Form # P 04

DISPLAY THIS CARD ON PRINCIPAL FRONTAGE OF WORK

CITY OF PORTLAND

BUILDING INSPECTION

PERMIT

Please Read Application And Notes, If Any, Attached

Permit Number: 080609

PERMIT ISSUED

JUN 13 2008

CITY OF PORTLAND

This is to certify that ST JOHN STREET ASSOCIATES LIMITED PARTNERSHIP TB

has permission to install foundation for new 5[illegible]n carbon [illegible]oxide [illegible]age tank

AT 70 ST JOHN ST [illegible] 070 A001001

provided that the person or persons, firm or corporation accepting this permit shall comply with all of the provisions of the Statutes of Maine and of the Ordinances of the City of Portland regulating the construction, maintenance and use of buildings and structures, and of the application on file in this department.

Apply to Public Works for street line and grade if nature of work requires such information.	[illegible]ification of inspection must be [illegible]n and when permission procured before this [illegible]ilding or part there[illegible]s [illegible]ed or [illegible]osed-in [illegible]4 [illegible]UR NOTICE IS REQUIRED.	A certificate of occupancy must be procured by owner before this building or part thereof is occupied.

OTHER REQUIRED APPROVALS

Fire Dept. Greg Cass

Health Dept. _____

Appeal Board _____

Other _____

Department Name

John Burke 6/12/08

Director - Building & Inspection Services

PENALTY FOR REMOVING THIS CARD

City of Portland, Maine - Building or Use Permit Application

389 Congress Street, 04101 Tel: (207) 874-8703, Fax: (207) 874-8716

Permit No:	Issue Date:	CBL:
08-0609		070 A001001

Location of Construction:	Owner Name:	Owner Address:	Phone:
70 ST JOHN ST	ST JOHN STREET ASSOCIATES	PO BOX 4821	
Business Name:	**Contractor Name:**	**Contractor Address:**	**Phone**
	TBD	Portland	
Lessee/Buyer's Name	**Phone:**	**Permit Type:**	**Zone:**
		Additions - Commercial	I-Mb

Past Use:	Proposed Use:	Permit Fee:	Cost of Work:	CEO District:
Barber Foods, Inc.	Barber Foods, Inc. - install foundation for new 50 ton carbon dioxide storage tank	$620.00	$60,000.00	2

FIRE DEPT: ☑ Approved ☐ Denied

Signature: Greg Cass

INSPECTION:
Use Group: F2 Type: 1
CO_2 TANK
IBC-2003

Signature: JMB 6/12/08

PEDESTRIAN ACTIVITIES DISTRICT (P.A.D.)

Action: ☐ Approved ☐ Approved w/Conditions ☐ Denied

Signature: Date:

Proposed Project Description:
install foundation for new 50 ton carbon dioxide storage tank

Permit Taken By:	Date Applied For:
ldobson	05/30/2008

1. This permit application does not preclude the Applicant(s) from meeting applicable State and Federal Rules.
2. Building permits do not include plumbing, septic or electrical work.
3. Building permits are void if work is not started within six (6) months of the date of issuance. False information may invalidate a building permit and stop all work..

Zoning Approval

Special Zone or Reviews	Zoning Appeal	Historic Preservation
☐ Shoreland	☐ Variance	☑ Not in District or Landmark
☐ Wetland	☐ Miscellaneous	☐ Does Not Require Review
☐ Flood Zone	☐ Conditional Use	☐ Requires Review
☐ Subdivision	☐ Interpretation	☐ Approved
☑ Site Plan exemption #2008-0065 scanned	☐ Approved	☐ Approved w/Conditions
Maj ☑ Minor ☐ MM ☐	☐ Denied	☐ Denied
Date: 6/3/09	Date:	Date:

FEE PAID ISSUED

CITY OF PORTLAND

CERTIFICATION

I hereby certify that I am the owner of record of the named property, or that the proposed work is authorized by the owner of record and that I have been authorized by the owner to make this application as his authorized agent and I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in the application is issued, I certify that the code official's authorized representative shall have the authority to enter all areas covered by such permit at any reasonable hour to enforce the provision of the code(s) applicable to such permit.

SIGNATURE OF APPLICANT ADDRESS DATE PHONE

RESPONSIBLE PERSON IN CHARGE OF WORK, TITLE DATE PHONE

City of Portland, Maine - Building or Use Permit

389 Congress Street, 04101 Tel: (207) 874-8703, Fax: (207) 874-8716

Permit No:	Date Applied For:	CBL:
08-0609	05/30/2008	070 A001001

Location of Construction:	Owner Name:	Owner Address:	Phone:
70 ST JOHN ST	ST JOHN STREET ASSOCIATES	PO BOX 4821	
Business Name:	**Contractor Name:**	**Contractor Address:**	**Phone**
	TBD	Portland	
Lessee/Buyer's Name	**Phone:**	**Permit Type:**	
		Additions - Commercial	

Proposed Use:	Proposed Project Description:
Barber Foods, Inc. - install foundation for new 50 ton carbon dioxide storage tank	install foundation for new 50 ton carbon dioxide storage tank

Dept: Zoning	**Status:** Approved	**Reviewer:** Marge Schmuckal	**Approval Date:** 06/03/2008	
Note:			**Ok to Issue:** ☑	
Dept: Building	**Status:** Approved with Conditions	**Reviewer:** Jeanine Bourke	**Approval Date:** 06/12/2008	
Note:			**Ok to Issue:** ☑	
Dept: Fire	**Status:** Approved	**Reviewer:** Capt Greg Cass	**Approval Date:** 06/05/2008	
Note:			**Ok to Issue:** ☑	

Comments:

6/10/2008-jmb: Contacted Aaron W. For geotech report, he will provide

6/11/2008-jmb: Received the report, ok to issue

General Building Permit Application

If you or the property owner owes real estate or personal property taxes or user charges on any property within the City, payment arrangements must be made before permits of any kind are accepted.

Location/Address of Construction: 70 ST JOHN ST		
Total Square Footage of Proposed Structure/Area: 256 SF	Square Footage of Lot: 5.9 A	
Tax Assessor's Chart, Block & Lot Chart# 70 Block# A Lot# 01 070 A001001	Applicant *must be owner, Lessee or Buyer* Name BARBER FOODS INC. Address 70 ST. JOHN ST. City, State & Zip PORTLAND ME 04112	Telephone: 541-2816
Lessee/DBA (If Applicable)	Owner (if different from Applicant) Name Address City, State & Zip	Cost Of Work: $ 60,000 C of O Fee: $ 630.00 Total Fee: $ 630.00

Current legal use (i.e. single family) MANUFACTURING

If vacant, what was the previous use? _____

Proposed Specific use: _____

Is property part of a subdivision? N If yes, please name _____

Project description: INSTALL FOUNDATION FOR NEW 50 TON CARBON DIOXIDE STORAGE TANK.

Contractor's name: T.B.D.

Address: _____ MAY 30

City, State & Zip _____ Telephone: _____

Who should we contact when the permit is ready: MIKE CUSHING Telephone: 541-2816

Mailing address: 70 ST. JOHN ST PORTLAND ME 04112

Please submit all of the information outlined on the applicable Checklist. Failure to do so will result in the automatic denial of your permit.

In order to be sure the City fully understands the full scope of the project, the Planning and Development Department may request additional information prior to the issuance of a permit. For further information or to download copies of this form and other applications visit the Inspections Division on-line at www.portlandmaine.gov, or stop by the Inspections Division office, room 315 City Hall or call 874-8703.

I hereby certify that I am the Owner of record of the named property, or that the owner of record authorizes the proposed work and that I have been authorized by the owner to make this application as his/her authorized agent. I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in this application is issued, I certify that the Code Official's authorized representative shall have the authority to enter all areas covered by this permit at any reasonable hour to enforce the provisions of the codes applicable to this permit.

AGENT: AARON S. WILSON 878-1751

Signature: _____ Date: 5.29.08

This is not a permit; you may not commence ANY work until the permit is issue

ASSOCIATED DESIGN PARTNERS INC.

80 Leighton Road ▪ Falmouth, Maine 04105

Office: 207.878.1751
Fax: 207.878.1788
e-mail: adp@adpengineering.com
web: www.adpengineering.com

May 30, 2008 07249

Ms. Jeanie Bourke
Inspection Services Manager
389 Congress St
Portland, ME 04101

Re: Proposed CO2 Tank
Barber Foods Facility
70 St. John Street
Portland, ME 04112

Dear Jeanie,

Associated Design Partners, Inc. is pleased to submit this application and supporting documents for building permits for the proposed construction of a 15,000 Gal CO2 tank at the existing Barber Foods Facility at 70 St. John Street. We have submitted for a site plan exemption, and have gotten a verbal approval from Barbara Barhydt, but have yet to receive final documentation of the exemption.

A summary of the project scope is listed below:

1. Existing Mobile Training Office Trailer has been removed.
2. Install new 16ftx16ft concrete pile supported foundation (see attached partial site plan on S101 for proposed location).
3. Install fencing and steel bollards around tank foundation.
4. Erect new 40ft, 15,000 gallon Carbon Dioxide storage tank, supplied by Linde gasses.

Please find the General Building Permit Application, One set of 24"x36" plans, a disc with 11"x17" pdf (not to scale), and the permit fee. If you have any questions regarding this project or the information contained within, please do not hesitate to call.

Sincerely,

Aaron S Wilson, P. E.
Engineering Project Manager
Associated Design Partners Inc
207-878-1751
ASW

Certificate of Design Application

From Designer: ASSOCIATED DESIGN PARTNERS, INC

Date: 5-29-08

Job Name: BARBER FOODS CO_2 TANK FOUNDATION

Address of Construction: 70 ST. JOHN ST. PORTLAND

2003 International Building Code

Construction project was designed to the building code criteria listed below:

Building Code & Year 2003 Use Group Classification (s) F-2

Type of Construction I

Will the Structure have a Fire suppression system in Accordance with Section 903.3.1 of the 2003 IRC N

Is the Structure mixed use? N If yes, separated or non separated or non separated (section 302.3) _____

Supervisory alarm System? N Geotechnical/Soils report required? (See Section 1802.2) Y (ATTACHED)

Structural Design Calculations

_____ Submitted for all structural members (106.1 – 106.11)

Design Loads on Construction Documents (1603)

Uniformly distributed floor live loads (7603.11, 1807)

Floor Area Use	Loads Shown
N.A.	
FOUNDATION DESIGNED FOR TANK REACTIONS	

Wind loads (1603.1.4, 1609)

ANALYTICAL Design option utilized (1609.1.1, 1609.6)

120 Basic wind speed (1809.3)

1.15 Building category and wind importance Factor, I_w table 1604.5, 1609.5)

C Wind exposure category (1609.4)

N.A. Internal pressure coefficient (ASCE 7)

N.A. Component and cladding pressures (1609.1.1, 1609.6.2.2)

+/- 33PSF Main force wind pressures (7603.1.1, 1609.6.2.1)

Earth design data (1603.1.5, 1614-1623)

E.L.F. Design option utilized (1614.1)

IV Seismic use group ("Category")

0.324/0.125 Spectral response coefficients, SDs & SD1 (1615.1)

D Site class (1615.1.5)

NA Live load reduction

NA Roof *live* loads (1603.1.2, 1607.11)

NA Roof snow loads (1603.7.3, 1608)

60PSF Ground snow load, P_g (1608.2)

NA If P_g > 10 psf, flat-roof snow load P_f

NA If P_g > 10 psf, snow exposure factor, C_e

NA If P_g > 10 psf, snow load importance factor, I_s

NA Roof thermal factor, C_t (1608.4)

NA Sloped roof snowload, P_s (1608.4)

B Seismic design category (1616.3)

BRACED LEGS Basic seismic force resisting system (1617.6.2)

3/2.5 Response modification coefficient, R_I and ASCE 7-05 T15.4-2 deflection amplification factor C_d (~~1617.6.2~~)

E.L.F. Analysis procedure (1616.6, 1617.5)

17 K Design base shear (1617.4, 16175.5.1)

Flood loads (1803.1.6, 1612)

NA Flood Hazard area (1612.3)

NA Elevation of structure

Other loads

NA Concentrated loads (1607.4)

NA Partition loads (1607.5)

NA Misc. loads (Table 1607.8, 1607.6.1, 1607.7, 1607.12, 1607.13, 1610, 1611, 2404

New Commercial
Permit Application Checklist

All of the following information is required and must be submitted. Checking off each item as you prepare your application package will ensure your package is complete and will help to expedite the permitting process.

One (1) complete Set of construction drawings must include:

Note: Construction documents for costs in excess of $50,000.00 must be prepared by a Design Professional and bear their seal.

- ☑ Cross sections w/framing details
- ☐ Detail of any new walls or permanent partitions N/A
- ☑ Floor plans and elevations
- ☐ Window and door schedules N/A
- ☑ Foundation plans with rebar specifications and required drainage and damp proofing (if applicable)
- ☐ Detail egress requirements and fire separations N/A
- ☑ Insulation R-factors of walls, ceilings, floors and U-factors of windows as per the IEEC 2003
- ☐ Complete the Accessibility Certificate and The Certificate of Design N/A
- ☑ A statement of special inspections as required per the IBC 2003
- ☐ Complete electrical and plumbing layout. N/A
- ☐ Mechanical drawings for any specialized equipment such as furnaces, chimneys, gas equipment, HVAC equipment (air handling) or other types of work that may require special review. N/A
- ☑ Reduced plans or electronic files in PDF format are required if originals are larger than 11" x 17".
- ☐ Per State Fire Marshall, all new bathrooms must be ADA compliant. N/A

Separate permits are required for internal & external plumbing, HVAC and electrical installations.

Nine (9) copies of the minor (< 10,000 sf) or major (> 10,000 sf) site plan application is required that includes:

SEE SITE PLAN EXEMPTION

- ☐ A stamped boundary survey to scale showing north arrow, zoning district and setbacks to a scale of ≥ 1" = 20' on paper ≥ 11" x 17"
- ☐ The shape and dimension of the lot, footprint of the proposed structure and the distance from the actual property lines. Photocopies of the plat or hand draw footprints not to scale will not be accepted.
- ☐ Location and dimensions of parking areas and driveways, street spaces and building frontage
- ☐ Finish floor or sill elevation (based on mean sea level datum)
- ☐ Location and size of both existing utilities in the street and the proposed utilities serving the building
- ☐ Existing and proposed grade contours
- ☐ Silt fence (erosion control) locations

Building Inspections Division • 389 Congress Street • Portland, Maine 04101 • (207) 874-8703 • FACSIMILE (207) 874-8716 • TTY (207) 874-8936

Fire Department requirements.

The following shall be submitted on a separate sheet:

- ☑ Name, address and phone number of applicant **and** the project architect.
- ☑ Proposed use of structure (NFPA and IBC classification)
- ☑ Square footage of proposed structure (total and per story)
- ☑ Existing and proposed fire protection of structure.
- ☐ Separate plans shall be submitted for NA
    a) Suppression system
    b) Detection System (separate permit is required)
- ☐ A separate Life Safety Plan must include: NA
    a) Fire resistance ratings of all means of egress
    b) Travel distance from most remote point to exit discharge
    c) Location of any required fire extinguishers
    d) Location of emergency lighting
    e) Location of exit signs
    f) NFPA 101 code summary
- ☐ Elevators shall be sized to fit an 80" x 24" stretcher. NA

For questions on Fire Department requirements call the Fire Prevention Officer at (207) 874-8405.

Please submit all of the information outlined in this application checklist. If the application is incomplete, the application may be refused.

In order to be sure the City fully understands the full scope of the project, the Planning and Development Department may request additional information prior to the issuance of a permit. For further information or to download copies of this form and other applications visit the Inspections Division on-line at www.portlandmaine.gov, or stop by the Inspections Division office, room 315 City Hall or call 874-8703.

Permit Fee: $30.00 for the first $1000.00 construction cost, $10.00 per additional $1000.00 cost

This is not a Permit; you may not commence any work until the Permit is issued.

Certificate of Design

Date: 5/29/08

From: Aaron S. Wilson

These plans and / or specifications covering construction work on:

Barber Foods CO_2 Tank Foundation

Have been designed and drawn up by the undersigned, a Maine registered Architect / Engineer according to the ***2003 International Building Code*** and local amendments.

Signature:

Title: Structural Engineer

Firm: Associated Design Partners INC.

Address: 80 Leighton Rd

Falmouth ME 04105

Phone: 207-878-1757

For more information or to download this form and other permit applications visit the Inspections Division on our website at www.portlandmaine.gov

STATEMENT OF SPECIAL CONSTRUCTION MONITORING

PROJECT: **Barber Foods CO2 Tank Foundation**
Portland, Maine

PERMIT APPLICANT: **Mike Cushing – Barber Foods**
APPLICANT'S ADDRESS: **70 St. John St, Portland ME 04112**

STRUCTURAL ENGINEER OF RECORD
Associated Design Partners, Inc
CONTRACTOR: **TBD**

This Statement of Special Construction Monitoring is submitted as a condition for building permit issuance in accordance with Section 1704.0 of the 2003 International Building Code. It includes the Schedule of Special Construction Monitoring and Testing as applicable to this project. Also included is a listing of agents and other approved agencies to be retained for conducting the monitoring and testing applicable to this project.

The Special Construction Monitoring Coordinator shall keep records of all observations listed herein, and shall furnish field reports to the Registered Design Professional of Record. All discrepancies shall be brought to the immediate attention of the Contractor for correction, and to the Registered Design Professional of Record. If the discrepancies are not corrected, the discrepancies shall be brought to the attention of the Registered Design Professional of Record. Interim reports shall be submitted to the Registered Design Professional of Record monthly, unless more frequent submissions are requested.

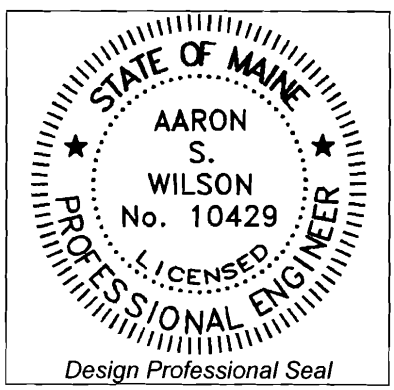
The Special Construction Monitoring program does not relieve the Contractor of his or her responsibilities. Job site safety is solely the responsibility of the Contractor. Materials and activities covered under the monitoring schedule are not to include the Contractor's equipment and methods used to erect or install the materials listed.

Prepared by:

Aaron S. Wilson
(type or print name)

Signature

4/16/08
Date



Design Professional Seal

Owner's Authorization:

Signature Date

Building Official's Acceptance:

Signature Date

QUALITY ASSURANCE FOR LATERAL SYSTEMS

Quality Assurance for Seismic Requirements

Seismic Design Category IBC 1705 *B*

Quality Assurance Plan Required (Y/N) *N*

If seismic design category C, and plan is not required, explain (see exceptions to 1705.1)

Description of seismic force resisting system and designated seismic systems:
Self Supporting Steel Tank Structure

Quality Assurance for Wind Requirements

Basic Wind Speed (3 second gust) IBC 1706 *97MPH*

Quality Assurance Plan Required (Y/N) *N*

Description of wind force resisting system and designated wind resisting components:
Self Supporting Steel Tank Structure

Statement of Responsibility

Each contractor responsible for the construction or fabrication of a system or component designated above must submit a Statement of Responsibility in accordance with section 1705.3, and 1706.3 of the 2003 IBC code.

SPECIAL CONSTRUCTION MONITORING AGENTS

This Statement of Special Construction Monitoring / Quality Assurance Plan includes the following building systems:

- ☒ Soils and Foundations
- ☐ Cast-in-Place Concrete Retaining walls
- ☐ Precast Concrete
- ☐ Masonry
- ☐ Structural Steel
- ☐ Cold-Formed Steel Framing
- ☐ Spray Fire Resistant Material
- ☐ Wood Construction
- ☐ Exterior Insulation and Finish System
- ☐ Mechanical & Electrical Systems
- ☐ Architectural Systems
- ☐ Special Cases

AGENT	FIRM	CONTACT INFORMATION
1. Engineer of Record	**Associated Design Partners**	**80 Leighton Rd** **Falmouth ME 04105** **Ph: 878-1751**
2. Special Construction Monitoring Coordinator	**Associated Design Partners**	**80 Leighton Rd** **Falmouth ME 04105** **Ph: 878-1751**
3. Field Monitor	**S.W. Cole Engineering Inc**	**286 Portland Road** **Gray, ME 04039-9586** **P: (207) 657.2866**
4. Testing Agency	**S.W. Cole Engineering Inc**	**286 Portland Road** **Gray, ME 04039-9586** **P: (207) 657.2866**
5. Contractor	**TBD**	

Note: The testing agency shall be engaged by the Owner or the Owner's Agent, and not by the Contractor or Subcontractor whose work is to be inspected or tested. Any conflict of interest must be disclosed to the Building Official, prior to commencing work.

The qualifications of all personnel performing Special Inspection and testing activities are subject to the approval of the Building Official. The credentials of all Inspectors and testing technicians shall be provided if requested.

Key for Minimum Qualifications of Inspection Agents:

When the Registered Design Professional in Responsible Charge deems it appropriate that the individual performing a stipulated test or inspection have a specific certification or license as indicated below, such designation shall appear below the *Agency Number* on the Schedule.

PE/SE	Structural Engineer – a licensed SE or PE specializing in the design of building structures
PE/GE	Geotechnical Engineer – a licensed PE specializing in soil mechanics and foundations
EIT	Engineer-In-Training – a graduate engineer who has passed the Fundamentals of Engineering examination

American Concrete Institute (ACI) Certification

ACI-CFTT	Concrete Field Testing Technician – Grade 1
ACI-CCI	Concrete Construction Inspector
ACI-LTT	Laboratory Testing Technician – Grade 1&2
ACI-STT	Strength Testing Technician

American Welding Society (AWS) Certification

AWS-CWI	Certified Welding Inspector
AWS/AISC-SSI	Certified Structural Steel Inspector

American Society of Non-Destructive Testing (ASNT) Certification

ASNT	Non-Destructive Testing Technician – Level II or III.

International Code Council (ICC) Certification

ICC-SMSI	Structural Masonry Special Inspector
ICC-SWSI	Structural Steel and Welding Special Inspector
ICC-SFSI	Spray-Applied Fireproofing Special Inspector
ICC-PCSI	Prestressed Concrete Special Inspector
ICC-RCSI	Reinforced Concrete Special Inspector

National Institute for Certification in Engineering Technologies (NICET)

NICET-CT	Concrete Technician – Levels I, II, III & IV
NICET-ST	Soils Technician - Levels I, II, III & IV
NICET-GET	Geotechnical Engineering Technician - Levels I, II, III & IV

Exterior Design Institute (EDI) Certification

EDI-EIFS	EIFS Third Party Inspector

TABLE 1 – SCHEDULE OF SPECIAL CONSTRUCTION MONITORING

MATERIAL / ACTIVITY		EXTENT of MONITORING (Continuous, Periodic, Other, Exempt, None)	COMMENTS	AGENT #	DATE COMPLETED	REV #
1704.3 STEEL CONSTRUCTION						
1. Material Verification of high strength bolts, nuts, and washers.	a. Identification markings to conform to ASTM standards specified in the approved construction documents.	None	No steel construction in project. Tank in self supporting.			
	b. Manufacturers Certificate of Compliance required.					
2. Inspection of High – Strength Bolting	a. Bearing type connections					
	b. Slip – critical connections					
3. Material Verification of structural steel	a. Identification marking to conform to ASTM standards specified in the contract documents.					
	b. Manufacturers certified mill test Reports.					
4. Material Verification of weld filler materials:	a. Identification marking to conform to AWS standards specified in the contract documents.					
	b. Manufacturers Certificate of Compliance required.					
5. Inspection of Welding – Structural Steel	a. Single Pass fillet welds < 5/16"					
	b. Roof deck welds					
6. Inspection of Steel Frame Joint details for compliance with approved documents.	a. Bracing / moment frame connections					
	b. Member locations					
	c. Application of joint details at each connection.					

TABLE 1 – STATEMENT OF SPECIAL INSPECTIONS, cont.

MATERIAL/ACTIVITY		EXTENT of INSPECTION (Continuous, Periodic, Other, None)	COMMENTS	AGENT #	DATE COMPLETED	REV #
1704.4 CONCRETE CONSTRUCTION						
1. Inspection of reinforcing steel, including placement.		Periodic		3		
2. Inspection of reinforcing steel welding		None	No welding of rebar specified in contract drawings			
3. Inspect bolts embedded into concrete prior to and during placement of concrete where allowable loads have been increased.		Exempt	All anchors are post installed epoxy into new or existing concrete			
4. Verify concrete mix design(s)		Periodic	SER to review and approve mix design(s) prior to delivery. Field agent to verify delivery ticket matches approved mix design.	1,3		
5. Sample fresh concrete for strength tests, perform slump and air content tests, and determine temperature of concrete.		Continuous		3,4		
6. Inspection of concrete placement for proper techniques.		Continuous		3		
7. Inspection for maintenance of specified curing temperature and techniques.		Periodic		3		
1704.5 MASONRY CONSTRUCTION - Level 1 Special Inspection for non-essential facility – 1704.5.2			None Masonry Construction in Project			
1. As Masonry Construction begins, the following shall be verified to ensure conformance	a. Proportions of site-prepared mortar	None				
	b. Construction of mortar joints	None				
	c. Location of reinforcement	None				
	d. Pre-stressing technique	None	No pre-stressing in building			
	e. Grade and size of pre-stressing tendons.	None	No pre-stressing in building			
2. The Inspection program shall verify the following:	a. Size and location of structural elements.	None				
	b. Type, size, and location of embedded anchors.	None				
	c. Size, grade, and type of reinforcing	None				

TABLE 1 – STATEMENT OF SPECIAL INSPECTIONS, cont.

MATERIAL/ACTIVITY		EXTENT of INSPECTION (Continuous, Periodic, Other, None)	COMMENTS	AGENT #	DATE COMPLETED	REV #
1704.5 MASONRY CONSTRUCTION - Level 1 Special Inspection for non-essential facility – 1704.5.2			None Masonry Construction in Project			
2. The Inspection program shall verify the following, cont:	d. welding of reinforcing bars	None				
	e. Protection of Masonry during cold weather (temp. below 40 deg F.)	None				
	f. Application and measurement of pre-stressing reinforcement	None				
3. Prior to grouting, the following shall be verified to ensure compliance.	a. Grout space is clean	None				
	b. Placement of reinforcement	None				
	c. Proportions of site-prepared grout	None				
	d. Construction of mortar joints	None				
4. Grout placement shall be verified to ensure compliance with code and construction document provisions.		None				
5. Preparation of any grout specimens, mortar specimens and/or prisms shall be observed		None				
6. Compliance with required inspection provisions of the construction documents and the approved submittals shall be verified.		None				
1704.6 WOOD CONSTRUCTION						
1. Horizontal Diaphragms and Vertical Shearwalls	a. Inspect sheathing size, grade, and thickness for conformance with construction documents.	None				
	b. Inspect sheathing fastener size and pattern for conformance with construction documents.	None				
	c. Verify attachment to supporting elements is per contract documents.	None				
2. Wood truss fabricator certification / quality control procedures	Verify shop fabrication and quality control procedures for wood truss plant.	None				
3. Material Grading	Verify material grading for sawn lumber for compliance with construction documents. Verify manufactured lumber (LVL'S, PSL's) for conformance with construction documents.	None				

BUILDING PERMIT INSPECTION PROCEDURES

Please call 874-8703 or 874-8693 (ONLY)

to schedule your inspections as agreed upon

Permits expire in 6 months, if the project is not started or ceases for 6 months.

The Owner or their designee is required to notify the inspections office for the following inspections and provide adequate notice. Notice must be called in 48-72 hours in advance in order to schedule an inspection:

By initializing at each inspection time, you are agreeing that you understand the inspection procedure and additional fees from a "Stop Work Order" and "Stop Work Order Release" will be incurred if the procedure is not followed as stated below.

A Pre-construction Meeting will take place upon receipt of your building permit.

__X__ **The final report of Special Inspections shall be submitted at the completion of the work.**

__X__ **Form inspection prior to pouring concrete for the pile cap**

Certificate of Occupancy is not required for certain projects. Your inspector can advise you if your project requires a Certificate of Occupancy. All projects DO require a final inspection.

If any of the inspections do not occur, the project cannot go on to the next phase, REGARDLESS OF THE NOTICE OR CIRCUMSTANCES.

CERIFICATE OF OCCUPANICES MUST BE ISSUED AND PAID FOR, BEFORE THE SPACE MAY BE OCCUPIED.

Signature of Applicant/Designee — Date: 6/16/08

Signature of Inspections Official — Date: 6/16/08

PERMIT ISSUED

JUN 13

CITY OF PORTLAND

CBL: 070 A001001 **Building Permit #:** 08-0609

TABLE 1 – STATEMENT OF SPECIAL INSPECTIONS, cont.

MATERIAL/ACTIVITY		EXTENT of INSPECTION (Continuous, Periodic, Other, None)	COMMENTS	AGENT #	DATE COMPLETED	REV #
1704.6 WOOD CONSTRUCTION						
4. Wood Connections	Verify that connections are made as shown in the contract documents. For connections not specifically detailed, verify conformance with IBC 2003 Ch. 23	None				
5. Framing	Verify that framing is installed in accordance with construction documents.	None				
6. Pre-Fabricated Wood Trusses	Inspect truss and all bracing installation. Bracing to be installed per fabricator's recommendations and BCSI 1-03	None				
1704.7 SOILS						
1. Site Preparation	Inspect preparation of site for conformance with Geotechnical recommendations prior to placement of prepared fill.	None	Pile supported foundation does not require specific soil preparation techniques.			
2. Fill Placement	During Fill Placement verify that material and lift thickness comply with approved Geotechnical report.	None				
3. In-Place Soil Density	Verify compliance of in-place compacted dry density with approved Geotechnical report.	None				
1704.8 PILE FOUNDATIONS	Provide pile driving records of each pile. Submit reports to building official and EOR. Reports to include pile tip cutoff elevation relative to a common benchmark.	Continuous		3		
1704.10 ARCHITECTURAL WALL PANELS AND VENEERS	Verify compliance of attachment of interior and exterior Architectural veneers to supporting structure for building in Seismic Design Category E or F.	None				

TABLE 1 – STATEMENT OF SPECIAL INSPECTIONS, cont.

MATERIAL/ACTIVITY		EXTENT of INSPECTION (Continuous, Periodic, Other, None)	COMMENTS	AGENT #	DATE COMPLETED	REV #
1704.11 SPRAYED FIRE-RESISTANT MATERIAL	a. Verify conformance of the prepared surface with manufacturer's specifications prior to application of material.	None	No Sprayed Fire-Resistant material in project			
	b. Verify that substrate's ambient temperature meet manufacturer's specifications.	None				
	c. Verify that material thickness meets design specifications.	None				
	d. Verify that the material density meets the design specifications. Test in accordance with ASTM E 605.	None				
	e. Verify that bond strength between material and substrate is greater than or equal to 150 psf. Test in accordance with ASTM E 736 and IBC 2003 1704.11.5.1 – 1704.11.5.2	None				
1704.12 EXTERIOR AND INSULATION AND FINISH SYSTEMS (EIFS)	Verify conformance of EFIS installation with manufacturers and design specifications.	None	No EIFS in project			
1704.13 SPECIAL CASES						
1704.10 SMOKE CONTROL						
	a. Test ductwork for leakage and recode device locations prior to concealment of mechanical systems.	None				
	b. Prior to building occupation, perform pressure difference testing, flow measurements and detection, and control monitoring.	None				

Geotechnical Engineering Services
Proposed Carbon Dioxide Tank
Barber Foods Building
54 & 70 St. John Street
Portland, Maine

00-0695.2 **May 13, 2008**

PREPARED FOR:
Associated Design Partners, Inc.
Attn: Aaron Wilson, P.E.
70 Leighton Road
Falmouth ME 04105

PREPARED BY

S.W.COLE ENGINEERING INC

S.W.COLE ENGINEERING INC • Geotechnical Engineering • Field & Lab Testing • Scientific & Environmental Consulting

00-0695.2

May 13, 2008

Associated Design Partners, Inc.
Attn: Aaron Wilson, P.E.
70 Leighton Road
Falmouth ME 04105

Subject: Geotechnical Engineering Services
Proposed Carbon Dioxide Tank
Barber Foods Building
54 & 70 St. John Street
Portland, Maine

Dear Mr. Wilson:

As discussed, we have reviewed the subsurface information from previous work at the Barber Foods Building for the proposed carbon dioxide tank. The purpose of the review was to determine if the design geotechnical parameters presented in our previous report (SWCE project number 00-0695, dated December 15, 2000) we applicable to the proposed tank and to provide additional recommendations as necessary.

PROPOSED CONSTRUCTION

Based on the information you provided, we understand the proposed construction consists of a tank supported on three legs. We understand piles are being considered for a foundation. The tank will be placed on a reinforced concrete pile cap. We understand the tank weighs on the order of 20 tons and the capacity is about 50 tons for a total dead load of 70 tons.

SUBSURFACE CONDITIONS

Based on the soils encountered in test boring B-101 completed on November 20, 2000 and test borings TB-5 and TB-9 (by others), we anticipate that soils in the area of the proposed tank will consist of gravelly sand fill overlying sand with some silt overlying silty sand with some gravel (glacial till). Bedrock is anticipated to be about 17 to 20 feet below the ground surface in the location of the proposed carbon dioxide tank. The test boring logs for B-101, TB-5, and TB-9 are attached. An exploration location plan showing the area of the proposed tank is also attached.

RECOMMENDATIONS

Based on the subsurface conditions at the site, the recommendations for foundation support on driven H-piles as given in our previous report (dated December 15, 2000) are applicable.

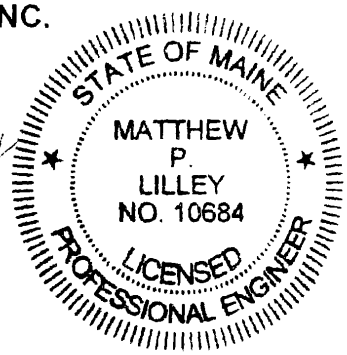
Alternatively, consideration could also be given to small diameter steel pipe piles. The small diameter drill casing pile would likely consist of a drilling contractor installing 4 ½ O.D., ½ inch thick drill casing to bedrock. The casing would be washed out and then an NQ rock core bit would be used to core into the rock, creating a 2 ¾ inch diameter hole to allow for a single reinforcing steel rod to be placed and grouted into the bedrock for uplift capacity. The steel casing would also be grouted from bottom up, using a tremmie pipe. A 4,000 psi grout is typically used. The foundation contractor typically installs the reinforcing steel and grout. A steel plate would be welded to the top of the casing to provide tie-in to the foundation cap. An allowable downward axial working capacity of 30 kips or less is typically achievable. This assumes a 24-inch deep rock core below the bottom of the casing and a rock compressive strength of at least 8 ksi. We can provide additional information for this option, if requested.

CLOSURE

It has been a pleasure to be of assistance to you with this phase of your project. If you have any questions, please do not hesitate to contact us.

Sincerely,

S. W. COLE ENGINEERING, INC.

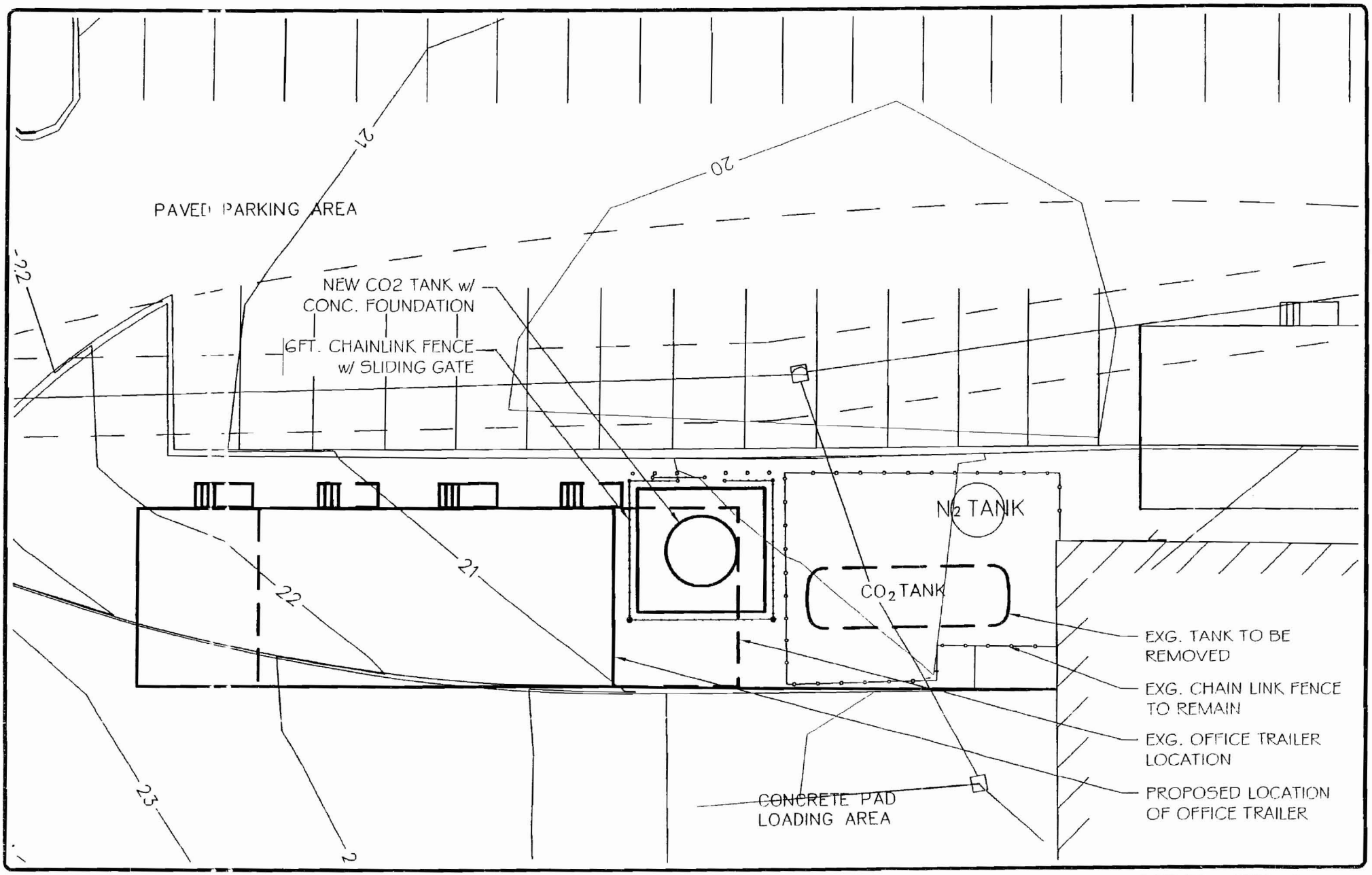


Matthew P. Lilley, P.E.
Geotechnical Engineer

MPL:mpl/jlw

Attachments

P:\2000\00-0695.2 C- Associated Design Partners - Portland, ME - Proposed Carbon Dioxide Tank - Barber Foods - MPL\Reports and Letters\00-0695.2 Report.doc



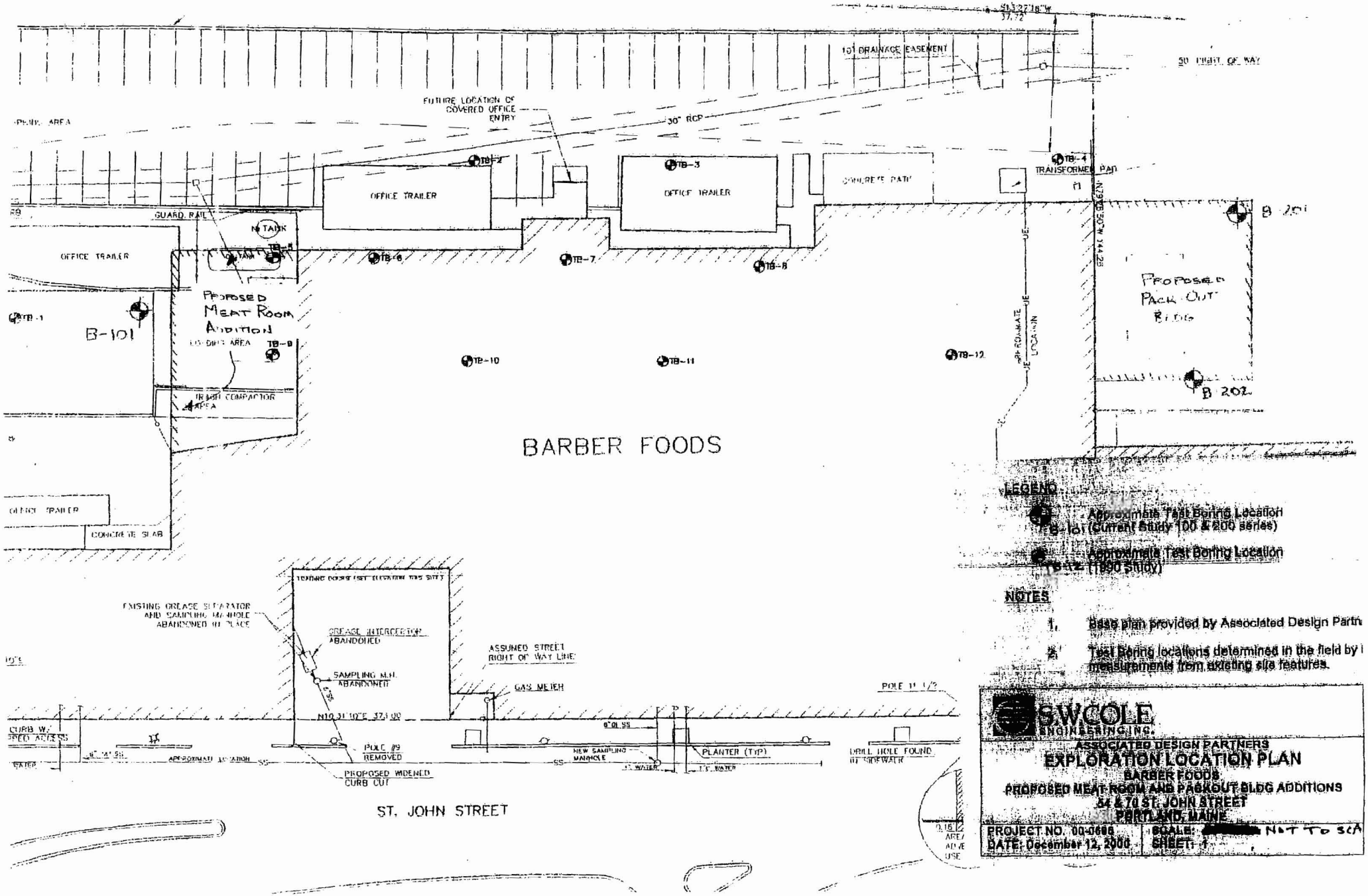
ASSOCIATED DESIGN PARTNERS INC.
80 Leighton Road
Falmouth, Maine 04105
Office: (207) 878-1751
Fax: (207) 878-1788
E-Mail: adp@adpengineering.com

PROJECT: BARBER FOODS CO2 TANKS
PORTLAND MAINE
FOR: Owner

SHEET TITLE:
PARTIAL SITE PLAN

REVISIONS			
No.	BY	DESCRIPTION	DATE
1			
2			
3			
4			

DATE : 1-24-08
SCALE : AS NOTED
DESIGN BY: ASW
DRAWN BY: RSC
FILE #:07249-C101.dwg
PROJECT NUMBER: 07249
SHEET NO: C101



LEGEND

- TB-101 Approximate Test Boring Location (Current Study 100 & 200 series)
- TB-2 Approximate Test Boring Location (1990 Study)

NOTES

1. Base plan provided by Associated Design Partn
2. Test Boring locations determined in the field by measurements from existing site features.

SWCOLE ENGINEERING

ASSOCIATED DESIGN PARTNERS

EXPLORATION LOCATION PLAN

BARBER FOODS

PROPOSED MEAT ROOM AND PACKOUT BLDG ADDITIONS

54 & 70 ST. JOHN STREET

PORTLAND, MAINE

PROJECT NO. 00-0606 | SCALE: [redacted] NOT TO SCA

DATE: December 12, 2000 | SHEET: 1

S.W.COLE
ENGINEERING, INC.
GEOTECHNICAL CONSULTANTS

BORING LOG

BORING NO.: B-101
SHEET: 1 OF 1
PROJECT NO.: 00-0695
DATE START: 11/20/2000
DATE FINISH: 11/20/2000
ELEVATION: 21' + -
SWC REP.: GWB

PROJECT / CLIENT: PROPOSED BARBER FOODS ADDITION / ASSOCIATED DESIGN PARTNERS, INC.
LOCATION: ST. JOHN STREET / PORTLAND, MAINE
DRILLING FIRM: GREAT WORKS TEST BORINGS DRILLER: DON BOLSTRETCH

	TYPE	SIZE I.D.	HAMMER WT.	HAMMER FALL
CASING:	HW	4"	300 lb	18"
SAMPLER:	SS	1 3/8"	140 lb	30"
CORE BARREL:				

WATER LEVEL INFORMATION
Soils Saturated @ 7.0'

CASING BLOWS PER FOOT	SAMPLE NO	PEN.	REC.	DEPTH @ BOT	SAMPLER BLOWS PER 6" 0-6	6-12	12-18	18-24	DEPTH	STRATA & TEST DATA
									0.3'	BITUMINOUS PAVEMENT
	1D	21"	14"	2.0'	13	32	19	14	1.8'	BROWN GRAVELLY SAND (FILL) ~ DENSE ~
	2D	24"	18"	7.0'	9	8	10	15		BROWN FINE TO MEDIUM SAND SOME SILT TRACE GRAVEL ~ MEDIUM DENSE ~
	3D	24"	20"	12.0'	7	13	14	20		
	4D	24"	6"	17.0'	5	17	8	6	18.0'	
									19.5'	GRAY SILTY SAND SOME GRAVEL (TILL) ~ DENSE ~
										REFUSAL @ 19.5' (PROBABLE BEDROCK)

SAMPLES:
D=SPLIT SPOON
C=3" SHELBY TUBE
U=3.5" SHELBY TUBE

SOIL CLASSIFIED BY:
[ ] DRILLER - VISUALLY
[X] SOIL TECH.-VISUALLY
[X] LABORATORY TEST

REMARKS:
STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY BETWEEN SOIL TYPES AND THE TRANSITION MAY BE GRADUAL.

2

BORING NO.: B-101

Tel. (508) 583-2680
ENVIRO-TECH DRILLING, INC.
45 Turnpike Street, West Bridgewater
Massachusetts 02379

BORING / WELL LOG

CLIENT Stahlman Engineering

PROJECT Barber Foods
St. John Street
Portland, ME

DRILLER J. Marks
INSPECTOR
LINE & STA ____ OFFSET ____
GR. ELEV.
START September 26, 1990
FINISH September 26, 1990

BORING NO. TB5
SHEET 1 OF 1
FILE NO. 90095

	CASING	SAMPLER	CORE BARREL
TYPE	HSA	SS	
SIZE ID	4.25in.	1.375in.	
HAMMER WT.		140lb.	
HAMM. FALL		30in.	

DEPTH	CSG BLOWS PER FT	SAMPLE NO.	SAMPLE DEPTH RANGE FEET	SAMPLE BLOWS / 6" ON SAMPLER	SAMPLE TYPE	MOISTURE, DENSITY OR CONSIST.	STRAT. CHANGE FEET	SAMPLE CLASSIFICATION AND REMARKS	SAMPLE PEN. / REC.
		1	0'0"-1'6"	8-9	S	Moist		Tan to black, fine to medium	18"/13"
				9		Med	1.0	silty sand, trace cobble	
						Dense		fragments, trace coal (FILL)	
5		2	5'0"-6'6"	5-6	S	Wet		Brown, fine to medium SAND,	18"/12"
				5		Med		some silty sand layers, trace	
						Dense		clay	
10		3	10'0"-11'6"	2-4	S	Wet		Gray, fine to medium silty	18"/18"
				4		Loose		SAND	
15		4	15'0"-16'11"	10-17	S	Wet		Gray fine to medium silty	21"/21"
				34-120/3"		Very		SAND, some rock fragments	
						Dense			
20									
25									

SAMPLE IDENTIFICATION
S – SPLIT SPOON
T – THIN WALL TUBE
U – UNDISTURBED PISTON
O – OPEN END ROD
W – WASH SAMPLE

PENETRATION RESISTANCE
140 lb. Wt. falling 30" on 2" O.D. Sampler

Cohesionless Density		Cohesive Consistency	
0-4	Very Loose	0-2	Very Soft
5-9	Loose	3-4	Soft
10-29	Med. Dense	5-8	Med. Stiff
30-49	Dense	9-15	Stiff

PROPORTIONS USED

trace	0-10%
little	10 to 20%
some	20 to 35%
and	35 to 50%

GROUNDWATER OBSERVATIONS
AT 2 FT. AFTER 0 HRS
AT ____ AFTER ____ HRS
NOTE: Levels may vary with seasonal fluctuation

Tel. (508) 583-2680
ENVIRO-TECH DRILLING, INC.
45 Turnpike Street, West Bridgewater
Massachusetts 02379

BORING / WELL LOG

CLIENT Stahlman Engineering

PROJECT Barber Foods
St. John Street
Portland, ME

DRILLER J. Marks
INSPECTOR
LINE & STA OFFSET
SUR. ELEV.
START September 26, 1990
FINISH September 26, 1990

BORING NO. TB9
SHEET 1 OF 1
FILE NO. 90095

	CASING	SAMPLER	CORE BARREL
TYPE	HSA	SS	
SIZE ID	4.25in.	1.375in.	
HAMMER WT.		140lb.	
HAMM. FALL		30in.	

DEPTH	CSG BLOWS PER FT	SAMPLE NO.	SAMPLE DEPTH RANGE FEET	SAMPLE BLOWS/6" ON SAMPLER	SAMPLE TYPE	MOISTURE, DENSITY OR CONSIST.	STRAT. CHANGE FEET	SAMPLE CLASSIFICATION AND REMARKS	SAMPLE PEN./ REC.
		1	0'0"-1'6"	7-8	S	Moist		Black loamy SAND and rust to	18"/18"
				10		Med	1.5	gray, fine to medium sand	
						Dense		(FILL)	
5		2	5'0"-6'6"	14-22	S	Wet		Gray, fine to medium SAND,	18"/18"
				25		Dense		little silt	
10		3	10'0"-11'6"	5-8	S	Wet	10.0	Gray, organic SILT, some	18"/18"
				10		Very		fine to medium sand, little	
						Stiff		clay	
15		4	15'0"-16'6"	6-6	S	Wet		Gray, silty fine to coarse	18"/18"
				8		Med		SAND, some clay and rock	
						Dense		fragments	
		5	17'0"-17'10"	59/120				Same as #4	10"/10"
20				/4				(Note: Auger Refusal at 17 Ft.)	
								END OF BORING AT 17'10"	
25									

SAMPLE IDENTIFICATION
S – SPLIT SPOON
T – THIN WALL TUBE
U – UNDISTURBED PISTON
O – OPEN END ROD
W – WASH SAMPLE

PENETRATION RESISTANCE
140 lb. Wt. falling 30" on 2" O.D. Sampler

Cohesionless Density		Cohesive Consistency	
0-4	Very Loose	0-2	Very Soft
5-9	Loose	3-4	Soft
10-29	Med. Dense	5-8	Med. Stiff
30-49	Dense	9-15	Stiff

PROPORTIONS USED

trace	0-10%
little	10 to 20%
some	20 to 35%
and	35 to 50%

GROUNDWATER OBSERVATIONS
AT 4 FT. AFTER 0 HR
AT AFTER HR
NOTE: Levels may vary with seasonal fluctuation

GEOTECHNICAL ENGINEERING SERVICES
PROPOSED BARBER FOODS BUILDING ADDITIONS
54 & 70 ST. JOHN STREET
PORTLAND, MAINE

00-0695 S December 15, 2000

TABLE OF CONTENTS

S.W.COLE ENGINEERING, INC. • Geotechnical Engineering • Field & Lab Testing • Scientific & Environmental Consulting

00-0695 S

December 15, 2000

Associated Design Partners, Inc.
Attn: Jim Thibodeau, P.E.
70 Leighton Road
Falmouth ME 04105

Subject: Geotechnical Engineering Services
Proposed Barber Foods Building Additions
54 & 70 St. John Street
Portland, Maine

Dear Mr. Thibodeau:

In accordance with our Agreement dated August 31, 2000, we have made a subsurface investigation at the site of the proposed Packout Building and Meat Room Additions to the existing Barber Foods Facility on St. John Street in Portland, Maine. We received verbal authorization to proceed on November 2, 2000. The contents of this report are subject to the limitations set forth in Attachment A.

1.0 INTRODUCTION

1.1 Scope of Work

The purpose of the investigation was to explore the subsurface conditions at the site of the proposed Packout and Meat Room Building Additions in order to provide recommendations relative to foundation design and earthwork associated with the proposed construction. The investigation included the making of three test borings, soils laboratory testing, review and interpretation of existing test boring data, and a geotechnical evaluation of the findings as they relate to the proposed construction.

1.2 Proposed Construction

The proposed Meat Room Addition is located at the southwesterly corner of the existing facility and the Packout Addition at the northwesterly corner. Based on information provided by Associated Design Partners (project civil & structural engineer), we

understand the Meat Room Addition will be a second story, steel-framed addition creating a canopy over an existing loading dock area. It is not known if the existing building, which the Meat Room will adjoin, is supported by a spread footing or driven pile foundation system.

We understand the proposed Packout Addition will be a two-story, dock-high, steel-framed structure with an on-grade floor slab at a finished floor elevation of about 23.5 feet. The first floor will match portions of the existing lower level at about elevation 23.5 feet and will be slightly above an adjacent depressed slab area at about elevation 21.5 feet. We understand the adjoining building was built around 1994 and is supported on a driven H-pile foundation with on-grade slabs. Exterior grades in the area of the proposed Packout Addition range from about elevation 23.5 to 25.5 feet; thus, tapered cuts approaching 2.5 feet will be needed to establish bottom of slab grade.

Based on preliminary information provided by Associated Design Partners, Inc., we understand that column loads for the new additions will range from 80 to 100 kips and may approach 150 kips for transient loads. A plan view of the proposed building addition footprints are shown on the "Exploration Location Plan" attached as Sheet 1.

2.0 EXPLORATION AND TESTING

2.1 Exploration

Three test borings were made at the site by Great Works Test Boring, Inc. of Rollinsford, New Hampshire on November 20, 2000. The test boring locations were selected and established in the field by S.W.COLE ENGINEERING, INC. based on information provided by Associated Design Partners, Inc. The test boring locations are shown on the "Exploration Location Plan" attached as Sheet 1. Logs of the explorations are attached as Sheets 2 through 4. A log of rock core obtained in test boring B-202 is attached as Sheet 5. A key to the notes and symbols used on the logs is attached as Sheet 6. It should be noted that ground surface elevations shown on the logs are based on interpolation between topographic contours shown on site grading plans provided by Associated Design Partners, Inc. Logs of pertinent explorations from a previous geotechnical investigation (by others) in September 1990 are attached in Appendix A.

2.2 Testing

In-situ strength test results are noted on the logs. Laboratory testing was performed on selected soil samples recovered from the test borings. Moisture content and Atterberg Limits test results are noted on the log sheets. The results of two grain size analyses are attached as Sheets 7 and 8.

3.0 SITE AND SUBSURFACE CONDITIONS

3.1 Site Conditions

The site of the proposed Meat Room Addition is mostly concrete pavement with grassed areas on the westerly and easterly edges. An above-ground storage tank exists on the westerly edge. Surface relief in this area generally slopes gently downward from south to north toward the building. The concrete pavements are for the existing truck docks on the southerly face of the building.

The site of the proposed Packout Addition is covered with bituminous pavement or crushed gravel. An existing mechanical building on a concrete pad exists in the central portion of this area. Surface relief in the proposed Packout Addition slopes gently downward from the northeast to the southwest. The paved areas are presently used for car parking.

3.2 Subsurface Conditions

Test boring B-101 was made for the proposed Meat Room Addition. Below a surficial layer of asphaltic pavement, B-101 generally encountered a sequence of dense brown sand with gravel (fill) over medium-dense brown fine to medium sand with silt and gravel overlying dense gray silty sand with gravel (glacial till). The upper stratum of sandy fill was about 1.5 feet thick. The middle stratum of fine to medium sand was about 16 feet thick overlying a thin layer of glacial till. A refusal surface (probable bedrock) was encountered at a depth of about 19.5 feet below the ground surface. The subsurface conditions encountered in test borings TB-1, TB-5, TB-6 and TB-9 from a previous study (by others - September 1990) in this area are generally consistent with the subsurface findings at test boring B-101.

Test borings B-201 and B-202 were made in the area of the proposed Packout Building Addition. Below a surficial layer of asphaltic pavement or crushed gravel, test borings B-201 and B-202 generally encountered a sequence of dense to medium-dense brown to rust-brown sand with gravel (fill) over a deposit of alternating layers of gray fine sand and gray silty clay overlying dense gray silty sand with gravel (glacial till). The upper stratum of sandy fill ranged from 7.5 to 13.5 feet thick. The middle stratum of layered gray fine sand and gray silty clay was encountered at depth of 7.5 to 13.5 feet below the ground surface and varied in thickness from about 11 to 12 feet overlying a thin layer of glacial till. Refusal surfaces (probable bedrock) were encountered at depths of 26.5 and 20.0 feet in borings B-201 and B-202, respectively. A 5-foot bedrock core was collected in boring B-202 between a depth interval of 20 to 25 feet. The bedrock was observed to be fractured gray Schist with an RQD (Rock Quality Designation) of 74 percent. The subsurface conditions encountered in test borings TB-4 and TB-12 from a previous study (by others - September 1990) in this area are generally consistent with the subsurface findings at test borings B-201 and B-202.

Refer to the attached logs, Sheets 2 through 5 and Appendix A, for more detail of the subsurface conditions encountered at the exploration locations.

3.3 Groundwater

Based on moisture conditions of the test boring samples and observations made during drilling, groundwater appeared to be at a depth of about 7 feet or greater below the ground surface at the time of exploration work. Due to the short time period the boreholes remained open, actual long-term groundwater levels were not determined. Groundwater levels will fluctuate seasonally and in response to precipitation, variations in subsurface conditions, nearby tidal influence, construction activities, and other factors.

3.4 Seismic and Frost Conditions

According to BOCA 1999, we interpret the subsurface conditions encountered in the area of the proposed Meat Room Addition to correspond to a soil profile type S_1 with a seismic site coefficient of 1.0. We interpret the subsurface conditions encountered in the area of the proposed Packout Building to correspond to a soil profile type S_3 with a seismic site coefficient of 1.5. The design freezing index for the Portland, Maine area is approximately 1200 Fahrenheit degree-days, which corresponds to a frost penetration

depth on the order of 4.0 feet.

4.0 EVALUATION AND RECOMMENDATIONS

4.1 Foundations

Based on the findings at the exploration locations and our understanding of the project, it is our opinion that the proposed Meat Room Addition can be supported by conventional spread footings; whereas, the proposed Packout Addition should be supported by a driven H-pile foundation system consistent with the foundation system used for the adjacent 1994 addition to reduce the risk of adverse movement due to differential settlement between the proposed and existing construction. In any case, excavations should not undermine existing foundations or pavements.

4.1.1 Meat Room Foundations

Spread footings should bear on a 12-inch (min.) layer of compacted gravel overlying stable deposits of densified native sand. The compacted gravel below footings should meet the requirements of MDOT Standard Specification 703.06 Type-A Gravel and should be densified to at least 95 percent of its maximum dry density as determined by ASTM D-1557. This will require overexcavation of existing native soils below footings to a depth of at least 1 foot. The overexcavated area should extend laterally outward from the edge of footings a distance equal to the depth below the footing (1H:1V slope).

For spread footings founded on properly prepared subgrades, we recommend that design of the proposed Meat Room Addition foundations consider the following geotechnical parameters:

- Allowable Bearing Pressure = 3.0 ksf (properly prepared subgrades, see above)
- Base Friction Factor = 0.45 (mass concrete to granular fill)
- Passive Lateral Earth Pressure Coeff. = 3.3 (compacted granular backfill)
- Unit Weight Compacted Foundation Backfill = 130 pcf (compacted granular backfill)
- Footing Depth for Frost Protection = 4.0 feet
- Seismic Site Coefficient = 1.0 (BOCA 1999 Soil Profile S_1)

We anticipate post-construction settlements will not exceed ½-inch for foundations bearing on properly prepared subgrades.

4.1.2 Packout Building Foundations

In our opinion, steel H-Piles driven to end-bearing in bedrock are best suited for foundation support of the proposed Packout Building Addition considering the subsurface conditions encountered at the site and that the adjoining building is supported on H-Piles. All grade beams, pile caps and foundations exposed to freezing temperatures should extend at least 4.0 feet below exterior finished grade. Alternatively, grade beams and pile caps may be protected with insulation to reduce the depth of embedment for frost protection. The insulation could be particularly useful in the truckdock area and other areas where the adjacent exterior grade may be lowered. S.W.COLE ENGINEERING, INC should be consulted to assist in design of insulation-protected foundations.

We recommend that the piles be driven to refusal in bedrock with cast steel driving shoes for tip protection. Considering the depths to refusal encountered at the test borings, we estimate pile tips will likely range from elevation 5.0 to -5.0 feet (project datum); however, because subsurface conditions vary across the site, the actual tip elevations of driven piles will also vary with location. Considering the proposed column loads of 80 to 100 kips, we recommend the following pile sections and axial compressive capacities. Our estimate of pile capacities assumes a working stress not exceeding 12 ksi in the steel piling and a reduction of the cross-sectional pile area by 1/8-inch on all exposed pile surfaces due to corrosion.

H-Pile Section	Allowable Compressive Capacity
HP12 x 53	80 kips
HP10 x 57	80 kips (see Note 1)
HP10 x 42	60 kips
HP8 x 36	57 kips
Note (1) Available capacity reduced to preclude pile load test	

Post-construction settlement of piles driven into bedrock should not exceed about ½ inch. Piles should be spaced a minimum center-to-center distance of at least 3 pile diameters, but no less than 30 inches. Lateral loads can be resisted by battered piles

and/or by passive earth pressures acting on the grade beams and pile caps provided these foundation elements are backfilled with compacted granular fill.

The piling contractor should submit information relative to the pile driving equipment and proposed stop driving criteria for geotechnical review prior to driving of production piles. We recommend that S.W.COLE ENGINEERING, INC. be on-site during the driving of production piles to monitor vibrations from pile driving, maintain pile driving records and to modify the stop driving criteria, if necessary, based on actual site driving conditions. The BOCA Building Code (1999) requires that a pile load test be performed on piles with design capacities over 40 tons (80 kips); thus, a pile load test is not required for the recommended pile sections.

4.2 Truck Dock Walls

Based on our understanding of the project, we understand the proposed Packout Building will be constructed with truck dock walls. All retaining walls should be backfilled with compacted select fill. We recommend the following soil parameters be considered in backfilled wall design:

A total unit weight of granular backfill (γ_t) = 130 pcf (compacted select fill)
An angle of internal friction = 30 degrees (compacted select fill)
An active lateral earth pressure coefficient (K_a) = 0.33 (compacted select fill)
A passive lateral earth pressure coefficient (K_p) = 3.3 (compacted select fill)
An at-rest lateral earth pressure coefficient (K_o) = 0.50 (compacted select fill)

Walls restrained from rotating should be designed using at-rest lateral earth pressure.

4.3 Floor Slabs

The on-grade floor slab for the Packout Addition may be soil-supported on a base of compacted Type 'A' Gravel over densified existing sandy fill soils. We recommend design consider a subgrade reaction modulus of 300 pci. A vapor retarder to limit the upward migration of moisture and ground vapors should underlie the first floor slab. The vapor retarder should have a permeance that is less than the floor covering being applied on the slab and should be durable enough to withstand puncture during construction. We recommend consulting flooring suppliers relative to selection and installation of acceptable vapor retarder systems for use with their products.

Floor slabs should be wet-cured for a period of at least 7 days after casting to reduce the potential for curling of the concrete and excessive drying/shrinkage. Additionally, we recommend that control joints be installed within floor slabs to accommodate shrinkage in the concrete as it cures. Contraction joints are typically installed at 10 to 15 foot spacing, but should be designed with consideration to slab thickness

4.4 Foundation Drainage

The existing foundation drains should be rerouted and connected to a new perimeter foundation drainage system around the proposed Packout Room Addition. In our opinion, foundation drains do not appear warranted for the proposed Meat Room Addition. The foundation drains should be installed near pile cap subgrade. The underdrain pipe should be 4 or 6-inch diameter (match existing) rigid PVC with perforations of ¼ to ½ inch enveloped with at least 6 inches of crushed stone bedding. The entire crushed stone layer should be wrapped in a non-woven geotextile filter fabric having an apparent opening size of at least 70. The underdrain should have a positive gravity outlet. Exterior foundation backfill should be sealed with a surficial layer of clayey or loamy soil in areas that are not to be paved or occupied by entrance slabs in order to reduce surface water infiltration into the foundation backfill.

4.5 Excavation Work

An erosion control system should be instituted prior to any construction activity at the site to help protect adjacent drainageways. We recommend that pavements and fills be removed from foundation and pile areas. However, as much pavement should remain in-place in order to provide stable construction access and to lessen the potential for erosion. Excavation work will encounter sandy fills and native fine to medium sands with varying amounts of silt and gravel. To reduce disturbance of foundation subgrades, the use of a smooth-edged bucket is recommended.

Groundwater and wet soil conditions may be encountered in the foundation excavations. In our opinion, ditching with sump and pump dewatering techniques should be adequate to control groundwater in excavations less than about 7 feet deep. Deeper excavations, such as for utilities, will likely require braced sheetpile shoring for groundwater cutoff and excavation stability. In any case, excavations must be properly shored and/or sloped in accordance with OSHA trenching regulations to prevent

sloughing and caving of the sidewalls during construction. Further, excavations should not undermine existing foundations or adjacent sidewalks or pavements.

4.6 Backfill and Compaction Requirements

We recommend that compacted granular backfill placed against foundation walls (both inside and out) and below floor slabs and sidewalks meet the gradation requirements for Select Fill. The on-site sandy fills appear suitable for reuse below slabs and as backfill against foundations. The native fine to medium sand with varying amounts of silt and gravel are not suitable for reuse as foundation backfill due to adfreeze considerations. Slab base material should meet the gradation requirements for MDOT Standard Specification 703.06 Type 'A' Gravel. Crushed stone placed around footing drains should meet the gradation requirements for Underdrain Aggregate.

Sieve Size	Percent Finer By Weight	
	Select Fill	Underdrain Aggregate
4 inch	100	----
3 inch	90-100	----
1 inch	----	100
¾ inch	----	90-100
¼ inch	25-90	20-55
# 4	----	0-15
# 10	----	0-5
# 40	0-30	----
# 200	5 max.	1.5 max.

Foundation backfill and fills placed beneath slabs, paved areas and walkways should be compacted to at least 95 percent of its maximum dry density as determined by the Modified Proctor (ASTM-D1557). Backfill against truckdock walls and retaining walls should be compacted to between 92 to 95 percent of ASTM-D1557.

4.7 Weather Considerations

If foundation construction takes place during cold weather, subgrades, foundations, and concrete must be protected during freezing conditions. Concrete must not be placed on frozen soil and once placed, the soil and concrete must be protected from freezing. Further, the on-site fine to medium sands can be sensitive to moisture and as such exposed soil surfaces will be susceptible to disturbance during wet conditions.

Consequently, sitework and construction activities should take appropriate measures to protect exposed soils, particularly when wet. A layer of crushed stone may be necessary over pile cap/grade beam subgrades to provide a stable working surface.

4.8 Construction Quality Assurance Testing

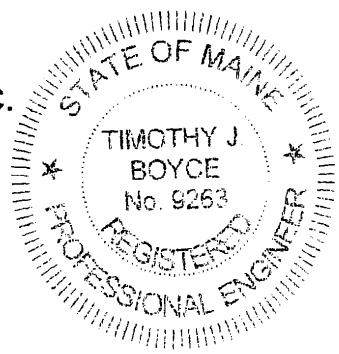
S.W.COLE ENGINEERING, INC. should be retained to provide consultation and quality assurance testing services for the piling, excavation and foundation phases of construction. This is to observe compliance with the design recommendations, drawings and specifications and to allow design changes in the event that subsurface conditions are found to differ from those anticipated prior to the start of construction. S.W.COLE ENGINEERING, INC. is available to provide vibration monitoring, pile installation monitoring, and testing of soils, concrete, masonry, steel and fireproofing.

5.0 CLOSURE

We request the opportunity to review the sitework and foundation design drawings to confirm that our recommendations have been appropriately interpreted and implemented. It has been a pleasure to be of assistance to you with this phase of your project. We look forward to working with you as the design progresses and during the construction phase.

Sincerely,

S.W.COLE ENGINEERING, INC.



Timothy J. Boyce, P. E.
Geotechnical Engineer

G:\Files\Projects\2000\00-0695_ADP_Portland_Proposed Barber Foods Additions\00-0695 Report.doc

Attachment A
Limitations

This report has been prepared for the exclusive use of Associated Design Partners, Inc. for specific application to the Proposed Meat Room and Packout Building Additions to the existing Barber Foods Facility on St. John Street in Portland, Maine. S.W.COLE ENGINEERING, INC. has endeavored to conduct the work in accordance with generally accepted soil and foundation engineering practices. No other warranty, expressed or implied, is made.

The soil profiles described in the report are intended to convey general trends in subsurface conditions. The boundaries between strata are approximate and are based upon interpretation of exploration data and samples.

The analyses performed during this investigation and recommendations presented in this report are based in part upon the data obtained from subsurface explorations made at the site. Variations in subsurface conditions may occur between explorations and may not become evident until construction. If variations in subsurface conditions become evident after submission of this report, it will be necessary to evaluate their nature and to review the recommendations of this report.

Observations have been made during exploration work to assess site groundwater levels. Fluctuations in water levels will occur due to variations in rainfall, temperature, and other factors.

Recommendations contained in this report are based substantially upon information provided by others regarding the proposed project. In the event that any changes are made in the design, nature, or location of the proposed project, S.W.COLE ENGINEERING, INC. should review such changes as they relate to analyses associated with this report. Recommendations contained in this report shall not be considered valid unless the changes are reviewed by S.W.COLE ENGINEERING, INC.

S.W.COLE
ENGINEERING, INC.
GEOTECHNICAL CONSULTANTS

BORING LOG

BORING NO.: B-101
SHEET: 1 OF 1
PROJECT NO.: 00-0695
DATE START: 11/20/00
DATE FINISH: 11/20/00
ELEVATION: 21 +/-
SWC REP.: GWB

PROJECT / CLIENT: PROPOSED BARBER FOODS ADDITION / ASSOCIATED DESIGN PARTNERS, INC.
LOCATION: ST. JOHN STREET / PORTLAND, MAINE
DRILLING FIRM: GREAT WORKS TEST BORINGS DRILLER: DON BOLSTRETCH

	TYPE	SIZE I.D.	HAMMER WT.	HAMMER FALL
CASING:	HW	4"	300 lb	18"
SAMPLER:	SS	1 3/8"	140 lb	30"
CORE BARREL:				

WATER LEVEL INFORMATION
Soils Saturated @ 7.0'

CASING BLOWS PER FOOT	SAMPLE NO.	PEN.	REC.	DEPTH @ BOT	SAMPLER BLOWS PER 6 0-6	6-12	12-18	18-24	DEPTH	STRATA & TEST DATA
									0.3'	BITUMINOUS PAVEMENT
	1D	21"	14"	2.0'	13	32	19	14		BROWN GRAVELLY SAND (FILL)
									1.8'	~ DENSE ~
	2D	24"	18"	7.0'	9	8	10	15		BROWN FINE TO MEDIUM SAND SOME SILT TRACE GRAVEL
	3D	24"	20"	12.0'	7	13	14	20		~ MEDIUM DENSE ~
	4D	24"	6"	17.0'	5	17	8	6	18.0'	
									19.5'	GRAY SILTY SAND SOME GRAVEL (TILL) ~ DENSE ~
										REFUSAL @ 19.5' (PROBABLE BEDROCK)

SAMPLES:
D=SPLIT SPOON
C=3" SHELBY TUBE
U=3.5" SHELBY TUBE

SOIL CLASSIFIED BY:
[ ] DRILLER - VISUALLY
[X] SOIL TECH.-VISUALLY
[X] LABORATORY TEST

REMARKS:
STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY BETWEEN SOIL TYPES AND THE TRANSITION MAY BE GRADUAL.

BORING NO.: B-101

S.W.COLE ENGINEERING, INC.
GEOTECHNICAL CONSULTANTS

BORING LOG

BORING NO.: B-201
SHEET: 1 OF 1
PROJECT NO.: 00-0695
DATE START: 11/20/00
DATE FINISH: 11/20/00
ELEVATION: 25' + / -
SWC REP.: GWB

PROJECT / CLIENT: PROPOSED BARBER FOODS ADDITION / ASSOCIATED DESIGN PARTNERS, INC.
LOCATION: ST. JOHN STREET / PORTLAND, MAINE
DRILLING FIRM: GREAT WORKS TEST BORINGS DRILLER: DON BOLSTRETCH

	TYPE	SIZE I.D.	HAMMER WT.	HAMMER FALL
CASING:	HW	4"	300 lb	18"
SAMPLER:	SS	1 3/8"	140 lb	30"
CORE BARREL:				

WATER LEVEL INFORMATION
Soils Saturated @ 13.5'

CASING BLOWS PER FOOT	SAMPLE NO.	PEN.	REC.	DEPTH @ BOT	SAMPLER BLOWS PER 6" 0-6	6-12	12-18	18-24	DEPTH	STRATA & TEST DATA
									0.3'	BITUMINOUS PAVEMENT
	1D	21"	12"	2.0'	14	30	15	16	3.5'	BLACK-BROWN GRAVELLY SAND (FILL) ~ DENSE ~
	2D	24"	16"	7.0'	13	13	19	20		RUST-BROWN SILTY SAND WITH FRACTURED GRAVEL (FILL) ~ MEDIUM DENSE ~
	3D	24"	14"	12.0'	9	10	11	9	13.5'	
	4D	24"	20"	17.0'	2	4	6	6		W_L = 28 W_P = 14 LAYERED GRAY FINE SAND AND GRAY SILTY CLAY ~ LOOSE ~ AND ~ SOFT ~
	5D	24"	22"	22.0'	3	4	6	4	25.0'	
	6D	18"	10"	26.5'	3	2	21	25/0"	26.5'	GRAY SILTY SAND SOME GRAVEL (TILL) ~ DENSE ~ REFUSAL @ 26.5' (PROBABLE BEDROCK)

SAMPLES:
D=SPLIT SPOON
C=3" SHELBY TUBE
U=3.5" SHELBY TUBE

SOIL CLASSIFIED BY:
[ ] DRILLER - VISUALLY
[X] SOIL TECH.-VISUALLY
[X] LABORATORY TEST

REMARKS:
STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY BETWEEN SOIL TYPES AND THE TRANSITION MAY BE GRADUAL.

BORING NO.: B-201

S.W.COLE
ENGINEERING, INC.
GEOTECHNICAL CONSULTANTS

BORING LOG

BORING NO.: B-202
SHEET: 1 OF 1
PROJECT NO.: 00-0695
DATE START: 11/20/00
DATE FINISH: 11/20/00
ELEVATION: 26 + / -
SWC REP.: GWB

PROJECT / CLIENT: PROPOSED BARBER FOODS ADDITION / ASSOCIATED DESIGN PARTNERS, INC.
LOCATION: ST. JOHN STREET / PORTLAND, MAINE
DRILLING FIRM: GREAT WORKS TEST BORINGS DRILLER: DON BOLSTRETCH

	TYPE	SIZE I.D.	HAMMER WT.	HAMMER FALL
CASING:	HW	4"	300 lb	18"
SAMPLER:	SS	1 3/8"	140 lb	30"
CORE BARREL:				

WATER LEVEL INFORMATION
Soils Saturated @ 7.5'

CASING BLOWS PER FOOT	SAMPLE NO.	PEN.	REC.	DEPTH @ BOT	SAMPLER BLOWS PER 6" 0-6	6-12	12-18	18-24	DEPTH	STRATA & TEST DATA
									0.2'	CRUSHED GRAVEL
	1D	24"	20"	2.0'	10	11	16	16	3.0'	BLACK-BROWN GRAVELLY SAND (FILL) ~ DENSE ~
	2D	24"	0"	7.0'	9	13	5	4	7.5'	BROWN SILTY SAND WITH FRACTURED GRAVEL (FILL) ~ MEDIUM DENSE ~
	3D	24"	16"	12.0'	1	2	2	2		LAYERED GRAY SILTY FINE SAND AND GRAY SILTY CLAY ~ LOOSE ~ AND ~ SOFT ~
	4D	24"	22"	17.0'	1	1	1	2	18.0'	
	5D	0"	0"	20.0'	25/0"				20.0'	GRAY SILTY SAND SOME GRAVEL (TILL) ~ DENSE ~ ~ DENSE ~
	1R	60"		25.0'					25.0'	BEDROCK - WEATHERED SCHIST RQD = 74% (FAIR)
										BOTTOM OF EXPLORATION @ 25.0'

SAMPLES:
D=SPLIT SPOON
C=3" SHELBY TUBE
U=3.5" SHELBY TUBE

SOIL CLASSIFIED BY:
[ ] DRILLER - VISUALLY
[X] SOIL TECH.-VISUALLY
[X] LABORATORY TEST

REMARKS:
STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY BETWEEN SOIL TYPES AND THE TRANSITION MAY BE GRADUAL.

BORING NO.: B-202

S.W.COLE
ENGINEERING, INC.
GEOTECHNICAL CONSULTANTS

ROCK CORE LOG

BORING NO. B-202
PROJECT NO. 00-0965
SHEET 1 OF 1
CORE SIZE NQ2

PROJECT NAME / LOCATION: Barber Foods / Portland
LOGGED BY MTT DATE 11/21/00
CHECKED BY GWB DATE 11/28-00

DEPTH BELOW SURFACE (ft)	CORE RUN	CORE INTERVAL (in)	CORE RECOVERY (in)	RQD%	ROCK QUALITY	GRAPHIC LOG	ROCK DESCRIPTION AND IDENTIFICATION
20.0'							← very fractured
		5.0'	4.4'	3.7/5.0			GRAY Schist - rich w/ quartz veins - fractures @ 60° from horizontal along foliations - weathered - moderately hard
	R1	60"	53"	74%	Fair		
25.0'							← Zone of CORE LOSS
							BOE @ 25.0'

KEY TO THE NOTES & SYMBOLS
Test Boring and Test Pit Explorations

All stratification lines represent the approximate boundary between soil types and the transition may be gradual.

Key to Symbols Used:

w	-	water content, percent (dry weight basis)
q_u	-	unconfined compressive strength, kips/sq. ft. - based on laboratory unconfined compressive test
S_v	-	field vane shear strength, kips/sq. ft.
L_v	-	lab vane shear strength, kips/sq. ft.
q_p	-	unconfined compressive strength, kips/sq. ft. based on pocket penetrometer test
O	-	organic content, percent (dry weight basis)
W_L	-	liquid limit - Atterberg test
W_P	-	plastic limit - Atterberg test
WOH	-	advance by weight of hammer
WOM	-	advance by weight of man
WOR	-	advance by weight of rods
HYD	-	advance by force of hydraulic piston on drill
RQD	-	Rock Quality Designator - an index of the quality of a rock mass. RQD is computed from recovered core samples.
γ_T	-	total soil weight
γ_B	-	buoyant soil weight

Description of Proportions:

0 to 5% TRACE
5 to 12% SOME
12 to 35% "Y"
35+% AND

REFUSAL: **Test Boring Explorations** - Refusal depth indicates that depth at which, in the drill foreman's opinion, sufficient resistance to the advance of the casing, auger, probe rod or sampler was encountered to render further advance impossible or impracticable by the procedures and equipment being used.

REFUSAL: **Test Pit Explorations** - Refusal depth indicates that depth at which sufficient resistance to the advance of the backhoe bucket was encountered to render further advance impossible or impracticable by the procedures and equipment being used.

Although refusal may indicate the encountering of the bedrock surface, it may indicate the striking of large cobbles, boulders, very dense or cemented soil, or other buried natural or man-made objects or it may indicate the encountering of a harder zone after penetrating a considerable depth through a weathered or disintegrated zone of the bedrock.

S. W. COLE ENGINEERING, INC.

R E P O R T O F G R A D A T I O N
ASTM C-117, C-136

Project No. 000695
Date 11/27/2000

Project BARBER FOODS

Client Associated Design Partners

Sample No. B-101, S-2, 5-7'

Sieve Size	Percent Passing	Specifications %
3/4 "	100.0	
1/2 "	97.1	
1/4 "	96.7	
# 4	96.5	
# 10	96.0	
# 20	93.8	
# 40	83.3	
# 60	55.2	
# 100	21.7	
# 200	7.2	

fine to medium SAND some silt trace gravel

S. W. COLE ENGINEERING, INC.

R E P O R T O F G R A D A T I O N
ASTM C-117, C-136

Project No. 000695
Date 11/27/2000

Project BARBER FOODS

Client Associated Design Partners

Sample No. B-101, S-3, 10-12'

Sieve Size	Percent Passing	Specifications %
1/4 "	100.0	
# 4	99.6	
# 10	99.2	
# 20	96.3	
# 40	79.1	
# 60	45.5	
# 100	17.2	
# 200	6.9	

fine to medium SAND some silt

Tel. (508) 583-2680
ENVIRO-TECH DRILLING, INC.
45 Turnpike Street, West Bridgewater
Massachusetts 02379

BORING / WELL LOG

CLIENT Stahlman Engineering

PROJECT Barber Foods
St. John Street
Portland, ME

DRILLER J. Marks
INSPECTOR
LINE & STA OFFSET
SUR. ELEV.
START September 27, 1990
FINISH September 27, 1990

BORING NO. TB1
SHEET 1
OF 1
FILE NO. 90095

	CASING	SAMPLER	CORE BARREL
TYPE	HSA	SS	
SIZE ID	4.25in.	1.375 in.	
HAMMER WT.		140lb.	
HAMM. FALL		30in.	

DEPTH	CSG BLOWS PER FT	SAMPLE NO.	SAMPLE DEPTH RANGE FEET	SAMPLE BLOWS / 6" ON SAMPLER	SAMPLE TYPE	MOISTURE, DENSITY OR CONSIST.	STRAT. CHANGE FEET	SAMPLE CLASSIFICATION AND REMARKS	SAMPLE PEN./ REC.
		1	0'0"-1'6"	13-12	S	Dry		Tan to black, fine to coarse	18"/11"
				8		Med		SAND, trace gravel and silt	
						Dense			
5		2	5'0"-6'6"	8-10	S	Wet		Tan, fine to coarse SAND,	18"/15"
				15		Med		trace fine gravel	
						Dense			
10		3	10'0"-11'6"	5-5	S	Wet		Tan, fine to coarse SAND,	18"/18"
				12		Med		trace fine to medium gravel	
						Dense		and rock fragments	
15		4	15'0"-16'6"	8-13	S	Wet		Tan, fine to medium SAND,	18"/15"
				15		Med		some silt	
						Dense			
20		5	20'0"-21'6"	13-11	S	Wet		Tan to gray, fine to medium	18"/18"
				12		Med	21.0	SAND, some silt and clay	
						Dense			
								AUGER REFUSAL AT 24'6"	
25		6	24'6"-24'6"	120/0"	S			NO RECOVERY	0"/0"
								END OF BORING AT 24'6"	

SAMPLE IDENTIFICATION
S - SPLIT SPOON
T - THIN WALL TUBE
U - UNDISTURBED PISTON
O - OPEN END ROD
W - WASH SAMPLE
A - AUGER SAMPLE

PENETRATION RESISTANCE
140 lb. Wt. falling 30" on 2" O.D. Sampler

Cohesionless Density		Cohesive Consistency	
0-4	Very Loose	0-2	Very Soft
5-9	Loose	3-4	Soft
10-29	Med. Dense	5-8	Med. Stiff
30-49	Dense	9-15	Stiff
50 +	Very Dense	16-30	Very Stiff

PROPORTIONS USED

trace	0-10%
little	10 to 20%
some	20 to 35%
and	35 to 50%

GROUNDWATER OBSERVATIONS
AT 5 FT. AFTER 0 HR
AT AFTER HR

NOTE: Levels may vary with seasonal fluctuatio and the degree of soil saturation when the borin was taken

Tel. (508) 583-2680
ENVIRO-TECH DRILLING, INC.
45 Turnpike Street, West Bridgewater
Massachusetts 02379

BORING / WELL LOG

CLIENT Stahlman Engineering

PROJECT Barber Foods
St. John Street
Portland, ME

DRILLER J. Marks
INSPECTOR
LINE & STA ____ OFFSET ____
SUR. ELEV.
START September 27, 1990
FINISH September 27, 1990

BORING NO. TB4
SHEET 1 OF 1
FILE NO. 90095

	CASING	SAMPLER	CORE BARREL
TYPE	HSA	SS	
SIZE ID	4.25in.	1.375in.	
HAMMER WT.		140lb.	
HAMM. FALL		30in.	

DEPTH	CSG BLOWS PER FT	SAMPLE NO.	SAMPLE DEPTH RANGE FEET	SAMPLE BLOWS / 6" ON SAMPLER	SAMPLE TYPE	MOISTURE, DENSITY OR CONSIST.	STRAT. CHANGE FEET	SAMPLE CLASSIFICATION AND REMARKS	SAMPLE PEN. / REC.
		1	0'0"-1'6"	12-16	S	Dry		Black to brown, fine to coarse	18"/18
				20		Dense		SAND, some gravel, trace coal	
								(FILL)	
5		2	5'0"-6'6"	12-16	S	Wet	5.0	Brown, fine to coarse SAND,	18"/11
				22		Dense	5.5	Gray fine to medium SAND,	
								trace silt	
10		3	10'0"-11'6"	7-10	S	Wet	11.0	Gray SILT, some fine to medium	18"/18
				11		Very		SAND, trace rock fragments and	
						Stiff		clay	
15		4	15'0"-16'6"	3-2	S	Wet		Gray SILT, some fine sand and	18" /
				2		Soft		clay	
20		5	20'0"-21'6"	3-3	S	Wet		Gray SILT, some medium to	18"/15
				2		Med		coarse sand layers	
						Stiff			
25		6	25'0"-26'6"	4-4	S	Wet		Gray fine to medium silty	18"/18
				3		Loose		SAND, trace gravel	
								(Note: Auger Refusal at 28 Ft.)	
30		7	28'0"-28'8"	34/120/2"	S	Wet		Gray fine to coarse silty	8"/8"
						Very		SAND, some gravel and rock	
						Dense		fragments, trace clay	
								END OF BORING AT 28'8"	

SAMPLE IDENTIFICATION
S – SPLIT SPOON
T – THIN WALL TUBE
U – UNDISTURBED PISTON
O – OPEN END ROD
W – WASH SAMPLE

PENETRATION RESISTANCE
140 lb. Wt. falling 30" on 2" O.D. Sampler

Cohesionless Density		Cohesive Consistency	
0-4	Very Loose	0-2	Very Soft
5-9	Loose	3-4	Soft
10-29	Med. Dense	5-8	Med. Stiff
30-49	Dense	9-15	Stiff

PROPORTIONS USED

trace	0-10%
little	10 to 20%
some	20 to 35%
and	35 to 50%

GROUNDWATER OBSERVATIONS
AT 4 Ft. AFTER 0 Hr
AT ____ AFTER ____ Hr
NOTE: Levels may vary with seasonal fluctuati
and the degree of soil saturation when the bori

Tel. (508) 583-2680
ENVIRO-TECH DRILLING, INC.
45 Turnpike Street, West Bridgewater
Massachusetts 02379

BORING / WELL LOG

CLIENT Stahlman Engineering

PROJECT Barber Foods
St. John Street
Portland, ME

DRILLER J. Marks
INSPECTOR
LINE & STA ____ OFFSET ____
SUR. ELEV.
START September 26, 1990
FINISH September 26, 1990

BORING NO. TB5
SHEET 1 OF 1
FILE NO. 90095

	CASING	SAMPLER	CORE BARREL
TYPE	HSA	SS	
SIZE ID	4.25in.	1.375in.	
HAMMER WT.		140lb.	
HAMM. FALL		30in.	

DEPTH	CSG BLOWS PER FT	SAMPLE NO.	SAMPLE DEPTH RANGE FEET	SAMPLE BLOWS / 6" ON SAMPLER	SAMPLE TYPE	MOISTURE, DENSITY OR CONSIST.	STRAT. CHANGE FEET	SAMPLE CLASSIFICATION AND REMARKS	SAMPLE PEN./ REC.
		1	0'0"-1'6"	8-9	S	Moist		Tan to black, fine to medium	18"/13
				9		Med	1.0	silty sand, trace cobble	
						Dense		fragments, trace coal (FILL)	
5		2	5'0"-6'6"	5-6	S	Wet		Brown, fine to medium SAND,	18"/12"
				5		Med		some silty sand layers, trace	
						Dense		clay	
10		3	10'0"-11'6"	2-4	S	Wet		Gray, fine to medium silty	18"/18"
				4		Loose		SAND	
15		4	15'0"-16'11"	10-17	S	Wet		Gray fine to medium silty	21"/22
				34-120/3"		Very		SAND, some rock fragments	
						Dense			
20									
25									

SAMPLE IDENTIFICATION
S – SPLIT SPOON
T – THIN WALL TUBE
U – UNDISTURBED PISTON
O – OPEN END ROD

PENETRATION RESISTANCE
140 lb. Wt. falling 30" on 2" O.D. Sampler

Cohesionless Density		Cohesive Consistency	
0-4	Very Loose	0-2	Very Soft
5-9	Loose	3-4	Soft
10-29	Med. Dense	5-8	Med. Stiff
30-49	Dense	9-15	Stiff
		16-30	Very Stiff

PROPORTIONS USED

trace	0-10%
little	10 to 20%
some	20 to 35%
and	35 to 50%

GROUNDWATER OBSERVATIONS
AT 12 FT. ____ AFTER 0 HR
AT ____ AFTER ____ HR

NOTE: Levels may vary with seasonal fluctuation and the degree of soil saturation when the boring

Tel. (508) 583-2680
ENVIRO-TECH DRILLING, INC.
45 Turnpike Street, West Bridgewater
Massachusetts 02379

BORING / WELL LOG

CLIENT Stahlman Engineering

PROJECT Barber Foods
St. John Street
Portland, ME

DRILLER J. Marks
INSPECTOR
LINE & STA OFFSET
SUR. ELEV.
START September 26, 1990
FINISH September 26, 1990

BORING NO. TB6
SHEET 1 OF 1
FILE NO. 90095

	CASING	SAMPLER	CORE BARREL
TYPE	HSA	SS	
SIZE ID	4.25in.	1.375in.	
HAMMER WT.		140lb.	
HAMM. FALL		30in.	

DEPTH	CSG BLOWS PER FT	SAMPLE NO.	SAMPLE DEPTH RANGE FEET	SAMPLE BLOWS / 6" ON SAMPLER	SAMPLE TYPE	MOISTURE, DENSITY OR CONSIST.	STRAT. CHANGE FEET	SAMPLE CLASSIFICATION AND REMARKS	SAMPLE PEN. / REC.
		1	0'0"-1'6"	8-7	S	Moist		Tan to black, fine to coarse	18"/9"
				11		Med	1.0	SAND, trace silt and fine	
						Dense		gravel, trace root fibers	
								(FILL)	
5		2	5'0"-6'6"	5-6	S	Wet		Brown, silty fine to medium	18"/18"
				7		Med		SAND, trace clay	
						Dense			
10									
		3	10'0"-11'6"	16-7	S	Wet		Gray, fine to medium silty	18"/6"
				6		Med		SAND	
						Dense		(Note: Auger Refusal at 13 Ft.)	
		4	13'0"-13'0"	120/0"	S			NO RECOVERY	0"/0"
15									
								END OF BORING AT 13'0"	
20									
25									

SAMPLE IDENTIFICATION
S – SPLIT SPOON
T – THIN WALL TUBE
U – UNDISTURBED PISTON
O – OPEN END ROD
W – WASH SAMPLE
A – AUGER SAMPLE

PENETRATION RESISTANCE
140 lb. Wt. falling 30" on 2" O.D. Sampler

Cohesionless Density		Cohesive Consistency	
0-4	Very Loose	0-2	Very Soft
5-9	Loose	3-4	Soft
10-29	Med. Dense	5-8	Med. Stiff
30-49	Dense	9-15	Stiff
50 +	Very Dense	16-30	Very Stiff

PROPORTIONS USED

trace	0-10%
little	10 to 20%
some	20 to 35%
and	35 to 50%

GROUNDWATER OBSERVATIONS
AT 3 FT. AFTER 0 HRS
AT AFTER HRS

NOTE: Levels may vary with seasonal fluctuation and the degree of soil saturation when the boring

Tel. (508) 583-2680
ENVIRO-TECH DRILLING, INC.
45 Turnpike Street, West Bridgewater
Massachusetts 02379

BORING / WELL LOG

CLIENT Stahlman Engineering

PROJECT Barber Foods
St. John Street
Portland, ME

DRILLER J. Marks
INSPECTOR
LINE & STA OFFSET
SUR. ELEV.
START September 26, 1990
FINISH September 26, 1990

BORING NO. TB9
SHEET 1
OF 1
FILE NO. 90095

	CASING	SAMPLER	CORE BARREL
TYPE	HSA	SS	
SIZE ID	4.25in.	1.375in.	
HAMMER WT.		110lb.	
HAMM. FALL		30in.	

DEPTH	CSG BLOWS PER FT	SAMPLE NO.	SAMPLE DEPTH RANGE FEET	SAMPLE BLOWS / 6" ON SAMPLER	SAMPLE TYPE	MOISTURE, DENSITY OR CONSIST.	STRAT. CHANGE FEET	SAMPLE CLASSIFICATION AND REMARKS	SAMPLE PEN./ REC.
		1	0'0"-1'6"	7-8	S	Moist		Black loamy SAND and rust to	18"/1
				10		Med	1.5	gray, fine to medium sand	
						Dense		(FILL)	
5		2	5'0"-6'6"	14-22	S	Wet		Gray, fine to medium SAND,	18"/1
				25		Dense		little silt	
10		3	10'0"-11'6"	5-8	S	Wet	10.0	Gray, organic SILT, some	18"/1
				10		Very		fine to medium sand, little	
						Stiff		clay	
15		4	15'0"-16'6"	6-6	S	Wet		Gray, silty fine to coarse	12"/1
				8		Med		SAND, some clay and rock	
						Dense		fragments	
		5	17'0"-17'10"	59/120				Same as #4	10"/1
20				/4				(Note: Auger Refusal at 17 Ft.)	
								END OF BORING AT 17'10"	
25									

SAMPLE IDENTIFICATION
S – SPLIT SPOON
T – THIN WALL TUBE
U – UNDISTURBED PISTON
O – OPEN END ROD

PENETRATION RESISTANCE
140 lb. Wt. falling 30" on 2" O.D. Sampler

Cohesionless Density		Cohesive Consistency	
0-4	Very Loose	0-2	Very Soft
5-9	Loose	3-4	Soft
10-29	Med. Dense	5-8	Med. Stiff
30-49	Dense	9-15	Stiff
		16-30	Very Stiff

PROPORTIONS USED

trace	0-10%
little	10 to 20%
some	20 to 35%
and	35 to 50%

GROUNDWATER OBSERVATIONS
AT 4 FT. AFTER 0
AT AFTER

NOTE: Levels may vary with seasonal fluctuat and the degree of soil saturation when the bor

Tel. (508) 583-2680
ENVIRO-TECH DRILLING, INC.
45 Turnpike Street, West Bridgewater
Massachusetts 02379

BORING / WELL LOG

CLIENT Stahlman Engineering

PROJECT Barber Foods
St. John Street
Portland, ME

DRILLER J. Marks
INSPECTOR
LINE & STA OFFSET
SUR. ELEV.
START September 27, 1990
FINISH September 27, 1990

BORING NO. TB12
SHEET 1
OF 1
FILE NO. 90095

	CASING	SAMPLER	CORE BARREL
TYPE	HSA	SS	
SIZE ID	4.25in.	1.375in.	
HAMMER WT.		140lb.	
HAMM. FALL		30in.	

DEPTH	CSG BLOWS PER FT	SAMPLE NO.	SAMPLE DEPTH RANGE FEET	SAMPLE BLOWS / 6" ON SAMPLER	SAMPLE TYPE	MOISTURE, DENSITY OR CONSIST.	STRAT. CHANGE FEET	SAMPLE CLASSIFICATION AND REMARKS	SAMPLE PEN. / REC.
		1	0'0"-1'6"	12-10	S	Dry		Brown to black, fine to coarse	18"/12"
				14		Med	1.0	SAND, little gravel and rock	
						Dense		fragments (FILL)	
5		2	5'0"-6'6"	7-6	S	Wet	5.0	Gray, fine to coarse SAND,	18"/18"
				3		Med	6.0	gray organic SILT, trace	
						Dense		clay	
10		3	10'0"-11'6"	2-3	S	Wet		Gray, fine to medium, silty	18"/18"
				4		Loose		SAND	
		4	15'0"-16'6"	3-5	S	Wet		Gray, fine to medium silty	18"/18"
				7		Med		SAND, trace coarse sand and	
						Dense		fine gravel	
20		5	20'0"-20'3"	120/3"	S	Wet		Gray silty fine to coarse	3"/3"
						Very		sand, some rock fragments,	
						Dense		trace clay	
25								END OF BORING AT 20'3"	

SAMPLE IDENTIFICATION
S - SPLIT SPOON
T - THIN WALL TUBE
U - UNDISTURBED PISTON
O - OPEN END ROD

PENETRATION RESISTANCE
140 lb. Wt. falling 30" on 2" O.D. Sampler

Cohesionless Density		Cohesive Consistency	
0-4	Very Loose	0-2	Very Soft
5-9	Loose	3-4	Soft
10-29	Med. Dense	5-8	Med. Stiff
30-49	Dense	9-15	Stiff
		16-30	Very Stiff

PROPORTIONS USED

trace	0-10%
little	10 to 20%
some	20 to 35%
and	35 to 50%

GROUNDWATER OBSERVATIONS
AT 4 FT. AFTER 0 HR
AT AFTER HR

NOTE: Levels may vary with seasonal fluctuation and the degree of soil saturation when the boring

ASSOCIATED DESIGN
PARTNERS INC.
80 Leighton Road, Falmouth, ME. 04105

FAX MEMO

DATE: 5/11/08

TO: BARBARA BRADHYDT FAX: 756-8258

FROM: AARON WILSON

PHONE: 207-878-1751 FAX: 207-878-1788

RE: BARBER FOODS
CO_2 TANK

Number of pages including cover sheet: 2

Message

REMOVAL ORDER FOR EXISTING TRAILER ATTACHED. WILL ALSO SEND HARD COPY VIA MAIL.

AARON.

Mobile Offices • Storage Products And More

MICHAEL CUSHING SR.

TARGET DATE: 5-20-08 OFFSITE

Confirmation and Procedures ("Confirmation")
For Scheduled Pick-up of
Standard Leased Equipment

Williams Scotsman, Inc.
325 Riodman Road
Auburn, ME 04210
Phone: 207-783-3200
Fax: 207-783-6183

Please sign and return this document to Fax Number 207-783-6183 as your 30-DAY ADVANCE WRITTEN NOTICE TO TERMINAT the rental of the Equipment ("Equipment") listed below. We will attempt to schedule the pick up as close to your requested date as possible. **This form MUST be completed, signed and returned by fax PRIOR to scheduling your Pick-up.**

Date of Call:	05/02/2008	Early Return?	No
Requested P/U Date:	05/16/2008	Min. Lease Term:	24
Bill Thru Date:		Actual Lease Term:	93
Company Name:	HARBOR FOODS INC	On-Site Takeover?	No
Site Address:	St Johns Street	Equipment Serial #:	CPX-11185
		Size:	64 x 24
		Site Phone:	
City/State	PORTLAND, ME	Fax:	
Site Hours:		Union/Government Site:	Not Applicable
Site Contact:			
Directions:			
Comments/Instructions:			

Thank you for choosing Williams Scotsman. We will make every effort to accommodate the requested pickup date. To ensure a smooth pickup, please review the following off rent process. **Failure to perform any of these procedures will result in additional charges on your final invoice.**

1. Disconnect all utilities, cut back all utility lines (waste/water/electric) to 3" below belly board. (If applicable, blow out **water lines and drain the water heater BEFORE disconnecting utilities.**)
2. Remove all items from the Equipment that were not delivered with the Equipment. Williams Scotsman will not be responsible for storage or the return of any customer owned items and/or any items stored in the Equipment. Items left in the Equipment will be held for 15 days and then disposed of as Williams Scotsman sees fit. Equipment should be broom swept.
3. Remove all skirting, decks, ramps and steps that were not provided by Williams Scotsman.
4. Ensure that all tires, axles, hitches are on the Equipment and operational. If not, notify Williams Scotsman.
5. Return keys with the Equipment to avoid lock replacement charges.
6. Ensure that the Equipment is readily accessible by truck with no obstacles of any kind impeding easy access. There MUST be no less than 120 ft of **UNOBSTRUCTED ACCESS** for our truck to maneuver. (If applicable, remove any snow or ice from the roof of the Equipment, as it cannot be transported unless removed.) Williams Scotsman is **NOT** responsible for site conditions. Should you find that the site is inaccessible due to weather, mud, snow, ice, sugar sand etc. call Williams Scotsman immediately to reschedule your pickup. If a driver is sent to pick up this Equipment and it is not accessible for any reason, you will be billed for wait time and/or an attempted pick-up charge, and the rental agreement shall remain in full force and effect until such time as Williams Scotsman picks up the Equipment.

Name of person authorized to release this Equipment: Michael M. Cushing SR. Facility Eng.
*I acknowledge early termination charges will be billed on the final invoice for the early return of this Equipment, if applicable. In the event the Lease extends beyond the Minimum Lease Term, knockdown and return freight shall be billed at the Lessor's prevailing rate of:
Knockdown: $1,203.00 **Freight:** $712.00

The parties hereby covenant and agree that: a) Notwithstanding anything contained in the rental agreement to the contrary, in the event of any conflict between the rental agreement and the terms of this Confirmation, the terms of this Confirmation will apply; and b) each party hereto may rely on a telefacsimile signature of either party on this Confirmation. Any such signature shall be treated as an original signature and the faxed Confirmation shall be deemed to be the original Confirmation for all purposes.

Customer Signature: Michael M Cushing SR
Printed Name/Title: Michael M Cushing SR
Date: 5-5-08

Williams Scotsman, Inc: _____
Printed Name/Title: _____
Date: _____

Thank you for choosing Williams Scotsman.
It was our pleasure to provide you with temporary space on your project.

ASSOCIATED DESIGN PARTNERS INC.

80 Leighton Road • Falmouth Maine 04105

Office: 207.878.1751
Fax: 207.878.1788
e-mail: adp@adpengineering.com
web: www.adpengineering.com

May 1, 2008 07248

Ms. Barbara Barhydt
Development Review Manager
389 Congress St
Portland, ME 04101

Re: Proposed CO2 Tank
Barber Foods Facility
70 St. John Street
Portland, ME 04112

Dear Barbara,

Associated Design Partners, Inc. is pleased to submit this application and supporting documents for exemption from site plan review relating to the proposed construction of a 15,000 Gal CO2 tank at the existing Barber Foods Facility at 70 St. John Street.

A summary of the project scope is listed below:

1. Remove Existing Mobile Training Office Trailer (see attached photos).
2. Install new 16ftx16ft concrete pile supported foundation (see attached aerial photo and partial site plan C101 for proposed location).
3. Erect new 40ft, 15,000 gallon Carbon Dioxide storage tank, supplied by BOC gasses.

An itemized list in accordance with Art V, Sec 14-523 is presented below:

a. The concrete pad is outside the existing facility envelope, but is less than 500sf
b. The footprint increase is 256 sf.
c. There are no new curb cuts, driveways, or parking areas proposed.
d. The curbs and sidewalks along St. John St are in sound condition as can be verified by Public Works Authority.
e. The construction of the CO2 tank does not require additional or reduced parking.
f. There are no known stormwater management issues, and the construction of the tank will not change the stormwater management characteristics of the site.
g. There are no known evident deficiencies in screening from adjoining properties. The tank will be located on the west side of the building and screened by the building from St. John St.
h. Existing utilities that serve the building are adequate to accommodate the proposed CO2 tank. No work is proposed in the public right of way.

Please find the Exemption for Site Plan Review Application, photos of the existing site, and partial site plan. If you have any questions regarding this project or the information contained within, please do not hesitate to call.

Sincerely,

Aaron S Wilson, P. E.
Engineering Project Manager
Associated Design Partners Inc
ASW

City of Portland, Maine
Planning And Urban Development
Application For Exemption From Site Plan Review

Barber Foods	(207) 772-1934	05/01/2008	20080065
Applicant	**Phone**	**Application Date**	**Application ID**
P.O. Box 4821		Exemption	
Address		**Project Name/Description**	
Portland ME 04112		070 A001001	
City State Zip		**CBL**	
Aaron Wilson/Associated Design Part	(207) 878-1751	70 St John St	
Consultant/Agent	**Phone**	**Address of Proposed Site**	

Descsription of Proposed Development:

Construction of 16'x16' pile-supported foundation and erection of 15,000-gallon CO2 tank.

PLEASE ATTACH SKETCH/PLAN OF PROPOSAL/DEVELOPMENT

Criteria for Exemptions:	**Applicant (Yes, No, N/A)**	**Planning Office**
a) within existing structures: No New Buildings, Demolitions or Additions	No	No
b) footprint increase less than 500 sq ft	Yes	Yes
c) no new curb cuts, driveways, parking areas	Yes	Yes
d) curbs and sidewalks in sound condition and comply with ADA	Yes	Yes
e) no additional parking / no traffic increase	Yes	Yes
f) no stormwater problems	Yes	Yes
g) sufficient property screening	Yes	Yes
h) adequate utilities	Yes	Yes

Planning Office Use Only:

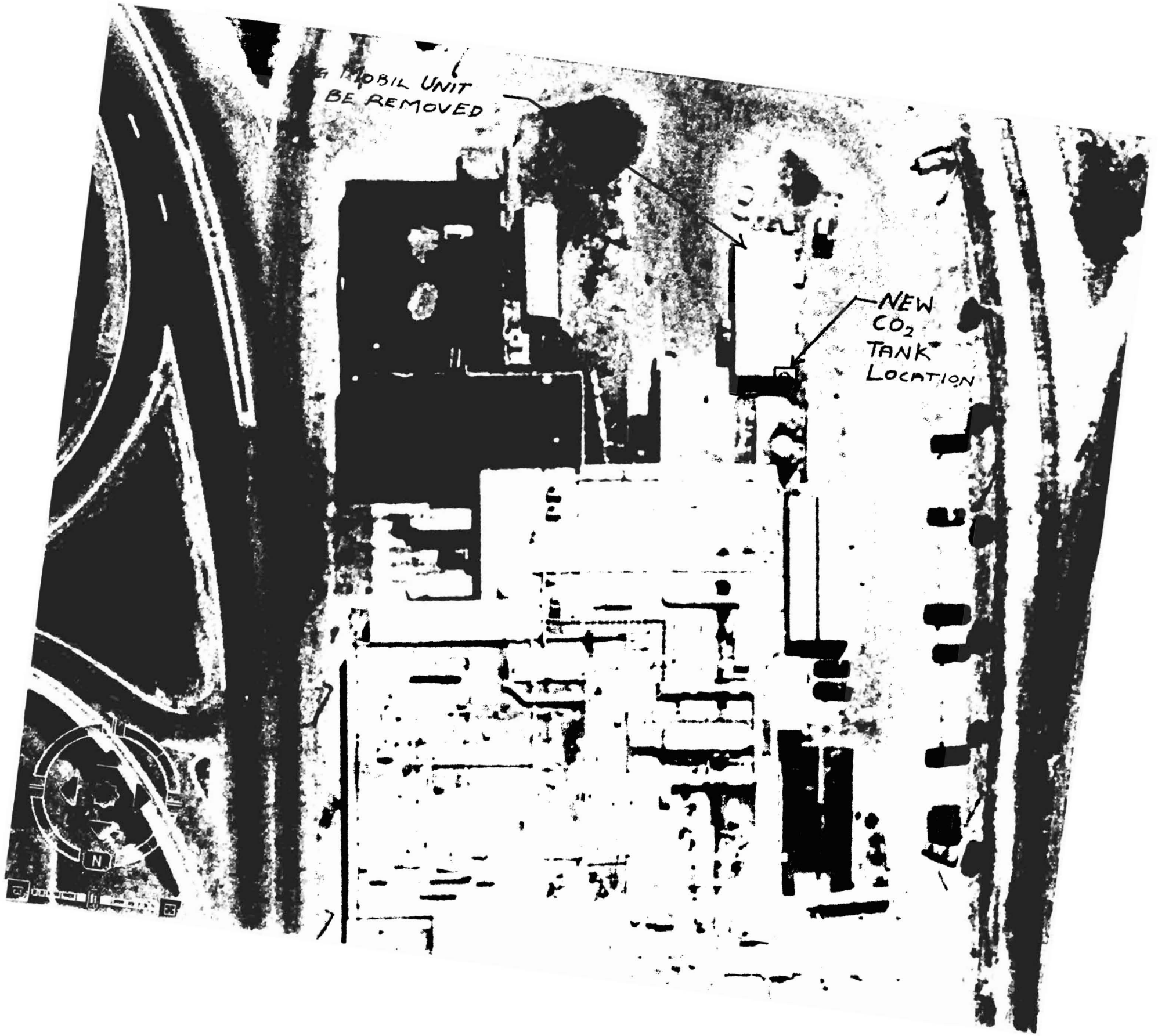
Exemption Granted 05/21/2008 **Partial Exemption** _____ **Exemption Denied** _____

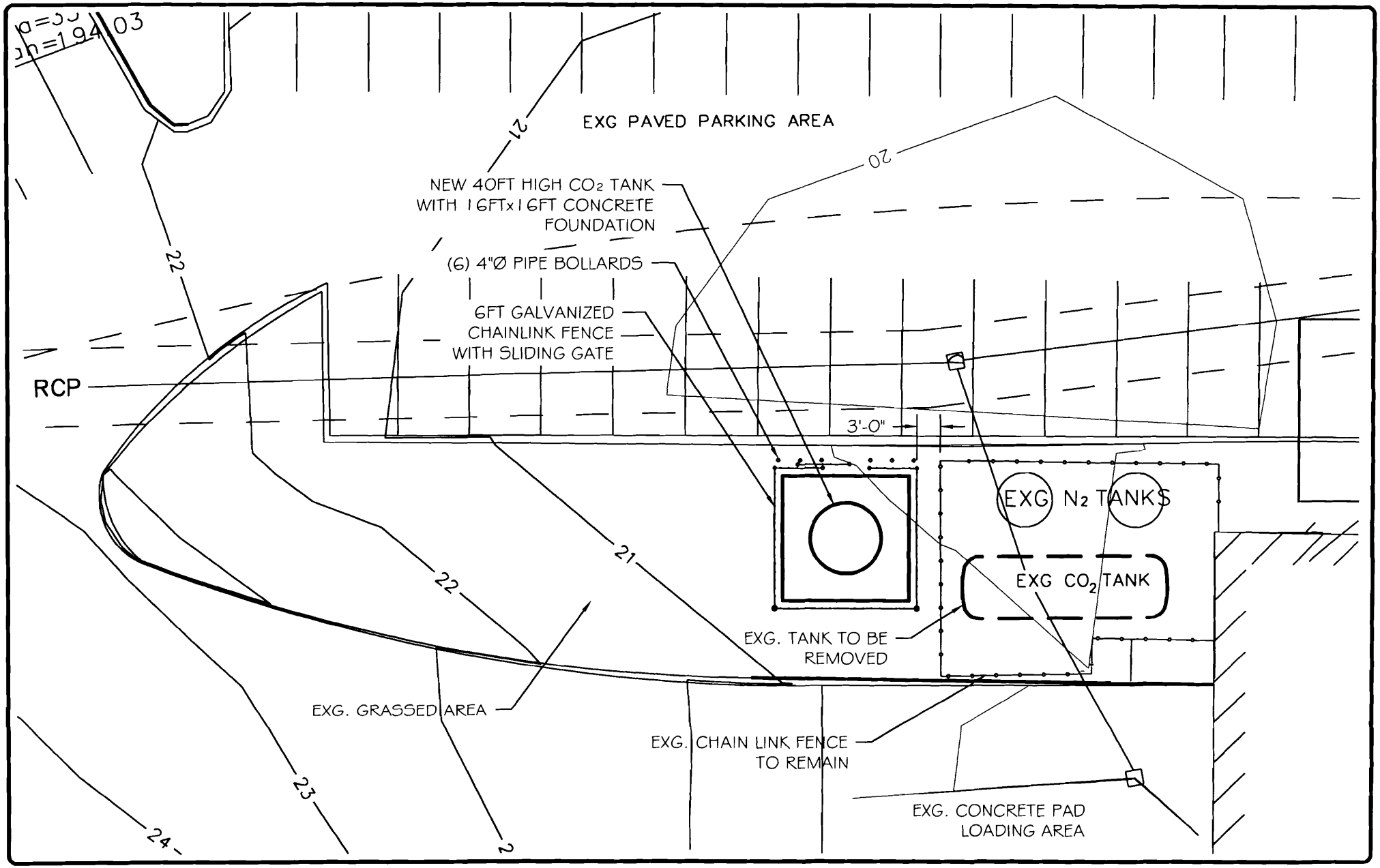
Conditions (if any)

Dept	***Condition***
Planning	A building permit for this installation is required. Please contact Portland's Inspection Division to apply for the appropriate permits.

_____ _____

Planner's Signature **Date**


MOBIL UNIT
BE REMOVED
NEW
CO_2
TANK
LOCATION



ASSOCIATED DESIGN PARTNERS INC.

80 Leighton Road
Falmouth, Maine 04105

Office: (207) 878-1751
Fax: (207) 878-1788
E-Mail: adp@adpengineering.com

PROJECT: **BARBER FOODS CO2 TANKS**
PORTLAND MAINE
FOR: Owner

SHEET TITLE:
SITE SKETCH PLAN AT PROPOSED CO2 TANK

REVISIONS

No.	BY	DESCRIPTION	DATE
1			
2			
3			
4			

DATE : 5-1-08
SCALE : AS NOTED
DESIGN BY: ASW
DRAWN BY: RSC
FILE #: 07249-C101.dwg
PROJECT NUMBER: 07249
SHEET NO: C101

#F2008 0065

APPLICATION FOR EXEMPTION FROM SITE PLAN REVIEW

Applicant

Application Date

Applicant's Mailing Address

Project Name/Description

Consultant/Agent/Phone Number

Address of Proposed Site

CBL: 070 A001

Description of Proposed Development:

Please Attach Sketch/Plan of Proposal/Development

Criteria for Exemptions:
See Section 14-523 (4) on back side of form

	Applicant's Assessment (Yes, No, N/A)	Planning Office Use Only
a) Within Existing Structures; No New Buildings, Demolitions or Additions		
b) Footprint Increase Less Than 500 Sq. Ft.		
c) No New Curb Cuts, Driveways, Parking Areas		
d) Curbs and Sidewalks in Sound Condition/Comply with ADA		
e) No Additional Parking/ No Traffic Increase		
f) No Stormwater Problems		
g) Sufficient Property Screening		
h) Adequate Utilities		

MAY 28 2008

Planning Division Use Only

Exemption Granted X Partial Exemption ✓ Exemption Denied _____