJUSTMENTS MADE FOR CODE COMPLIANCE AND PRODUCTION CAPABILITY

DRAWING MAY BE REVERSED

DRAWN: YYD PIF MJC SLP RT
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NO:	DATE:	DESCRIPTION
QN1	10-21-14	PERMIT SET
QN2	11-12-14	
QN3	11-18-14	
KH-1	11-20-14	
KH-2	12/2/14	

DRAWING TITLE: COVER SHEET

SCALE: 1/8" = 1'-0"
SHEET: P1

REAR "EAST" ELEVATION

LEFT "NORTH" ELEVATION

RIGHT "SOUTH" ELEVATION

NOTES:

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QN2	11-12-14		PIF	
QN3	11-18-14		MJC	
KH-1	11-20-14		SLP	
KH-2	12/2/14	PERMIT SET	RT	

DRAWING TITLE:

ELEVATIONS

SCALE: 3/32" = 1'-0"

SHEET: P1.1

MAINE

2009 INTERNATIONAL RESIDENTIAL CODE w/EXCEPTIONS
2011 NFPA 31, STD FOR THE INSTALLATION OF OIL BURNING EQUIP
2009 NFPA 54, NATIONAL FUEL GAS CODE
2011 NFPA 70, NATIONAL ELECTRIC CODE w/EXCEPTIONS
2010 NFPA 211 STANDARDS FOR CHIMNEYS, FIREPLACES, VENTS AND SOLID FUEL BURNING APPLIANCES
2009 UNIFORM PLUMBING CODE w/EXCEPTIONS
2011 STATE OF MAINE OIL AND SOLID FUEL BOARD LAW AND RULES
2011 NATIONAL ELECTRIC CODE w/EXCEPTIONS

ALL PLUMBING VENT TERMINATIONS ARE 24" ABOVE THE ROOF. WATER CLOSET VENTS & ALL WET VENTS ARE MIN. 2" PER THE 2009 UNIFORM PLUMBING CODE WITH MAINE EXCEPTIONS.

* IN ATTIC, RADON VENT TO RUN TOWARDS CENTER OF HOUSE TO GET THE PROPER CLEARANCE.

NOTE: SHINGLES TO BE ATTACHED PER THE MANUFACTURES INSTALLATION INSTRUCTIONS FOR COASTAL REGIONS

NOTES:
1. 2x6 EXT WALLS @ 16" O.C./2x4 MARR WALLS @ 16" O.C. (EXCEPT AS NOTED)
2. 8'-0" CLG HT.
3. 2x10 SPF#2 FLOOR JOISTS @ 16" O.C.
4. ROOF SYSTEM TO BE 16" O.C.
5. PARADIGM HYBRID SINGLE HUNG, CASEMENT & AWNING WINDOWS
6. BASED ON <100 MPH WIND LOAD & EXPOSURE "B"
7. SITE LOCATION: PEAKS ISLAND, ME; CUBMERLAND COUNTY; 50 PSF GROUND SNOW LOAD
8. CLG GIRDER OVER LIVING/DINING TO BE: 4-1 1/2"x11 1/4" M.L. (2-PER MODULE) -- FASTEN PLIES TOGETHER USING (2) ROWS OF 0.131"x3" NAILS @ 8"O.C.
9. CLG BEAM FOR SINGLE STORY ROOF TO BE: 2-1 1/2"x11 1/4" M.L. -- FASTEN PLIES TOGETHER USING (3) ROWS OF 0.131"x3" NAILS @ 8"O.C.
10. CLG BEAM UNDER 2nd FLR EXTERIOR WALL TO BE: 2-1 1/2"x11 1/4" M.L. -- FASTEN PLIES TOGETHER USING (2) ROWS OF 0.131"x3" NAILS @ 8"O.C.

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Date: 12/12/14

MAINE ENERGY SPECIFICATION TABLES

MINIMUM INSUL. R-VALUES		MAXIMUM U-FACTORS	
CEILINGS	R-38	ENTRANCE DOORS	.35
ROOF/CEILINGS	R-38	SPECIALTY DOORS	.45
WALLS	R-19	WINDOWS	.35
FLOORS	R-19	SKYLIGHTS	.60

HOUSE TO BE BUILT OVER UNCONDITIONED SPACE. BUILDER IS RESPONSIBLE TO PROVIDE & INSTALL FLOOR INSULATION ON THE 1ST FLOOR PER THE MAINE ENERGY CODE. THE BUILDER ALSO REQUIRED TO PROVIDE & INSTALL A DOOR SWEEP & WEATHER STRIPPING AT BASEMENT DOOR.
EXCEL TO PROVIDE & INSTALL R-11 INSULATION IN WALLS AND R-19 INSULATION IN THE CEILING OF ANY BASEMENT STAIR ENCLOSURES.

ADDITIONAL MAINE REQUIREMENTS

* SET MANUAL MUST BE IN RESIDENCE.
* COPY OF APPROVED SETS MUST BE IN RESIDENCE.
* WINDOWS REQUIRE ARGON UPGRADE TO MEET MAINE U-FACTOR REQUIREMENTS.

FOR THE STATE OF MAINE, FOR BASEMENTS HEIGHTS FROM 7'-3" TO 8'-0" BASEMENT STAIRS ARE A COMPONENT OF THIS DESIGN WITH A MAXIMUM RISER HEIGHT OF 8-1/4": A MINIMUM TREAD DEPTH OF 9" AND A 1" NOSING WILL BE PROVIDED ON ALL TREADS WITH TREAD WIDTH LESS THAN 10"

RANGE SHALL BE EQUIPPED WITH A SEPARATE FAN/HOOD WITH A MIN. RATING OF 100 CFM. EACH BATHROOM WILL BE EQUIPPED WITH A SEPARATE VENTILATING FAN THAT HAS A MIN. RATING OF 50 CFM AND BE RATED FOR SOUND AT A MAX. SOUND RATING OF 3 SONE. ALL RANGE & BATH FANS TO BE VENTED TO THE EXTERIOR.

PER MAINE RADON REQUIREMENTS, THE RADON PIPE SHALL BE A MIN. 12" ABOVE THE ATTIC SPACE AND HAVE A 36" HIGH BY 24" DIAMETER CLEARANCE FOR COMPLETION.

PARADIGM WINDOW SCHEDULE

WINDOW CALL SIZE	UNIT SIZE	ROUGH OPENING	TYPE	LIGHT FT.	VENT FT.	SQ. FT.	U-VALUE
A2424	23 1/2" x 23 1/2"	24 1/2" x 24 1/2"	AWNING	2.17	1.52	4.0	.22
A2424-2	47" x 23 1/2"	48" x 24 1/2"	AWNING	4.35	3.05	8.0	.22
2436-2	47" x 35 1/2"	48" x 36 1/2"	HYBRID SH	7.51	3.88	12.0	.22
* C2848-2	55" x 47 1/2"	56" x 48 1/2"	CASEMENT	12.56	11.95	18.67	.22
* 2860-2	55" x 59 1/2"	56" x 60 1/2"	HYBRID SH	16.65	8.59	23.34	.22

NOTE: SAFETY GLAZING TO BE PROVIDED FOR WINDOWS IN HAZARDOUS LOCATIONS
NOTE: WINDOWS ARE NFRC RATED
* MEETS EGRESS REQUIREMENTS

STANDARD -EXTERIOR (INSWING) DOOR SCHEDULE

DOOR CALL SIZE	WIDTH	HEIGHT	ROUGH OPENING	MATERIAL	MANUFACTURER	TYPE	U-VALUE
3068 4-LITE	3'-0"	6'-8"	38 3/4" x 82 3/4"	INSUL. CORE	THERMA-TRU	EXT HINGED	.16
PAR 5068 SGD	5'-0"	6'-8"	60 1/2" x 80"	INSUL. CORE	PARADIGM	EXT-SLIDER	.30

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KEISER HOMES BRAND
BUILT BY EXCEL HOMES OF MAINE

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QN1	10-21-14	
QN2	11-12-14	
QN3	11-18-14	
KH-1	11-20-14	
KH-2	12/2/14	PERMIT SET

DRAWING TITLE: FIRST FLOOR PLAN

SCALE: 3/16" = 1'-0"
SHEET: P2

NOTES:
1. 2x6 EXT WALLS @ 16" O.C./2x4 MARR WALLS @ 16" O.C. (EXCEPT AS NOTED)
2. 8'-0" CLG HT.
3. 2x10 SPF#2 FLOOR JOISTS @ 16" O.C.
4. ROOF SYSTEM TO BE 16" O.C.
5. PARADIGM HYBRID SINGLE HUNG WINDOWS
6. FLR GIRDER TO BE: 4-1 1/2"x9 1/4"x 20'-8" M.L. (2-PER MODULE) -- FASTEN PLIES TOGETHER USING (2) ROWS OF 0.131"x3" NAILS @ 8"O.C.

PFS CORPORATION
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State: 0DLQH
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Title: Staff Plan Reviewer
Date: 12/12/14

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Inspections Division
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CHECKED: PIF

NO:	DATE:	DESCRIPTION
QN1	10-21-14	
QN2	11-12-14	
QN3	11-18-14	
KH-1	11-20-14	
KH-2	12/2/14	PERMIT SET

DRAWING TITLE: SECOND FLOOR PLAN

SCALE: 3/16" = 1'-0"
SHEET: P2.1

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State: 0DLQH
Signature: Renee Moist
Title: Staff Plan Reviewer
Date: 12/12/14

2011NEC

* THIS PLAN MAY HAVE ADDITIONAL APPLIANCES/FIXTURES ADDED TO THE ELECTRICAL SCHEMATIC AND/OR PANEL BOX PROVIDING THE LOADING DOESN'T EXCEED THE MAXIMUM ALLOWED BY STATE & LOCAL CODES.
* E-CUTOFF SWITCH ON-SITE BY OTHERS PER ALL STATE & LOCAL CODES.
* ALL BRANCH CIRCUITS SUPPLYING 15 & 20 AMPERE OUTLETS IN LIVING SPACES ARE PROTECTED BY AN ARC-FAULT CIRCUIT INTERRUPTER IN ACCORDANCE WITH SECTION 210.12.2011 NEC.
* PER 406.12 OF 2011 NEC ALL 125 -VOLT, 15 AND 20 AMP RECEPTS INSTALLED IN AREAS SPECIFIED BY 210.52, SHALL BE LISTED TAMPER RESISTANT TYPE.
* BUILDER IS RESPOSIBLE FOR INSTALLING GAS LINES
* 50# LIGHT BOXES REQUIRED

PTL#: KIM 4350
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BUILDER: HALLMARK HOMES

CUSTOMER/PROJECT: CARTER (32307)

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QN1	10-21-14	
QN2	11-12-14	
QN3	11-18-14	
KH-1	11-20-14	
KH-2	12/2/14	PERMIT SET

DRAWING TITLE:
FIRST FLOOR ELECTRICAL PLAN

SCALE: NTS
SHEET: P6

PFS CORPORATION
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State: 0DLQH
Signature: Renee Moisa
Title: Staff Plan Reviewer
Date: 12/12/14

2011NEC

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* E-CUTOFF SWITCH ON-SITE BY OTHERS PER ALL STATE & LOCAL CODES.
* ALL BRANCH CIRCUITS SUPPLYING 15 & 20 AMPERE OUTLETS IN LIVING SPACES ARE PROTECTED BY AN ARC-FAULT CIRCUIT INTERRUPTER IN ACCORDANCE WITH SECTION 210.12.2011 NEC.
* PER 406.12 OF 2011 NEC ALL 125 -VOLT, 15 AND 20 AMP RECEPTS INSTALLED IN AREAS SPECIFIED BY 210.52, SHALL BE LISTED TAMPER RESISTANT TYPE.
* BUILDER IS RESPOSIBLE FOR INSTALLING GAS LINES
* 50# LIGHT BOXES REQUIRED

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QN2	11-12-14	
QN3	11-18-14	
KH-1	11-20-14	
KH-2	12/2/14	PERMIT SET

DRAWING TITLE:
SECOND FLOOR ELECTRICAL PLAN

SCALE: NTS
SHEET: P6.1

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Signature: Renee Moist
Title: Staff Plan Reviewer
Date: 12/12/14

CS-WSP 6d COMMON (0.113" x 2") NAILS @ 6" O.C. SPACING (PANEL EDGES) AND 12" O.C. (INTERMEDIATE SUPPORTS)
or 16 GAUGE x 1 3/4" STAPLES @ 3" O.C. SPACING (PANEL EDGES) AND 6" O.C. (INTERMEDIATE SUPPORTS)

EXTERIOR WALL LINE	TOTAL WALL LINE LENGTH	BRACED WALL SPACING	BRACED WALL METHOD	TABULATED MIN. BRACED WALL TOTAL Table R602.10.1.2(1)	WIND EXPOSURE FACTOR Table R602.10.1.2(1)a	RIDGE TO EAVE HEIGHT FACTOR Table R602.10.1.2(1)c	WALL HEIGHT FACTOR Table R602.10.1.2(1)d	BRACED WALL LINE QUANTITY FACTOR Table R602.10.1.2(1)e	ADJUSTED MIN. BRACED WALL LENGTH REQ'D	BRACED WALL LENGTH PROV'D	
1	32.78'	27.06'	CS-WSP	1.0 (<100mph)	1.0 ("B")	1.031 (11'-0.5")	.9 (8'-0")	1.0 (2)	9.21'	20.9'	PASSES
2	32.78'	27.06'	CS-WSP	1.0	1.0	1.031	.9	1.0	9.21'	20.6'	PASSES
A	27.06'	32.78'	CS-WSP	1.0	1.0	1.031	.9	1.0	11.16'	18.1'	PASSES
B	27.06'	32.78'	CS-WSP	1.0	1.0	0.7 (<5')	.9	1.0	4.13'	22.4'	PASSES

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QN3	11-18-14	
KH-1	11-20-14	
KH-2	12/2/14	PERMIT SET

DRAWING TITLE:

FIRST FLOOR SHEAR WALL PLAN

SCALE: NTS
SHEET: 10A

CS-WSP 6d COMMON (0.113" x 2") NAILS @ 6" O.C. SPACING (PANEL EDGES) AND 12" O.C. (INTERMEDIATE SUPPORTS)
or 16 GAUGE x 1 3/4" STAPLES @ 3" O.C. SPACING (PANEL EDGES) AND 6" O.C. (INTERMEDIATE SUPPORTS)

EXTERIOR WALL LINE	TOTAL WALL LINE LENGTH	BRACED WALL SPACING	BRACED WALL METHOD	TABULATED MIN. BRACED WALL TOTAL Table R602.10.1.2(1)	WIND EXPOSURE FACTOR Table R602.10.1.2(1)a	RIDGE TO EAVE HEIGHT FACTOR Table R602.10.1.2(1)c	WALL HEIGHT FACTOR Table R602.10.1.2(1)d	BRACED WALL LINE QUANTITY FACTOR Table R602.10.1.2(1)e	ADJUSTED MIN. BRACED WALL LENGTH REQ'D	BRACED WALL LENGTH PROV'D	
3	20.67'	27.06'	CS-WSP	1.0 (<100mph)	1.0 ("B")	1.063 (11'-0.5")	.9 (8'-0")	1.0 (2)	5.18'	16.0'	PASSES
4	20.67'	27.06'	CS-WSP	1.0	1.0	1.063	.9	1.0	5.18'	11.3'	PASSES
C	27.06'	20.67'	CS-WSP	1.0	1.0	1.063	.9	1.0	3.95'	23.1'	PASSES
D	27.06'	20.67'	CS-WSP	1.0	1.0	1.063	.9	1.0	3.95'	17.7'	PASSES

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KH-2	12/2/14	

DRAWING TITLE:

SECOND FLOOR SHEAR WALL PLAN

SCALE: NTS

SHEET: 10B

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PERIMETER INSULATION
(NTS)

CEILING RIM LEDGER DETAILS FOR SUPPORT OF FLOOR FRAME ABOVE

2nd FLR JST SUPT. LEDGERS @ EXTERIOR SIDE
(NTS)

2nd FLR JST SUPT. LEDGERS @ MATE WALL SIDE
(NTS)

A WEATHER PROTECTIVE BARRIER THAT MEETS THE REQUIREMENTS OF ASTM D226 WILL BE INSTALLED UNDER THE VINYL SIDING

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QN3	11-18-14	
KH-1	11-20-14	
KH-2	12/2/14	

DRAWING TITLE: SECTION

SCALE: NTS
SHEET: P13

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KH-1	11-20-14	
KH-2	12/2/14	PERMIT SET

DRAWING TITLE: SECTION

SCALE: NTS
SHEET: P13.1

ALL BASEMENT WALLS ARE 8'-0" HIGH (EXPOSED HEIGHT VARIES)

NOTES:

1) THIS FOUNDATION DRAWING AND NOTES ARE FOR REVIEW ONLY. ACTUAL FOUNDATION DESIGN SHALL BE DESIGNED FOR YOUR SPECIFIC SITE AS REQUIRED PER LOCAL CODE.
2) ALL CONSTRUCTION AND MATERIALS BELOW THE BOTTOM OF THE FLOOR JOISTS IS THE RESPONSIBILITY OF EXCEL HOMES' BUILDER. FOUNDATION WALL, FOOTER, LOLLY COLUMNS & PADS ARE ALL TO BE DETERMINED ON-SITE IN ACCORDANCE WITH LOCAL CODES & CONDITIONS
3) REINFORCED MASONRY UNIT REQUIRED AT CENTER BEARING LOCATIONS.
4) LOLLY COLUMNS ARE TO BE SPACED ACCORDING TO SPAN SCHEDULE BASED ON TYPE OF HOUSE, LOCAL SNOW LOAD & WIDTH OF UNIT. ADD'L COLUMNS MAY BE REQ'D & ARE NOTED ON FLOOR PLAN. LOLLY COLUMN SPACING IS BASED ON MIN. 1/2"x6"x12" STEEL PLATE BETWEEN COLUMN & CENTER BEAM.
5) FOUNDATION IS TO BE CONSTRUCTED IN ACCORDANCE w/ALL APPLICABLE CODES.
6) FOUNDATION SIZES REFLECT WOOD TO WOOD DIMENSIONS OF MODULAR UNITS, ALLOWING SHEATHING AND SIDING TO OVERHANG THE FOUNDATION. IF STYROFOAM IS USED FOUNDATION MAY BE INCREASED IN LENGTH AND WIDTH TO ACCOMODATE.
7) PERIMETER FLOOR JOISTS TO BE ATTACHED TO SILL PLATE w/16d NAILS AT 16" O.C.
8) INSTALLATION OF WASHER, DRYER AND/OR WATER HEATER IN BASEMENT PER STATE AND LOCAL CODES IS THE RESPONSIBILITY OF MODULAR MANUFACTURERS, BUILDER.
9) SMOKE DETECTORS IN BASEMENT SHALL BE THE RESPONSIBILITY OF THE BUILDER TO PROVIDE AND INSTALL. (COIL WIRE IN BSMT BY MODULAR MANUFACTURER, INC)
10) CRAWL SPACE FOUNDATION REQUIRES A MINIMUM 18"x24" ACCESS OPENING, INSECT & RODENT PROOF CROSS VENTS WITHIN 3' OF CORNERS AND PROVIDE 1/150 OF FLOOR AREA WITH VENTILATION.
11) SEE MODULAR MANUFACTURERS' SUBMISSION SET PAGE FOR ADDITIONAL NOTES AND DETAILS.
12) GFCI RECPT AND LIGHTS FOR BASEMENT AND CRAWLSPACES PER ALL STATE AND LOCAL CODES
13) "BACKFILLING AND TAMPING TO BE DONE PER LOCAL REQUIREMENTS"

PFS CORPORATION
Approval Limited to Factory Built Portion Only

State: 0DLQH
Signature: Renee Moisa
Title: Staff Plan Reviewer
Date: 12/12/14

PTL#: KIM 4350
☐ PD ■ QN ☐ SN
Inspections Division
Date: 02/12/15

BUILDER: **HALLMARK HOMES**

CUSTOMER/PROJECT: **CARTER (32307)**

KEISER HOMES BRAND

BUILT BY EXCEL HOMES OF MAINE

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NO:	DATE:	DESCRIPTION	DRAWN:	CHECKED:
QN1	10-21-14		YYD	PIF
QN2	11-12-14		PIF	
QN3	11-18-14		MJC	
KH-1	11-20-14		SLP	
KH-2	12/2/14	PERMIT SET	RT	

DRAWING TITLE: **FOUNDATION PLAN**

SCALE: **3/16" = 1'-0"**

SHEET: **P21**

ALL FASTENINGS SHOWN TO BE HOUSE SPECIFIC. SEE MODEL SPECIFIC FASTENING SCHEDULE CONTAINED WITHIN THE PERMIT SET. FASTENING SCHEDULE BASED ON SECTIONS 8 & 10 OF THE CALCULATION MANUAL, STAMPED & APPROVED BY A REGISTERED PROFESSIONAL ENGINEER & THIRD PARTY INSPECTION AGENCY (PFS).

THESE STEPS ARE THE SAME FOR BOTH SIDES

TYP CONNECTIONS FOR TWO-STORY W/ STORAGE TRUSS

SEE HOUSE SPECIFIC FASTENING SCHEDULE AT RIGHT.

SECTION LEGEND

- C6 = CONNECTION REQUIRED. SEE CHART AT RIGHT.
- M1 = CONNECTION BY MODULAR MANUFACTURER. SEE SCHEDULE
- S1 = CONNECTION TO BE COMPLETED ON-SITE. SEE SCHEDULE AT RIGHT.

FASTENING SCHEDULE

HOUSE-SPECIFIC INFO

QUOTE#: 4350	STATE: ME
WIND SPEED: <100 mph	SNOW LOAD: 50 psf
MEAN ROOF HT.: 24'-10 1/4"	
ROOF O.C.: 16 in	
WALL O.C.: 16 in	
OVERHANG DEPTH: 10 in	
EAVE OVERHANG: FIXED	
LOCATION: MAIN HOUSE	
EXTERIOR WALLS: 2x6	
MARRIAGE WALLS: 2x4	
WALL HT.: 8.0 ft	
GABLE WALL HT.: 11 ft	

TRUSS-SPECIFIC CONNECTION INFO

CONNECTION LOCATION	FORCE (lbs)		
	TENSION	SHEAR	COMPRESSION
RIDGE TO RIDGE	153	149	
RIDGE FLIP TO TOP CHORD	102	200	
COLLAR TIE TO TOP CHORD	220	19	626
KNEEWALL TO TOP CHORD			
KNEEWALL TO BOTTOM CHORD (OR KING POST)	339	161	
TRUSS HEEL UPLIFT	33		
TRUSS HEEL HORIZONTAL	198		
TRUSS MATEWALL UPLIFT	59		
TRUSS MATEWALL HORIZONTAL	121		

FASTENING TO BE COMPLETED "ON-SITE"

CONN.#	DES	CONNECTION AREA	CONNECTION REQUIRED	CALC MANUAL PAGE REF#
C1	S1	RIDGE TO RIDGE	(2) 0.131" x 3-1/4" FACE-NAILS PER TRUSS BAY	8.0.4
	S2	FLIP RAFTER TO FLIP RAFTER	(1) 1.25" x 20 GA STRAP w/ (14) 8d NAILS EVERY THIRD TRUSS	8.0.5
C2	S1	TOP CHORD CONTINUOUS TO FLIP CONTINUOUS	(3) 0.131" x 3-1/4" FACE-NAILS PER TRUSS BAY	8.0.7
C3	S1	COLLAR TIE TO RAFTER	(7) 0.131" x 3-1/4" FACE-NAILS BOTH SIDES OF COLLAR-TIE	8.0.13
C4	S1	KNEEWALL TO TRUSS CHORD (STORAGE TRUSS)	1 SIMPSON H8	8.0.12
	S2	KNEEWALL TO TRUSS CHORD (SHED TRUSS)	(1) 1.25" x 26 GA STRAP w/ (8) 8d NAILS EVERY THIRD TRUSS	8.0.12
C5	S1	SHEATHING TO TOP CHORD	(3) 0.131" x 2-1/2" FACE-NAILS PER TRUSS	10.18.0
	S2	FLIP CONTINUOUS TO STUD	N/A	10.22.0
C7	S1	DECKING ACROSS MATEWALL (TRUSS TO TRUSS)	N/A	8.0.11
C12	S1	SHEATHING BAND TO RIM JOISTS	(2) ROWS OF 0.131" x 2-1/2" FACE-NAILS AT 16" O.C.	10.4.0
	S2	RIM JOIST TO RIM JOIST (HORIZONTAL LOADING)	(1) ROW OF 0.131" x 3-1/4" TOE-NAILS AT 6" O.C.	2009 IRC
C13	S1	SHEATHING BAND TO RIM	(2) ROWS OF 0.131" x 2-1/2" FACE-NAILS AT 16" O.C.	10.4.0
	S2	SHEATHING BAND TO SILL PLATE	(1) ROW OF 0.131" x 2-1/2" FACE-NAILS AT 16" O.C.	10.5.0
	S3	FLOOR RIM TO SILL PLATE	(1) ROW OF 0.131" x 3-1/4" TOE-NAILS AT 6" O.C.	10.14.0
C14	S1	WALL PLATE TO RIM & RAFTER	(1) ROW OF 0.131" x 3-1/4" FACE NAILS AT 8" O.C.	10.20.0

FASTENING TO BE COMPLETED BY "MANUFACTURER"

CONN.#	DES	CONNECTION AREA	CONNECTION REQUIRED	CALC MANUAL PAGE REF#
C1	M1	RIDGE TO FLIP RAFTER	(3) 0.131" x 3-1/4" FACE-NAILS PER TRUSS	8.0.2
C2	M1	SHEATHING TO ROOF CONTINUOUS	(2) 0.131" x 2-1/2" FACE-NAILS PER TRUSS EA. SIDE	8.0.8
	M2	CONTINUOUS TO FLIP RAFTER OR TOP CHORD	(4) 0.131" x 3-1/4" FACE-NAILS PER TRUSS	8.0.6
C4	M1	KNEEWALL PLATE TO KNEEWALL OR KINGPOST	(3) 0.131" x 3-1/4" FACE-NAILS PER TRUSS	8.0.9
	M2	KINGPOST TO KNEEWALL STUD	(1) 1.25" x 26 GA STRAP w/ (8) 8d NAILS EVERY THIRD TRUSS	8.0.10
C6	M1	SHEATHING TO WALL PLATES	(2) ROWS OF 0.131" x 2-1/2" NAILS AT 16" O.C.	10.3.0
	M2	TRUSS TO TOP PLATE (HORIZONTAL LOADING)	(2) 0.131" x 3-1/4" TOE-NAILS	10.10.0
	M3	TRUSS TO TOP PLATE (OR WALL STUD)	(1) SIMPSON MTS30 - EVERY THIRD TRUSS	10.2.0
	M4	CONTINUOUS TO BOTTOM CHORD	(1) 0.131" x 3-1/4" FACE-NAILS PER TRUSS	10.15.0
C9	M1	PLATE TO STUD	(2) 0.131" x 3-1/4" FACE-NAILS PER STUD	10.12.0
	M2	PLATE TO PLATE	(1) ROW OF 0.131" x 3-1/4" FACE-NAILS AT 13" O.C.	10.11.0
C10	M1	CONTINUOUS TO WALL STUD	(1) LSTA12 STRAP w/ (10) 0.148" x 2-1/2" NAILS AT 48" O.C.	10.17.0
	M2	CONTINUOUS TO TRUSS (OR CEILING JOIST)	(2) 0.131" x 3-1/4" FACE-NAILS PER TRUSS	10.15.0
	M3	CONTINUOUS TO WALL PLATES	(1) ROW OF 0.131" x 3-1/4" TOE-NAILS AT 6" O.C.	2009 IRC
C11	M1	FLOOR RIM TO WALL STUD	(1) LSTA12 STRAP w/ (10) 0.148" x 2-1/2" NAILS AT 48" O.C.	10.17.0
	M2	WALL PLATE TO FLOOR RIM	(2) ROWS OF 0.131" x 3-1/4" FACE-NAILS AT 16" O.C.	2009 IRC
C12	M1	SHEATHING TO RIM JOIST	(2) ROWS OF 0.131" x 2-1/2" FACE-NAILS AT 16" O.C.	10.4.0
	M2	RIM JOIST TO WALL PLATE	(1) ROW OF 0.131" x 3-1/4" TOE-NAILS AT 6" O.C.	2009 IRC
	M3	PLATE TO FLOOR RIM JOIST	(1) ROW OF 0.131" x 3-1/4" FACE-NAILS AT 7" O.C.	10.13.0
C13	M1	SHEATHING TO RIM JOIST	(2) ROWS OF 0.131" x 2-1/2" FACE-NAILS AT 16" O.C.	10.4.0
	M2	PLATE TO FLOOR RIM JOIST	(1) ROW OF 0.131" x 3-1/4" FACE-NAILS AT 7" O.C.	10.13.0
C14	M1	SHEATHING TO RIM AND GABLE WALL	(1) ROW OF 0.131" x 3-1/4" FACE-NAILS AT 6" O.C.	10.21.0
	M2	TRUSS BOTTOM CHORD TO WALL PLATE	(1) ROW OF 0.131" x 3-1/4" FACE-NAILS AT 6" O.C.	10.21.0

PFS CORPORATION
Approval Limited to Factory Built Portion Only
State: 0DLQH
Signature: Renee Moisa
Title: Staff Plan Reviewer
Date: 12/12/14

FASTENING REQUIREMENTS FOR TWO-STORY W/ STORAGE TRUSS

FASTENING SHOWN IS HOUSE SPECIFIC TO THE MODEL CONTAINED WITHIN THIS PERMIT SET.
ALTERNATE FASTENERS OF EQUAL OR GREATER VALUE MAY BE SUBSTITUTED
FOR THOSE SHOWN, PROVIDED THEY RESIST THE LOADS/FORCES IMPOSED PER CONNECTION.

PTL#: KIM 4350
☐PD ■QN ☐SN
Inspections Division
Date: 02/12/15

BUILDER: HALLMARK HOMES

CUSTOMER/PROJECT: CARTER (32307)

KEISER HOMES BRAND

BUILT BY EXCEL HOMES OF MAINE

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QN2	11-12-14		PIF	
QN3	11-18-14		MJC	
KH-1	11-20-14		SLP	
KH-2	12/2/14	PERMIT SET	RT	

DRAWING TITLE: -

SCALE: NTS

SHEET: -

Inspecti
Date: 02/1

Google earth

feet 1000

meters 400

HEAT LOSS CALC QN- 4350 (32307) "U" VALUES:

DATE 11/13/2014 CEILING: 0.026 R30=.040 R38=.026 R42=.024 R49=.020

BY PIF FLOOR: 0.053 R19=.053 R30=.040

STATE: ME WALL: 0.053

DELTA T: 85

WALL TYPE 2: 6 USE H6 FOR R21/C6 FOR R17

(ONLY 2x4 & 2x6)

Inspecti

Date: 02/1

DATA:	LR	DR	KIT	BATH#1	BATH#2	BR#1	BR#2	BR#3	LAUNDRY	HALL							TOTAL
FLOOR(1,2,3SINGLE)	1	1	1	2	1	1	2	2	1	2							
# OF EXT. WALL(S):	2	2	1	2	2	2	2	2	1	2							
LENGTH	20.5	13.0	12.0	7.0	8.0	13.0	13.5	10.0	8.0	11.0	0.0	0.0	0.0	0.0	0.0	0.0	27.0
WIDTH	13.50	13.50	13.50	13.50	6.50	13.50	13.50	13.50	7.00	13.50	0.00	0.00	0.00	0.00	0.00	0.00	27.00
CLG HGT	9.0	9.0	9.0	9.0	9.0	9.0	9.0	9.0	9.0	9.0	0.0	0.0	0.0	0.0	0.0	0.0	18.0
EXT WALL	34.00	26.50	12.00	20.50	14.50	26.50	27.00	23.50	8.00	24.50	0.00	0.00	0.00	0.00	0.00	0.00	107.50
.30 WIND	39.1	0.0	13.9	8.0	4.0	26.5	37.0	37.0	0.0	18.5	0.0	0.0	0.0	0.0	0.0	0.0	184.0
.32 WIND	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
.34 WIND	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
.36 WIND	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
.38 WIND	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
.42 WIND	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
.14 DOOR (SOLID)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
.25 DOOR (GLASS)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
.30 DOOR (GLASS)	0.0	32.7	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	32.7
.39 DOOR (GLASS)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
WALL LOSS	2200	1817	778	999	672	1631	1872	1730	324	1382	0	0	0	0	0	0	13323
CLG LOSS	612	388	358	209	115	388	403	298	124	328	0	0	0	0	0	0	1611
FLR LOSS	1247	791	730	426	234	791	821	608	252	669	0	0	0	0	0	0	3284
AIR INF	3811	2417	1487	1301	716	2417	2510	1859	514	2045	0	0	0	0	0	0	20077
WATT LOSS	2125	1471	877	735	475	1417	1401	1138	319	1100	0	0	0	0	0	0	11214
BTUH LOSS	7258	5025	2995	2509	1622	4839	4785	3887	1090	3755	0	0	0	0	0	0	38295
WATT PROV	2500	2000	1000	1000	1000	1500	1500	1500	500	1500	0	0	0	0	0	0	18500
BTUH PROV	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
REQ'D ELEC SIZE(IN	60	50	30	30	30	40	40	40	20	40	20	20	20	20	20	20	
REQ'D HWBB SIZE(FT	14.00	10.00	6.00	5.00	4.00	10.00	9.00	8.00	3.00	8.00	1.00	1.00	1.00	1.00	1.00	1.00	
ACTUAL ELEC INST'D	60	50	30	30	30	40	40	40	20	40							380.00
ACTUAL HWBB INST'D																	

NOTES:

PFS CORPORATION

Approval Limited to Factory Built Portion Only

State: 0DLQH

Signature: Renee Moise

Title: Staff Plan Reviewer

Date: 12/12/14

Linear Convector™ LC Series

Heat that fits

Revolutionary design provides a sleek, compact heater profile, while improving heater performance, reducing energy consumption, and improving comfort.

Features

- Faster heating of the room than a conventional baseboard due to rapid vertical laminar air flow, directing heat to the ceiling, speeding dispersal of warm air into the center of the room
- Improved performance and reduced length through use of top heat discharge and new fin design
- Superior shark-fin blade design on a steel tubular element for improved heat transfer and longer life
- Discreet styling, reduced length and added versatility of placement provide more options when designing a room
- May be used with wall or top mounted built-in thermostat (not included)
- Full length automatic overheat reset for safety

Applications

- All residential applications, commercial offices, lobbies, washrooms

Specifications

Voltage	120, 208, 240, 347V
Wattage	Sizes ranging from 500W to 2500W
Color	White or Almond
Finish	Specially-formulated epoxy / polyester powder coating is environmentally friendly and resists fading and abrasion.
Construction	Robust, 20 gauge steel construction.
Heating Element	A nickel chromium element is totally enclosed within a steel sheath, providing superior life expectancy and resistance to rust. Shark-fin shaped aluminum fins are firmly staked in an upright position to provide directional wicking for top discharge heat transfer.
Installation	Easily removed front caps, knock-outs on both sides of the convector, and pre-stamped mounting holes make installation easy.
Warranty	Ten year element warranty. One year warranty on complete unit.

C US

Dimplex®
Better solutions through innovation

LC Series

Ordering Guide

Cat. No.	Watts	Volts	BTU	Length mm/in.	Weight kg/lbs.
LC2005W11	500	120	1706	508/20	1.7/3.75
LC2005W21	500	208	1706	508/20	1.7/3.75
LC2005W31	500/375	240/208	1706/1280	508/20	1.7/3.75
LC2005W51	500	347	1706	508/20	1.7/3.75
LC2507W11	750	120	2559	635/25	2/4.40
LC2507W21	750	208	2559	635/25	2/4.40
LC2507W31	750/563	240/208	2559/1919	635/25	2/4.40
LC2507W51	750	347	2559	635/25	2/4.40
LC3010W11	1000	120	3412	762/30	2.3/5.00
LC3010W21	1000	208	3412	762/30	2.3/5.00
LC3010W31	1000/750	240/208	3412/2559	762/30	2.3/5.00
LC3010W51	1000	347	3412	762/30	2.3/5.00
LC3512W11	1250	120	4265	889/35	2.9/6.40
LC3512W21	1250	208	4265	889/35	2.9/6.40
LC3512W31	1250/938	240/208	4265/3199	889/35	2.9/6.40
LC3512W51	1250	347	4265	889/35	2.9/6.40
LC4015W11	1500	120	5118	1016/40	3.2/7.50
LC4015W21	1500	208	5118	1016/40	3.2/7.50
LC4015W31	1500/1125	240/208	5118/3839	1016/40	3.2/7.50
LC4015W51	1500	347	5118	1016/40	3.2/7.50
LC5020W21	2000	208	6824	1270/50	3.7/8.20
LC5020W31	2000/1500	240/208	6824/5120	1270/50	3.7/8.20
LC5020W51	2000	347	6824	1270/50	3.7/8.20
LC6025W21	2500	208	8530	1524/60	4.4/9.70
LC6025W31	2500/1875	240/208	8530/6398	1524/60	4.4/9.70
LC6025W51	2500	347	8530	1524/60	4.4/9.70

Note: 1) Standard color is white.
2) To order almond, omit "W" in Cat. No.

Control Options (field installed)

Thermostat Kits

Cat. No.	Description	Rating
DTK-SP	Single pole built-in thermostat kit (adjustable)	120-240V, 17A 347V, 14A
DTK-DP	Double pole built-in thermostat (adjustable)	120-240V, 17A 347V, 11A
DTKT-SP	Single pole built-in tamperproof thermostat kit	120-240V, 17A 347V, 11A
DTKT-DP	Double pole built-in tamperproof thermostat kit	120-240V, 25A 347V, 11A

Note: Each adjustable kit contains white and almond control knobs and hardware

Relays

Cat. No.	Description	Rating
BLLVC11	Low voltage relay & transformer kit	120V, 22A
BLLVC21	Low voltage relay & transformer kit	208V, 22A
BLLVC31	Low voltage relay & transformer kit	240V, 22A
BLLVC51	Low voltage relay & transformer kit	347V, 17A
BLLVD	Low voltage relay less transformer kit	120/208/240, 22A 347V, 17A

Dimensions

"A" – see Length in Ordering Guide

NOTE: ALL KNOCK OUTS ARE 7/8" FOR 1/2" CONDUIT

Dimplex North America Limited • 1367 Industrial Road Cambridge, ON Canada N1R 7G8
Telephone: 1-800-668-6663 Fax: 1-800-668-6665 • www.dimplexheating.com
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Printed in Canada 05/11 LI-933

Job	Truss	Truss Type	Qty	Ply	
76114	HMC43101	HINGE MONO	1	1	Excel Homes of Maine 212 U-1241 Designer:SM (PA 30040)

Universal Forest Products Inc., Grand Rapids, MI 49525, Steve Minahan — 7.520 e May 8 2014 MiTek Industries, Inc. Thu Dec 04 10:55:17 2014 Page 1

Inspecti Date: 02/1

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Plate Offsets (X,Y)-- [2:0-3-1,Edge], [3:0-1-8,0-1-8], [4:0-1-0,0-0-0], [5:0-0-4,0-1-0], [8:0-1-10,0-1-12]

LOADING (psf)		SPACING-	1-4-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	38.5	Plates Increase	1.15	TC	0.98	Vert(LL)	-0.11	8-9	>999	240	MT20	197/144
(Ground Snow=50.0)		Lumber Increase	1.15	BC	0.58	Vert(TL)	-0.29	8-9	>477	180	MT18HS	197/144
TCDL	10.0	Rep Stress Incr	YES	WB	0.91	Horz(TL)	0.03	8	n/a	n/a		
BCLL	0.0 *	Code IBC2009/TPI2007		(Matrix)							Weight: 37 lb	FT = 0%
BCDL	10.0											

LUMBER-
TOP CHORD 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x3 SPF Stud *Except*
W4: 2x4 SPF Stud

BRACING-
TOP CHORD Structural wood sheathing directly applied, except end verticals. [P]
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 3-8
JOINTS 1 Brace at Jt(s): 10

REACTIONS. (lb/size) 2=532/0-3-8, 8=437/0-3-8, 7=0/Mechanical
Max Horz 2=116(LC 14), 7=-116(LC 14)
Max Uplift2=-76(LC 7), 8=-59(LC 7)
Max Grav 2=573(LC 14), 8=516(LC 14)

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FORCES. (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/15, 2-3=-1560/196, 3-4=-340/43, 4-11=-321/44, 5-11=-269/54, 5-6=-134/33, 6-7=-120/35, 8-10=-283/87
BOT CHORD 2-9=-233/1379, 8-9=-233/1379
WEBS 3-9=0/196, 3-8=-1244/190, 5-10=-318/98

REQUIRED FIELD JOINT CONNECTIONS - Maximum Compression (lb)/ Maximum Tension (lb)/ Maximum Shear (lb)/ Maximum Moment (lb-in)
6=127/33/28/0, 10=318/98/145/0

NOTES-
1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-05; Pg=50.0 psf (ground snow); Ps=38.5 psf (roof snow); Category II; Exp B; Partially Exp.; Ct=1.1
3) Roof design snow load has been reduced to account for slope.
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 20.0 psf or 2.00 times flat roof load of 38.5 psf on overhangs non-concurrent with other live loads.
6) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
7) All plates are MT20 plates unless otherwise indicated.
8) See HINGE PLATE DETAILS for plate placement.
9) Provisions must be made to prevent lateral movement of hinged member(s) during transportation.
10) All additional member connections shall be provided by others for forces as indicated.
11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
12) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 76 lb uplift at joint 2 and 59 lb uplift at joint 8.
14) This truss has been designed in accordance with the 2009 IBC Section 2303.4.6, 2009 IRC Section 802.10.2.
15) Take precaution to keep the chords in plane, any bending or twisting of the hinge plate must be repaired before the building is put into service.
16) The field-installed members are an integral part of the truss design. Retain a design professional to specify final field connections and temporary supports. All field-installed members must be properly fastened prior to applying any loading to the truss. This design anticipates the final set position.

E-signed by Kevin Freeman

STATE OF MAINE
KEVIN W. FREEMAN
No. 9365
LICENSED
PROFESSIONAL ENGINEER
12/4/2014

The professional engineering seal indicates that a licensed professional has reviewed the design under the standards referenced within this document, not necessarily the current state building code. The engineering seal is not an approval to use in a specific state. The final determination on whether a truss design is acceptable under the locally adopted building code rest with the building official or designated appointee.

WARNING - ***Verify design parameters and READ NOTES***

Universal Forest Products, Inc. 2801 EAST BELTLINE RD, NE
PHONE (616)-364-6161 FAX (616)-365-0060 GRAND RAPIDS, MI 49525

Truss shall not be cut or modified without approval of the truss design engineer.
This component has only been designed for the loads noted on this drawing. Construction and lifting forces have not been considered. The builder is responsible for lifting methods and system design. Builder responsibilities are defined under TPI1. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult BCSI 1-06 from the Wood Truss Council of America and Truss Plate Institute Recommendation available from WTCA, 6300 Enterprise LN, Madison, WI 53719 J:\support\MitekSupp\templates\ufp.tpe

Job	Truss	Truss Type	Qty	Ply	
76114	P1461101	HINGED ATTIC	1	1	Excel Homes of Maine 212 U-1240 Designer:SM (PA 30040)

Universal Forest Products Inc., Grand Rapids, MI 49525, Steve Minahan — 7.520 e May 8 2014 MiTek Industries, Inc. Thu Dec 04 11:29:48 2014 Page 1 of 1

Inspecti Date: 02/1

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Plate Offsets (X,Y)-- [2:0-2-3,0-1-15], [3:0-1-8,0-3-12], [4:0-0-0,0-1-0], [10:0-0-0,0-1-0], [11:0-1-8,0-3-12], [12:0-2-3,0-1-15]

LOADING (psf)		**SPACING-**	1-4-0	**CSI.**		**DEFL.**	in	(loc)	l/defl	L/d	**PLATES**	**GRIP**
TCLL	29.6	Plates Increase	1.15	TC	0.74	Vert(LL)	-0.30	14-16	>806	240	MT20	197/144
(Ground Snow=50.0)		Lumber Increase	1.15	BC	0.57	Vert(TL)	-0.55	14-16	>445	180	MT18HS	197/144
TCDL	10.0	Rep Stress Incr	YES	WB	0.22	Horz(TL)	0.01	12	n/a	n/a		
BCLL	0.0 *	Code IBC2009/TPI2007		(Matrix)		Attic	-0.13	14-16	1074	360	Weight: 108 lb	FT = 0%
BCDL	10.0											

LUMBER-
TOP CHORD 2x6 SPF No.2 *Except*
T1: 2x4 SPF No.2
BOT CHORD 2x10 SPF No.2
WEBS 2x3 SPF Stud *Except*
W2: 2x6 SPF No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. [P]
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=759/0-3-8, 12=759/0-3-8
Max Horz 2=200(LC 8)
Max Uplift2=-56(LC 9), 12=-56(LC 10)
Max Grav 2=830(LC 2), 12=830(LC 2)

FORCES. (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-806/44, 3-4=-675/47, 4-5=-528/84, 5-6=-380/112, 6-7=-41/149, 7-8=-38/142, 8-9=-380/112, 9-10=-527/84, 10-11=-678/48, 11-12=-806/45, 12-13=0/51
BOT CHORD 2-16=-7/429, 15-16=-4/432, 14-15=-4/432, 12-14=-3/429
WEBS 10-14=-138/342, 4-16=-138/343, 6-8=-636/217

REQUIRED FIELD JOINT CONNECTIONS - Maximum Compression (lb)/ Maximum Tension (lb)/ Maximum Shear (lb)/ Maximum Moment (lb-in)
5=418/102/205/0, 6=639/218/19/0, 7=31/157/153/0, 8=636/217/11/0, 9=415/102/205/0, 14=138/342/0/0, 16=138/343/0/0

NOTES-
1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-05; Pg=50.0 psf (ground snow); Ps=29.6 psf (roof snow); Category II; Exp B; Partially Exp.; Ct=1.1
3) Roof design snow load has been reduced to account for slope.
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 38.5 psf on overhangs non-concurrent with other live loads.
6) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
7) All plates are MT20 plates unless otherwise indicated.
8) See HINGE PLATE DETAILS for plate placement.
9) Provisions must be made to prevent lateral movement of hinged member(s) during transportation.
10) All additional member connections shall be provided by others for forces as indicated.
11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
12) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
13) Ceiling dead load (5.0 psf) on member(s). 4-6, 8-10, 6-8
14) Bottom chord live load (30.0 psf) and additional bottom chord dead load (0.0 psf) applied only to room. 14-16
15) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 56 lb uplift at joint 2 and 56 lb uplift at joint 12.
16) This truss has been designed in accordance with the 2009 IBC Section 2303.4.6, 2009 IRC Section 802.10.2.
17) Attic room checked for L/360 deflection.
18) Take precaution to keep the chords in plane, any bending or twisting of the hinge plate must be repaired before the building is put into service.
19) The field-installed members are an integral part of the truss design. Retain a design professional to specify final field connections and temporary supports. All field-installed members must be properly fastened prior to applying any loading to the truss. This design anticipates the final set position.
20) Temporary supports are required to maintain the bottom chord in a level position during storage, transportation, and setup. Retain a design professional to specify all temporary bracing to support the truss until setup is complete. Temporary support(s) must not be removed until all field connections are completed.
21) The bottom chord must be laterally braced during shipment and setup to prevent damage to the splice plate.

The professional engineering seal indicates that a licensed professional has reviewed the design under the standards referenced within this document, not necessarily the current state building code. The engineering seal is not an approval to use in a specific state. The final determination on whether a truss design is acceptable under the locally adopted building code rest with the building official or designated appointee.

E-signed by Kevin Freeman

12/4/2014

WARNING - ***Verify design parameters and READ NOTES***

Universal Forest Products, Inc. 2801 EAST BELTLINE RD, NE
PHONE (616)-364-6161 FAX (616)-365-0060 GRAND RAPIDS, MI 49525

Truss shall not be cut or modified without approval of the truss design engineer.
This component has only been designed for the loads noted on this drawing. Construction and lifting forces have not been considered. The builder is responsible for lifting methods and system design. Builder responsibilities are defined under TPI1. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult BCSI 1-06 from the Wood Truss Council of America and Truss Plate Institute Recommendation available from WTCA, 6300 Enterprise LN, Madison, WI 53719 J:\support\MitekSupp\templates\ufp.tpe

PFS APPROVED
DATE 12/12/14
PFS CORPORATION
Bloomsburg, PA

JOB	4350 (32307) SHED GIRDER		
SHEET NO.		OF	
CALCULATED BY	RT	DATE	12/2/2014
		REVISED	

Inspecti
Date: 02/1

Beam & Header Spans - Non Composite

Beam1 =	1.5" x 11.25"	Species1 =	LVL	Grade1 =	NA	NA	NA
Beam2 =	None	Species2 =	LVL	Grade2 =	NA	NA	NA

Quantity Per Box, Q1 =	2	d1=	11.25	in	b1=	3.00	in
Quantity Per Box, Q2 =	0	d2=	0.00	in	b2=	0.00	in

Enter Beam w/ Largest Quantity in Q1

Area, A1 =	33.75	in^2	Section Modulus, S1 =	63.28	in^3	Moment of Inertia, I1 =	355.96 in^4
Area, A2 =	0.00	in^2	Section Modulus, S2 =	0.00	in^3	Moment of Inertia, I2 =	0.00 in^4

Load Duration		Beam Stability		Size Factor		Repetitve Factor		Deflection Criteria	
C_D =	1.15	C_L =	1.00	C_{F1} =	1.00	C_{R1} =	1.00	delta LL =	240
				C_{F2} =	1.00	C_{R2} =	1.00	delta TL =	180

Shear			Moment			Deflection			Load Share		
F_{v1} =	285	psi	F_{b1} =	2750	psi	E1 =	2000000	psi	Beam1 =	100.00	%
F_{v2} =	0	psi	F_{b2} =	0	psi	E2 =	0	psi	Beam2 =	0.00	%

Controlling Load w = 666.5 plf

Max Allowable Length =	169.8	in	Actual Length Needed =	153.0	in
Reaction at Each End =	4248.9	lb	Minimum Bearing Length Required =	3.3	in

Species & Grade	Number of Matewall Column Studs Per Box					
	Max 8ft Tall Column			Max 9ft Tall Column		
	2x3	2x4	2x6	2x3	2x4	2x6
spf #2	NG	2	2	NG	3	2
spf #3	NG	3	2	NG	3	2
spf stud	NG	3	2	NG	3	2
sp#2	4	2	2	NG	2	2

BEAM FASTENING REQUIREMENTS

CONDITION 1 - TOP LOADED - 2 TO 4 PLY BEAMS

12" deep or less - (2) rows of 0.131"x3" nails @ 8" o.c.
> 12" & < 18" deep - (3) rows of 0.131"x3" nails @ 8" o.c.
> 18" deep - (4) rows of 0.131"x3" nails @ 8" o.c.

CONDITION 2 - SIDE LOADED

2-Ply <= 465plf (2) rows of 0.131"x3" nails @ 8" o.c.
2-Ply <= 700plf (3) rows of 0.131"x3" nails @ 8" o.c.
2-Ply <= 870plf (2) rows of 1/2" Bolts @ 12" o.c.
2-Ply > 870plf (3) rows of 1/2" Bolts @ 12" o.c.

3-Ply <= 350plf (2) rows of 0.131"x3" nails @ 8" o.c.
3-Ply <= 525plf (3) rows of 0.131"x3" nails @ 8" o.c.
3-Ply <= 650plf (2) rows of 1/2" Bolts @ 12" o.c.
3-Ply > 650plf (3) rows of 1/2" Bolts @ 12" o.c.

4-Ply (3) rows of 1/2" Bolts @ 12" o.c.

Note: Stagger fastener rows and locate fasteners minimum 2" from all ends and edges. Space rows equally apart vertically. Fastener spacing is per row.

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JOB	4350 (32307) COLONIAL CLG EXTERIOR GIRDER		
SHEET NO.		OF	
CALCULATED BY	RT	DATE	12/2/2014
		REVISED	

Date: 02/1

Load Calculation Procedure

INPUTS

Location:	SIDEWALL
Supporting Roof:	YES
Number of Floor Supporting:	1
Cape Roof:	YES
Simply Supported Rafter:	NO

CONSTANTS

Floor Live Load =	40	psf
Floor Dead Load =	10	psf
Wall Dead Load =	62	plf
Ceiling Dead Load =	6	psf

Reaction from Truss or Rafter =	830	lb
Truss spacing =	16	in o.c.
Unit Width =	0	ft

Required Deflection Criteria

Live Load =	L / 360
Total Load =	L / 240

Roof Live Load / Unbalanced Snow Load =	50	psf
Roof Dead Load =	20	psf
Roof/Snow Load =	622.5	plf
% Roof Live Load =	50.0	%
% Roof Dead Load =	20.0	%
% Attic Live Load =	30.0	%

Attic Live Load = 30 psf

Load Cases

					Load Duration for Wood Members
1.	D + F	=	186.5	plf	Cd = 0.9
2.	D + H + F + L + T	=	373.3	plf	Cd = 1.0
3.	D + H + F + (L_r or S or R)	=	497.8	plf	Cd = 1.15
4.	D + H + F + 0.75(L + T) + 0.75(L_r or S or R)	=	606.7	plf	Cd = 1.15
5.	D + H + F + (W or 0.7E)	=	186.5	plf	Cd = 0.9
6.	D + H + F + 0.75(W or 0.7E) + 0.75L + 0.75(L_r or S or R)	=	606.7	plf	Cd = 1.15
7.	0.6D + W + H	=	111.9	plf	Cd = 0.9
8.	0.6D + 0.7E + H	=	111.9	plf	Cd = 0.9

Check the highest load for each load duration factor when sizing wood members.
Check the highest load and apply no load duration factor when sizing steel members.

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JOB	4350 (32307) CLG GIRDER		
SHEET NO.		OF	
CALCULATED BY	RT	DATE	12/2/2014
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Date: 02/1

Beam & Header Spans - Non Composite

Beam1 =	1.5" x 11.25"	Species1 =	LVL	Grade1 =	NA	NA	NA
Beam2 =	1.5" x 9.25"	Species2 =	LVL	Grade2 =	NA	NA	NA

Quantity Per Box, Q1 =	2	d1=	11.25	in	b1=	3.00	in
Quantity Per Box, Q2 =	2	d2=	9.25	in	b2=	3.00	in

Enter Beam w/ Largest Quantity in Q1

Area, A1 =	33.75	in^2	Section Modulus, S1 =	63.28	in^3	Moment of Inertia, I1 =	355.96	in^4
Area, A2 =	27.75	in^2	Section Modulus, S2 =	42.78	in^3	Moment of Inertia, I2 =	197.86	in^4

Load Duration		Beam Stability		Size Factor		Repetitve Factor		Deflection Criteria	
C_D =	1.00	C_L =	1.00	C_{F1} =	1.00	C_{R1} =	1.00	delta LL =	360
				C_{F2} =	1.04	C_{R2} =	1.00	delta TL =	240

Shear			Moment			Deflection			Load Share		
F_{v1} =	285	psi	F_{b1} =	2750	psi	E1 =	2000000	psi	Beam1 =	64.27	%
F_{v2} =	285	psi	F_{b2} =	2750	psi	E2 =	2000000	psi	Beam2 =	35.73	%

Controlling Load w = 377.5 plf

Max Allowable Length =	218.9	in	Actual Length Needed =	211.9	in
Reaction at Each End =	3332.6	lb	Minimum Bearing Length Required =	2.6	in

Species & Grade	Number of Matewall Column Studs Per Box					
	Max 8ft Tall Column			Max 9ft Tall Column		
	2x3	2x4	2x6	2x3	2x4	2x6
spf #2	4	2	2	NG	2	2
spf #3	NG	2	2	NG	3	2
spf stud	NG	2	2	NG	3	2
sp#2	4	2	1	4	2	2

BEAM FASTENING REQUIREMENTS

CONDITION 1 - TOP LOADED - 2 TO 4 PLY BEAMS

12" deep or less - (2) rows of 0.131"x3" nails @ 8" o.c.
> 12" & < 18" deep - (3) rows of 0.131"x3" nails @ 8" o.c.
> 18" deep - (4) rows of 0.131"x3" nails @ 8" o.c.

CONDITION 2 - SIDE LOADED

2-Ply <= 465plf (2) rows of 0.131"x3" nails @ 8" o.c.
2-Ply <= 700plf (3) rows of 0.131"x3" nails @ 8" o.c.
2-Ply <= 870plf (2) rows of 1/2" Bolts @ 12" o.c.
2-Ply > 870plf (3) rows of 1/2" Bolts @ 12" o.c.

3-Ply <= 350plf (2) rows of 0.131"x3" nails @ 8" o.c.
3-Ply <= 525plf (3) rows of 0.131"x3" nails @ 8" o.c.
3-Ply <= 650plf (2) rows of 1/2" Bolts @ 12" o.c.
3-Ply > 650plf (3) rows of 1/2" Bolts @ 12" o.c.

4-Ply (3) rows of 1/2" Bolts @ 12" o.c.

Note: Stagger fastener rows and locate fasteners minimum 2" from all ends and edges. Space rows equally apart vertically. Fastener spacing is per row.

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DATE 12/12/14
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Bloomsburg, PA

JOB	4350 (32307) 1st FLR HEADER		
SHEET NO.		OF	
CALCULATED BY	RT	DATE	12/2/2014
		REVISED	

Inspecti
Date: 02/1

Beam & Header Spans - Non Composite

						SPF	
Beam1 =	2x10	Species1 =	SPF	Grade1 =	NA	No.1/No.2	NA
Beam2 =	None	Species2 =	LVL	Grade2 =	NA	NA	NA

Quantity Per Box, Q1 =	2	d1=	9.25	in	b1=	3.00	in
Quantity Per Box, Q2 =	0	d2=	0.00	in	b2=	0.00	in

Enter Beam w/ Largest Quantity in Q1

Area, A1 =	27.75	in^2	Section Modulus, S1 =	42.78	in^3	Moment of Inertia, I1 =	197.86 in^4
Area, A2 =	0.00	in^2	Section Modulus, S2 =	0.00	in^3	Moment of Inertia, I2 =	0.00 in^4

Load Duration		Beam Stability		Size Factor		Repetitve Factor		Deflection Criteria	
C_D =	1.00	C_L =	1.00	C_{F1} =	1.10	C_{R1} =	1.00	delta LL =	360
				C_{F2} =	1.00	C_{R2} =	1.00	delta TL =	240

Shear			Moment			Deflection			Load Share		
F_{v1} =	135	psi	F_{b1} =	875	psi	E1 =	1400000	psi	Beam1 =	100.00	%
F_{v2} =	0	psi	F_{b2} =	0	psi	E2 =	0	psi	Beam2 =	0.00	%

Controlling Load w = 399.5 plf

Max Allowable Length =	99.5	in	Actual Length Needed =	60.5	in
Reaction at Each End =	1007.1	lb	Minimum Bearing Length Required =	0.8	in

Species & Grade	Number of Jack Studs Per Box					
	2-ply Header			3-ply Header		
	2x3	2x4	2x6	2x3	2x4	2x6
spf #2	1	1	1	1	1	1
spf #3	1	1	1	1	1	1
spf stud	1	1	1	1	1	1
sp#2	1	1	1	1	1	1

BEAM FASTENING REQUIREMENTS

CONDITION 1 - TOP LOADED - 2 TO 4 PLY BEAMS

12" deep or less - (2) rows of 0.131"x3" nails @ 8" o.c.
> 12" & < 18" deep - (3) rows of 0.131"x3" nails @ 8" o.c.
> 18" deep - (4) rows of 0.131"x3" nails @ 8" o.c.

CONDITION 2 - SIDE LOADED

2-Ply <= 465plf (2) rows of 0.131"x3" nails @ 8" o.c.
2-Ply <= 700plf (3) rows of 0.131"x3" nails @ 8" o.c.
2-Ply <= 870plf (2) rows of 1/2" Bolts @ 12" o.c.
2-Ply > 870plf (3) rows of 1/2" Bolts @ 12" o.c.

3-Ply <= 350plf (2) rows of 0.131"x3" nails @ 8" o.c.
3-Ply <= 525plf (3) rows of 0.131"x3" nails @ 8" o.c.
3-Ply <= 650plf (2) rows of 1/2" Bolts @ 12" o.c.
3-Ply > 650plf (3) rows of 1/2" Bolts @ 12" o.c.

4-Ply (3) rows of 1/2" Bolts @ 12" o.c.

Note: Stagger fastener rows and locate fasteners minimum 2" from all ends and edges. Space rows equally apart vertically. Fastener spacing is per row.