

Report on Subsurface and Foundation Investigation

Proposed Accessory Building Portland, Maine

for

PropSys, Inc. P.O. Box 660 Lewiston, ME 04243-0660

March 5, 2010



Engineering Expertise You Can Build On

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March 5, 2010 09492

Chris Thompson PropSys, Inc. P.O. Box 660 Lewiston, ME 04243-0660

Report on Subsurface and Foundation Investigation Proposed Accessory Building, Portland, Maine

Dear Chris:

This report presents the results of our subsurface and foundation investigation for the proposed accessory building at 501 Danforth Street in Portland, Maine. These services were completed in accordance with our proposal dated December 30, 2009.

In summary, it is our opinion that the building may be supported on spread and continuous footings bearing on undisturbed, naturally deposited sand and clay or on compacted structural fill placed after removal of unsuitable soils. In addition, an earth-supported slab-on-grade may be used for the lowest floor. Specific recommendations regarding foundation design and construction considerations are presented below.

Introduction

The site is located at 501 Danforth Street in Portland. It presently consists of a paved parking lot. We understand the proposed building will have plan dimensions of 60 feet by 100 feet, located within a portion of the paved parking lot. Ground surface elevations within the building limits vary from approximately El. 42.0 to El. 45.0. The building will have the lowest floor level at approximately El. 45.0 with a mezzanine in one corner. It will have a steel braced frame with steel roof joists. Column loads will vary from approximately 10 kips at the corners to 56 kips in the interior.

Subsurface Explorations

On February 15, 2010 Maine Test Borings, Inc. (MTB) drilled four borings, B1 to B4, at the site at locations shown on Sheet 1, Boring Plan. MTB drilled the borings to depths below ground surface varying from 8.5 feet to 17.4 feet. Sebago Technics, Inc. monitored the borings and prepared the logs included in Appendix A. Table I summarizes the results of borings. MTB backfilled the borings with the drilled material and placed a bituminous concrete patch at the surface.

Borings were drilled using 3.0-inch diameter solid stem augers. Samples were generally recovered at five feet intervals. Standard penetration resistance (N) was measured at each sample in accordance with ASTM procedures. Borings were drilled to refusal judged to be bedrock.

Sebago Technics determined the locations of borings by pacing from existing site features.

The boring logs and related information depict the subsurface conditions and water levels encountered at the locations and during the times indicated on the logs. Subsurface conditions at other locations may differ from those encountered in the borings. The passage of time may result in a change in groundwater conditions at the borings.

Subsurface Conditions

The borings disclosed five principal soil units below bituminous concrete at the site: fill, clay, sand, glacial till and weathered bedrock. Encountered thickness and generalized descriptions of the strata encountered are presented below in order of increasing depth below ground surface.

Fill – Fill consists of medium dense; gray brown well-graded GRAVEL (GW); to poorly-graded SAND (SP). Encountered thickness varies from 1.3 feet to 3.8 feet.

Marine Clay – Marine clay consists of stiff to soft, gray brown to gray lean CLAY (CL). Boring B2 encountered 12.0 feet of clay.

Marine Sand – Marine sand consists of loose to medium dense, brown to gray poorly-graded SAND (SP); to silty SAND (SM). Encountered thickness varies from 6.0 feet to 9.0 feet.

Glacial Till – Glacial till consists of loose to medium dense, gray silty SAND with gravel (SM). Encountered thickness varies from 0.5 foot to 5.7 feet.

Weathered Bedrock – Weathered bedrock consists of bedrock that has been weathered to sand, gravel and cobble size pieces of rock fragments. Encountered thickness varies from 0.1 foot to 0.4 foot.

All borings terminated in refusal on what is judged to be bedrock.

Groundwater was encountered in B2 at a depth of 8.6 feet below ground surface. Groundwater was not encountered in the other borings. However, observations of groundwater were made over a relatively short period of time and may not reflect the stabilized groundwater level. In addition, water levels at the site will vary with season, precipitation, temperature and construction activity in the area. Therefore, water levels during and following construction will vary from those observed in the borings.

Strength and Compressibility Characteristics of Clay Stratum

We estimated the stress history of the clay deposit by correlations with strength ratio, the ratio of shear strength to overburden stress, of similar clays in the area. The undrained shear strength of the clay stratum was estimated by correlations with the N value. Estimated shear strength varies from 1,500 pounds per square foot (psf) at 6 feet to 500 psf at 11 feet below ground surface. The

stress history of the deposit was estimated by comparing the strength ratio with correlations of strength ratio and stress history of clay from other projects with similar conditions.

The stress-strain or compressibility characteristics (settlement) of clays are highly dependent upon their stress history. If clay is stressed within the limits of the maximum previous stress, σ_{vm} , the strain (settlement) will be a function of the recompression ratio (RR) of the clay. If the applied stress exceeds the maximum previous stress, the strain will be proportional to both the recompression ratio and the virgin compression ratio (CR). The compression ratio is typically 10 to 15 times the recompression ratio.

The stress history and appropriate compression ratios were estimated for the clay deposit as discussed above. The correlations indicate that the deposit is moderately overconsolidated, that is, the existing overburden stress is at least 1,000 psf less than the maximum previous. The deposit likely became overconsolidated due to desiccation (drying) resulting from a lowering of the groundwater level for an extended period at some time in the geologic past which created a stiff upper crust and also increased the effective overburden stress throughout the stratum.

Recommendations for Foundation Design

Recommended Foundation Type and Design Criteria

The bituminous concrete and fill are not considered suitable for support of the building. All bituminous concrete and existing fill should be removed from within the foundation limits. In our opinion, the building may be supported on spread and continuous footings bearing on undisturbed, naturally-deposited clay or sand or on compacted structural fill placed after removal of unsuitable soil or for raises-in-grade.

Footings may be proportioned for an allowable bearing stress in pounds per square foot (psf) equal to 800 multiplied by the least lateral dimension of the footing in feet up to a maximum of 2,500 psf. All footings should be a minimum of 1.5 feet wide.

Exterior footings should be founded at least 4.5 feet below the lowest adjacent ground surface exposed to freezing. Interior footings should be founded a minimum of 1.5 feet below the ground floor slab.

Compacted structural fill supporting footings should extend laterally from the footings to at least the limits defined by 1 horizontal to 1 vertical lines sloped outward and downward from points located at least 1 foot horizontally beyond the bottom edges of the footings.

In order to consider foundations in the northwest corner bearing above the clay stratum we estimated the settlement of the clay resulting from the increased stress from the column load, ground floor slab and raise-in-grade. Loading information was provided by Structural Design Consulting. We estimate that the total settlement in the northwest corner will be on the order of 0.5 inch or less with differential settlement on the order of 0.4 inch in 40 feet. We estimate that this settlement will occur within 5 years of completion of construction. We anticipate that settlement of this magnitude is acceptable. However, Structural Design Consulting should determine final acceptability of settlement.

Ground Floor Slab

We recommend that the lowest level floor slab be designed as an earth-supported slab-on-grade bearing on a minimum of 6 inches of compacted structural fill. All bituminous concrete and fill containing organics or debris should be removed from within the building limits prior to placing fill. All fill placed below the floor slab for raise-in-grade should consist of compacted structural fill, Normal dampproofing and vapor barriers should be provided for the slab.

In general, the existing fill consists of gravel subbase and poorly-graded sand. In our opinion, the existing fill is suitable to remain in-place following proofrolling. The subgrade should be proofrolled using fully-loaded ten-wheel dump trucks or equivalent equipment. Any soft or yielding soil should be excavated and replaced with compacted structural fill prior to placing fill for raise-in-grade.

We recommend that the lowest level floor slab be designed with a modulus of subgrade reaction of 200 pounds per cubic inch.

Seismic Design Considerations

We understand that the City of Portland requires design using the 2003 edition of the International Building Code (IBC). We recommend that the building be designed in accordance with the seismic requirements of the 2003 edition of the IBC. The site classification is Class D based on a calculation of the weighted average of overburden strength in the top 100 feet of the site; the site response coefficient F_a is 1.5 for a short period spectral response acceleration S_s of 0.375g; the site response coefficient F_v is 2.4 for the 1-second period spectral response acceleration S_1 of 0.10g. The subgrade soils are not considered liquefaction susceptible.

Lateral Foundation Loads

We recommend that lateral loads be resisted by bottom friction on footings and that a coefficient of friction equal to 0.35 be used for footings. If this does not provide sufficient lateral resistance, we will consider the problem in more detail to take into account other factors.

Backfill Materials

Structural fill used below foundations and floor slabs and for backfill adjacent to walls should consist of sandy gravel to gravelly sand. It should be free of organic material, loam, trash, snow, ice, frozen soil and other objectionable material, and should conform to the following gradation:

Sieve Size	Percent Finer by Weight
6 inches	100
No. 4	30 to 90
No. 40	10 to 50
No. 200	0 to 8

Compacted structural fill should be placed in layers not exceeding eight inches in loose measure and compacted by self propelled vibratory equipment at the approximate optimum moisture content to a dry density of at least 95 percent of the maximum dry density, as determined in

accordance with ASTM Test Designation D1557. In confined areas, the maximum particle size should be reduced to 3 inches and the loose layer thickness should be reduced to 6 inches and compaction performed by hand-guided vibratory equipment.

Compacted structural fill on the exterior of the foundation walls should extend laterally a minimum of 2 feet from the wall. Backfill beyond this limit on the exterior of the building may consist of common fill. The top 12 inches of fill on the exterior of the building should consist of low permeability material to minimize water infiltration next to the building. Grading should provide for runoff away from the building.

Common fill may consist of inorganic mineral soil that can be placed in layers and compacted. Common fill should be placed and spread in layers not exceeding 12 inches in thickness and compacted at the approximate optimum moisture content to a dry density of at least 92 percent of the maximum dry density, as determined in accordance with ASTM Test Designation D1557.

Pavement Section

We recommend the following pavement section for roadway and parking areas:

Roadway and Automobile Parking Areas

- 3 inches bituminous concrete, placed in two layers
- 3 inches screened or crushed gravel base course
- 15 inches sand or gravel subbase course

Base and subbase course materials should conform to the following gradations:

Base Course

<u>Screened or Crushed Gravel</u> (Maine DOT Standard Specification, Highways and Bridges; Section 703.06a, Type A)

Sieve Size	Percent Finer by Weight
2 inches	100
1/2 inch	45 to 70
1/4 inch	30 to 55
No. 40	0 to 20
No. 200	0 to 5

Subbase Course

Sand or Gravel (Maine DOT, Section 703.06b, Type D)

Sieve Size	Percent Finer by Weight
4 inches	100
1/4 inch	25 to 70
No. 40	0 to 30
No. 200	0 to 7

(Note: Type D aggregate should be modified to a maximum 4 inch size. Compacted structural fill may be substituted for gravel subbase course).

All fill containing debris should be removed from within the limits of pavement.

Subbase course material should be placed in maximum 8-inch thick loose lifts and compacted at approximately optimum moisture content to a dry density of at least 95 percent of maximum dry density, as determined in accordance with ASTM Test Designation D1557. Base course material should be placed in one lift and compacted with a minimum of two coverages with self-propelled vibratory compaction equipment.

Fill below the pavement section for raises-in-grade may consist of common fill. Common fill may consist of inorganic mineral soil that can be placed in layers and compacted. Common fill should be placed and spread in layers not exceeding 12 inches in thickness and compacted at the approximate optimum moisture content to a dry density of at least 92 percent of the maximum dry density, as determined in accordance with ASTM Test Designation D1557. In our opinion, the existing clay is not suitable for common fill.

It should be noted that the subgrade soils may be frost-susceptible. Therefore, pavement roughness due to non-uniform frost movement may occur. To eliminate such non-uniform frost movement would require approximately 4.5 feet of structural fill subbase. However, it is common practice to tolerate seasonal movement to avoid the high cost of the added thickness of subbase.

Construction Considerations

General

The primary purpose of this section of the report is to comment on items related to excavation, earthwork and related geotechnical aspects of proposed construction. It is written primarily for the engineer having responsibility for preparation of plans and specifications. Since it identifies potential construction problems related to foundations and earthwork, it will also aid personnel who monitor the construction activity. The contractor must evaluate the construction problems on the basis of their own knowledge and experience in the Portland, Maine area, and on the basis of similar projects in other localities, taking into account their proposed construction methods, procedures, equipment and personnel.

Excavation, Lateral Support and Control of Water

We anticipate that foundation excavation can be accomplished with sloped open excavation through the overburden soils provided safe side slopes can be maintained. Some sloughing and raveling should be anticipated in temporary slopes. Temporary excavations should be made in accordance with all OSHA and other applicable regulatory agency requirements.

Groundwater may be encountered at footing excavations. We anticipate that groundwater and water from other sources can be controlled by sumps and open pumping. In general, the contractor should control groundwater and water from runoff and other sources by methods which prevent disturbance of bearing surfaces or adjacent soils and allow construction in-the-dry.

Subgrade Preparation

The subgrade soil is susceptible to disturbance from construction traffic. Equipment and personnel should not be permitted to travel across exposed footing bearing surfaces or exposed slab subgrades. Any subgrade areas that are disturbed should be recompacted or excavated and replaced with compacted structural fill or fabric and crushed stone prior to placing concrete. Subgrades should be protected against freezing temperatures if exposed during construction. Final excavation to subgrade should be performed using equipment with smooth-edge buckets.

Construction Monitoring

The foundation recommendations contained herein are based on the known and predictable behavior of a properly engineered and constructed foundation. Monitoring of the foundation construction is required to enable the geotechnical engineer to keep in contact with procedures and techniques used in construction. Therefore, we recommend that a person qualified by training and experience be present to provide monitoring at the site during excavation, preparation of foundation bearing surfaces, and placement of compacted structural fill.

Limitations of Recommendations

This report has been prepared for specific application to the subject project in accordance with generally accepted geotechnical engineering practices. In the event that any changes in the nature, design or location of the building is planned, the conclusions and recommendations contained in this report should not be considered valid, unless the changes are reviewed and the conclusions of this report modified or verified in writing.

The recommendations presented herein are based in part on the data obtained from the referenced borings. The nature and extent of variations between the borings may not become evident until construction. If variations then appear evident, it will be necessary to reevaluate the recommendations of this report.

We request that we be provided the opportunity for a general review of final design and specifications in order to determine that our earthwork and foundation recommendations have been interpreted and implemented in the design and specifications as they were intended.

It has been a pleasure to work with you on this project. Please do not hesitate to contact us if you have any questions or need additional information.

RECKER

Sincerely,

SEBAGO TECHNICS, INC.

Kenneth L. Recker, P.E.

Geotechnical Engineering Manager

KLR:klr/dlf

Enclosures:

Table I - Summary of Borings

Sheet 1 - Boring Plan

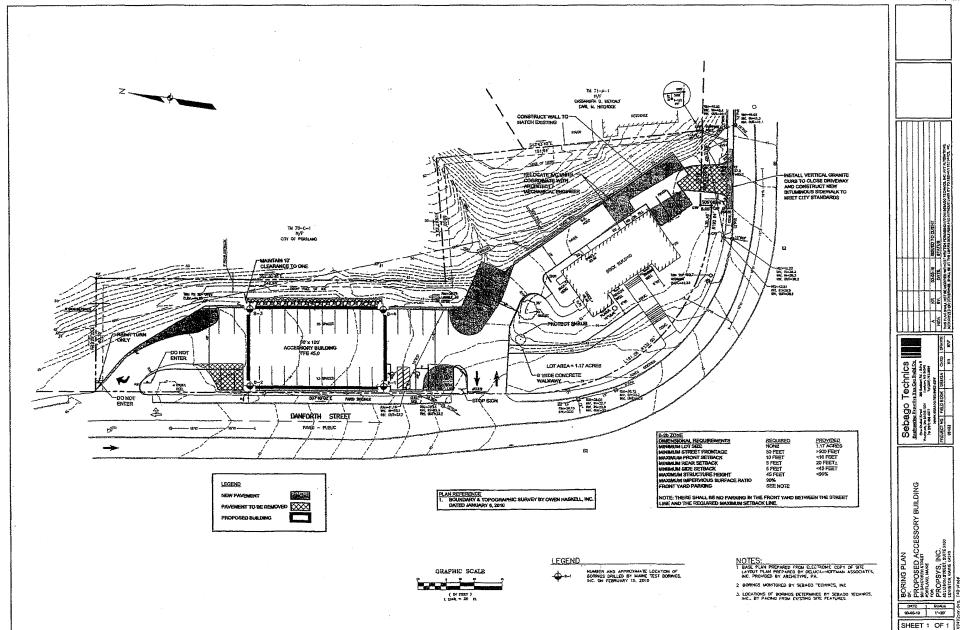
Appendix A - Logs of Borings

TABLE I SUMMARY OF BORINGS PROPOSED ACCESSORY BUILDING 501 DANFORTH STREET PORTLAND, MAINE

Boring	Depth	Depth to				Strata T	hickness (Ft)		
Number	(Ft)	Water (Ft)	Bituminous	Fill	Clay	Sand	Glacial Till	Weathered Rock	Bedrock
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B2	17.4	8.6	0.2	3.8	12.0		1.0	0.4	0.0*
B3	16.3	NE	0.2	1.3		9.0	5.7	0.1	0.0*
B4	8.5	NE	0.2	1.8		6.0	0.5		0.0*

NOTES:

- 1. NE INDICATES GROUNDWATER NOT ENCOUNTERED WITHIN DEPTH OF BORING.
- 2. INDICATES STRATUM NOT ENCOUNTERED WITHIN DEPTH OF BORING.
- 3. * INDICATES DEPTH OF PENETRATION INTO STRATUM.



Appendix A

Logs of Borings

PROJECT PROPOSED ACCESSORY BUILDING STI JOB NO. 09492 LOCATION 501 DANFORTH STREET, PORTLAND, MAINE PROJECT MGR. K. RECKER CLIENT PROPSYS, INC. FIELD REP. K. B. STEPHENSON CONTRACTOR MAINE TEST BORINGS, INC. DATE STARTED 2/15/2010 DRILLER R. LEONARD DATE FINISHED 2/15/2010 Elevation ft. Datum Boring Location See Plan Item Casing Sampler Core Barrel Rig Make & Model Mobile B53 Hammer Type Drilling Mud Casing Advance	SEBAG TECHN INC,		TEST BORING REPORT														B1	NO.		
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G Geoprobe EXT Concrete BORING NO. B1 Fleid Tests Dilatancy: R - Rapid S - Slow N - None Plasticity: N - Nonplastic L - Low M - Medium H - High Dry Strength: N - None L - Low M - Medium H - High Dry Strength: N - None L - Low M - Medium H - High V - Very High *NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.	302000		ļ	Casing			U	Undisturbed Sample	Cuttings			-							_	
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SEBAGO TECHNI		TEST BORING REPORT														BORING NO. B2 Page 1 of									
INC. PROJECT LOCATIO CLIENT CONTRAC	N	PROPSYS,	ORTH STRE INC ST BORING	ET, PORTL	NG AND, MAIN	IE .				PRO. FIEL DATE	JOB NO. JECT MGR. D REP. E STARTED E FINISHED		1		2 ECK ST 201										
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Hammer W Hammer F		_	140 30		∏ Tra				oller Bil adling Head	Drillin	Automatic ng Notes:	V	·······································	vone							-				
Tianinio 7		Sample		19075788557								F	Grav	/el	·	nd	Ī		ield	Tes					
Depth (fl.)	Sampler Blows per in,	No &	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	(density/consistency,	color, GRO	Identification & De DUP NAME & SYMBO ptional descriptions, g	OL, max	dmum particle siz	··.	% Coarse	% Fine	% Coarse	% Fine	% Fines	Dilatancy	Touglmess	Phasticity	Strength				
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	3	S1	1.0	674 -18 ME18 100 100	1.0		Note: gray gravel with Medium dense, brown						10	{-	- {•	5 80	5	+•1	-		٠.				
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	12	20	3.0		Construent in the state of the	SP	Medium dense, brown Note: cobble fragmen	poorly-gr s in tip of	aded SAND (SP), m split-spoon	nps = C	0.1 in., damp					0 85									
					~				-FILL-	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,							1			-11	****				
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	3	S2	5,0	ale brains despera	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		Stiff, gray-brown lean 0.02 in., damp	CLAY (C	L), frequent sand se	eams, n	nps =					20	80	N	M	M.					
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_ 10 _	WOH	\$3	10.0		10.4	ČL	Soft, gray-brown lean	CLÁY (C	L). frequent sand ser	ams. m	nos = 0.02 in.	,				20	80	Ñ	M	M	~~~				
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- 15 -		S4	15.0		***************************************	CL	Soft, gray lean CLAY	(CL), free	uent sand seams, m	nps = 0).02 in., wet				***************************************	20	8ô	N	M	M	_				
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		Water L	evel Data				Sample ID		Well Diagram					Sun	nmai	у					7				
Date	Time	Elapsed Time (hr.)	Bottom of Casing	Bottom of Hole	to: Water	Т	Open End Rod Thin Wall Tube		Filter Sand	R	Overburden (Lir Rock Cored (Lir	ear fi					17.0								
2/15/201	1510	-		14.6	8.6	U S	Undisturbed Sample Split Spoon Sample	RAE		L	iumber of Sam	nes					4\$								
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Field	Tests	Dilatancy:			w N - None	=	Plasticity:	1	I - Nonplastic L	- Low											٦				
		Toughness			ım H - Higi		Dry Strength: termined by direct of		L - Low M - M				ery l	High							4				
			NOTE: Soi	identificati	ons based o	n visual-	manual methods of	the USC	system as pract	ticed t	by Sebago Tec	hnic	s, ir	ic.							l				

SEBAGO TECHN INC.		TEST BORING REPORT PROPOSED ACCESSORY BUILDING STI JOB NO. 09492														BORING NO B3							
PROJECT LOCATION CLIENT CONTRAIN DRILLER	CTOR	501 DANF PROPSYS	ORTH STR , INC. EST BORIN	eet, portl	******	NE			K. K. 2/1	REC	TE 010												
Elevation			Datum			Location																	
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Type Inside Dia	meter (in)	SSA	1.3						inch	Safety Doughnul			lyme		-	VSPII	_	-	мри				
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	Sample	Sample			Stratum	İ	Vicus	t Monuel le	dentification & De	ecrintian	-61	avel	┼	San	7		-		Test				
Depth (ft.)	Blows per		Sample Depth (ft.)	Well Diagram	Change (ft.)	USCS Symbol	(density/consistency,	color, GROL	JP NAME & SYMBO	eologic interpretation)	% Coarse	% Fine	% Coarse	% Medium	% Fine	% Fines	Dilatancy	Toughness	Plasticity				
- 0 -	AND REPORT OF THE PARTY.	****	-		0.2		man	-BITUMI	NOUS CONCRET	E-		ļ			11				5				
	and the second of the second of the second of				1.0	1	Note: gray-brown filty	sand with	eravel on sucer flip	hts-SUBBASE/FILL-					<u> </u>				1				
	4	S1	1.0		1.5	GW.	Medium dense, gray w	vell-graded (GRAVEL with san	1 (GW), mps = 1.25	20	30	20	10	15	5							
	<u>. 6</u>				************	SP	in., damp Medium dense, brown	poorly ara	-FILL-	wowlet midly		-	┼	10	85	5	\vdash	-+					
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- 5	4	S2	5.0		5.5	SP	Loose, brown poorly-	graded SAN	D (SP), mps = 0.1	In. damp			·	10	85	5							
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- 15 -	2	S4	15.0	eller manerant setted	tal of the Warrent State Conf.	SM	Medium dense, gray si	lry SAND v	vith gravel (SM), n	nps = 0.3 in., wet		10	20	10	40	20							
	3	***) 14, m; 34, 15, 10, 10, 10, 10, 20, 20;	*A to be Kellentone to a second	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	wad-100 mpac 11.8 p. d. 100, 1901 1801 1806 1806 1806 1806 1806 1806 18					ļ						-					
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SEBAGO TECHN INC.		TEST BORING REPORT PROPOSED ACCESSORY BUILDING STI JOB NO. 09492														BORING NO. B4 Page i of								
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