GENERAL NOTES:

1. THE NOTES ON THESE DRAWINGS ARE NOT INTENDED TO REPLACE THE SPECIFICATIONS. SEE SPECIFICATIONS FOR REQUIREMENTS IN ADDITION TO THE GENERAL NOTES. INCONSISTENCIES BETWEEN THE DRAWINGS AND THE SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER PRIOR TO PROCEEDING WITH THE AFFECTED PORTION OF THE WORK.

2. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUCTION WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL AND SITE DRAWINGS. G.C. SHALL COORDINATE LOCATIONS AND DIMENSIONS OF OPENINGS, CHASES, INSERTS, REGLETS, SLEEVES, DEPRESSIONS, ETC.

3. ALL DIMENSIONS AND COORDINATES SHALL BE FIELD VERIFIED. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH THE AFFECTED PORTION OF THE WORK.

4. THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE BUILDING IS COMPLETE. IT IS THE CONTRACTOR'S SOLE REPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO ENSURE THE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS, OR TIEDOWNS. SUCH MATERIAL SHALL REMAIN THE PROPERTY OF THE CONTRACTOR AFTER COMPLETION OF THE PROJECT.

5. SECTIONS AND DETAILS SHOWN ON ANY STRUCTURAL DRAWINGS SHALL BE CONSIDERED TYPICAL FOR SIMILAR CONDTIONS.

6. THE CONTRACTOR SHALL PERFORM ALL WORK IN CONFORMANCE WITH ALL APPLICABLE FEDERAL, STATE AND MUNICIPAL REGULATIONS.

DESIGN NOTES:

1. THIS BUILDING IS DESIGNED TO COMPLY W/ THE 2009 EDITION OF THE INTERNATIONAL BUILDING CODE AND ASCE 7-05, "MINIMUM DESIGN LOADS FOR BUILDINGS & OTHER STRUCTURES".

2. DEAD LOADS = ACTUAL WEIGHTS OF COMPONENTS PLUS 5 PSF ALLOWANCE FOR MISCELLANEIOUS DUCTWORK, SPRINKLER PIPING AND OTHER HUNG ITEMS.

3. DESIGN FOR SNOW LOAD IS IN ACCORDANCE WITH ASCE 7.
   A. GROUND SNOW LOAD PG = 60 PSF
   B. FLAT ROOF SNOW LOAD PF = 50.4 PSF
   C. SNOW LOAD IMPORTANCE FACTOR I =1.0
   D. SNOW EXPOSURE FACTOR CE = 1.0
   E. SNOW THERMAL FACTOR CT = 1.2
   F. SNOW DRIFTING IN ACCORDANCE WITH ASCE 7

4. DESIGN FOR WIND LOAD IS IN ACCORDANCE WITH ASCE 7,
   A. BASIC WIND SPEED = 99 MPH
   B. WIND LOAD IMPORTANCE FACTOR I = 1.0
   C. WIND EXPOSURE = EXPOSURE C
   D. WIND INTERNAL PRESSURE COEFFICIENT GCPI = ±0.18
   E. DESIGN WIND LOADS:
   F. COMPONENTS AND CLADDING IN CANOPY CONSTRUCTION
      33 PSF UPLIFT ON TOP SURFACE.
      20 PSF UPWARDS ON BOTTOM SURFACE

CONCRETE NOTES:

1. ALL CONCRETE WORK SHALL CONFORM TO ACI 318-08 AND 301-05. CEMENT SHALL COMPLY WITH ASTM C150 TYPE II; AGGREGATES (NORMAL WEIGHT) SHALL COMPLY WITH ASTM C33.

2. CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS SHALL BE 4000 PSI WITH MOISTURE VAPOR REDUCTION ADMIXTURE (VAPOR LOCK 20/20). SUBMIT DESIGN MIX FOR REVIEW AND APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD. INCLUDE MATERIAL CERTIFICATES FOR CEMENTITIOUS MATERIALS, ADMIXTURES AND AGGREGATES. FOLLOW ALL MVRA MANUFACTURER'S REQUIREMENTS FOR WARRANTY, AND PROVIDE WARRANTY TO OWNER.

3. ALL CONCRETE EXPOSED TO FREEZING SHALL HAVE A 6% AIR CONTENT (+/-1%), ALL THE OTHER CONCRETE SHALL BE N0N-AIR ENTRAINED, WITH 3.0% MAXIMUM AIR CONTENT.

4. CONSOLIDATE CONCRETE WITH MECHANICALLY VIBRATING EQUIPMENT IN ACCORDANCE WITH ACI 301; CONTRACTOR IS TO HAVE TWO (2) OPERATIONAL VIBRATORS AT THE WORK SITE TO SAFEGUARD AGAINST MECHANICAL FAILURE DURING PLACEMENT.

5. REINFORCING BARS SHALL CONFORM TO ASTM A-615 GRADE 60 DEFORMED BARS AND SHALL BE DETAILED, FABRICATED, AND PLACED IN ACCORDANCE WITH ACI 315-LATEST EDITION. REINFORCING SHALL BE INSPECTED PRIOR TO PLACEMENT OF CONCRETE. SPLICES OF REINFORCING BARS SHALL BE IN ACCORDANCE WITH ACI 318

6. REINFORCEMENT SHALL HAVE THE FOLLOWING COVER:
   A. CAST AGAINST EARTH: 3"
   B. EXPOSED TO EARTH AND/OR WEATHER:#5 AND SMALLER: 1 1/2"

7. CONCRETE SHALL BE TESTED BY THE CONTRACTOR'S AGENT. TESTING SHALL BE PERFORMED IN ACCORDANCE WITH ASTM C172. COPIES OF TEST REPORT SHALL BE SENT TO THE OWNER AND THE PROJECT STRUCTURAL ENGINEER.
   A. OBTAIN ONE COMPOSITE SAMPLE FOR EACH DAY'S PLACEMENT EXCEEDING 5 CUBIC YARDS;
   B. TEST MIXTURE SLUMP AT POINT OF PLACEMENT ACCORDING TO ASTM C143, RETEST IF CONCRETE CONSISTENCY APPEARS TO CHANGE.
   C. TEST AIR ENTRAINMENT ACCORDING TO ASTM C231; PERFORM ONE TEST PER COMPOSITE SAMPLE.
   D. COMPRESSION TEST: ASTM C39, TEST ONE SET OF TWO CURED SPECIMENS AT 7 DAYS AND ONE SET OF TWO CURED SPECIMENS AT 28 DAYS.
   E. PERFORM ADDITIONAL TESTS AS DIRECTED BY STRUCTURAL ENGINEER OF RECORD WHEN TESTING INDICATES CONCRETE REQUIREMENTS HAVE NOT BEEN MET. ADDITIONAL TESTING SHALL BE AT CONTRACTOR'S EXPENSE.

8. CORRECT ALL DEFICIENCIES IN THE WORK THAT TEST REPORTS OR INSPECTIONS INDICATE DO NOT COMPLY WITH THE CONSTRUCTION DOCUMENTS.

9. EMBEDDED ITEM INSTALLATION: PLACE AND SECURE ANCHORAGE DEVICES AND OTHER EMBEDDED ITEMS REQUIRED FOR ADJOINING WORK THAT IS ATTACHED TO OR SUPPORTED BY CAST-IN-PLACE CONCRETE. USE SETTING DRAWINGS, TEMPLATES, DIAGRAMS, INSTRUCTIONS, AND DIRECTIONS FURNISHED WITH ITEMS TO BE EMBEDDED.

10. CONCRETE PLACEMENT: BEFORE PLACING CONCRETE, VERIFY THAT INSTALLATION OF FORMWORK, REINFORCEMENT, AND EMBEDDED ITEMS IS COMPLETE AND THAT REQUIRED INSPECTIONS ARE COMPLETED. DO NOT ADD WATER TO CONCRETE DURING DELIVERY, AT PROJECT SITE, OR DURING PLACEMENT UNLESS APPROVED BY ARCHITECT. BEFORE TEST SAMPLING AND PLACING CONCRETE, WATER MAY BE ADDED AT PROJECT SITE, SUBJECT TO LIMITATIONS OF ACI 301. DO NOT ADD WATER TO CONCRETE AFTER ADDING HIGH-RANGE WATER-REDUCING ADMIXTURES TO MIXTURE. DEPOSIT CONCRETE CONTINUOUSLY IN ONE LAYER OR IN HORIZONTAL LAYERS OF SUCH THICKNESS THAT NO NEW CONCRETE IS PLACED ON CONCRETE THAT HAS HARDENED ENOUGH TO CAUSE SEAMS OR PLANES OF WEAKNESS. IF A SECTION CANNOT BE PLACED CONTINUOUSLY, PROVIDE CONSTRUCTION JOINTS AS INDICATED. DEPOSIT CONCRETE TO AVOID SEGREGATION.

11. AFTER 7 DAYS OF CURING, THE FLOOR SHALL BE CLEANED OF ALL DIRT, OIL, AND OTHER FOREIGN MATTER. FLOORS THAT RECEIVE A SEALER SHALL BE SEALED AT THIS TIME.

12. ISOLATION JOINTS SHALL HAVE 3/8" PREFORMED, CLOSED-CELL FOAM JOINT MATERIAL. THE TOP 1/2" OF THE JOINT SHALL BE FILLED WITH POLYURETHANE SEALANT.

13. EXISTING SOILS SHALL BE COMPACTED WITH A PLATE COMPACTOR AS REQUIRED TO REACH 95% OF MDD. THE TOP SURFACE OF THE BASE SHALL BE LEVEL TO WITHIN 3/8" OF DESIGN GRADE. MDOT TYPE C FILL SHALL BE USED WHERE ADDITIONAL MATERIAL IS NEEDED.

14. SLABS SHALL BE REINFORCED WITH 4X4-W2.9XW2.9 PLACED AT MID-DEPTH, UNLESS OTHER REINFORCING IS INDICATED. REINFORCEMENT

STRUCTURAL STEEL NOTES:

1. STRUCTURAL STEEL FARICATION, ERECTION, AND CONNECTION DESIGN SHALL CONFORM TO AISC "SPECIFICATION FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL" - THIRTEENTH EDITION.

2. STRUCTURAL STEEL WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A572, GRADE 50 OR ASTM A992 STEEL WITH A YIELD STRENGTH OF 50 KSI. STEEL FOR PLATES, ANGLES, AND CHANNELS SHALL CONFORM TO ASTM A36 WITH A YIELD STRENGTH OF 36 KSI. COLD FORM HOLLOW STRUCTURAL SECTIONS ASTM A500 GRADE B WITH A MINIMUM YIELD STRENGTH OF 46 KSI.

3. FIELD CONNECTIONS SHALL BE BOLTED USING 3/4" DIAMETER A325N HIGH STRENGTH BOLTS EXCEPT WHERE FIELD WELDING IS INDICATED ON THE DRAWINGS OR WHERE OTHERWISE REQUIRED TO SUPPORT DESIGN LOADS.

4. ALL WELDING SHALL CONFORM TO AWS D1.1-LATEST EDITION. ELECTRODES SHALL BE E70XX.

5. STEEL FRAMING MEMBERS AND COMPONENTS THAT REQUIRE FIREPROOFING PER THE ARCHITECTURAL DRAWINGS SHALL NOT BE PRIMED. REMAINDER OF STEEL SHALL BE PAINTED IN ACCORDANCE WITH THE SPECIFICATIONS.

6. DO NOT APPLY SHOP PRIMER TO THE TOP OF BEAM TOP FLANGES SPECIFIED TO RECEIVE FIELD WELDED SHEAR CONNECTORS.

7. CONNECTIONS NOT DETAILED WITHIN THE DRAWINGS SHALL BE DESIGNED BY THE FABRICATOR UNDER THE RESPONSIBLE CHARGE OF AN ENGINEER LICENSED IN THE STATE WHERE THE BUILDING IS TO BE BUILT. WHERE A DETAIL IS REQUIRED TO SPECIFY FIELD ATTACHMENT, THAT DETAIL SHALL BE PROVIDED ON THE ERECTION DRAWINGS.

8. SHOP DRAWINGS DETAILING FABRICATION AND ERECTION OF EACH METAL FABRICATION INDICATED SHALL BE SUBMITTED TO THE PROJECT ENGINEER FOR REVIEW PRIOR TO FABRICATION AND CONSTRUCTION.

METAL DECK NOTES:

1. STEEL ROOF DECK SHALL BE 1 1/2" DEEP X20 GA. WIDE RIB (TYPE B) GALVANIZED (G60) ROOF DECK. VULCRAFT 1.5B20, SINGLE SPAN CONDITION IS THE BASIS OF DESIGN.

2. ROOF DECK SHALL BE FASTENED AS FOLLOWS, UNLESS OTHERWISE SHOWN IN THE DRAWINGS.
   AT SUPPORTS: HILTI X-ENP-19 L15 PINS, 36/4 PATTERN
   AT SIDE LAPS: HILTI SLC SCREW AT 30" O.C. (MAX)
   AT PERIMETER SUPPORTS: HILTI X-ENP-19 L15 PINS @ 6" O.C. (MAX)

3. SHOP DRAWINGS FOR STEEL DECKING SHALL BE SUBMITTED TO THE PROJECT ENGINEER FOR REVIEW PRIOR TO FABRICATION AND CONSTRUCTION.

LOOSE LINTEL NOTES:

MASONRY OPENINGS	LINTEL SIZE
6'-0" AND LESS	L4X4X5/16

1. PROVIDE 8" MINIMUM BEARING AT EACH END OF ALL LINTELS.

2. ALL EXTERIOR LINTELS SHALL BE HOT-DIP GALVANIZED. ALL INTERIOR LINTELS SHALL BE PRIME PAINTED.

3. REFER TO ARCHITECTURAL DRAWINGS FOR LOCATIONS. DO NOT CUT OR REMOVE ANY EXISTING EXTERIOR WALL KICKERS OR BRACES. ADJUST OPENING LOCATIONS AS REQUIRED.

TYP LINTEL DETAIL AT MASONRY VENEER (UNO)

NOTE: -SEE ARCHITECTURAL DETAILS FOR ADDITIONAL LINTEL INFORMATION.
(*) EXTEND PL TO FACE OF STUD OR FACE OF CMU.

REV	DESCRIPTION	DATE
0	PERMIT SET	7-1-16

PERMIT SET
7-1-16
CURRENT ISSUE STATUS:

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MERCY FORE RIVER MOB

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STRUCTURAL GENERAL NOTES

SHEET TITLE:

SCALE: AS NOTED

PROJECT MANAGER:	KD	PROJECT NO:	16087
A/E OF RECORD:	ADB		
JOB CAPTAIN:	SHK		
DRAWN BY:	DSF		
SMRT FILE:	SG001-16087	SHEET No.	SG001

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Date: 09/06/16