THIS HOME HAS BEEN DESIGNED SPECIFICALLY FOR:

19 MERRIAM STREET (ON PEAKS ISLAND) CITY OF PORTLAND, ME 04108 **CUMBERLAND COUNTY**

BUILDER:

HALLMARK HOMES



Title: Date: **Staff Plan Reviewer** 12/12/14

Digitally signed by Renee Moist o=PFS Corporation, ou=NorthEast Region, oration.Com, c=US Date: 2014.12.12

Renee Moist,

11:24:33 -05'00'

+/- 29'-10" TOP OF SILL TO PEAK THERE ARE NO LOT LINE FIRE SEPARATION REQUIREMENTS 8'-0" 2ND FLOOR CEILING HEIGHT 2ND FLOOR FLOOR 8'-0" 1ST FLOOR CEILING HEIGHT ROOF, COLUMNS, STEPS, RAILING, PORCH FLR. AND HEADER TO BE SUPPLIED AND INSTALLED BY BUILDER OUTLINE OF FOUNDATION BELOW GRADE 32'-9%"

FRONT "WEST" ELEVATION

SITE CONDITIONS:

GROUND SNOW LOAD: WIND SPEED: <100 MPH EXPOSURE: SEISMIC CATEGORY: USE GROUP: SINGLE FAMILY CONSTRUCTION TYPE: VB WOOD FRAME UNPROTECTED

SQUARE FOOTAGE:

FIRST FLOOR: 887 SQ. FT. SECOND FLOOR: 559 SQ. FT. BONUS ROOM: SQ. FT. GARAGE: SQ. FT. TOTAL: 1,446 SQ. FT.

27'-0 3/4" x 20'-8"/32'-9 3/8" OVERALL SIZE MODEL: CUSTOM COLONIAL

DRAWING TITLE:

COVER SHEET

SHEET: 1/8" = 1'-0" P1

NOTES:

■ EXCEL HOMES OF ME.

OXFORD, ME 04270

PHONE:

FAX:

56 MECHANIC FALLS ROAD

(888) 333-1748

(207) 539-0944

1. ITEMS SHOWN ON THE EXTERIOR ELEVATION DRAWINGS ARE FOR ILLUSTRATIVE PURPOSES ONLY 2. GRILLS SHOWN ARE FOR ILLUSTRATIVE PURPOSES ONLY (SEE WINDOW MANUFACTURER CATALOG FOR ACTUAL GRILL PATTERN) HALLMARK HOMES

CUSTOMER/PROJECT:

BUILDER:

KIM 4350 □ PD ■QN □ SN

CARTER (32307) KEISER HOMES

BRAND

BUILT BY EXCEL HOMES OF MAINE

THIS DRAWING WAS EXTRACTED FROM APPROVED PLANS AND/OR APPROVED SYSTEMS DRAWINGS

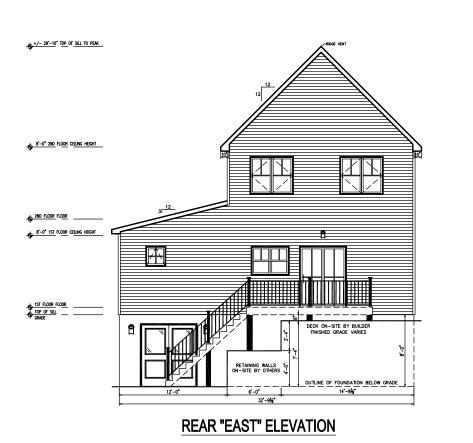
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ADJUSTMENTS MADE FOR CODE COMPLIANCE AND PRODUCTION CAPABILITY

DRAWING MAY BE REVERSED

WYYD YYYD PIF MJC SLP

SCALE:





Approval Limited to Factory Built Portion Only

State:

Signature:

PFS Renee Moiss

Staff Plan Reviewer Title: Date:

12/12/14

PTL#:

KIM 4350 □PD ■QN □SN

BUILDER:

HALLMARK HOMES

CUSTOMER/PROJECT: **CARTER (32307)**

KEISER HOMES BRAND

BUILT BY EXCEL HOMES OF MAINE

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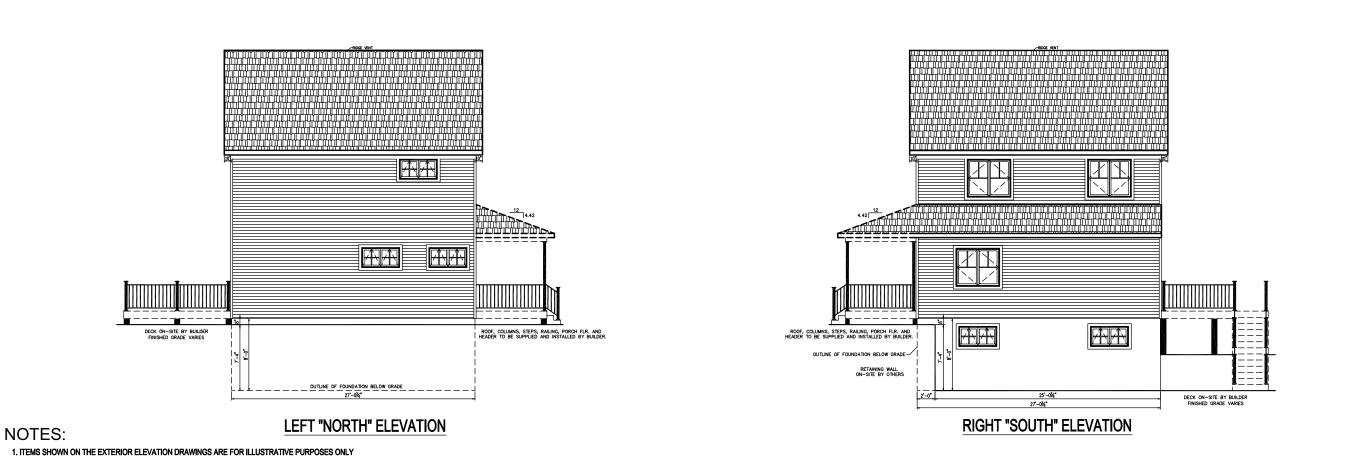
MUC WIC

NO: ON2 ON3 CH-1 KH-2

DRAWING TITLE:

ELEVATIONS

SCALE: SHEET: 3/32" = 1'-0" P1.1



PFS **MAINE** 2009 INTERNATIONAL RESIDENTIAL CODE w/EXCEPTIONS
2011 NFPA 31, STD FOR THE INSTALLATION OF OIL BURNING EQUIP
2009 NFPA 54, NATIONAL FUEL GAS CODE
2011 NFPA 70, NATIONAL ELECTRIC CODE w/EXCEPTIONS
2010 NFPA 211 STANDARDS FOR CHIMNEYS, FIREPLACES, VENTS AND
SOLID FUEL BURNING APPLIANCES
2009 UNIFORM PLUMBING CODE w/EXCEPTIONS
2011 STATE OF MAINE OIL AND SOLID FUEL BOARD LAW AND RULES
2011 NATIONAL ELECTRIC CODE w/EXCEPTIONS **PFS CORPORATION Approval Limited to Factory Built Portion Only** HOLD BACK CEILING DRYWALL 21" EACH SIDE OF MATE WALL OPENINGS INSTALL 16" OF SOLID BLOCKING WHERE State: CEILING DRYWALL IS HELD BACK Renee Moiss Signature: _1'-9"__1'-9"_ 47'-03/4" Title: **Staff Plan Reviewer** ALL PLUMBING VENT TERMINATIONS ARE 24"
ABOVE THE ROOF. WATER CLOSET VENTS & ALL
WET VENTS ARE MIN. 2" PER THE 2009 UNIFORM 27'-03/4" 8'-0" 12'-0" 12/12/14 Date: 13'-6" UNIT "A" 13'-6" UNIT "B" PLUMBING CODE WITH MAINE EXCEPTIONS. 3'-0" 7'-6" 3'-0" * IN ATTIC, RADON VENT TO RUN TOWARDS CENTER OF HOUSE TO GET THE PROPER CLEARANCE. SAFFTY NOTE: SHINGLES TO BE ATTACHED PER THE MANUFACTURES INSTALLATION STAIR GEOM'Y (8 1/4" MAX R. 9" MIN TREAD) INSTRUCTIONS FOR COASTAL REGIONS R F 36" HIGH SLOPED WALL 2-2x6 EACH U 2-2x4 SPF#2 SLOPED CEILING STORAGE UNDER STAIRS **DINING ROOM** ON-SITE BY BUILDER FOR ON-SITE DOOR BY BUILDER ABOVE LIVING ROOM 3. L CLG LINE **DECK** STATE LOC'N ABEL 5'-6" 511.67 SQ.FT. of ROOM HELD 40.9 LIGHT REQ'D. SUPPLIED SO VENT REQ'D. 61.6 LIGHT PROV'D. SUBFRICIENT LT & VT PROVIDED. A DOOR SWEEP & WEATHER STRIPPING AT BASEMENT DOOR. **FOYER** 36" 4-LITE THERMA TRU S1209 GBGF KITCHEN ADDITIONAL MAINE REQUIREMENTS SB * SET MANUAL MUST BE IN RESIDENCE. * COPY OF APPROVED SETS MUST BE IN RESIDENCE. 24" 24" RAISE 3/4" **FRONT** WINDOWS REQUIRE ARGON UPGRADE TO MEET MAINE TRIM CASING <u>4'-</u>10½"CLO PORCH ON-SITE BY BUILDER -2-2x4 SPF#2 EACH UNIT ADD'L LOLLY COL. REQ'D FOR WINDOW -4-2x6 SPF#2 FOR THE STATE OF MAINE, FOR BASEMENTS HEIGHTS FROM 7'-3" TO 8'-0" -EXPANDING INSUL'N AROUND H20 LINE 4-2x6 SPF#2 EACH UNIT 4-2x4 SPF#2-EACH UNIT ADD'L LOLLY COL. REQ'D 8'-9½" ROUGH PLUMB FOR TEPF1-84-ON-SITE SHOWER FRAME ONLY FOR RANGE SHALL BE EQUIPPED WITH A SEPARATE FAN/HOOD WITH A MIN. 3'-101/2" ON-SITE MED - CAB BY BUILDER 4'-0" 3'-0" WR3612 BEDROOM #1 RANGE & BATH FANS TO BE VENTED TO THE EXTERIOR 16.9 LIGHT PROV'D. 15.0 VENT PROV'D. SUFFICIENT LT & VT PROVIDED. HALL : I PAN DIAMETER CLEARANCE FOR COMPLETION. 22" x 30" STACK W/D CLO 4'-0¾" 12'-8" [∠]15"x24" [∠]DBX1000-6 ∠2" FUT VENT HORIZONTAL w/CTSB (2) 6'-9" 6'-9 PARADIGM WINDOW SCHEDULE 27'-034" - DOUBLE TRUSSES @ 16" O.C. FOR SNOW DRIFT LOADING ROUGH OPENING WINDOW CALL SIZE UNIT 512" × 23 1/2" 24 1/2" × 24 1/2" AWNING 2.17 1.52 4.0 .22 47" × 23 1/2" 48" × 24 1/2" AWNING 4.35 3.05 8.0 .22 47" × 35 1/2" 48" × 24 1/2" AWNING 4.35 3.05 8.0 .22 47" × 35 1/2" 48" × 36 1/2" HYBRID SH 7.51 3.88 12.0 .22 55" × 59 1/2" 56" × 60 1/2" HYBRID SH 16.65 8.59 23.34 .22 A2424 A2424-2

PTL#

ODLQH

KIM 4350 □ PD ■ QN □ SN

BUII DER:

HALLMARK HOMES

CUSTOMER/PROJECT **CARTER (32307)**

KEISER HOMES

02/12/15

BUILT BY EXCEL HOMES OF MAINE

MAINE ENERGY SPECIFICATION TABLES

1	MINIMUM INSUL. R-VALUES			MAXIMUM U-FAC	TORS
	CEILINGS	R-38		ENTRANCE DOORS	.35
	ROOF/CEILINGS	R-38		SPECIALTY DOORS	.45
1	WALLS	R-19		WINDOWS	.35
	FLOORS	R-19		SKYLIGHTS	.60
- 1		•	•	•	

HOUSE TO BE BUILT OVER UNCONDITIONED SPACE. BUILDER IS RESPONSIBLE TO PROVIDE & INSTALL FLOOR INSULATION ON THE 1ST FLOOR PER THE MAINE ENERGY CODE. THE BUILDER ALSO REQUIRED TO PROVIDE & INSTALL EXCEL TO PROVIDE & INSTALL R-11 INSULATION IN WALLS AND R-19 INSULATION IN THE CEILING OF ANY BASEMENT STAIR ENCLOSURES.

BASEMENT STAIRS ARE A COMPONENT OF THIS DESIGN WITH A MAXIMUM RISER HEIGHT OF 8-1/4": A MINIMUM TREAD DEPTH OF 9" AND A 1" NOSING WILL BE PROVIDED ON ALL TREADS WITH TREAD WIDTH LESS THAN 10"

RATING OF 100 CFM. EACH BATHROOM WILL BE EQUIPPED WITH A SEPARATE VENTILATING FAN THAT HAS A MIN. RATING OF 50 CFM AND BE RATED FOR SOUND AT A MAX. SOUND RATING OF 3 SONE. ALL

PER MAINE RADON REQUIREMENTS, THE RADON PIPE SHALL BE A MIN. 12" ABOVE THE ATTIC SPACE AND HAVE A 36" HIGH BY 24"

NOTE: SAFETY GLAZING TO BE PROVIDED FOR WINDOWS IN HAZARDOUS LOCATIONS

STANDARD -EXTERIOR (INSWING) DOOR SCHEDULE
 DOOR CALL SIZE
 WDTH
 HEIGHT
 ROUGH OPENING
 MATERIAL
 MANUFACTURER
 TYPE
 U-VALUE

 3068 4—LITE
 3'-0"
 6'-8"
 38 3/4" x 82 3/4"
 INSUL. CORE
 THERMA—TRU
 EXT HINGED
 .16

 PAR 5068 SGD
 5'-0"
 6'-8"
 60 1/2" x 80"
 INSUL. CORE
 PARADIGM
 EXT-SLIDER
 .30

NOTE: WINDOWS ARE NFRC RATED * MEETS EGRESS REQUIREMENTS

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MJC SLP RT

DATE 10-21-1 11-12-1 11-18-1 11-20-1 12/2/14 ON 2 ON 2 ON 3 ON 3 ON 4-1 OF 1-1

DRAWING TITLE:

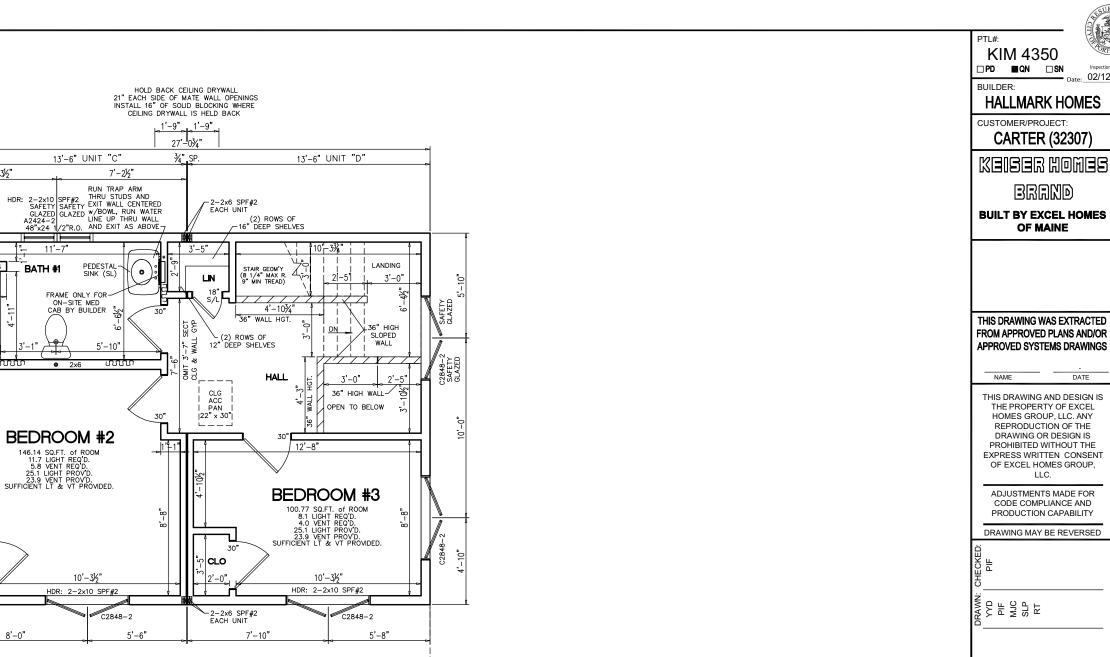
FIRST FLOOR PLAN

SHEET:

SCALE: 3/16" = 1'-0"

NOTES:

1. 2x6 EXT WALLS @ 16" O.C./2x4 MARR WALLS @ 16" O.C. (EXCEPT AS NOTED)
2. 8'-0" CLG HT.
3. 2x10 SPF#2 FLOOR JOISTS @ 16" O.C.
4. ROOF SYSTEM TO BE 16" O.C.
5. PARADIGM HYBRID SINGLE HUNG, CASEMENT & AWNING WINDOWS
6. BASED ON <100 MPH WIND LOAD & EXPOSURE "B"
7. SITE LOCATION: PEAKS ISLAND, ME; CUBMERLAND COUNTY; 50 PSF GROUND SNOW LOAD
8. CLG GIRDER OVER LIVING/DINING TO BE: 4-1 1/2"x11 1/4" M.L. (2-PER MODULE) -- FASTEN PLIES TOGETHER USING (2) ROWS OF 0.131"x3" NAILS @ 8"O.C.
9. CLG BEAM FOR SINGLE STORY ROOF TO BE: 2-1 1/2"x11 1/4" M.L. -- FASTEN PLIES TOGETHER USING (3) ROWS OF 0.131"x3" NAILS @ 8"O.C.
10. CLG BEAM UNDER 2nd FLR EXTERIOR WALL TO BE: 2-1 1/2"x11 1/4" M.L. -- FASTEN PLIES TOGETHER USING (2) ROWS OF 0.131"x3" NAILS @ 8"O.C.



FRONT

IOTES:
1. 2x6 EXT WALLS @ 16" O.C./2x4 MARR WALLS @ 16" O.C. (EXCEPT AS NOTED)
2. 8'-0" CLG HT.
3. 2x10 SPF#2 FLOOR JOISTS @ 16" O.C.
4. ROOF SYSTEM TO BE 16" O.C.
5. PARADIGM HYBRID SINGLE HUNG WINDOWS
6. FLR GIRDER TO BE: 4-1 1/2"x9 1/4"x 20'-8" M.L. (2-PER MODULE) -- FASTEN PLIES TOGETHER USING (2) ROWS OF 0.131"x3" NAILS @ 8"O.C.

6'-3½"

C2848-2 'x48 1/2"R.

်ကု CLO

BATH #1

PFS CORPORATION

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State:

Signature: Title:

Staff Plan Reviewer

12/12/14 Date:

ODLQH Renee Moiss

SECOND FLOOR PLAN

DATE: 10-21-14 11-12-14 11-20-14 12/2/14

NO: QN1 QN2 QN3 KH-1

DRAWING TITLE:

SCALE:

SHEET: 3/16" = 1'-0" P2.1

LLC.

02/12/15



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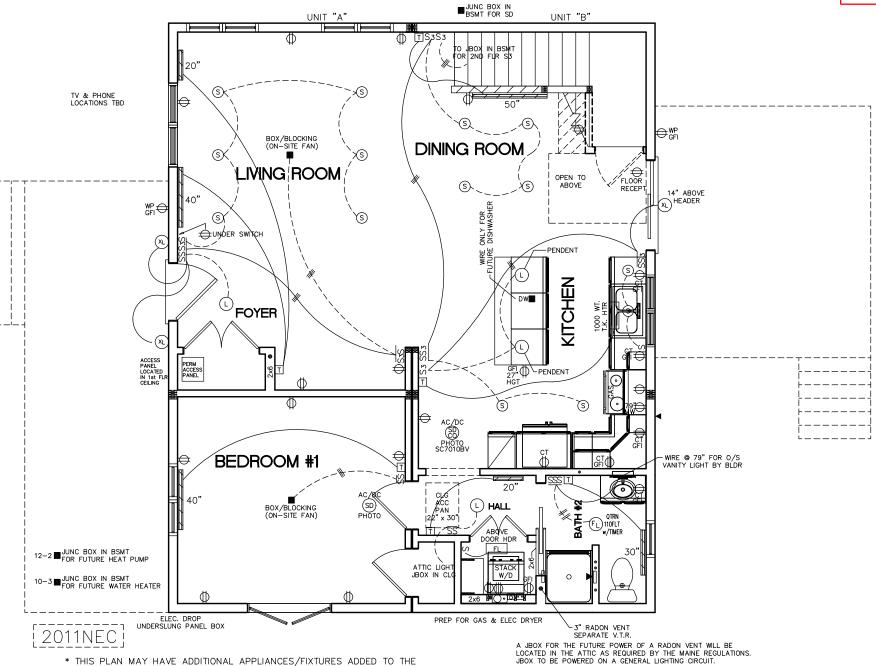
State:

Signature:

PFS Renee Moiss

Title: Date: **Staff Plan Reviewer**

12/12/14



* THIS PLAN MAY HAVE ADDITIONAL APPLIANCES/FIXTURES ADDED TO THE ELECTRICAL SCHEMATIC AND/OR PANEL BOX PROVIDING THE LOADING DOESN'T EXCEED THE MAXIMUM ALLOWED BY STATE & LOCAL CODES.

- * E-CUTOFF SWITCH ON-SITE BY OTHERS PER ALL STATE & LOCAL CODES.

 * ALL BRANCH CIRCUITS SUPPLYING 15 & 20 AMPERE OUTLETS IN LIVING SPACES ARE PROTECTED BY AN ARC-FAULT CIRCUIT INTERRUPTER IN ACCORDANCE WITH SECTION 210.12.2011 NEC.

 * PER 406.12 OF 2011 NEC ALL 125 -VOLT, 15 AND 20 AMP RECEPTS
- INSTALLED IN AREAS SPECIFIED BY 210.52, SHALL BE LISTED TAMPER RESISTANT TYPE.
- * BUILDER IS RESPOSIBLE FOR INSTALLING GAS LINES
- * 50# LIGHT BOXES REQUIRED

KIM 4350 □PD ■QN □SN

BUILDER:

HALLMARK HOMES

CUSTOMER/PROJECT:

CARTER (32307) KEISER HOMES

BUILT BY EXCEL HOMES OF MAINE

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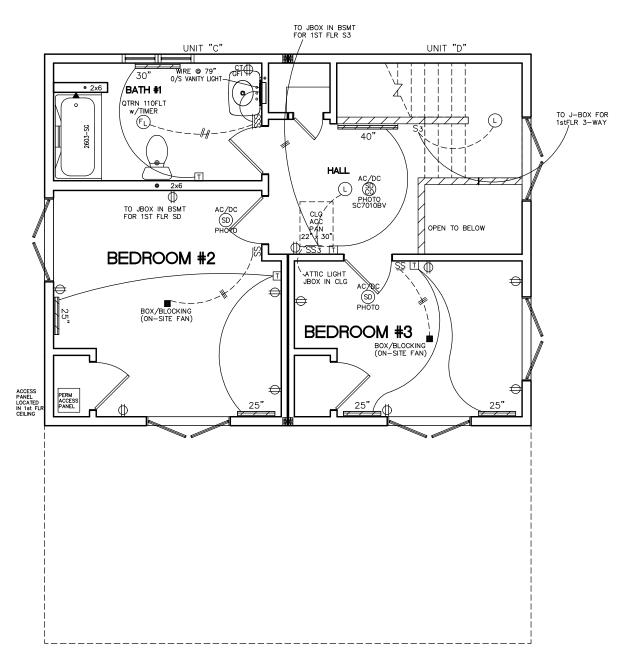
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AMP YYD MJC SLP SLP

DRAWING TITLE:

FIRST FLOOR **ELECTRICAL PLAN**

SCALE: NTS SHEET: P6



2011NEC

- * THIS PLAN MAY HAVE ADDITIONAL APPLIANCES/FIXTURES ADDED TO THE ELECTRICAL SCHEMATIC AND/OR PANEL BOX PROVIDING THE LOADING
- DOESN'T EXCEED THE MAXIMUM ALLOWED BY STATE & LOCAL CODES.
- * E-CUTOFF SWITCH ON-SITE BY OTHERS PER ALL STATE & LOCAL CODES.

 * ALL BRANCH CIRCUITS SUPPLYING 15 & 20 AMPERE OUTLETS IN LIVING SPACES ARE PROTECTED BY AN ARC-FAULT CIRCUIT INTERRUPTER IN ACCORDANCE
- WITH SECTION 210.12.2011 NEC.

 * PER 406.12 OF 2011 NEC ALL 125 -VOLT, 15 AND 20 AMP RECEPTS INSTALLED IN AREAS SPECIFIED BY 210.52, SHALL BE LISTED TAMPER RESISTANT TYPE.
- * BUILDER IS RESPOSIBLE FOR INSTALLING GAS LINES
- * 50# LIGHT BOXES REQUIRED

PFS.

PFS CORPORATION

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State:

Date:

ODLQH PFS Renee Moiss

Signature: Title:

Staff Plan Reviewer 12/12/14

KIM 4350

□PD ■QN □SN BUILDER:

HALLMARK HOMES CUSTOMER/PROJECT:

CARTER (32307)

KEISER HOMES

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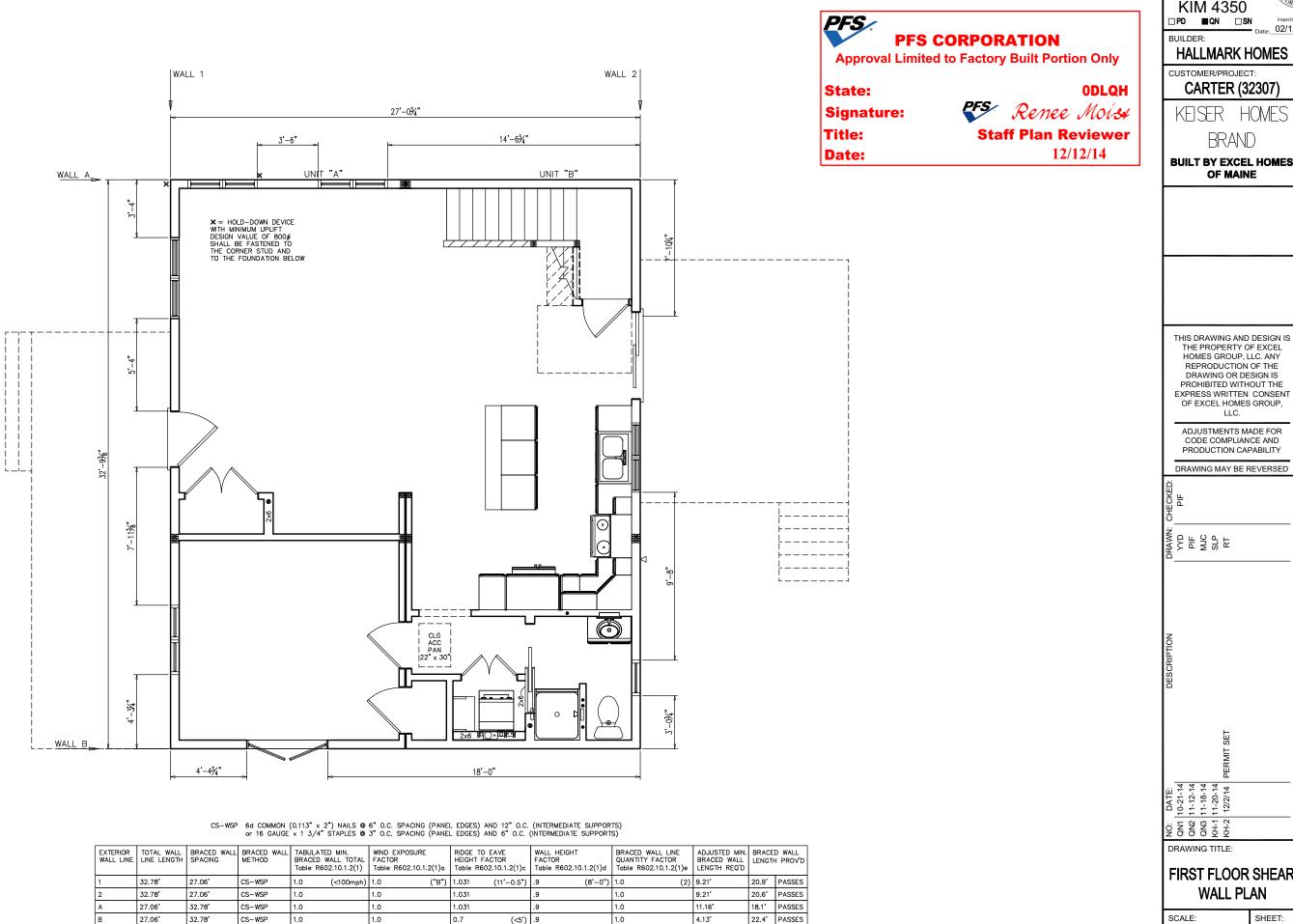
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SECOND FLOOR **ELECTRICAL PLAN**

SHEET:

P6.1

SCALE: NTS



KIM 4350

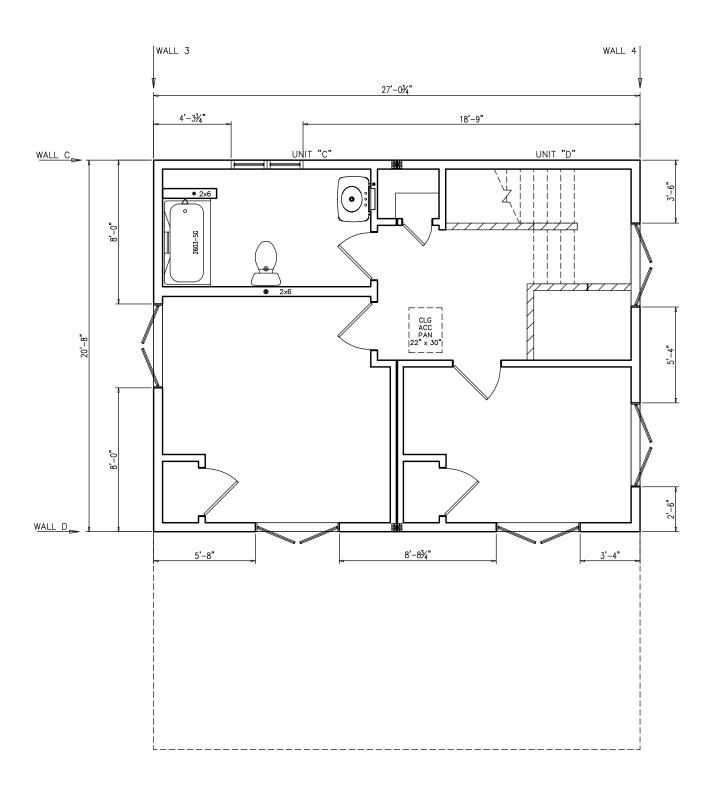
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CODE COMPLIANCE AND PRODUCTION CAPABILITY

FIRST FLOOR SHEAR WALL PLAN

SCALE: SHEET: NTS 10A



CS-WSP 6d COMMON (0.113" x 2") NAILS @ 6" O.C. SPACING (PANEL EDGES) AND 12" O.C. (INTERMEDIATE SUPPORTS) or 16 GAUGE x 1 3/4" STAPLES @ 3" O.C. SPACING (PANEL EDGES) AND 6" O.C. (INTERMEDIATE SUPPORTS)

EXTERIOR WALL LINE	TOTAL WALL LINE LENGTH		BRACED WALL METHOD	BRACED WALL TOTAL		RIDGE TO EAVE HEIGHT FACTOR Table R602.10.1.2(1)c	WALL HEIGHT FACTOR Table R602.10.1.2(1)d		ADJUSTED MIN. BRACED WALL LENGTH REQ'D		
3	20.67	27.06'	CS-WSP	1.0 (<100mph)	1.0 ("B")	1.063 (11'-0.5")	.9 (8'-0")	1.0 (2)	5.18'	16.0'	PASSES
4	20.67'	27.06'	CS-WSP	1.0	1.0	1.063	.9	1.0	5.18'	11.3'	PASSES
С	27.06'	20.67'	CS-WSP	1.0	1.0	1.063	.9	1.0	3.95'	23.1'	PASSES
D	27.06'	20.67'	CS-WSP	1.0	1.0	1.063	.9	1.0	3.95'	17.7'	PASSES

PFS

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State:

Renee Moiss

Signature: Title:

Staff Plan Reviewer

Date: 12/12/14

PTI

KIM 4350

BUILDER:

HALLMARK HOMES

CUSTOMER/PROJECT:

CARTER (32307)

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DRAWN YYD PIF MJC SLP SLP

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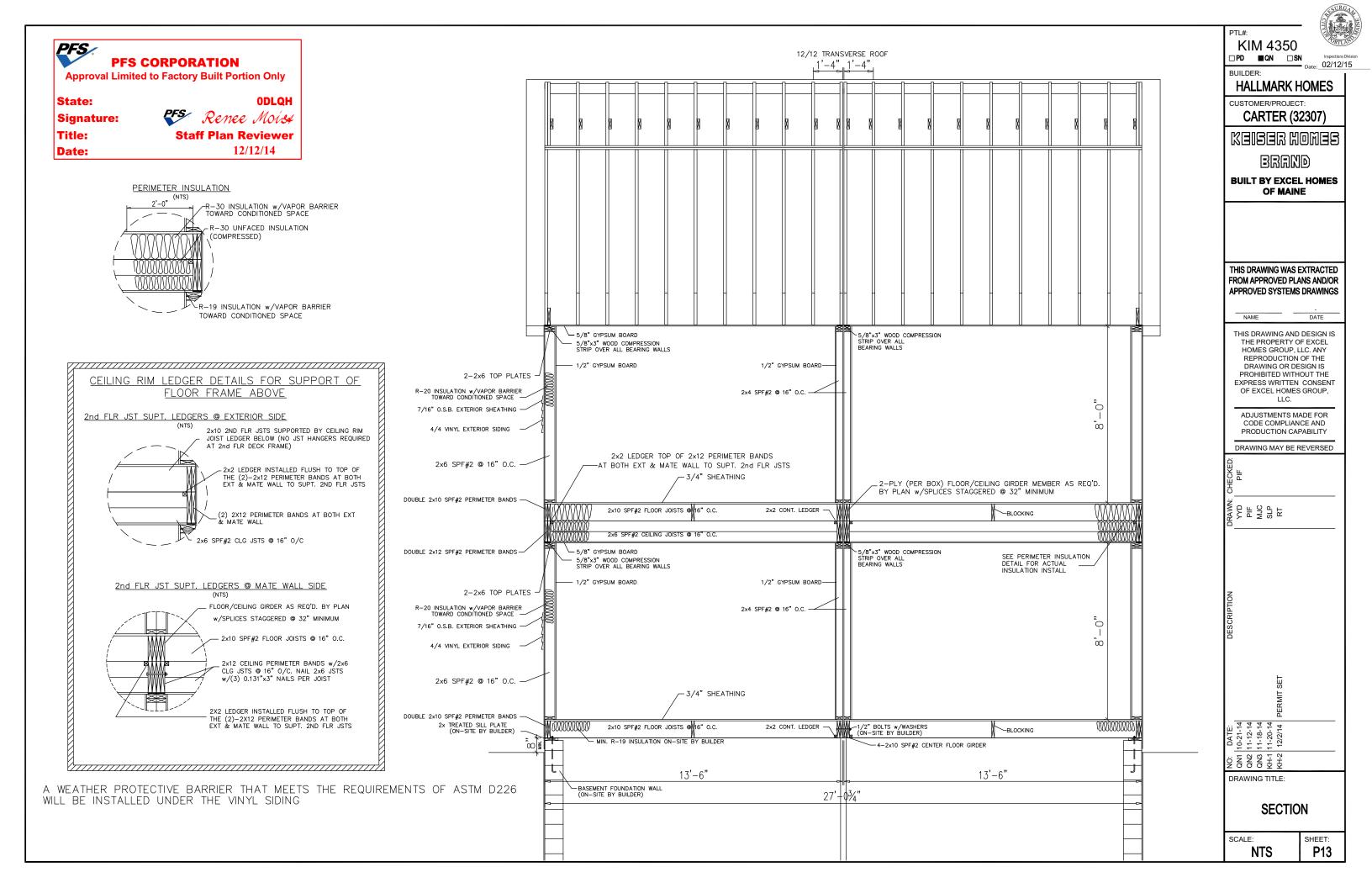
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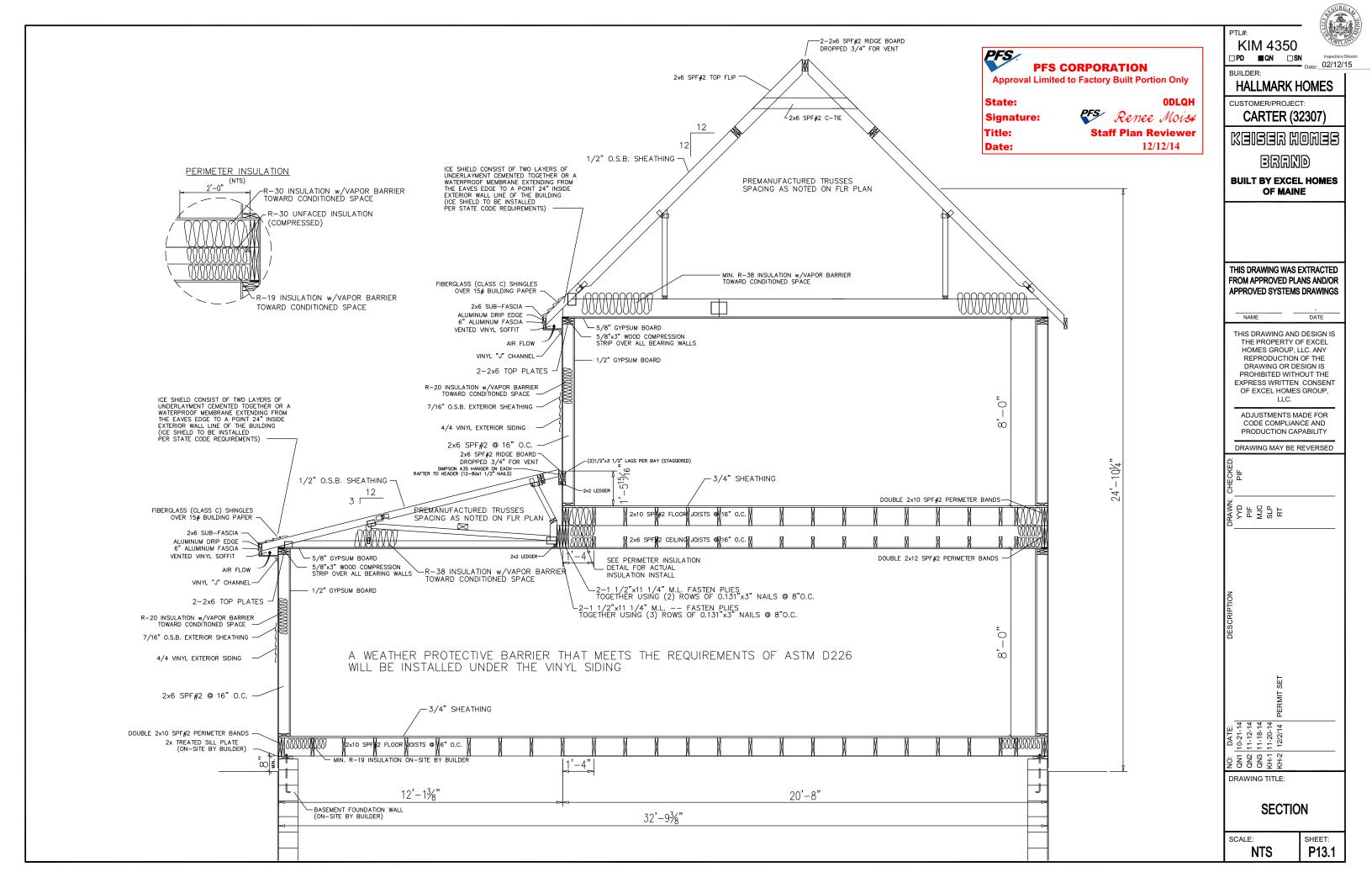
SECOND FLOOR
SHEAR WALL PLAN

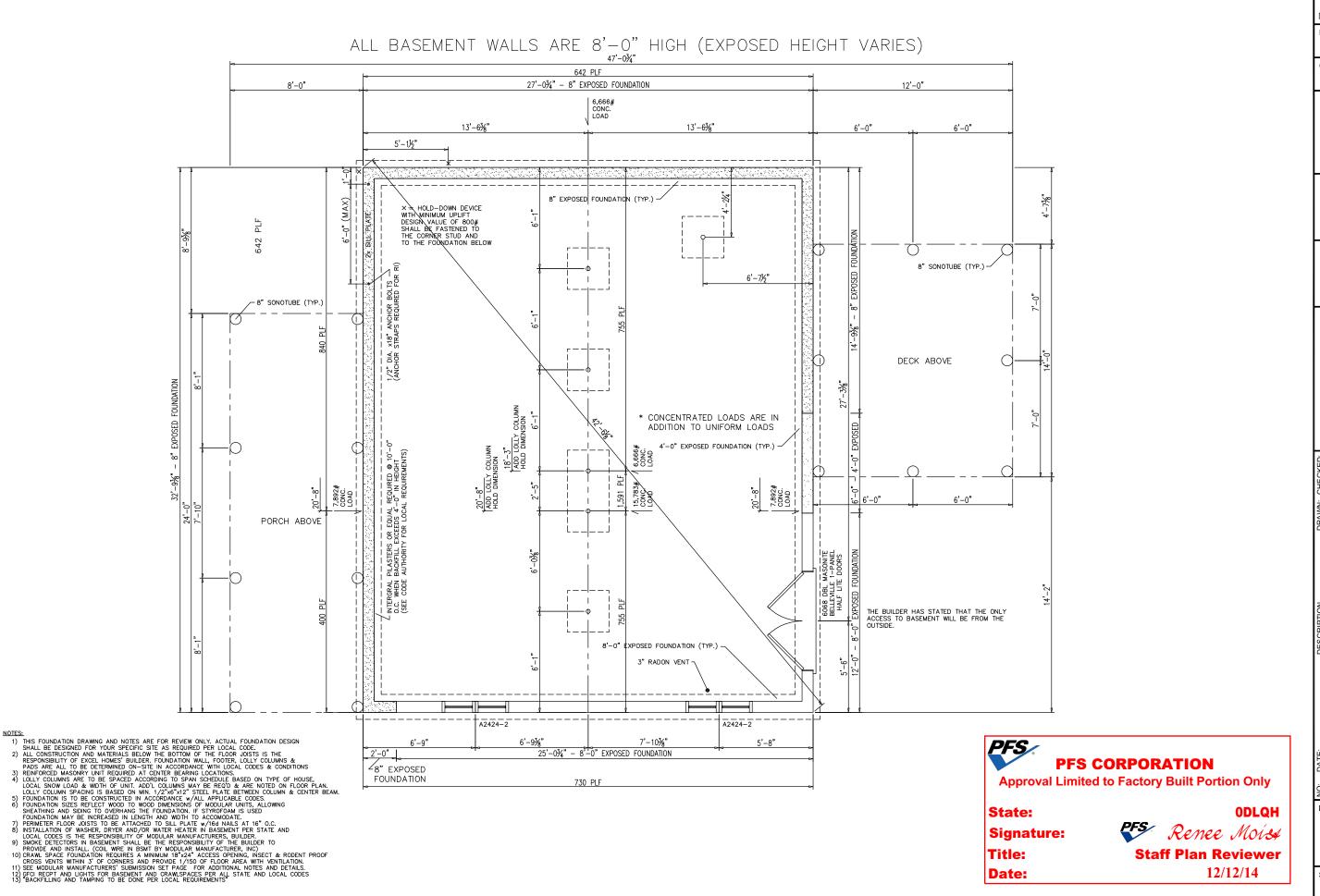
SHEET:

10B

SCALE:







KIM 4350 □PD ■QN □SN

BUILDER: HALLMARK HOMES

CUSTOMER/PROJECT:

CARTER (32307)

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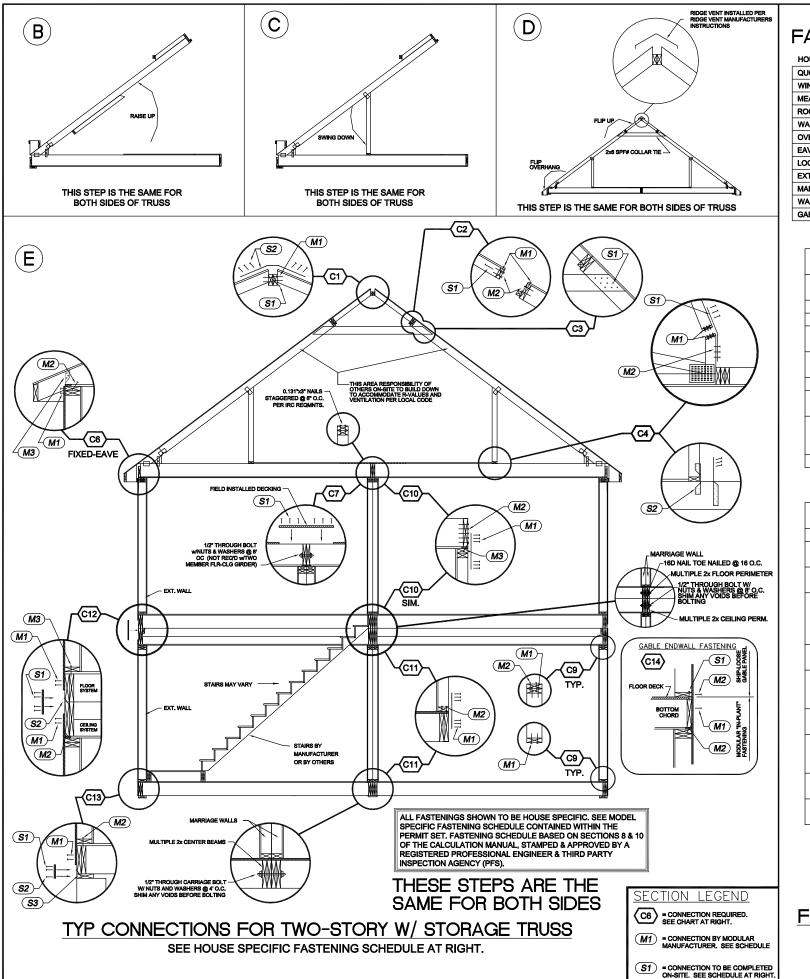
NO: QN2 QN3 KH-1

DRAWING TITLE:

FOUNDATION PLAN

SCALE:

SHEET: 3/16" = 1'-0"



FASTENING SCHEDULE

HOUSE-SPECIFIC INFO

QUOTE#: 4350 STATE: ME
WIND SPEED: <100 mph SNOW LOAD: 50 psf
MEAN ROOF HT.: 24'-10 1/4"
ROOF O.C.: 16 in
WALL O.C.: 16 in
OVERHANG DEPTH: 10 in
EAVE OVERHANG: FIXED
LOCATION: MAIN HOUSE
EXTERIOR WALLS: 2x6
MARRIAGE WALLS: 2x4
WALL HT.: 8.0 ft
GABLE WALL HT.: 11 ft

TRUSS-SPECIFIC CONNECTION INFO

CONNECTION LOCATION		FORCE (lbs)		
		SHEAR	COMPRESSION	
RIDGE TO RIDGI	153	149		
IP TO TOP CHORE	102	200		
TIE TO TOP CHORE	220	19	626	
LL TO TOP CHORE)			
D (OR KING POST	339	161		
RUSS HEEL UPLIF	•	33		
HEEL HORIZONTA	. 1	198		
MATEWALL UPLIF	•	59		
WALL HORIZONTA	. 1	121		

FASTENING TO BE COMPLETED "ON-SITE"

CONN.#	DES	CONNECTION AREA	CONNECTION REQUIRED	CALC MANUAL PAGE REF#
C1	S1	RIDGE TO RIDGE	(2) 0.131" x 3-1/4" FACE-NAILS PER TRUSS BAY	8.0.4
Ci	S2	FLIP RAFTER TO FLIP RAFTER	(1) 1.25" x 20 GA STRAP w/ (14) 8d NAILS EVERY THIRD TRUSS	8.0.5
C2	S1	TOP CHORD CONTINUOUS TO FLIP CONTINUOUS	(3) 0.131" x 3-1/4" FACE-NAILS PER TRUSS BAY	8.0.7
СЗ	S1	COLLAR TIE TO RAFTER	(7) 0.131" x 3-1/4" FACE-NAILS BOTH SIDES OF COLLAR-TIE	8.0.13
C4	S1	KNEEWALL TO TRUSS CHORD (STORAGE TRUSS)	1 SIMPSON H8	8.0.12
C4	S2	KNEEWALL TO TRUSS CHORD (SHED TRUSS)	(1) 1.25" x 26 GA STRAP w/ (8) 8d NAILS EVERY THIRD TRUSS	8.0.12
C5	S1	SHEATHING TO TOP CHORD	(3) 0.131" x 2-1/2" FACE-NAILS PER TRUSS	10.18.0
Co	S2	FLIP CONTINUOUS TO STUD	N/A	10.22.0
C7	S1	DECKING ACROSS MATEWALL (TRUSS TO TRUSS)	N/A	8.0.11
040	S1	SHEATHING BAND TO RIM JOISTS	(2) ROWS OF 0.131" x 2-1/2" FACE-NAILS AT 16" O.C.	10.4.0
C12	S2	RIM JOIST TO RIM JOIST (HORIZONTAL LOADING)	(1) ROW OF 0.131" x 3-1/4" TOE-NAILS AT 6" O.C.	2009 IRC
	S1	SHEATHING BAND TO RIM	(2) ROWS OF 0.131" x 2-1/2" FACE-NAILS AT 16" O.C.	10.4.0
C13	S2	SHEATHING BAND TO SILL PLATE	(1) ROW OF 0.131" x 2-1/2" FACE-NAILS AT 16" O.C.	10.5.0
	S3	FLOOR RIM TO SILL PLATE	(1) ROW OF 0.131" x 3-1/4" TOE-NAILS AT 6" O.C.	10.14.0
C14	S1	WALL PLATE TO RIM & RAFTER	(1) ROW OF 0.131" x 3-1/4" FACE NAILS AT 8" O.C.	10.20.0

FASTENING TO BE COMPLETED BY "MANUFACTURER"

CONN.#	DES	CONNECTION AREA	CONNECTION REQUIRED	CALC MANUAL PAGE REF#
C1	M1	RIDGE TO FLIP RAFTER	(3) 0.131" x 3-1/4" FACE-NAILS PER TRUSS	8.0.2
C2	M1	SHEATHING TO ROOF CONTINUOUS	(2) 0.131" x 2-1/2" FACE-NAILS PER TRUSS EA. SIDE	8.0.8
G2	M2	CONTINUOUS TO FLIP RAFTER OR TOP CHORD	(4) 0.131" x 3-1/4" FACE-NAILS PER TRUSS	8.0.6
C4	M1	KNEEWALL PLATE TO KNEEWALL OR KINGPOST	(3) 0.131" x 3-1/4" FACE-NAILS PER TRUSS	8.0.9
C4	M2	KINGPOST TO KNEEWALL STUD	(1) 1.25" x 26 GA STRAP w/ (8) 8d NAILS EVERY THIRD TRUSS	8.0.10
	M1	SHEATHING TO WALL PLATES	(2) ROWS OF 0.131" x 2-1/2" NAILS AT 16" O.C.	10.3.0
C6	M2	TRUSS TO TOP PLATE (HORIZONTAL LOADING)	(2) 0.131" x 3-1/4" TOE-NAILS	10.10.0
Co	М3	TRUSS TO TOP PLATE (OR WALL STUD)	(1) SIMPSON MTS30 - EVERY THIRD TRUSS	10.2.0
	M4	CONTINUOUS TO BOTTOM CHORD	(1) 0.131" x 3-1/4" FACE-NAILS PER TRUSS	10.15.0
C9	M1	PLATE TO STUD	(2) 0.131" x 3-1/4" FACE-NAILS PER STUD	10.12.0
Cs	M2	PLATE TO PLATE	(1) ROW OF 0.131" x 3-1/4" FACE-NAILS AT 13" O.C.	10.11.0
	M1	CONTINUOUS TO WALL STUD	(1) LSTA12 STRAP w/ (10) 0.148" x 2-1/2" NAILS AT 48" O.C.	10.17.0
C10	M2	CONTINUOUS TO TRUSS (OR CEILING JOIST)	(2) 0.131" x 3-1/4" FACE-NAILS PER TRUSS	10.15.0
	МЗ	CONTINUOUS TO WALL PLATES	(1) ROW OF 0.131" x 3-1/4" TOE-NAILS AT 6" O.C.	2009 IRC
C11	M1	FLOOR RIM TO WALL STUD	(1) LSTA12 STRAP w/ (10) 0.148" x 2-1/2" NAILS AT 48" O.C.	10.17.0
GII	M2	WALL PLATE TO FLOOR RIM	(2) ROWS OF 0.131" x 3-1/4" FACE-NAILS AT 16" O.C.	2009 IRC
	M1	SHEATHING TO RIM JOIST	(2) ROWS OF 0.131" x 2-1/2" FACE-NAILS AT 16" O.C.	10.4.0
C12	M2	RIM JOIST TO WALL PLATE	(1) ROW OF 0.131" x 3-1/4" TOE-NAILS AT 6" O.C.	2009 IRC
	МЗ	PLATE TO FLOOR RIM JOIST	(1) ROW OF 0.131" x 3-1/4" FACE-NAILS AT 7" O.C.	10.13.0
C13	M1	SHEATHING TO RIM JOIST	(2) ROWS OF 0.131" x 2-1/2" FACE-NAILS AT 16" O.C.	10.4.0
013	M2	PLATE TO FLOOR RIM JOIST	(1) ROW OF 0.131" x 3-1/4" FACE-NAILS AT 7" O.C.	10.13.0
C14	M1	SHEATHING TO RIM AND GABLE WALL	(1) ROW OF 0.131" x 3-1/4" FACE-NAILS AT 6" O.C.	10.21.0
U14	M2	TRUSS BOTTOM CHORD TO WALL PLATE	(1) ROW OF 0.131" x 3-1/4" FACE-NAILS AT 6" O.C.	10.21.0

PFS CORPORATION PFS Renee Moist

FASTENING REQUIREMENTS FOR TWO-STORY W/ STORAGE TRUSS

FASTENING SHOWN IS HOUSE SPECIFIC TO THE MODEL CONTAINED WITHIN THIS PERMIT SET. ALTERNATE FASTENERS OF EQUAL OR GREATER VALUE MAY BE SUBSTITUTED FOR THOSE SHOWN, PROVIDED THEY RESIST THE LOADS/FORCES IMPOSED PER CONNECTION.

KIM 4350 □PD ■QN □SN

BUILDER: HALLMARK HOMES

CUSTOMER/PROJECT:

CARTER (32307)

BRAND

BUILT BY EXCEL HOMES OF MAINE

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ADJUSTMENTS MADE FOR CODE COMPLIANCE AND PRODUCTION CAPABILITY

DRAWING MAY BE REVERSED

SUP RT

NO: QNZ QNZ QN3 CH-1

DRAWING TITLE:

SCALE: SHEET: NTS



HEAT LOSS CALC ON- 4350 (32307)

DATE: 11/13/2014 BY: PIF STATE: ME

0.026 CEILING: FLOOR: 0.053 WALL: 0.053

DELTA T

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"U" VALUES:

R30=.040 R38=.026 R42=.024 R49=.020 R19=.053 R30=.040

Date: 02/1

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1 00

WALL TYPE

2125

7258

2500

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60

60

14.00

5025

2000

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50

50

10 00

2995

1000

Ω

30

30

6 00

USE H6 FOR R21/C6 FOR R17 (ONLY 2x4 & 2x6) KIT BATH#1 BATH#2 BR#1 BR#2 BR#3 LAUNDRY HALL TOTAL FLOOR(1,2,3SINGLE) # OF EXT. WALL(S): LENGTH 20.5 13.0 12.0 7.0 8.0 13.0 13.5 10.0 8.0 11.0 0.0 0.0 0.0 0.0 0.0 0.0 27.0 WIDTH 13.50 13.50 13.50 13.50 6.50 13.50 13.50 13.50 7.00 13.50 0.00 0.00 0.00 0.00 0.00 0.00 27.00 CLG HGT 9.0 9.0 9.0 9.0 9.0 9.0 9.0 9.0 9.0 9.0 0.0 0.0 0.0 0.0 0.0 0.0 18.0 EXT WALL 34.00 26.50 12.00 20.50 14.50 26.50 27.00 23.50 8.00 24.50 0.00 0.00 0.00 0.00 0.00 0.00 107.50 .30 WIND 39.1 0.0 13.9 8.0 4.0 26.5 37.0 37.0 0.0 18.5 0.0 0.0 0.0 0.0 0.0 0.0 184.0 .32 WIND 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 .34 WIND 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 .36 WIND 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 .38 WIND 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 .42 WIND 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 .14 DOOR (SOLID) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 .25 DOOR (GLASS) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 .30 DOOR (GLASS) 0.0 32.7 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 32.7 .39 DOOR (GLASS) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 WALL LOSS 1817 1631 1872 1730 13323 CLG LOSS 612 388 358 209 115 388 403 298 124 328 1611 FLR LOSS 1247 791 730 234 791 821 608 252 669 0 3284 AIR INF 3811 2417 1487 1301 716 2417 2510 1859 2045 0 0 20077

1138

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ACTUAL HWBB INST'D NOTES:

WATT LOSS

BTUH LOSS

WATT PROV

BTUH PROV

REO'D ELEC SIZE (IN

REO'D HWBB SIZE (FT

ACTUAL ELEC INST'D

1417

4839

1500

10 00

Ω

40

40

1401

4785

1500

9.00

Ω

40

40

PFS CORPORATION

Approval Limited to Factory Built Portion Only

State:

PFS Renee Moiss Signature:

Title:

Staff Plan Reviewer

12/12/14 Date:

Linear Convector[™] LC Series



Revolutionary design provides a sleek, compact heater profile, while improving heater performance, reducing energy consumption, and improving comfort.

Features

- Faster heating of the room than a conventional baseboard due to rapid vertical laminar air flow, directing heat to the ceiling, speeding dispersal of warm air into the center of the room
- Improved performance and reduced length through use of top heat discharge and new fin design
- Superior shark-fin blade design on a steel tubular element for improved heat transfer and longer life
- Discreet styling, reduced length and added versatility of placement provide more options when designing a room
- May be used with wall or top mounted built-in thermostat (not included)
- Full length automatic overheat reset for safety

Applications

 All residential applications, commercial offices, lobbies, washrooms



Specifications

Voltage 120, 208, 240, 347V

Wattage Sizes ranging from 500W to 2500W

Color White or Almond

Finish Specially-formulated epoxy / polyester powder coating is environmentally

friendly and resists fading and abrasion.

Construction Robust, 20 gauge steel construction.

Heating Element A nickel chromium element is totally enclosed within a steel sheath,

providing superior life expectancy and resistance to rust. Shark-fin shaped aluminum fins are firmly staked in an upright position to provide directional

wicking for top discharge heat transfer.

Installation Easily removed front caps, knock-outs on both sides of the convector, and

pre-stamped mounting holes make installation easy.

Warranty Ten year element warranty.

One year warranty on complete unit.





Inspect

Ordering Guide

Cat. No.	Watts	Volts	BTU	Length mm/in.	Weight kg/lbs.
LC2005W11	500	120	1706	508/20	1.7/3.75
LC2005W21	500	208	1706	508/20	1.7/3.75
LC2005W31	500/375	240/208	1706/1280	508/20	1.7/3.75
LC2005W51	500	347	1706	508/20	1.7/3.75
LC2507W11	750	120	2559	635/25	2/4.40
LC2507W21	750	208	2559	635/25	2/4.40
LC2507W31	750/563	240/208	2559/1919	635/25	2/4.40
LC2507W51	750	347	2559	635/25	2/4.40
LC3010W11	1000	120	3412	762/30	2.3/5.00
LC3010W21	1000	208	3412	762/30	2.3/5.00
LC3010W31	1000/750	240/208	3412/2559	762/30	2.3/5.00
LC3010W51	1000	347	3412	762/30	2.3/5.00
LC3512W11	1250	120	4265	889/35	2.9/6.40
LC3512W21	1250	208	4265	889/35	2.9/6.40

Cat. No.	Watts	Volts	BTU	Length mm/in.	Weight kg/lbs.
LC3512W31	1250/938	240/208	4265/3199	889/35	2.9/6.40
LC3512W51	1250	347	4265	889/35	2.9/6.40
LC4015W11	1500	120	5118	1016/40	3.2/7.50
LC4015W21	1500	208	5118	1016/40	3.2/7.50
LC4015W31	1500/1125	240/208	5118/3839	1016/40	3.2/7.50
LC4015W51	1500	347	5118	1016/40	3.2/7.50
LC5020W21	2000	208	6824	1270/50	3.7/8.20
LC5020W31	2000/1500	240/208	6824/5120	1270/50	3.7/8.20
LC5020W51	2000	347	6824	1270/50	3.7/8.20
LC6025W21	2500	208	8530	1524/60	4.4/9.70
LC6025W31	2500/1875	240/208	8530/6398	1524/60	4.4/9.70
LC6025W51	2500	347	8530	1524/60	4.4/9.70

Note: 1) Standard color is white.

2) To order almond, omit "W" in Cat. No.

Control Options (field installed)

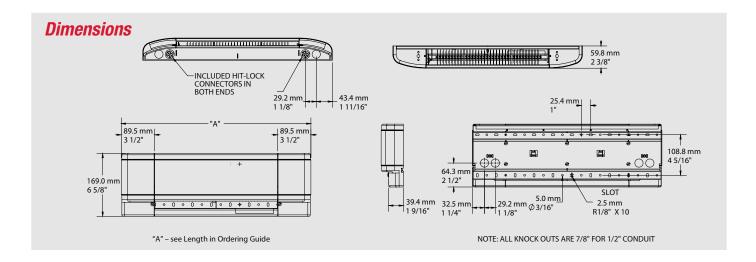
Thermostat Kits

Cat. No.	Description	Rating
DTK-SP	Single pole built-in thermostat kit (adjustable)	120-240V, 17A 347V, 14A
DTK-DP	Double pole built-in thermostat (adjustable)	120-240V, 17A 347V, 11A
DTKT-SP	Single pole built-in tamperproof thermostat kit	120-240V, 17A 347V, 11A
DTKT-DP	Double pole built-in tamperproof thermostat kit	120-240V, 25A 347V, 11A

Note: Each adjustable kit contains white and almond control knobs and hardware

Relays

Cat. No.	Description	Rating
BLLVC11	Low voltage relay & transformer kit	120V, 22A
BLLVC21	Low voltage relay & transformer kit	208V, 22A
BLLVC31	Low voltage relay & transformer kit	240V, 22A
BLLVC51	Low voltage relay & transformer kit	347V, 17A
BLLVD	Low voltage relay less transformer kit	120/208/240, 22A 347V, 17A



0-1-0 3.00 12 SMH18B 3-3-1 OPT, CUT OFF 5-3-2 0-10-8 3x4 =SMH18A 5-4 W B1 9 1x3 II 8 3.5x5 =1 1/2 x 1 1/2 3x4 =DADO 3-10-18-0-5 0-8-8 11-10-6 Plate Offsets (X,Y)-- [2:0-3-1,Edge], [3:0-1-8,0-1-8], [4:0-1-0,0-0-0], [5:0-0-4,0-1-0], [8:0-1-10,0-1-12]

F	` ' -				
	LOADING (psf) TCLL 38.5 (Ground Snow=50.0) TCDL 10.0 BCLL 0.0 *	SPACING- 1-4-0 Plates Increase 1.15 Lumber Increase 1.15 Rep Stress Incr YES Code IBC2009/TPI2007	CSI. TC 0.98 BC 0.58 WB 0.91 (Matrix)	DEFL. in (loc) l/defl L/d Vert(LL) -0.11 8-9 >999 240 Vert(TL) -0.29 8-9 >477 180 Horz(TL) 0.03 8 n/a n/a	PLATES GRIP MT20 197/144 MT18HS 197/144 Weight: 37 lb FT = 0%
	BCDL 10.0	Code IBC2009/1712007	(iviatrix)		weight: 37 lb F1 = 0%

LUMBER-

TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2

2x3 SPF Stud *Except* W4: 2x4 SPF Stud **WEBS**

BRACING-TOP CHORD BOT CHORD WFRS **JOINTS**

Structural wood sheathing directly applied, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Row at midpt

1 Brace at Jt(s): 10

REACTIONS. (lb/size) 2=532/0-3-8, 8=437/0-3-8, 7=0/Mechanical Max Horz 2=116(LC 14), 7=-116(LC 14)

Max Uplift2=-76(LC 7), 8=-59(LC 7) Max Grav 2=573(LC 14), 8=516(LC 14)

FORCES. (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/15, 2-3=-1560/196, 3-4=-340/43, 4-11=-321/44, 5-11=-269/54, 5-6=-134/33, 6-7=-120/35, 8-10=-283/87 **BOT CHORD**

2-9=-233/1379, 8-9=-233/1379

3-9=0/196, 3-8=-1244/190, 5-10=-318/98 WFBS

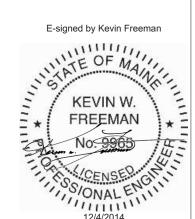
REQUIRED FIELD JOINT CONNECTIONS - Maximum Compression (Ib)/ Maximum Tension (Ib)/ Maximum Shear (Ib)/ Maximum Moment (Ib-in) 6=127/33/28/0. 10=318/98/145/0

- 1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-05; Pg=50.0 psf (ground snow); Ps=38.5 psf (roof snow); Category II; Exp B; Partially Exp.; Ct=1.1
- 3) Roof design snow load has been reduced to account for slope
- 4) Unbalanced snow loads have been considered for this design
- 5) This truss has been designed for greater of min roof live load of 20.0 psf or 2.00 times flat roof load of 38.5 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads 7) All plates are MT20 plates unless otherwise indicated.
- 8) See HINGE PLATE DETAILS for plate placement.
- 9) Provisions must be made to prevent lateral movement of hinged member(s) during transportation.

 10) All additional member connections shall be provided by others for forces as indicated.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 12) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 76 lb uplift at joint 2 and 59 lb uplift at joint 8. 14) This truss has been designed in accordance with the 2009 IBC Section 2303.4.6, 2009 IRC Section 802.10.2.

- 15) Take precaution to keep the chords in plane, any bending or twisting of the linge plate must be repaired before the building is put into service.

 16) The field-installed members are an integral part of the truss design. Retain a design professional to specify final field connections and temporary supports. All field-installed members must be properly fastened prior to applying any loading to the truss. This design anticipates the final set position.



12/4/2014

The professional engineering seal indicates that a licensed professional has reviewed the design under the standards referenced within this document, not necessarily the current state building code. The engineering seal is not an approval to use in a specific state. The final determination on whether a truss design is acceptable under the locally adopted building code rest with the building official or designated appointee.

WARNING - Verify design parameters and READ NOTES Universal Forest Products, Inc. PHONE (616)-364-6161 FAX (616)-365-0060 Truss shall not be cut or modified without approval of the truss design engineer.

2801 EAST BELTLINE RD. NE GRAND RAPIDS, MI 49525

This component has only been designed for the loads noted on this drawing. Construction and lifting forces have not been considered. The builder is responsible for lifting methods and system design. Builder responsibilities are defined under TPI1. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult BCSI 1-06 from the Wood Truss Council of America and Truss Plate Institute Recommendation available from WTCA, 6300 Enterprise LN, Madison, WI 53719 J:\support\MitekSupp\templates\ufp.tpe



02/1

[P]

APPROVED

12/12/14

PFS CORPORATION

Bloomsburg, PA

0-8-8 0-8-8 10-4-0 10-4-0 0-3-0 **APPROVED** 12/12/14 DATE PFS CORPORATION 12.00 12 Bloomsburg, PA 9.2.8 8-1-6 1-0-0 SMH18E A355 OPT. CUT OFF 0-10-8 X.S. 10 0-10-8 ONE SIDE 2x5 ∕∕ (ONE SIDE) SMH18E SMH18D SMH18D **B**1 B₂ 14 6x10 = 1516 * See Note 20 & 21* 4-5-8 5-10-8 5-10-8 4-5-8 0-8-8 0-8-8 20-8-0 Plate Offsets (X,Y)-- [2:0-2-3,0-1-15], [3:0-1-8,0-3-12], [4:0-0-0,0-1-0], [10:0-0-0,0-1-0], [11:0-1-8,0-3-12], [12:0-2-3,0-1-15]

LOADING (psf) TCLL 29.6 (Ground Snow=50.0) TCDL 10.0 BCLL 0.0 *	SPACING- 1-4-0 Plates Increase 1.15 Lumber Increase 1.15 Rep Stress Incr YES	CSI. TC 0.74 BC 0.57 WB 0.22	DEFL. in (loc) I/defl L/d Vert(LL) -0.30 14-16 >806 240 Vert(TL) -0.55 14-16 >445 180 Horz(TL) 0.01 12 n/a n/a	PLATES GRIP MT20 197/144 MT18HS 197/144
BCLL 0.0 * BCDL 10.0	Code IBC2009/TPI2007	(Matrix)	Attic -0.13 14-16 1074 360	Weight: 108 lb FT = 0%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SPF No.2 *Except* T1: 2x4 SPF No.2

BOT CHORD 2x10 SPF No.2 **WEBS** 2x3 SPF Stud *Except* W2: 2x6 SPF No.2

REACTIONS. (lb/size) 2=759/0-3-8, 12=759/0-3-8

Max Horz 2=200(LC 8)
Max Uplift2=-56(LC 9), 12=-56(LC 10)
Max Grav 2=830(LC 2), 12=830(LC 2)

FORCES. (lb) - Maximum Compression/Maximum Tension

1-2=0/51, 2-3=-806/44, 3-4=-675/47, 4-5=-528/84, 5-6=-380/112, 6-7=-41/149, 7-8=-38/142, 8-9=-380/112, 9-10=-527/84, 10-11=-678/48, 11-12=-806/45, 12-13=0/51, 1TOP CHORD

BOT CHORD 2-16=-7/429, 15-16=-4/432, 14-15=-4/432, 12-14=-3/429

10-14=-138/342, 4-16=-138/343, 6-8=-636/217 **WEBS**

REQUIRED FIELD JOINT CONNECTIONS - Maximum Compression (lb)/ Maximum Tension (lb)/ Maximum Shear (lb)/ Maximum Moment (lb-in)

5=418/102/205/0, 6=639/218/19/0, 7=31/157/153/0, 8=636/217/11/0, 9=415/102/205/0, 14=138/342/0/0, 16=138/343/0/0

NOTES-

- 1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-05; Pg=50.0 psf (ground snow); Ps=29.6 psf (roof snow); Category II; Exp B; Partially Exp.; Ct=1.1
- 3) Roof design snow load has been reduced to account for slope
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 38.5 psf on overhangs non-concurrent with other live loads
- 6) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads
- 7) All plates are MT20 plates unless otherwise indicated. 8) See HINGE PLATE DETAILS for plate placement.
- 9) Provisions must be made to prevent lateral movement of hinged member(s) during transportation.
- 10) All additional member connections shall be provided by others for forces as indicated.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 12) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 13) Ceiling dead load (5.0 psf) on member(s). 4-6, 8-10, 6-8
- 14) Bottom chord live load (30.0 psf) and additional bottom chord dead load (0.0 psf) applied only to room. 14-16
 15) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 56 lb uplift at joint 2 and 56 lb uplift at joint 12.
- 16) This truss has been designed in accordance with the 2009 IBC Section 2303.4.6, 2009 IRC Section 802.10.2.
- 17) Attic room checked for L/360 deflection
- 18) Take precaution to keep the chords in plane, any bending or twisting of the hinge plate must be repaired before the building is put into service.
- 19) The field-installed members are an integral part of the truss design. Retain a design professional to specify final field connections and temporary supports. All field-installed members must be properly fastened prior to applying any loading to the truss. This design anticipates the final set position. 20) Temporary supports are required to maintain the bottom chord in a level position during storage, transportation, and setup. Retain a design
- professional to specify all temporary bracing to support the truss until setup is complete. Temporary support(s) must not be removed until all field connections are completed.
- 21) The bottom chord must be laterally braced during shipment and setup to prevent damage to the splice plate.

The professional engineering seal indicates that a licensed professional has reviewed the design under the standards referenced within this document, not necessarily the current state building code. The engineering seal is not an approval to use in a specific state. The final determination on whether a truss design is acceptable under the locally adopted building code rest with the building official or designated appointee.

ONAL ENGINE WARNING - Verify design parameters and READ NOTES Universal Forest Products, inc. PHONE (616)-364-6161 FAX (616)-365-0060 2801 EAST BELTLINE RD, NE GRAND RAPIDS, MI 49525

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing

Truss shall not be cut or modified without approval of the truss design engineer This component has only been designed for the loads noted on this drawing. Construction and lifting forces have not been considered. The builder is responsible

for lifting methods and system design. Builder responsibilities are defined under TPI1. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult BCSI 1-06 from the Wood Truss Council of America and Truss Plate Institute Recommendation available from WTCA, 6300 Enterprise LN, Madison, WI 53719 J:\support\MittekSupp\templates\ufgartrightarrow\ufgartrightarrow\uppart\uppart\mittekSupp\templates\ufgartrightarrow\uppart



[P]

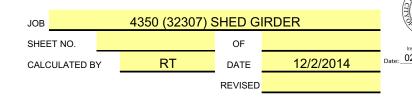
02/1

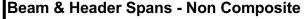
Date:

KEVIN W.
FREEMAN

SOONAL ENGINE







Beam1 =	1.5" x 11.25"	Species1 =	LVL	Grade1 =	NA	NA	NA
Beam2 =	None	Species2 =	LVL	Grade2 =	NA	NA	NA
Quantity Pe	r Box, Q1 =	2 d1=	11.25	in b1=	3.00 in	Enter Beam w/ L	argest Quantity in Q1

Quantity Per Box, Q1 =
$$\frac{2}{0}$$
 d1= $\frac{11.25}{0}$ in $\frac{11}{0}$ in $\frac{11}{0}$ 3.00 in Enter B

Area, A1 =
$$33.75$$
 in² Section Modulus, S1 = 63.28 in³ Moment of Inertia, I1 = 355.96 in⁴ Area, A2 = 0.00 in² Section Modulus, S2 = 0.00 in³ Moment of Inertia, I2 = 0.00 in⁴

Load D	<u>uration</u>	<u>Beam S</u>	Stability	<u>Size Fa</u>	<u>ctor</u>	<u>Repetity</u>	<u>/e Facto</u>	<u>Deflection C</u>	<u>Criteria</u>
C _D =	1.15	C _L =	1.00	$C_{F1} =$	1.00	C _{R1} =	1.00	delta LL =	240
				$C_{F2} =$	1.00	C _{R2} =	1.00	delta TL =	180

Shear Moment Deflection Load Share

$F_{v1} =$	285	psi	$F_{b1} =$	2750	psi	E1 =	2000000	psi	Beam1 =	100.00 %
$F_{v2} =$	0	psi	$F_{b2} =$	0	psi	E2 =	0	psi	Beam2 =	0.00 %

Controlling Load W = 666.5 plf

Max Allowable Length =	169.8	in	Actual Length Needed =	153.0	in
Reaction at Each End =	4248.9	lb	Minimum Bearing Length Required =	3.3	in

0	Number of Matewall Column Studs Per Box							
Species & Grade	Max 8	Sft Tall C	olumn	Max 9ft Tall Column				
0.000	2x3	2x4	2x6	2x3	2x4	2x6		
spf #2	NG	2	2	NG	3	2		
spf #3	NG	3	2	NG	3	2		
spf stud	NG	3	2	NG	3	2		
sp#2	4 2 2			NG	2	2		

BEAM FASTENING REQUIREMENTS

CONDITION 1 - TOP LOADED - 2 TO 4 PLY BEAMS

12" deep or less - (2) rows of 0.131"x3" nails @ 8" o.c.

> 12" & < 18" deep - (3) rows of 0.131"x3" nails @ 8" o.c.

> 18" deep - (4) rows of 0.131"x3" nails @ 8" o.c.

CONDITION 2 - SIDE LOADED

2-Ply <= 465plf (2) rows of 0.131"x3" nails @ 8" o.c.

2-Ply <= 700plf (3) rows of 0.131"x3" nails @ 8" o.c.

2-Ply <= 870plf (2) rows of 1/2" Bolts @ 12" o.c.

2-Ply > 870plf (3) rows of 1/2" Bolts @ 12" o.c.

3-Ply <= 350plf (2) rows of 0.131"x3" nails @ 8" o.c.

3-Ply <= 525plf (3) rows of 0.131"x3" nails @ 8" o.c.

3-Ply <= 650plf (2) rows of 1/2" Bolts @ 12" o.c.

3-Ply > 650plf (3) rows of 1/2" Bolts @ 12" o.c.

4-Ply (3) rows of 1/2" Bolts @ 12" o.c.

Note: Stagger fastener rows and locate fasteners minimum 2" from all ends and edges. Space rows equally apart vertically. Fastener spacing is per row.



6

psf

SHEET NO.

CALCULATED BY

RT

DATE 12/2/2014

REVISED

Load Calculation Procedure

<u>INPUTS</u> <u>CONSTANTS</u>

Location: SIDEWALL Floor Live Load = 40 psf
Supporting Roof: YES Floor Dead Load = 10 psf

Supporting Roof: YES Floor Dead Load = 10 psf
Number of Floor Supporting: 1 Wall Dead Load = 62 plf

Cape Roof: YES Ceiling Dead Load = Simply Supported Rafter: NO

Reaction from Truss or Rafter = 830 lb Required Deflection Criteria

Truss spacing = 16 in o.c. Live Load = L / 360
Unit Width = 0 ft Total Load = L / 240

Roof Live Load / Unbalanced Snow Load = 50 psf Attic Live Load = 30 psf

Roof Dead Load = 20 psf

Roof/Snow Load = 622.5 plf
% Roof Live Load = 50.0 %

% Roof Dead Load = 20.0 %

% Attic Live Load = 30.0 % Load Duration for Wood Members

Load Cases

1. D + F = 186.5 plf Cd = 0.9

2. D + H + F + L + T = 373.3 plf Cd = 1.0

3. $D + H + F + (L_r \text{ or } S \text{ or } R)$ = 497.8 plf Cd = 1.15

4. $D + H + F + 0.75(L + T) + 0.75(L_r \text{ or S or R})$ = 606.7 plf Cd = 1.15

5. D + H + F + (W or 0.7E) = 186.5 plf Cd = 0.9

6. D + H + F + 0.75(W or 0.7E) + 0.75L + 0.75(L_r or S or R) = 606.7 plf Cd = 1.15

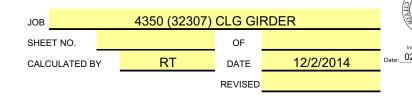
7. 0.6D + W + H = 111.9 plf Cd = 0.9

8. 0.6D + 0.7E + H = 111.9 plf Cd = 0.9

Check the highest load for each load duration factor when sizing wood members.

Check the highest load and apply no load duration factor when sizing steel members.







NA Beam1 = 1.5" x 11.25" Species1 = LVL Grade1 = NA NA LVL NA NA NA Beam2 = 1.5" x 9.25" Species2 = Grade2 = Enter Beam w/ Largest Quantity in Q1

Quantity Per Box, Q1 = 2 d1=11.25 in b1= 3.00 in Quantity Per Box, Q2 = 2 d2= 9.25 in b2= 3.00 in

Area, A1 = 33.75 in² Section Modulus, S1 = 63.28 in^3 Moment of Inertia, I1 = 355.96 in⁴ 27.75 in² Section Modulus, S2 = 42.78 in^3 Moment of Inertia, I2 = 197.86 in⁴ Area, A2 =

Load Duration **Beam Stability** Repetitve Factor Size Factor **Deflection Criteria** $C_D =$ 1.00 $C_L =$ 1.00 $C_{F1} =$ 1.00 $C_{R1} =$ 1.00 delta LL = 360 240 $C_{F2} =$ 1.04 $C_{R2} =$ 1.00 delta TL =

Shear Moment Deflection Load Share

285 $F_{v1} =$ $F_{b1} =$ 2750 2000000 E1 =psi psi psi Beam1 = 64.27 285 2750 2000000 E2 = $F_{v2} =$ $F_{b2} =$ 35.73 % psi psi psi Beam2 =

Controlling Load w = 377.5 plf

Max Allowable Length = 218.9 in Actual Length Needed = 211.9 in

Reaction at Each End = 3332.6 lb Minimum Bearing Length Required = 2.6 in

Cnasica 9	Number of Matewall Column Studs Per Box								
Species & Grade	Max 8	ft Tall C	olumn	Max 9ft Tall Column					
0.000	2x3	2x4	2x6	2x3	2x4	2x6			
spf #2	4	2	2	NG	2	2			
spf #3	NG	2	2	NG	3	2			
spf stud	NG	2	2	NG	3	2			
sp#2	4	2	1	4	2	2			

BEAM FASTENING REQUIREMENTS

CONDITION 1 - TOP LOADED - 2 TO 4 PLY BEAMS

12" deep or less - (2) rows of 0.131"x3" nails @ 8" o.c.

> 12" & < 18" deep - (3) rows of 0.131"x3" nails @ 8" o.c.

> 18" deep - (4) rows of 0.131"x3" nails @ 8" o.c.

CONDITION 2 - SIDE LOADED

2-Ply <= 465plf (2) rows of 0.131"x3" nails @ 8" o.c.

2-Ply <= 700plf (3) rows of 0.131"x3" nails @ 8" o.c.

2-Ply <= 870plf (2) rows of 1/2" Bolts @ 12" o.c.

2-Ply > 870plf (3) rows of 1/2" Bolts @ 12" o.c.

3-Ply <= 350plf (2) rows of 0.131"x3" nails @ 8" o.c.

3-Ply <= 525plf (3) rows of 0.131"x3" nails @ 8" o.c.

3-Ply <= 650plf (2) rows of 1/2" Bolts @ 12" o.c.

3-Ply > 650plf (3) rows of 1/2" Bolts @ 12" o.c.

4-Ply (3) rows of 1/2" Bolts @ 12" o.c.

Note: Stagger fastener rows and locate fasteners minimum 2" from all ends and edges. Space rows equally apart vertically. Fastener spacing is per row.



JOB 4350 (32307) 1st FLR HEADER

SHEET NO. OF

CALCULATED BY RT DATE 12/2/2014

REVISED

Beam & Header Spans - Non Composite

						SPF	
Beam1 =	2x10	Species1 =	SPF	Grade1 =	NA	No.1/No.2	NA
Beam2 =	None	Species2 =	LVL	Grade2 =	NA	NA	NA
Quantity Pe	r Box Q1 =	2 d1=	9.25	in b1=	3.00 in	Enter Beam w/ L	argest Quantity in Q1

Quantity Per Box, Q2 = 0 d2= 0.00 in b2= 0.00 in

Area, A1 = 27.75 in^2 Section Modulus, S1 = 42.78 in^3 Moment of Inertia, I1 = 197.86 in⁴ Area, A2 = in⁴ 0.00 in^2 Section Modulus, S2 = 0.00 in^3 Moment of Inertia, I2 = 0.00

Load Duration **Beam Stability** Repetitve Factor **Deflection Criteria** Size Factor $C_D =$ 1.00 $C_L =$ 1.00 $C_{F1} =$ 1.10 $C_{R1} =$ 1.00 delta LL = 360 240 $C_{F2} =$ 1.00 $C_{R2} =$ 1.00 delta TL =

Shear Moment Deflection Load Share

1400000 $F_{v1} =$ 135 $F_{b1} =$ 875 E1 =psi psi psi 100.00 % Beam1 = 0 0 0 $F_{v2} =$ $F_{b2} =$ E2 =0.00 % psi psi psi Beam2 =

Controlling Load w = 399.5 plf

Max Allowable Length = 99.5 in Actual Length Needed = 60.5 in

Reaction at Each End = 1007.1 lb Minimum Bearing Length Required = 0.8 in

Species & Grade	Number of Jack Studs Per Box								
	2-	ply Head	der	3-ply Header					
	2x3	2x4	2x6	2x3	2x4	2x6			
spf #2	1	1	1	1	1	1			
spf #3	1	1	1	1	1	1			
spf stud	1	1	1	1	1	1			
sp#2	1	1	1	1	1	1			

BEAM FASTENING REQUIREMENTS

CONDITION 1 - TOP LOADED - 2 TO 4 PLY BEAMS

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> 18" deep - (4) rows of 0.131"x3" nails @ 8" o.c.

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2-Ply > 870plf (3) rows of 1/2" Bolts @ 12" o.c.

3-Ply <= 350plf (2) rows of 0.131"x3" nails @ 8" o.c.

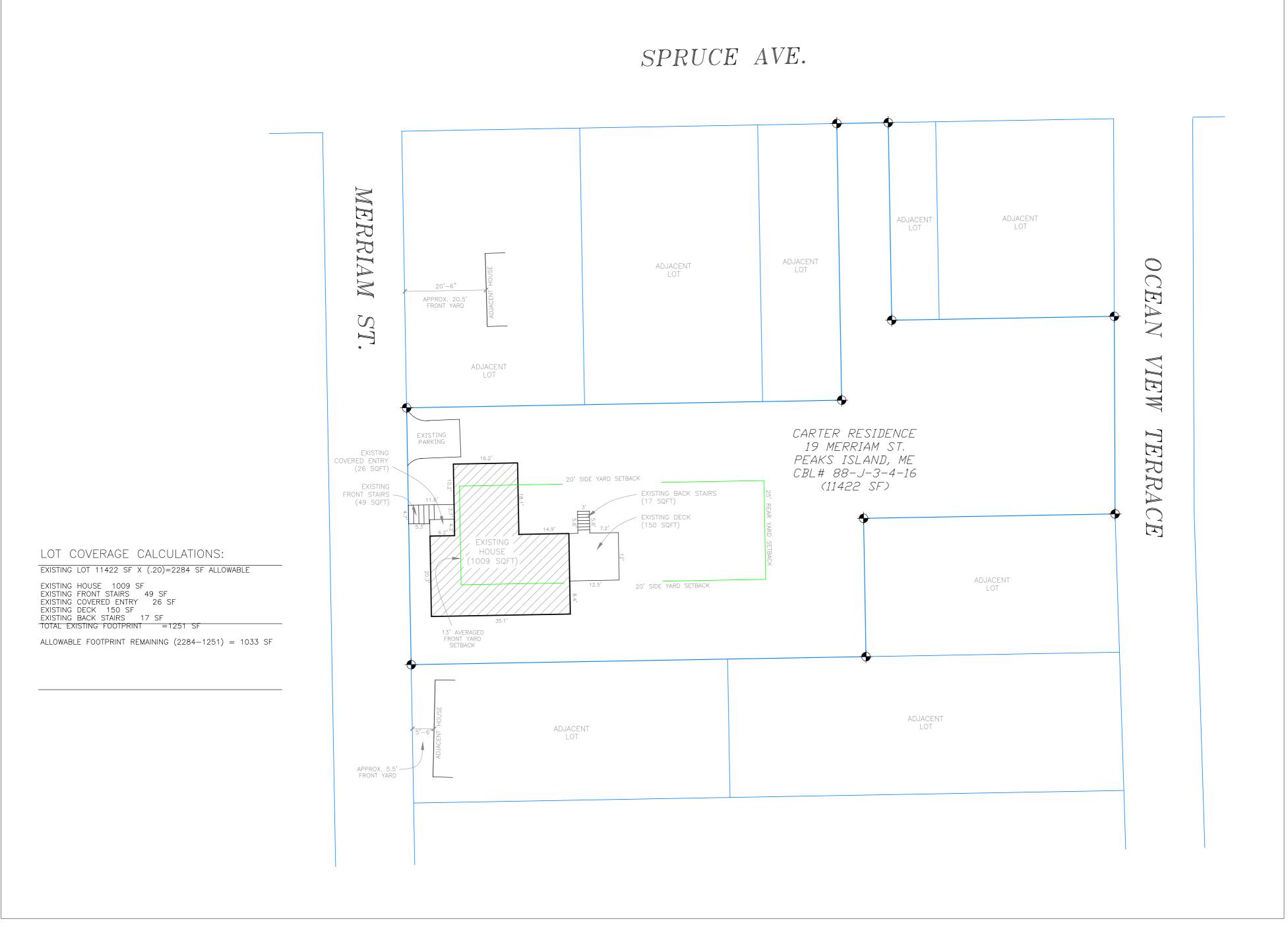
3-Ply <= 525plf (3) rows of 0.131"x3" nails @ 8" o.c.

3-Ply <= 650plf (2) rows of 1/2" Bolts @ 12" o.c.

3-Ply > 650plf (3) rows of 1/2" Bolts @ 12" o.c.

4-Ply (3) rows of 1/2" Bolts @ 12" o.c.

Note: Stagger fastener rows and locate fasteners minimum 2" from all ends and edges. Space rows equally apart vertically. Fastener spacing is per row.





26 Sterling Street
Peaks Island, Maine 04108
207.766.5625

Existing Site Plan

PROJECT

Carter Residence 19 Merriam St. Peaks Island, ME 04106

DATE REVISED

12.20.14

SCALE DRAWN BY

SCALE

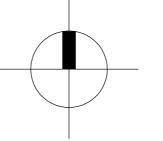
1/16"=1'-0"

| DRAWN BY | Rachel & | Harvey |



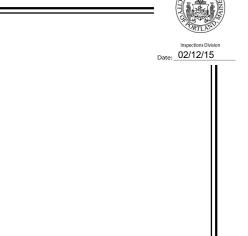
BOUNDARY SURVEY AND LAND AREA BY NORTHEASTERN LAND SURVEYING

A1





SPRUCE AVE.



Rachel Conly Architectural Design

26 Sterling Street Peaks Island, Maine 04108

207.766.5625

Proposed Site Plan

PROJECT

Carter Residence 19 Merriam St. Peaks Island, ME 04106

REVISED 02.10.15

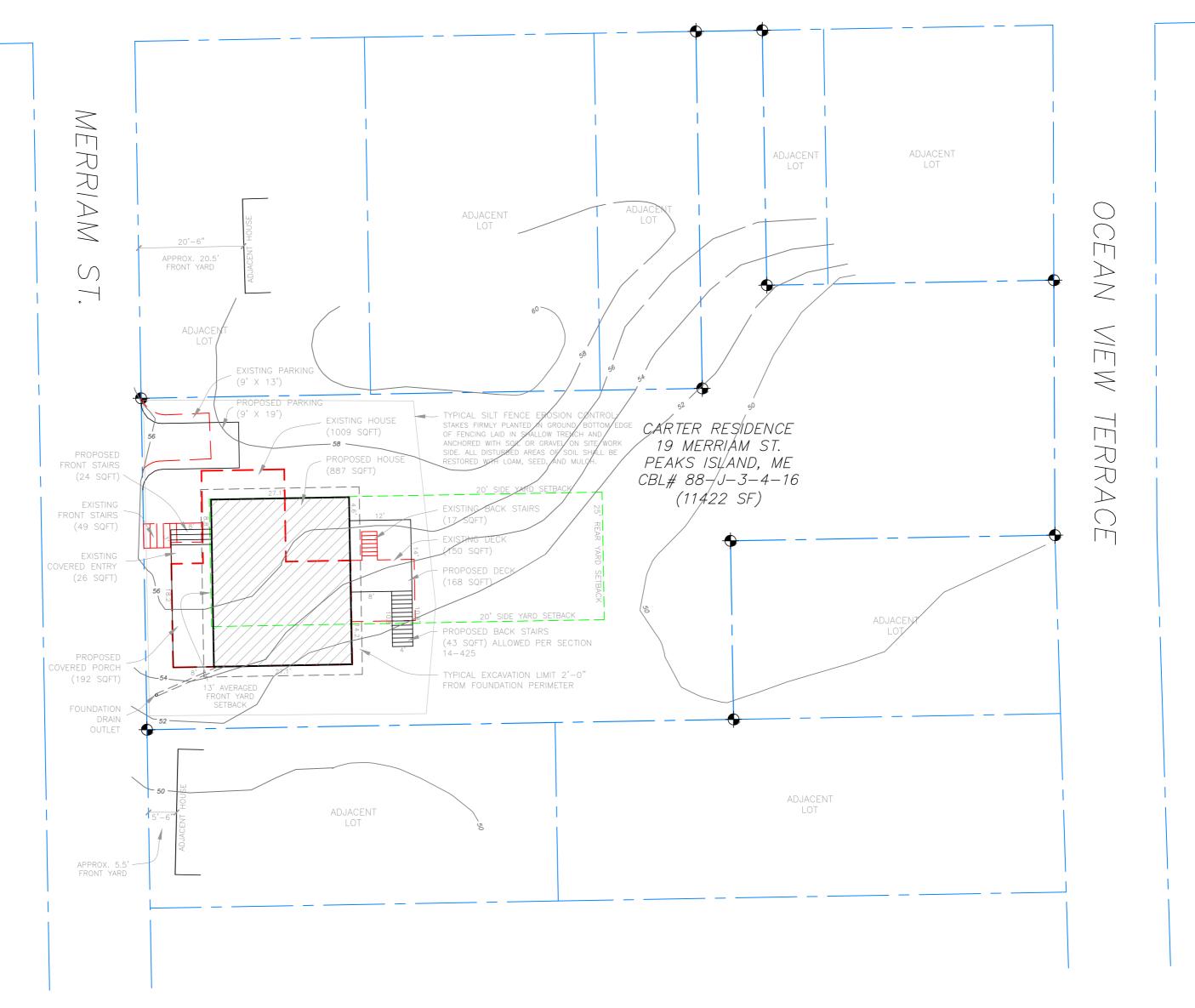
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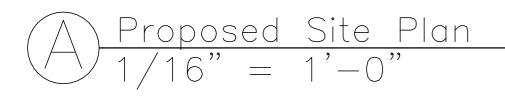
Rachel & Harvey



BOUNDARY SURVEY AND LAND AREA BY NORTHEASTERN LAND SURVEYING

A2





LOT COVERAGE CALCULATIONS: calculations assume total demolition of all existing structure

ALLOWABLE FOOTPRINT REMAINING (2284-1314) = 970 SF

EXISTING LOT 11422 SF X (.20)

EXISTING HOUSE EXISTING FRONT STAIRS EXISTING COVERED ENTRY

TOTAL EXISTING FOOTPRINT

PROPOSED HOUSE PROPOSED FRONT STAIRS PROPOSED COVERED PORCH

TOTAL PROPOSED FOOTPRINT

PROPOSED DECK
PROPOSED BACK STAIRS

EXISTING BACK STAIRS

EXISTING DECK

=2284 SF ALLOWABLE

1009 SF 49 SF 26 SF 150 SF

=1251 SF

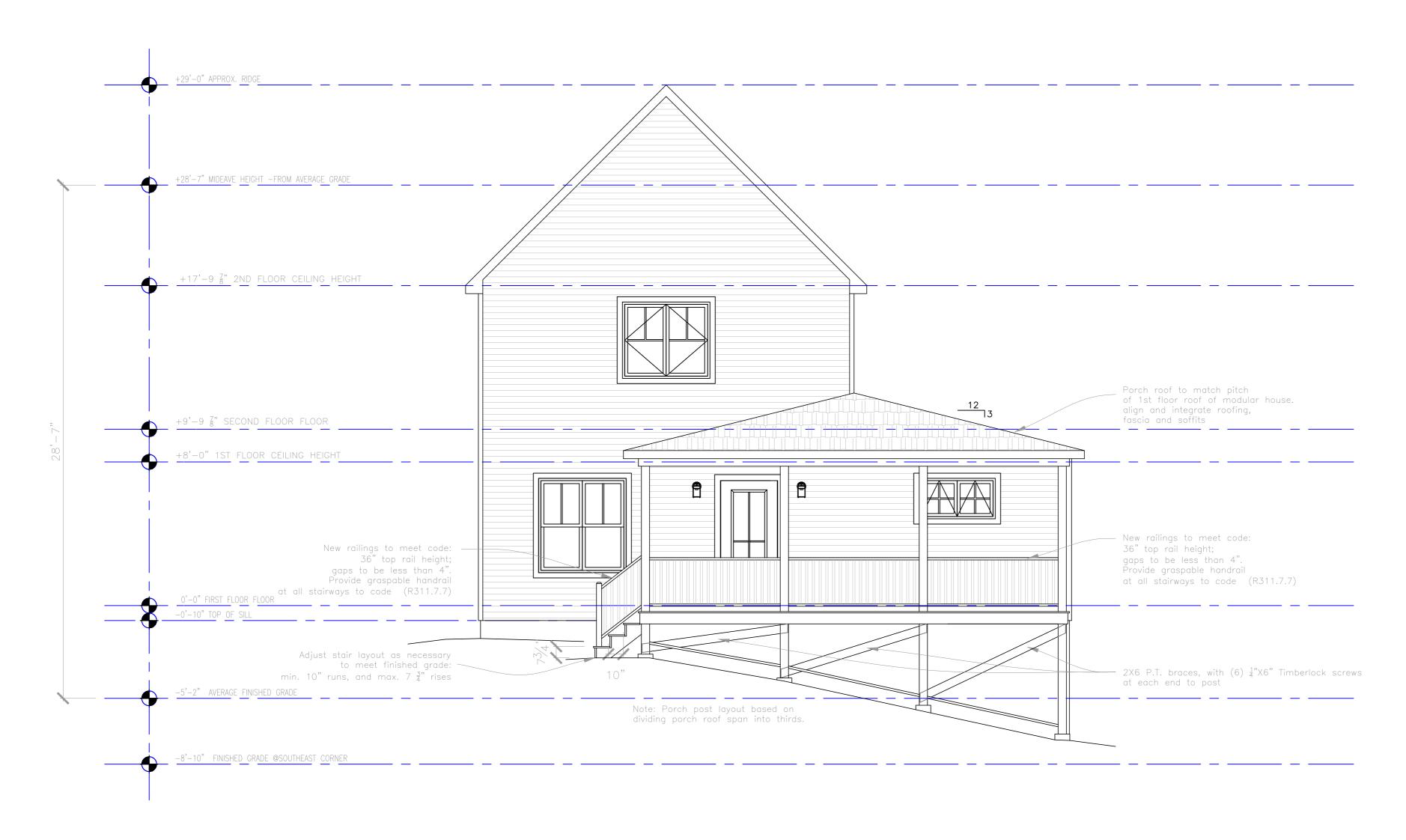
887 SF 24 SF 192 SF

168 SF











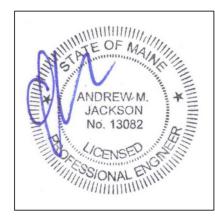


Rachel Conly

Architectural Design

26 Sterling Street Peaks Island, Maine 04108 207.766.5625

Proposed
West
Porch/Deck
Elevation



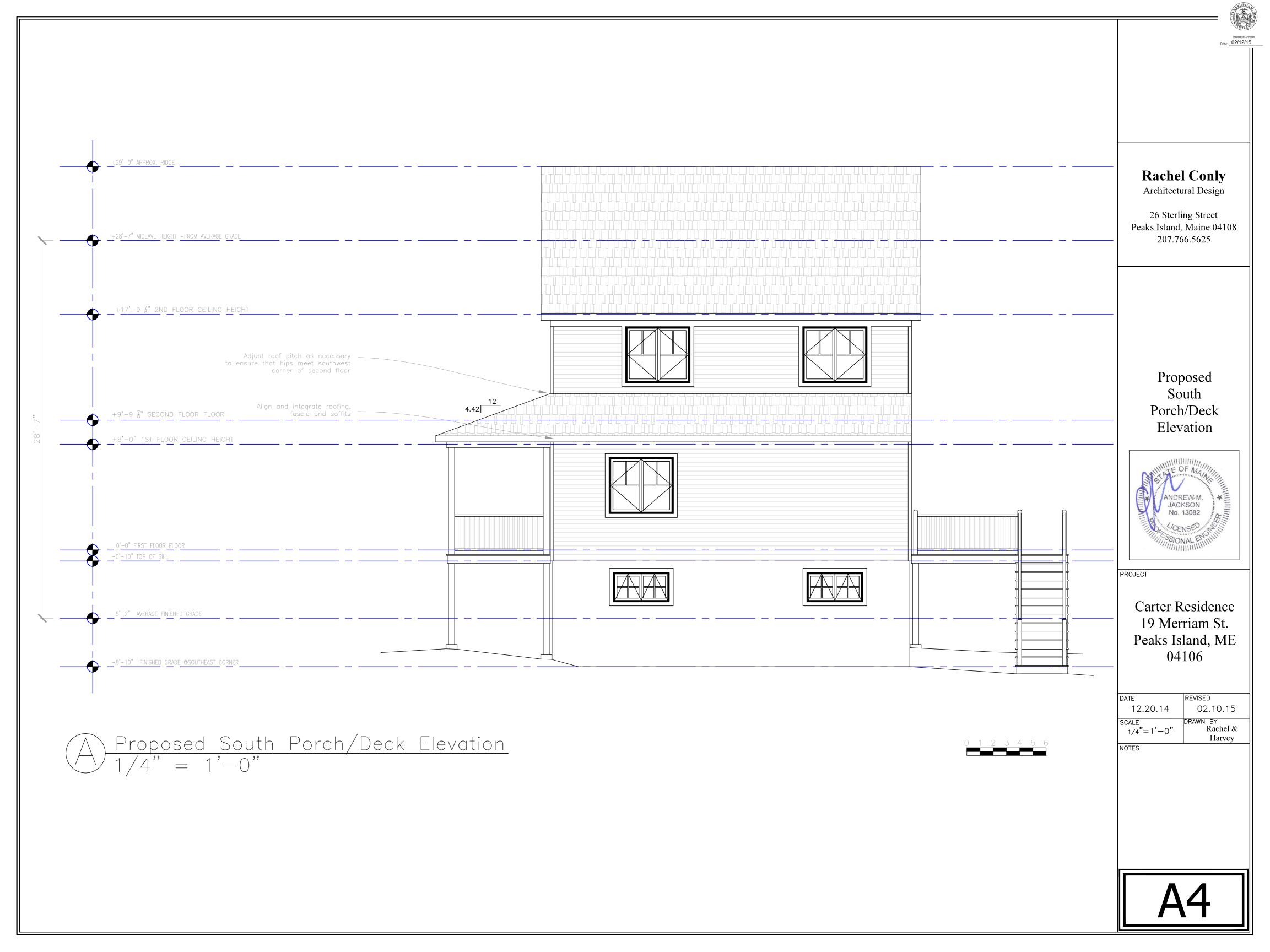
PROJECT

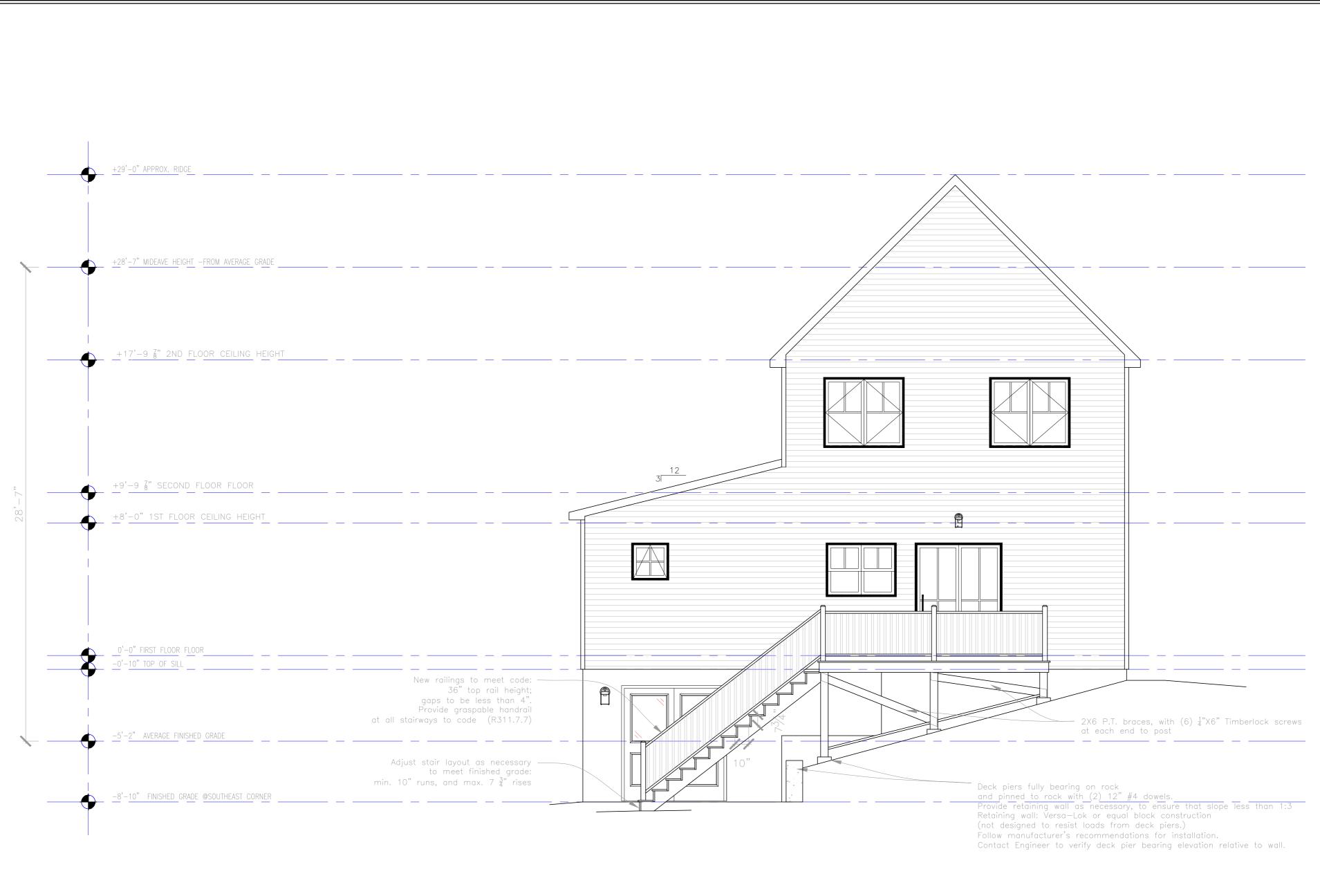
Carter Residence 19 Merriam St. Peaks Island, ME 04106

DATE	REVISED
12.20.14	02.10.15
SCALE 1/4"=1'-0"	DRAWN BY Rachel & Harvey

NOTE

A3







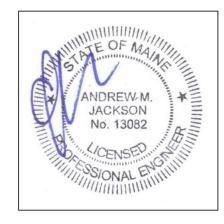


Rachel Conly

Architectural Design

26 Sterling Street Peaks Island, Maine 04108 207.766.5625

Proposed
East
Porch/Deck
Elevation



PROJECT

Carter Residence 19 Merriam St. Peaks Island, ME 04106

DATE	REVISED
12.20.14	02.10.15
SCALE 1/4"=1'-0"	DRAWN BY Rachel &

NOTES

A5





General Building Permit Application

If you or the property owner owes real estate or personal property taxes or user charges on any property within the City, payment arrangements must be made before permits of any kind are accepted.

Address/Location of Construction: 19 Merriam St.			
Total Square Footage of Proposed Structure:		1323 sq.ft	
Tax Assessor's Chart, Block & Lot Chart# Block# Lot# 88-J-3-4-16	Address 9 Fry St. City, State 8		Telephone: 207.772.7750 Email: bcpeaks@aol.com
Lessee/Owner Name : (if different than applicant) Address:	Portland, ME 04103 Contractor Name: (if different from Applicant) Address: 619 Lewiston Rd.		Cost Of Work: \$ 225,000 C of O Fee: \$
City, State & Zip: Telephone & E-mail:	City, State & Zip: Topsham, Maine 04086 Telephone & E-mail:		Historic Rev \$ Total Fees: \$
Current use (i.e. single family) single family If vacant, what was the previous use? n/a Proposed Specific use: single family residential (no change) Is property part of a subdivision? If yes, please name n/a Project description: Rebuild home which was destroyed by fire, within existing non-conforming footprint and setbacks			
Who should we contact when the permit is re	eady:Rachel	Conly, or Harvey Johnson	
Address: 26 Sterling St.			
City, State & Zip: Peaks Island, ME			
E-mail Address: rachelconlydesign@gmail.com and harvey.rachelconlydesign@gmail.com			
Telephone: 207.766.5625			

Please submit all of the information outlined on the applicable checklist. Failure to do so causes an automatic permit denial.

In order to be sure the City fully understands the full scope of the project, the Planning and Development Department may request additional information prior to the issuance of a permit. For further information or to download copies of this form and other applications visit the Inspections Division on-line at www.portlandmaine.gov, or stop by the Inspections Division office, room 315 City Hall or call 874-8703.

I hereby certify that I am the Owner of record of the named property, or that the owner of record authorizes the proposed work and that I have been authorized by the owner to make this application as his/her authorized agent. I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in this application is issued, I certify that the Code Official's authorized representative shall have the authority to enter all areas covered by this permit at any reasonable hour to enforce the provisions of the codes applicable to this permit.

Signature: Rachel Conly	Date: 12.24.14
Signature: Tablier Colly	Date: 12.27.17





PORTLAND MAINE

Strengthening a Remarkable City, Building a Community for Life • www.portlandmaine.gov

Jeff Levine, AICP, Director Director of Planning and Urban Development Tammy Munson Director, Inspections Division

Electronic Signature and Fee Payment Confirmation

Notice: Your electronic signature is considered a legal signature per state law.

By digitally signing the attached document(s), you are signifying your understanding this is a legal document and your electronic signature is considered a *legal signature* per Maine state law. You are also signifying your intent on paying your fees by the opportunities below.

I, the undersigned, intend and acknowledge that no permit application can be reviewed until payment of appropriate permit fees are *paid in full* to the Inspections Office, City of Portland Maine by method noted below:

4	Within 24-48 hours, once my complete permit appraperwork has been electronically delivered, I intend to care 207-874-8703 and speak to an administrative representationard over the phone.	all the Inspections Office at
	Within 24-48 hours, once my permit application and combeen electronically delivered, I intend to hand deliver Inspections Office, Room 315, Portland City Hall.	1 011
	I intend to deliver a payment method through the U.S. Popermit paperwork has been electronically delivered.	ostal Service mail once my
Applicant Sig	gnature: Rachel A. Conly	Date: 12.24.14
I have provid	ed digital copies and sent them on:	Date: 12.24.14

NOTE: All electronic paperwork must be delivered to <u>buildinginspections@portlandmaine.gov</u> or by physical means ie; a thumb drive or CD to the office.



Residential Additions/Alterations Permit Application Checklist



All of the following information is required and must be submitted. Checking off each item as you prepare your application package will ensure your package is complete and will help to expedite the permitting process.

The Maine Home Construction Contracts Act requires that any home construction or repair work for more than \$3000. in materials or labor must be based on a written contract unless the parties agree to exempt themselves. A sample contract is available on the City's website at www.portlandmaine.gov, in the Inspection Office, Room 315 of Portland City Hall or call (207)874-8703 to have one mailed to you.

One (1) complete set of construction drawings must include:

Floor plans and elevations existing & proposed

Detail removal of all partitions & any new structural beams

Cross sections w/framing details

incomplete, the application may be refused.

	Detail any new walls or permanent partitions
	Stair details including dimensions of: rise/run, head room, guards/handrails, baluster spacing
	Window and door schedules
	Foundation plans w/required drainage and damp proofing (if applicable)
	Detail egress requirements and fire separation/sound transmission ratings (if applicable)
	Insulation R-factors of walls, ceilings & floors & U-factors of windows per the IEEC 2009
	Deck construction including: pier layout, framing, fastenings, guards, stair dimensions
	Electronic files in pdf format are also required
	Proof of ownership is required if it is inconsistent with the assessors records
_	ate permits are required for internal & external plumbing, HVAC, and electrical installations
f the	ate permits are required for internal & external plumbing, HVAC, and electrical installations are any additions to the footprint or volume of the structure, any new or rebuilt ures or, accessory detached structures a plot plan is required. A plot must include:
f the	The shape and dimension of the lot, footprint of the existing and proposed structure and the distance from the actual property lines. Structures include decks, porches; bow windows, cantilever sections and roof overhangs, sheds, pools, garages and any other accessory structures must be shown with dimensions if not to scale.
If the	te are any additions to the footprint or volume of the structure, any new or rebuilt ures or, accessory detached structures a plot plan is required. A plot must include: The shape and dimension of the lot, footprint of the existing and proposed structure and the distance from the actual property lines. Structures include decks, porches; bow windows, cantilever sections and roof overhangs, sheds, pools, garages and any other accessory

Permit Fee: \$30.00 for the first \$1000.00 construction cost, \$10.00 per additional \$1000.00 cost

Please submit all of the information outlined in this application checklist. If the application is

Department may request additional information prior to the issuance of a permit. For further information visit us on-line at www.portlandmaine.gov, stop by the Building Inspections office, room 315 City Hall or call 874-8703.

In order to be sure the City fully understands the full scope of the project, the Planning and Development

This is not a Permit; you may not commence any work until the Permit is issued.



Building Inspections - Fwd: Re: Carter Residence, 19 Merriam St., Peaks Island

From: Laurie Leader

Inspections, Building; Messinger, Craig; Pirone, Chris To:

12/30/2014 8:12 AM Date:

Subject: Fwd: Re: Carter Residence, 19 Merriam St., Peaks Island

Pre-manufactured homes are not required to be sprinkled thus this is fast trackable.

>>> Building Inspections 12/30/2014 7:30 AM >>>

Could anyone chime in on this? It's about the ability to fast track and she didn't provide anything regarding sprinklers. Is what she is stating accurate?

-Brad.

>>> Rachel Conly <rachelconlydesign@gmail.com> 12/29/2014 4:41 PM >>> Hi,

Thank you for your reply to our submission.

This project is unusual in that it is modular construction and because the original structure burned in 2013. As a result, the new building was exempt from sprinklers by Captain Pirone of the Portland City Fire Department in a conversation we had on March 21st, 2014. I also had confirmation from Laurie Leader on December 19th that the third party design professional stamp from PFS and the engineering stamps from Andrew Jackson, LE, on the drawings meet the "Fast Track" requirements.

I hope this helps clarify things on your end and gets us back on track for a "Fast Track" review. Please let me know if you have any further questions.

Thank you!

On Mon, Dec 29, 2014 at 2:03 PM, Building Inspections < building inspections@portlandmaine.gov > wrote:

This is not fast trackable as requested. As stated on the form, the only item that can be fast tracked is:

New

Sprinklered One and Two Family Homes (bearing the seal of a licensed design professional stating code compliance) So you will need to provide all documents as stamped and a sprinkler system application as well. We will review this permit as normal until we receive the appropriate document(s).

>>> harvey johnson <harvey.rachelconlydesign@gmail.com> 12/24/2014 11:10 AM >>> Hello.

Please find attached a permit application, a fast-track form, and the associated drawings for the proposed rebuilding of the Carter residence.

Please note that this permit submission is unique, as it is for a burned structure, and was deemed not to need



a Level 1 Minor Residential Application, as it is an existing developed site, and we are rebuilding within existing non-conforming footprint and set backs.

Also, the drawings provided are from two sources: The manufactured home company has provided stamped drawings detailing the house itself. And then on our end, we have provided the siteplans, as well as the porch and deck elevations, and stamped structural drawings for the foundation, porch and deck.

Please let us know if you have any questions.

Thank you,

Harvey Johnson Rachel Conly Residential Design 26 Sterling Street Peaks Island, Maine 04108

207-766-5625

http://rachelconlyresidentialdesign.virb.com/

Notice: Under Maine law, documents - including e-mails - in the possession of public officials or city employees about government business may be classified as public records. There are very few exceptions. As a result, please be advised that what is written in an e-mail could be released to the public and/or the media if requested.

--

Rachel Conly

Residential Design, LEED AP 26 Sterling Street Peaks Island, Maine 04108

207-766-5625

http://rachelconlyresidentialdesign.virb.com/



Building Inspections - Carter Residence, 19 Merriam St., Peaks Island

From: harvey johnson harvey.rachelconlydesign@gmail.com

To: <buildinginspections@portlandmaine.gov>

Date: 12/24/2014 11:13 AM

Subject: Carter Residence, 19 Merriam St., Peaks Island CC: Rachel Conly <rachelconlydesign@gmail.com>

Attachments: Carter Permit Application 12.24.14.pdf; Carter Fast Track Form.pdf; S3 Proposed Porch

Roof Framing Plan.pdf; S2 Proposed Deck & Porch Floor Framing Plan.pdf; S1 Proposed Foundation Plan.pdf; A5 Proposed East Porch & Deck Elevation.pdf; A4 Proposed South Porch & Deck Elevation.pdf; A3 Proposed West Porch & Deck Elevation.pdf; A2 Proposed Site Plan.pdf; A1 Existing Site Plan.pdf; 4350 (ME)

Approved Permit Set (12-12-14).PDF

Hello,

Please find attached a permit application, a fast-track form, and the associated drawings for the proposed rebuilding of the Carter residence.

Please note that this permit submission is unique, as it is for a burned structure, and was deemed not to need a Level 1 Minor Residential Application, as it is an existing developed site, and we are rebuilding within existing non-conforming footprint and set backs.

Also, the drawings provided are from two sources: The manufactured home company has provided stamped drawings detailing the house itself. And then on our end, we have provided the siteplans, as well as the porch and deck elevations, and stamped structural drawings for the foundation, porch and deck.

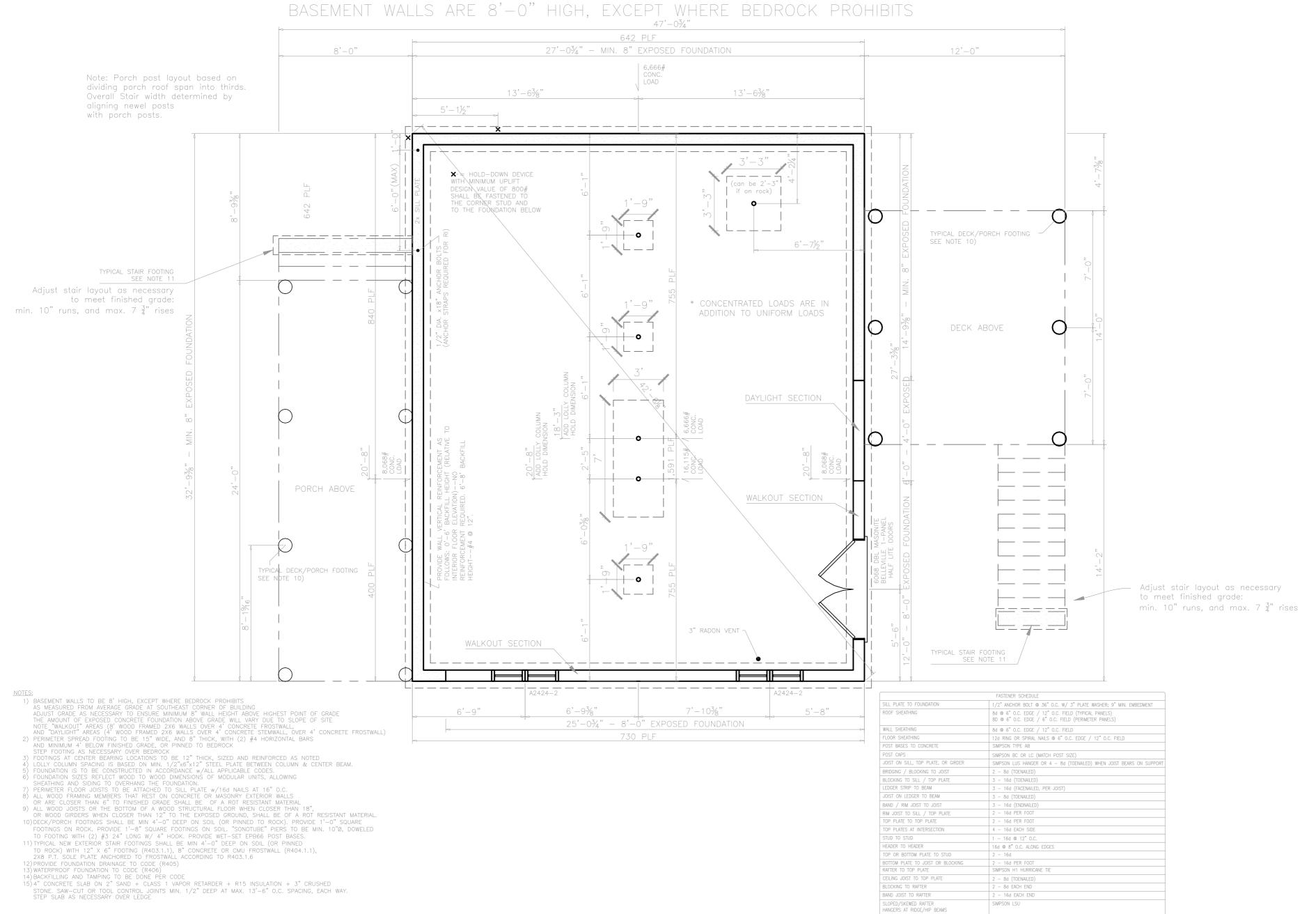
Please let us know if you have any questions.

Thank you,

Harvey Johnson Rachel Conly Residential Design 26 Sterling Street Peaks Island, Maine 04108

207-766-5625

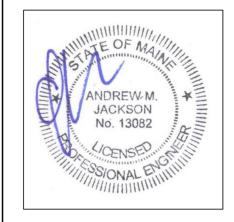
http://rachelconlyresidentialdesign.virb.com/



Rachel Conly
Architectural Design

26 Sterling Street Peaks Island, Maine 04108 207.766.5625

> Proposed Foundation Plan



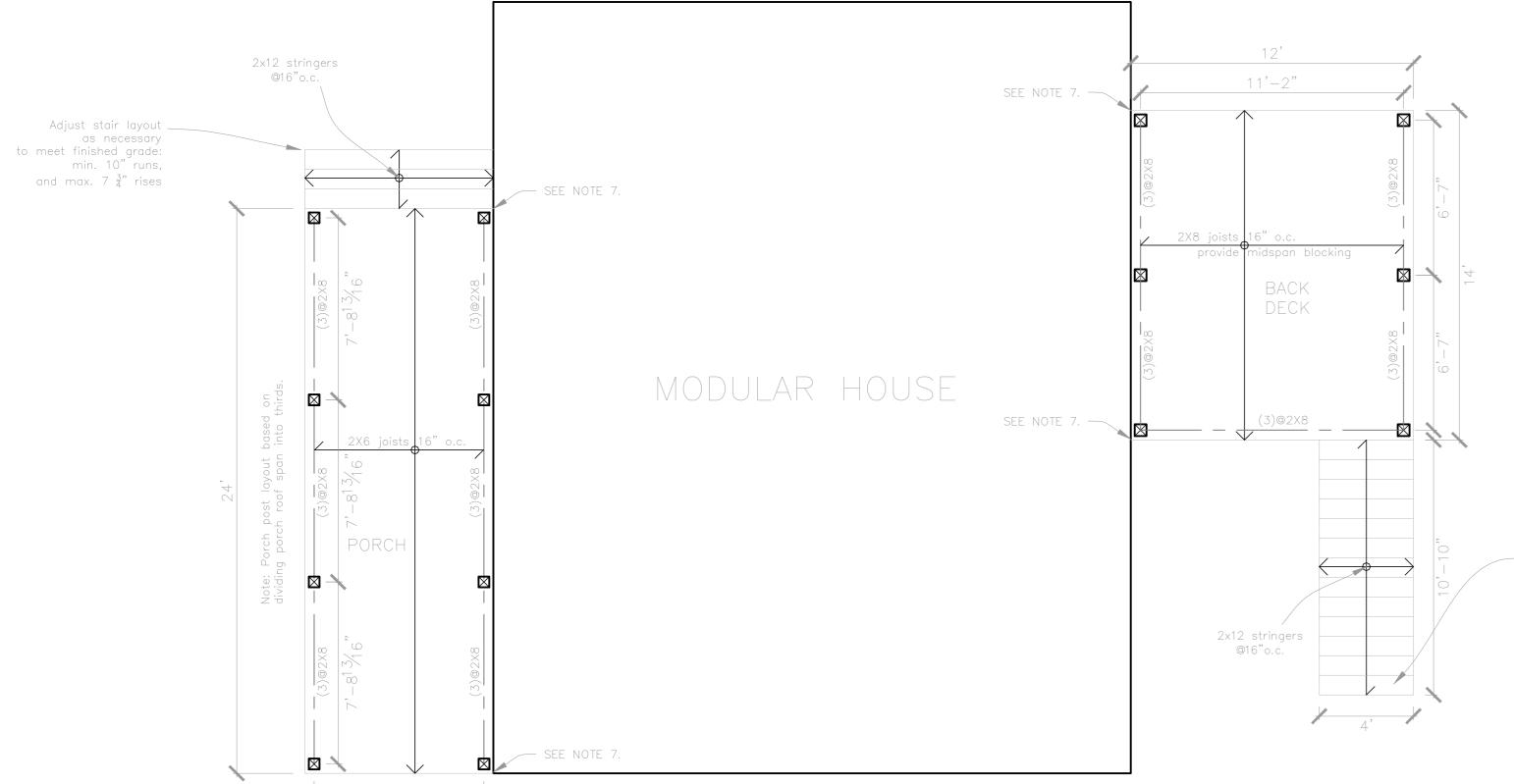
PROJECT

Carter Residence 19 Merriam St. Peaks Island, ME 04106

DATE	REVISED
12.20.14	02.10.15
SCALE 1/4"=1'-0"	DRAWN BY Rachel & Harvey

NOTES





1.)ALL WOOD FRAMING MEMBERS THAT REST ON CONCRETE OR MASONRY EXTERIOR WALLS OR ARE LESS THAN 8" FROM THE EXPOSED GROUND SHALL BE OF A ROT RESISTANT MATERIAL

2.)ALL WOOD JOISTS OR THE BOTTOM OF A WOOD STRUCTURAL FLOOR WHEN CLOSER THAN 18", OR WOOD GIRDERS WHEN CLOSER THAN 12", TO THE EXPOSED GROUND, SHALL BE OF A ROT REISTANT MATERIAL.

3.) NEW WOOD SIDING AND/OR SHEATHING TO BE ABOVE GRADE A MINIMUM OF 6" OR ELSE OF A ROT RESISTANT MATERIAL.

4.) ALL DECK/STAIR FRAMING/DECKING TO BE OF ROT RESISTANT WOOD.

5.) DECK/PORCH FLOOR BRACING: SÉE ELEVATIONS

6.) HANG JOISTS FROM GIRDERS WITH LU26-Z

7.) PROVIDE SIMPSON DTT2Z HOLDDOWN AT EDGE (NORTH AND SOUTH) DECK JOIST PERPENDICULAR TO CONCRETÉ WALL. EPOXY 1/2" THREADED ROD 6" INTO CONCRETE WALL (ALT.: THROUGH WALL W/ WASHER PLATE). CONNECT THREADED ROD TO HOLDDOWN PER MANUFACTURER'S DETAILS. 4 LOCATIONS TOTAL.

Adjust stair layout as necessary to meet finished grade: min. 10" runs, and max. $7\frac{3}{4}$ " rises

pressure treated 6X6 secured to footing with AB66

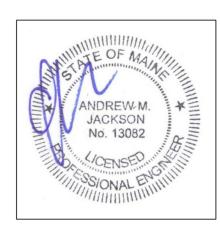
SILL PLATE TO FOUNDATION	1/2" ANCHOR BOLT @ 36" O.C. W/ 3" PLATE WASHER; 9" MIN. EMBEDMENT
ROOF SHEATHING	8d @ 6" O.C. EDGE / 12" O.C. FIELD (TYPICAL PANELS) 8D @ 6" O.C. EDGE / 6" O.C. FIELD (PERIMETER PANELS)
WALL SHEATHING	8d @ 6" O.C. EDGE / 12" O.C. FIELD
FLOOR SHEATHING	12d RING OR SPIRAL NAILS @ 6" O.C. EDGE / 12" O.C. FIELD
POST BASES TO CONCRETE	SIMPSON TYPE AB
POST CAPS	SIMPSON BC OR LC (MATCH POST SIZE)
JOIST ON SILL, TOP PLATE, OR GIRDER	SIMPSON LUS HANGER OR 4 - 8d (TOENAILED) WHEN JOIST BEARS ON SUF
BRIDGING / BLOCKING TO JOIST	2 - 8d (TOENAILED)
BLOCKING TO SILL / TOP PLATE	3 - 16d (TOENAILED)
LEDGER STRIP TO BEAM	3 - 16d (FACENAILED, PER JOIST)
JOIST ON LEDGER TO BEAM	3 - 8d (TOENAILED)
BAND / RIM JOIST TO JOIST	3 - 16d (ENDNAILED)
RIM JOIST TO SILL / TOP PLATE	2 - 16d PER FOOT
TOP PLATE TO TOP PLATE	2 - 16d PER FOOT
TOP PLATES AT INTERSECTION	4 - 16d EACH SIDE
STUD TO STUD	1 - 16d @ 12" O.C.
HEADER TO HEADER	16d @ 8" O.C. ALONG EDGES
TOP OR BOTTOM PLATE TO STUD	2 - 16d
BOTTOM PLATE TO JOIST OR BLOCKING	2 - 16d PER FOOT
RAFTER TO TOP PLATE	SIMPSON H1 HURRICANE TIE
CEILING JOIST TO TOP PLATE	2 - 8d (TOENAILED)
BLOCKING TO RAFTER	2 - 8d EACH END
BAND JOIST TO RAFTER	2 - 16d EACH END
SLOPED/SKEWED RAFTER HANGERS AT RIDGE/HIP BEAMS	SIMPSON LSU

Rachel Conly

Architectural Design

26 Sterling Street Peaks Island, Maine 04108 207.766.5625

> Proposed Deck & Porch Floor Framing Plan



PROJECT

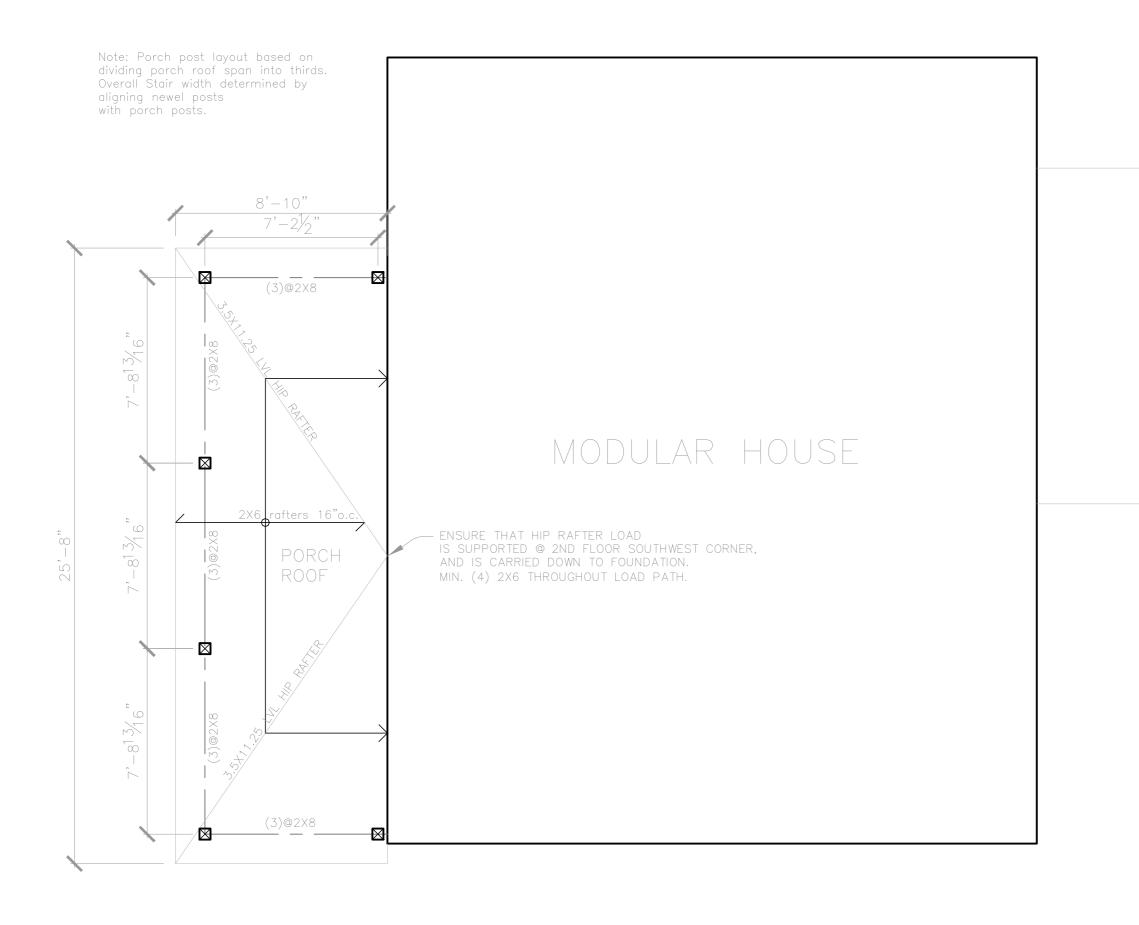
Carter Residence 19 Merriam St. Peaks Island, ME 04106

DATE	REVISED
12.20.14	02.10.15
SCALE 1/4"=1'-0"	DRAWN BY Rachel & Harvey



7'-2½"





NOTES: 1.)ALL WOOD FRAMING MEMBERS THAT RÉST ON CONCRETE OR MASONRY EXTERIOR WALLS OR ARE LESS THAN 8" FROM THE EXPOSED GROUND SHALL BE OF A ROT RESISTANT MATERIAL

2.)ALL WOOD JOISTS OR THE BOTTOM OF A WOOD STRUCTURAL FLOOR WHEN CLOSER THAN 18", OR WOOD GIRDERS WHEN CLOSER THAN 12", TO THE EXPOSED GROUND, SHALL BE OF A ROT REISTANT MATERIAL.

3.) NEW WOOD SIDING AND/OR SHEATHING TO BE ABOVE GRADE A MINIMUM OF 6" OR ELSE OF A ROT RESISTANT MATERIAL.

4.) ALL DECK/STAIR FRAMING/DECKING TO BE OF ROT RESISTANT WOOD.

pressure treated 6X6

FASTENER SCHEDULE		
SILL PLATE TO FOUNDATION	1/2" ANCHOR BOLT @ 36" O.C. W/ 3" PLATE WASHER; 9" MIN. EMBEDMENT	
ROOF SHEATHING	8d @ 6" O.C. EDGE / 12" O.C. FIELD (TYPICAL PANELS) 8D @ 6" O.C. EDGE / 6" O.C. FIELD (PERIMETER PANELS)	
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FLOOR SHEATHING	12d RING OR SPIRAL NAILS @ 6" O.C. EDGE / 12" O.C. FIELD	
POST BASES TO CONCRETE	SIMPSON TYPE AB	
POST CAPS	SIMPSON BC OR LC (MATCH POST SIZE)	
JOIST ON SILL, TOP PLATE, OR GIRDER	SIMPSON LUS HANGER OR 4 - 8d (TOENAILED) WHEN JOIST BEARS ON SUPPO	
BRIDGING / BLOCKING TO JOIST	2 - 8d (TOENAILED)	
BLOCKING TO SILL / TOP PLATE	3 - 16d (TOENAILED)	
LEDGER STRIP TO BEAM	3 - 16d (FACENAILED, PER JOIST)	
JOIST ON LEDGER TO BEAM	3 - 8d (TOENAILED)	
BAND / RIM JOIST TO JOIST	3 - 16d (ENDNAILED)	
RIM JOIST TO SILL / TOP PLATE	2 - 16d PER FOOT	
TOP PLATE TO TOP PLATE	2 - 16d PER FOOT	
TOP PLATES AT INTERSECTION	4 - 16d EACH SIDE	
STUD TO STUD	1 - 16d @ 12" O.C.	
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BLOCKING TO RAFTER	2 - 8d EACH END	
BAND JOIST TO RAFTER	2 - 16d EACH END	
SLOPED/SKEWED RAFTER HANGERS AT RIDGE/HIP BEAMS	SIMPSON LSU	

Rachel Conly

Architectural Design

26 Sterling Street Peaks Island, Maine 04108 207.766.5625

> Proposed Porch Roof Framing Plan



PROJECT

Carter Residence 19 Merriam St. Peaks Island, ME 04106

DATE	REVISED
12.20.14	02.10.15
SCALE 1/4"=1'-0"	DRAWN BY Rachel &



 $\frac{\text{Proposed Porch Roof Framing Plan}}{1/4" = 1'-0"}$

