Structural Special Inspections Report

Florence House

190 Valley Street
Portland, Maine
March 8, 2010

Report prepared by:

Structural Engineer of Record
Becker Structural Engineers, Inc.
75 York Street
Portland, Maine 04101

Florence House

Portland, Maine
March 8, 2010

Table of Contents

EXHIBIT A

01000 Special Inspections - General

BECKER
structural engineers, inc.

Statement of Special Inspections

Florence House
190 Valley Street
Portland, Maine

Statement Prepared by
Structural Engineer of Record:
Becker Structural Engineers, Inc.
75 York Street
Portland, ME 04101
207. 879. 1838

Owner:
Avesta Florence House, LP
307 Cumberland Ave
Portland, ME 04101

Architect of Record:
Gawron Turgeon Architects
29 Black Point Road
Scarborough, ME 04074
207.863.6307

Contractor:
Ganneston Construction
3025 North Belfast Avenue
Augusta, Maine 04332
207.621.8505

Project: Florence House
Date Prepared: September 2, 2008

Structural Statement of Special Inspections

Project: *Florence House*

Location: *190 Valley Street, Portland, Maine*

Owner: *Avesta Florence House, LP*

This *Statement of Special Inspections* encompass the following discipline: **Structural**

This *Statement of Special Inspections* is submitted as a condition for permit issuance in accordance with the Special Inspection and Structural Testing requirements of the Building Code. It includes a schedule of Special Inspection services applicable to this project as well as the name of the Structural Special Inspection Coordinator (SSIC) and the identity of other approved agencies to be retained for conducting these inspections and tests.

The Structural Special Inspection Coordinator shall keep records of all Structural inspections and shall furnish inspection reports to the Building Code Official (BCO) and the Structural Registered Design Professional in Responsible Charge (SRDP). Discovered discrepancies shall be brought to the immediate attention of the Contractor for correction. If such discrepancies are not corrected, the discrepancies shall be brought to the attention of the Building Official and the Structural Registered Design Professional in Responsible Charge. The Special Inspection program does not relieve the Contractor of his or her responsibilities.

Interim reports shall be submitted to the Building Official and the Structural Registered Design Professional in Responsible Charge at an interval determined by the SSIC and the BCO.

A *Final Report of Special Inspections* documenting completion of all required Special Inspections, testing and correction of any discrepancies noted in the inspections shall be submitted to the BCO prior to issuance of a Certificate of Use and Occupancy.

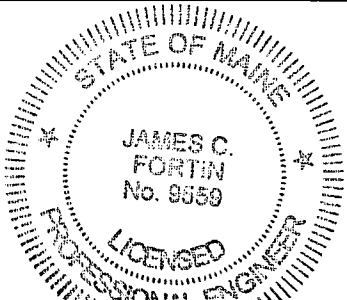
Job site safety and means and methods of construction are solely the responsibility of the Contractor.

Interim Report Frequency: ☒ *Upon request of Building Official* _____ or ☐ per attached schedule.

Prepared by:

James Fortin, P.E.
(type or print name of the Structural Registered Design Professional in Responsible Charge)

Signature — *09/02/2008* Date



Design Professional Seal

Owner's Authorization:

Signature Date

Building Code Official's Acceptance:

Signature Date

Structural Statement of Special Inspections (Continued)

List of Agents

Project: *Florence House*

Location: *190 Valley Street, Portland, Maine*

Owner: *Avesta Florence House, LP*

This *Statement of Special Inspections* encompass the following discipline: **Structural**

(Note: *Statement of Special Inspections* for other disciplines may be included under a separate cover)

This Statement of Special Inspections / Quality Assurance Plan includes the following building systems:

- ☒ Soils and Foundations
- ☒ Cast-in-Place Concrete
- ☐ Precast Concrete System
- ☒ Masonry Systems
- ☒ Structural Steel
- ☒ Wood Construction
- ☐ Special Cases

Special Inspection Agencies	Firm	Address, Telephone, e-mail
1. STRUCTURAL Special Inspections Coordinator (SSIC)	*James C. Fortin* *Becker Structural Engineers*	*75 York Street* *Portland, Maine 04101* *(207) 879-1838* *jim@beckerstructural.com*
2. Special Inspector (SI 1)	*James C. Fortin* *Becker Structural Engineers*	*75 York Street* *Portland, Maine 04101* *(207) 879-1838* *jim@beckerstructural.com*
3. Special Inspector (SI 2)	*Bill Peterlein, P.E.* *Summit Geoengineering Services*	*640 Main Street* *Lewiston, Maine 04240* *(207) 795-6009* *bpeterlein@summitenv.com*
4. Testing Agency (TA 1)	*Darrell Gilman* *Summit Environmental Consultants*	*640 Main Street* *Lewiston, Maine 04240* *(207) 795-6009* *dgilman@summitenv.com*
5. Testing Agency (TA 2)		
6. Other (O1)		

Note: The inspectors and testing agencies shall be engaged by the Owner or the Owner's Agent, and not by the Contractor or Subcontractor whose work is to be inspected or tested. Any conflict of interest must be disclosed to the Building Official, prior to commencing work.

Structural Statement of Special Inspections (Continued)

Final Report of Special Inspections (SSIC/SI 1)

[To be completed by the Structural Special Inspections Coordinator (SSIC/SI 1). Note that all Agent's Final Reports must be received prior to issuance.]

Project: *Florence House*

Location: *190 Valley Street, Portland, ME*

Owner: *Avesta Florence House, LP*

Owner's Address: *307 Cumberland Avenue*
Portland, ME 04101

Architect of Record: *Rebecca Dillon* (name) — *Gawron Turgeon Architects* (firm)

Structural Registered Design Professional in Responsible Charge: *Paul B Becker, P.E.* (name) — *Becker Structural Engineers, Inc* (firm)

To the best of my information, knowledge and belief, the Special Inspections required for this project, and itemized in the *Statement of Special Inspections* submitted for permit, have been performed and all discovered discrepancies have been reported and resolved.

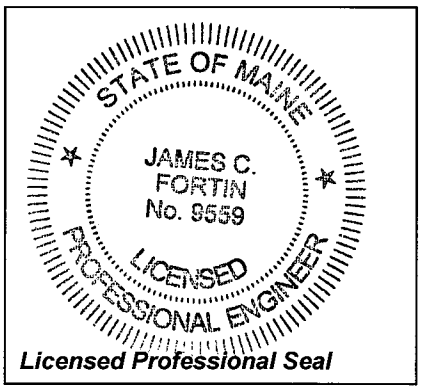
Interim reports submitted prior to this final report form a basis for and are to be considered an integral part of this final report.

Respectfully submitted,
Structural Special Inspection Coordinator

JAMES FORTIN
(Type or print name)

BECKER STRUCTURAL ENGINEERS, INC.
(Firm Name)

James Fortin — 3/5/10
Signature — Date



Licensed Professional Seal

SUMMIT

ENVIRONMENTAL CONSULTING • GEOTECHNICAL ENGINEERING • CONSTRUCTION MATERIALS TESTING

March 10, 2010
Summit #14194

Becker Structural Engineers, Inc.
75 York Street
Portland, ME 04101
Attn: Jim Fortin

Reference: Testing and Inspection Activity Certification
Florence House Project

Dear Jim:

Summit Geoengineering Services (Summit) has provided materials testing and special inspection services to Avesta Florence House, LP and Florence House Housing Corporation during construction activities at the Florence House project located on Valley Street in Portland, Maine. Summit's responsibilities for these services were identified in the Statement of Statement of Special Inspections dated September 2, 2008 and prepared by Becker Structural Engineers, Inc.

Summit was present to observe construction activities assigned within the earthwork, cast-in-place concrete and masonry sections of the Statement of Structural Special Inspections during construction of the Florence House. Summit subcontracted Quality Assurance Laboratories to provide AWS certified inspection of structural steel welding activities, as required. Summit certifies that, based on on-site observations and related test results, inspection activities were completed in accordance with the supplied project design plans and specifications.

Attached please find a signed copy of the Special Inspector's/Agent's Final Report form for the project. In addition, Darrell Gilman of Summit will provide you with a summary of laboratory testing results and onsite inspection observations for your records.

Thank you for the opportunity to work with Becker Structural Engineers on this project. Please contact us if there are any questions regarding the information provided.

Sincerely yours,
Summit Geoengineering Services

Michael J. Walsh, P.E.
Senior Engineer

Structural Statement of Special Inspections (Continued)

Special Inspector's/Agent's Final Report

Project: *Florence House*

Special Inspector or Agent: Michael J. Walsh (name) Summit Geoengineering Services (firm)

Designation:

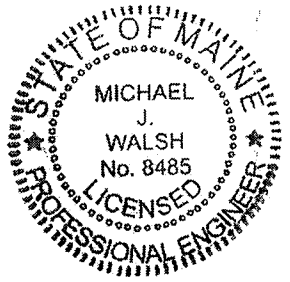
To the best of my information, knowledge and belief, the Special Inspections or testing required for this project, and designated for this Inspector/Agent in the *Statement of Special Inspections* submitted for permit, have been performed and all discovered discrepancies have been reported and resolved.

Interim reports submitted prior to this final report form a basis for and are to be considered an integral part of this final report.

Respectfully submitted,
Special Inspector or Agent:

Michael J. Walsh
(Type or print name)

Michael J. Walsh 3/10/10
Signature Date



Licensed Professional Seal or
Certification Number

01000.5 Disclaimers and Qualifications

The program of Structural/Special Tests and Inspections does not relieve the Contractor or its Subcontractors of their responsibilities and obligations for quality control of the work, for any design work which is included in the scope of services, and for full compliance with the requirements of the Construction Documents. Furthermore, the detection of, or the failure to detect, deficiencies or defects in work during testing and inspection conducted pursuant to the Program does not relieve the Contractor or its subcontractors of their responsibility to correct all deficiencies or defects, whether detected or undetected, in all parts of work, and to otherwise comply with all requirements of the Construction Documents. No warrantee is expressed or implied by the issuance of this document. Additional disclaimers and/or qualifications may be included in the Owner-Special Inspection agreement.

EXHIBIT B

01000 Qualifications of Inspectors and Testing Technicians
Schedule of Structural Inspections

Structural Schedule of Special Inspections

Qualifications of Inspectors and Testing Technicians

The qualifications of all personnel performing Special Inspection and testing activities are subject to the approval of the Building Official. The credentials of all Inspectors and testing technicians shall be provided to the Special Inspector for their records. *NOTE VERIFICATION THAT QUALIFIED INDIVIDUALS ARE AVAILABLE TO PERFORM STIPULATED TESTING AND/OR INSPECTION SHOULD BE PROVIDED PRIOR TO SUBMITTING STATEMENT. AGENT QUALIFICATIONS IN SCHEDULE ARE SUGGESTIONS ONLY; FINAL QUALIFICATIONS ARE SUBJECT TO THE DISCRETION OF THE REGISTERED DESIGN PROFESSIONAL PREPARING THE SCHEDULE.*

Key for Minimum Qualifications of Inspection Agents:

When the Registered Design Professional in Responsible Charge or Special Inspector of Record deems it appropriate that the individual performing a stipulated test or inspection have a specific certification, license or experience as indicated below, such requirement shall be listed below and shall be clearly identified within the schedule under the Agent Qualification Designation.

PE/SE	Structural Engineer – a licensed SE or PE specializing in the design of building structures
PE/GE	Geotechnical Engineer – a licensed PE specializing in soil mechanics and foundations
EIT	Engineer-In-Training – a graduate engineer who has passed the Fundamentals of Engineering examination

Experienced Testing Technician

ETT	Experienced Testing Technician – An Experienced Testing Technician with a minimum 5 years experience with the stipulated test or inspection

American Concrete Institute (ACI) Certification

ACI-CFTT	Concrete Field Testing Technician – Grade 1
ACI-CCI	Concrete Construction Inspector
ACI-LTT	Laboratory Testing Technician – Grade 1&2
ACI-STT	Strength Testing Technician

American Welding Society (AWS) Certification

AWS-CWI	Certified Welding Inspector
AWS/AISC-SSI	Certified Structural Steel Inspector

American Society of Non-Destructive Testing (ASNT) Certification

ASNT	Non-Destructive Testing Technician – Level II or III.

International Code Council (ICC) Certification

ICC-SMSI	Structural Masonry Special Inspector
ICC-SWSI	Structural Steel and Welding Special Inspector
ICC-SFSI	Spray-Applied Fireproofing Special Inspector
ICC-PCSI	Prestressed Concrete Special Inspector
ICC-RCSI	Reinforced Concrete Special Inspector

National Institute for Certification in Engineering Technologies (NICET)

NICET-CT	Concrete Technician – Levels I, II, III & IV
NICET-ST	Soils Technician - Levels I, II, III & IV
NICET-GET	Geotechnical Engineering Technician - Levels I, II, III & IV

Other

EXHIBIT B

02300 Soil and Foundation Construction

Project: Florence House
Date Prepared: September 2, 2008

Structural Schedule of Special Inspections

SOILS & FOUNDATION CONSTRUCTION

VERIFICATION AND INSPECTION IBC Section 1704.7, 1704.8, 1704.9	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS	AGENT	AGENT QUALIFICATION	TASK COMPLETED
1. Verify existing soil conditions, fill placement and load bearing requirements						
a. Prior to placement of prepared fill, determine that the site has been prepared in accordance with the approved soils report.	Y	P	IBC 1704.7.1	SI2	PE/GE, EIT or ETT	
b. During placement and compaction of fill material, verify material being used and maximum lift thickness comply with the approved soils report.	Y	P	IBC 1704.7.2	SI2	PE/GE, EIT or ETT	
c. Test in-place dry density of compacted fill complies with the approved soils report.	Y	p	IBC 1704.7.2	TA1	PE/GE, EIT or ETT	
2. Pile foundations:						
a. Observe and record procedures for static load testing of piles.	N/A	C	IBC 1704.8		PE/GE, EIT or ETT	
b. Observe and record procedures for dynamic load testing of piles.	N/A	C			PE/GE, EIT or ETT	
c. Record installation of each pile and results of load test. Include cutoff and tip elevations of each pile relative to permanent reference.	N/A	C			PE/GE, EIT or EIT	
d. Test welded splices of steel piles	N/A	C	AWS D1.1		AWS-CWI	
3. Pier foundations: Verify installation of pier foundations for buildings assigned to Seismic Design Category C, D, E or F.	N/A	C	IBC 1704.9		PE/GE, EIT or EIT	
a. Verify pier diameter and length	N/A	C			PE/GE, EIT or ETT	
b. Verify pier embedment (socket) into bedrock	N/A	P			PE/GE, EIT or ETT	
c. Verify suitability of end bearing strata	N/A	P			PE/GE, EIT or ETT	

Project: Florence House
Date Prepared: September 2, 2008

Structural Schedule of Special Inspections

SOILS & FOUNDATION CONSTRUCTION

VERIFICATION AND INSPECTION IBC Section 1704.7, 1704.8, 1704.9	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS	AGENT	AGENT QUALIFICATION	TASK COMPLETED
1. Verify existing soil conditions, fill placement and load bearing requirements						
a. Prior to placement of prepared fill, determine that the site has been prepared in accordance with the approved soils report.	Y	P	IBC 1704.7.1	SI2	PE/GE, EIT or ETT	
b. During placement and compaction of fill material, verify material being used and maximum lift thickness comply with the approved soils report.	Y	P	IBC 1704.7.2	SI2	PE/GE, EIT or ETT	
c. Test in-place dry density of compacted fill complies with the approved soils report.	Y	p	IBC 1704.7.2	TA1	PE/GE, EIT or ETT	
2. Pile foundations:						
a. Observe and record procedures for static load testing of piles.	N/A	C	IBC 1704.8		PE/GE, EIT or ETT	
b. Observe and record procedures for dynamic load testing of piles.	N/A	C			PE/GE, EIT or ETT	
c. Record installation of each pile and results of load test. Include cutoff and tip elevations of each pile relative to permanent reference.	N/A	C			PE/GE, EIT or ETT	
d. Test welded splices of steel piles	N/A	C	AWS D1.1		AWS-CWI	
3. Pier foundations: Verify installation of pier foundations for buildings assigned to Seismic Design Category C, D, E or F.	N/A	C	IBC 1704.9		PE/GE, EIT or ETT	
a. Verify pier diameter and length	N/A	C			PE/GE, EIT or ETT	
b. Verify pier embedment (socket) into bedrock	N/A	P			PE/GE, EIT or ETT	
c. Verify suitability of end bearing strata	N/A	P			PE/GE, EIT or ETT	

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: 626-9094

DAILY FIELD REPORT

Date: 2/3/09

Project: Avesta Florence House

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Preconstruction earthwork meeting.

Work Activities: Met onsite with Alan Nichols, Ganneston Construction, and RE Coleman Excavation personnel to discuss earthwork requirements related to the project Getechnical Report prepared by Summit. The major points to the discussion were as follows:

1. Ganneston proposes to stage the preload due to site constraints. Summit has no issue with this approach provided the required materials, placement techniques, and preload timelines are adhered to for both locations.
2. Underlying frost within the preload areas is to be removed and subgrade proof rolled prior to placement of the Structural Fill.
3. Proposed backfill materials should be submitted for laboratory analysis to confirm compliance with design requirements.
4. Summit requests notification to coordinate compaction testing of preload fill lifts to confirm required densities are achieved.
5. Summit recommends that settlement monitoring plates be installed in each of the preload areas, surveyed and elevations recorded daily through the preload periods to confirm the fill has adequately compressed underlying soils prior to building foundation construction.

Test Results: None.

Remarks: See above.

Portal to Portal		**Expenses**		**Signed:**	Michael J. Walsh, P.E.
Leave:	8:30am	Mileage:	11	**cc:**	
Return:	10:00am	Density Gauge:			
TOTAL:	1.5 hrs	Other:			

Sent: 2/3/09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: 626-9094

DAILY FIELD REPORT

Date: 3/11/09

Project: Avesta Florence House

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Soil sampling.

Work Activities: Obtained a sample of the existing onsite granular borrow for laboratory testing to the meet the project specification for structural fill and/or surcharge fill.

Test Results:

Remarks:

Portal to Portal		**Expenses**		**Signed:**	Darrell A. Gilman
Leave:	1:30	Mileage:	11	**cc:**	
Return:	2:30	Density Gauge:			
TOTAL:	1	Other:			

Reviewed: Darrell A. Gilman, CMT Manager
Sent: 3/11/09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: 626-9094

DAILY FIELD REPORT

Date: 3/30/09

Project: Avesta Florence House

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Compaction report from onsite test performed on 3-27-09.

Work Activities: A crew from Coleman Excavation placed and compacted 4" minus material along N-line from line 8.2 to 16. The material was compacted with a vibratory plate compactor.
Sample # 14194.1-S4 maximum density determined to be 136.5pcf at 6.0% moisture.

Test Results: Compaction range: 96.6% to 98.8%
All tests performed on line "N" from 8.2 to 16 exceed the 95% compaction requirement.

Remarks:

Portal to Portal		**Expenses**		**Signed:**	Darrell Gilman
Leave:	12:30	Mileage:		**cc:**	
Return:	12:46	Density Gauge:			
TOTAL:	0.25	Other:			

Reviewed: Darrell A. Gilman, CMT Manager
Sent: 3/31/09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: 626-9094

DAILY FIELD REPORT

Date: 4/10/09

Project: Avesta Florence House

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Special Inspections - Soils.

Work Activities: Upon arrival at the job site, Mr. Dick Beaulieu informed me that the embankment had caved in on the area of footing subgrade that was to be inspected per IBC section 1704.1, 1704.8 and 1704.9 - 1 - A. A small area that had been excavated was visible, though not enough to make a determination on the entire area. Mr. Beaulieu said that he would call to reschedule.

Test Results: N/A

Remarks:

Portal to Portal		**Expenses**		**Signed:**	Frank Clark
Leave:	11:00am	Mileage:	11	**cc:**	
Return:	12:45pm	Density Gauge:			
TOTAL:	1.75	Other:	1.75 tolls		

Reviewed: Darrell A. Gilman, CMT Manager
Sent: 4/14/09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: 626-9094

DAILY FIELD REPORT

Date: 4/13/09

Project: Avesta Florence House

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Soil Compaction Testing.

Work Activities: A crew from Coleman Excavation placed approximately a 12" lift and then compacted the 4" minus material along F-line from line 7 to 16 per special inspection, section 1704.7.2, 1-B. The material was compacted with a 5-ton vibratory roller. The area was tested for density per special inspection, section 1704.7.2, 1-C. All tests taken exceed the required 95 percent compaction. Prior to compaction the area was inspected and found to be suitable per special inspection, section 1704.7.1, 1-A.

Test Results:
Percent compaction: 95.5 to 98
Dry density: 130.4 to 133.8 pcf
Wet density: 136.9 to 137.8 pcf
Percent moisture : 3.0 to 3.6%

Remarks:

Portal to Portal

Leave:		**Expenses**		**Signed:**	Frank Clark
Return:	11:45am	Mileage:	11	**cc:**	
TOTAL:	2:30pm	Density Gauge:	1		
	2.75	Other:			

Reviewed: Darrell A. Gilman, CMT Manager
Date: 4/17/09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: 626-9094

DAILY FIELD REPORT

Date: 4/20/09

Project: Avesta Florence House

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Soil Compaction Testing.

Work Activities: A crew from Coleman Excavation placed approximately a 12" lift and then compacted the 4" minus material along A-line to line 7 per special inspection, section 1704.7.2, 1-B. The material was compacted with a 5-ton vibratory roller. The area was tested for density per special inspection, section 1704.7.2, 1-C. All tests taken exceed the required 95 percent compaction. Prior to compaction the area was inspected and found to be suitable per special inspection, section 1704.7.1, 1-A.
Picked up two sets of four test cylinders for lab testing and controlled storage.

Test Results:
Percent compaction: 97.6 to 97.7
Dry density: 126.4 to 126.5 pcf
Wet density: 130.1 to 130.5 pcf
Percent moisture : 3.7 to 4.0%

Remarks:

Portal to Portal

Leave:		**Expenses**		**Signed:**	Mike Sullivan
Return:	9:00	Mileage:	11	**cc:**	
TOTAL:	10:00	Density Gauge:	1		
	1	Other:			

Reviewed: Darrell A. Gilman, CMT Manager
Date: 4/21/09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: 626-9094

DAILY FIELD REPORT

Date: 4/28/09

Project: Avesta Florence House

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Soil Compaction Testing.

Work Activities: A crew from Coleman Excavation placed approximately a 12" lift and then compacted the 4" minus material along G-1,L.8-1,N-1&N-5.2 footing per special inspection, section 1704.7.2, 1-B. The material was compacted with a 5-ton vibratory roller. The area was tested for density per special inspection, section 1704.7.2, 1-C. All tests taken exceed the required 95 percent compaction. Prior to compaction the area was inspected and found to be suitable per special inspection, section 1704.7.1, 1-A.

Test Results:
Percent compaction: 95.0 to 98.7
Dry density: 123.0 to 127.9 pcf
Wet density: 128.4 to 131.7 pcf
Percent moisture : 3.0 to 4.6%

Remarks:

Portal to Portal

Leave:		**Expenses**		**Signed:**	Mike Sullivan
Return:	7:00	Mileage:	11	**cc:**	
TOTAL:	9:00	Density Gauge:	1		
	2	Other:			

Reviewed: Darrell A. Gilman, CMT Manager
Date: 4/30/09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: 626-9094

DAILY FIELD REPORT

Date: 5/4/09

Project: Avesta Florence House - Portland, Maine

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Subgrade soil inspection.

Work Activities: A crew from Coleman Excavation has excavated for the interior column footings at L-9, L-11, L-12, L-13, L-14, J-14, J-13, and H-10. The subgrade soil was inspected per special inspection section 1704.7.1ab and was found acceptable with the following observations:
-total material removed to bottom of footing ~6" to 8".
-soil was consistent with the geotechnical report dated 8-9-2007.
-a smooth edged bucket was used to excavate to final grade with no over excavation of subgrade soil.
-the surface of the subgrade was compacted with a 500 lb. plate compactor with no soft or yielding of the footing area.

Remarks: In conversation with Everett Stewart of Ganneston, a RFI had been issued for interior column footings to bear on native subgrade soil unless the material had been disturbed.

Portal to Portal		**Expenses**		**Signed:**	Darrell Gilman
Leave:	10:00	Mileage:	13	**cc:**	
Return:	11:00	Density Gauge:			
TOTAL:	1	Other:			

Reviewed: Darrell A. Gilman, CMT Manager
Sent: 5/6/09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: 626-9094

DAILY FIELD REPORT

Date: 7/28/09

Project: Avesta Florence House

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Soil Compaction Testing.

Work Activities: A crew from Ganneston Construction placed approximately a 12" lift and then compacted the onsite structural fill material along line 15 from L-line to M-line. The material was compacted with a walk behind compactor. The crew then compacted a 1" to 2" lift of the onsite barrow material with 1 1/2" stone to make it easier to shim grade. All tests taken exceed the required 95 percent compaction.

Test Results:
Percent compaction: 96.0 to 98.7
Dry density: 125.8 to 127.8 pcf
Wet density: 132.1 to 133.9 pcf
Percent moisture : 4.7 to 5.0%

Remarks:

Portal to Portal

Leave:		**Expenses**		**Signed:**	Justin Rouillard
Return:	2:00pm	Mileage:	11	**cc:**	
TOTAL:	3:30pm	Density Gauge:	X		
	1.5	Other:			

Reviewed: Darrell A. Gilman, CMT Manager
Date: 7/30/09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: 626-9094

DAILY FIELD REPORT

Date: 8/3/09

Project: Avesta Florence House

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Soil Compaction Testing.

Work Activities: A crew from Ganneston Construction placed approximately a 12" lift and then compacted the onsite structural fill material for the first floor. The material was compacted with a walk behind compactor. The crew then compacted a 1" to 2" lift of the onsite barrow material with 1 1/2" stone to make it easier to shim grade. All tests taken exceed the required 95 percent compaction.

Test Results:
Percent compaction: 96.9 to 101.1
Dry density: 125.4 to 130.9 pcf
Wet density: 128.7 to 134.2 pcf
Percent moisture : 2.5 to 3.8%

Remarks:

Portal to Portal

Leave:		**Expenses**		**Signed:**	Justin Rouillard
Return:	8:00	Mileage:	11	**cc:**	
TOTAL:	11:00	Density Gauge:	X		
	3	Other:			

Reviewed: Darrell A. Gilman, CMT Manager
Date: 8/10/09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: 626-9094

SUMMIT
GEOENGINEERING SERVICES

DAILY FIELD REPORT

Date: 8/27/09

Project: Avesta Florence House - Portland, Maine

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Density Testing

Work Activities: Three compaction tests were taken on the outside of the foundation wall between line F and line J and between line 10 and line 16. A sample was retreived and brought to the lab for testing. All three tests exceeded the 95% compaction requirement.

Test Results:

Percent compaction: 95.8% to 99.5%
Dry density: 122.6 to 127.4 pcf
Wet density: 130.2 to 134.1 pcf
Percent moisture : 5.3 to 6.6%

Remarks: Grout was being placed at the 3rd level of the stairwell.

Portal to Portal		**Expenses**		**Signed:**	Justin Rouillard
Leave:	8:45am	Mileage:	11	**cc:**	
Return:	10:15am	Density Gauge:	X		
TOTAL:	1.5	Other:			
				Reviewed:	Darrell A. Gilman, CMT Manager
				Sent:	9/8/09

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Phone: (207) 621-8334 Fax: 626-9094

DAILY FIELD REPORT

Date: 9/1/09

Project: Avesta Florence House - Portland, Maine

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Density Testing

Work Activities: Eight compaction tests were taken on the outside of the foundation wall along N line from 5 line to 14 line. Material was being placed via a conveyor truck and was being compacted with a walk behind wacker in 12" lifts. All tests exceeded 95% compaction.

Test Results:

Percent compaction: 95.3% to 98.0%
Dry density: 122.6 to 125.4 pcf
Wet density: 129.4 to 134.0 pcf
Percent moisture : 5.7 to 7.3%

Remarks: Grout was being placed at the 3rd level of the stairwell.

Portal to Portal		**Expenses**		**Signed:**	Justin Rouillard
Leave:	10:45am	Mileage:	22	**cc:**	
Return:	5:30am	Density Gauge:	X		
TOTAL:	6.75	Other:			
				Reviewed:	Darrell A. Gilman, CMT Manager
				Sent:	9/8/09

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434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: 626-9094

DAILY FIELD REPORT

Date: 9/2/09

Project: Avesta Florence House - Portland, Maine

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Density Testing

Work Activities: Fifteen compaction tests were taken on the outside of the foundation wall along N line from 5 line to 14 line. Material was being placed via a conveyor truck and was being compacted with a walk behind wacker in 12" lifts. All tests exceeded 95% compaction.

Test Results:

Percent compaction: 95.2% to 98.3%
Dry density: 121.9 to 125.8 pcf
Wet density: 128.3 to 132.7 pcf
Percent moisture : 4.2 to 6.5%

Remarks:

Portal to Portal		**Expenses**		**Signed:**	Justin Rouillard
Leave:	8:30am	Mileage:	22	**cc:**	
Return:	6:30am	Density Gauge:	X		
TOTAL:	10	Other:			
				Reviewed:	Darrell A. Gilman, CMT Manager
				Sent:	9/8/09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: 626-9094

DAILY FIELD REPORT

Date: 9/4/09

Project: Avesta Florence House - Portland, Maine

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Density Testing

Work Activities: Six compaction tests were taken on the outside of the foundation wall along N line from 5 line to 12 line. Material was being placed via a conveyor truck and was being compacted with a walk behind wacker in 12" lifts. All tests exceeded 95% compaction.

Test Results:

Percent compaction: 95.3% to 98.2%
Dry density: 122.0 to 125.8 pcf
Wet density: 133.3 to 128.2 pcf
Percent moisture : 4.6 to 6.1%

Remarks:

Portal to Portal		**Expenses**		**Signed:**	Justin Rouillard
Leave:	7:00am	Mileage:	22	**cc:**	
Return:	6:45pm	Density Gauge:	X		
TOTAL:	11.75	Other:			
				Reviewed:	Darrell A. Gilman, CMT Manager
				Sent:	9/10/09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: 626-9094

DAILY FIELD REPORT

Date: 9/9/09

Project: Avesta Florence House - Portland, Maine

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Density Testing

Work Activities: Eight compaction tests were taken on the outside of the foundation wall along N line from 4 line to 7 line. Material was being placed via frontend loader and was being compacted with a walk behind wacker in 12" lifts. All tests exceeded 95% compaction.

Test Results:

Percent compaction: 99.0% to 105.3%
Dry density: 113.4 to 120.6 pcf
Wet density: 116.8 to 123.7 pcf
Percent moisture : 3.0 to 4.2%

Remarks:

Portal to Portal		**Expenses**		**Signed:**	Justin Rouillard
Leave:	8:00am	Mileage:	22	**cc:**	
Return:	4:30pm	Density Gauge:	X		
TOTAL:	8.5	Other:			
				Reviewed:	Darrell A. Gilman, CMT Manager
				Sent:	9/10/09

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Phone: (207) 621-8334 Fax: 626-9094

DAILY FIELD REPORT

Date: 9/10/09

Project: Avesta Florence House - Portland, Maine

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Density Testing

Work Activities: One compaction test was taken on the outside of the foundation wall at the intersection of the M.5 line and the 4.5 line. Material was being placed via front end loader and was being compacted with a walk behind wacker in 12" lifts. All tests exceeded 95% compaction.

Test Results:

Percent compaction: 103.5%
Dry density: 118.6 pcf
Wet density: 122.3 pcf
Percent moisture : 3.1%

Remarks:

Portal to Portal		**Expenses**		**Signed:**	Justin Rouillard
Leave:	10:00am	Mileage:	22	**cc:**	
Return:	2:30pm	Density Gauge:	X		
TOTAL:	4.5	Other:			
				Reviewed:	Darrell A. Gilman, CMT Manager
				Sent:	9/14/09

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434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: 626-9094

DAILY FIELD REPORT

Date: 10/20/09

Project: Avesta Florence House - Portland, Maine

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Density Testing

Work Activities: Michael Walsh of Summit arrived onsite to provide compaction testing support during backfilling of the segmental block retaining wall. A crew from R.E. Coleman was placing the first course of block from approximately Station 0+92 to Station 1+12. Met with Everett Stewart of Ganneston and reviewed backfill requirements in project specifications and segmental wall plans by SRG Engineering, Inc. Justin Rouillard of Summit was onsite for testing at 11:15 am. However, lifts were not completed prior to leaving site at 2:00pm. Testing rescheduled for 10:00am tomorrow.

Test Results:

Remarks:

Portal to Portal		**Expenses**		**Signed:**	M. Walsh/Justin Rouillard
Leave:	8:45am	Mileage:	11	**cc:**	
Return:	2:00pm	Density Gauge:	X		
TOTAL:	3	Other:			
				Reviewed:	Darrell A. Gilman, CMT Manager
				Sent:	10/27/09

SUMMIT GEOENGINEERING SERVICES
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Phone: (207) 621-8334 Fax: 626-9094

DAILY FIELD REPORT

Date: 10/21/09

Project: Avesta Florence House - Portland, Maine

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Soil Compaction Testing Services.

Work Activities: Performed soil compaction testing inside the retaining wall south side. A crew from R.E. Coleman placed and compacted retaining wall backfill material in approximately 8" lifts. The maximum dry density of the material being placed had changed since laboratory testing was conducted, therefore a new sample was obtained and compaction percentages had to be back figured using the new dry density value. The back figured values revealed that two of the four compaction tests taken today did not meet the 95% compaction requirement. · All other compaction tests exceeded the 95% compaction requirement. Everett Stewart was made aware of the situation via telephone message.

Test Results:

Percent compaction: 92.2% to 97.1
Dry density: 104.2 to 109.7 pc
Moisture %: 3.4 to 9.0%

Remarks:

Portal to Portal		**Expenses**		**Signed:**	Neil Davis
Leave:	9:45am	Mileage:	11	**cc:**	
Return:	1:15pm	Density Gauge:	X		
TOTAL:	3.5	Other:			

Reviewed: Darrell A. Gilman, CMT Manager
Sent: 10/26/09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: 626-9094

DAILY FIELD REPORT

Date: 10/29/09

Project: Avesta Florence House

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Soil Compaction Testing.

Work Activities: Prior to my arrival, a crew from Coleman Excavation had placed and compacted the subbase fill for the upper and lower parking lots. We obtained a sample of the material for a grain size analysis and proctor. Compaction tests were taken and results will be back figured upon the completion of the proctor.

Test Results:
Percent compaction:
Dry density: 135.8 to 136.7 pcf
Wet density: 139.1 to 141.7 pcf
Percent moisture : 3.4 to 3.8 %

Remarks:

Portal to Portal

Leave:		**Expenses**		**Signed:**	Frank Clark
Return:	3:15pm	Mileage:	11	**cc:**	
TOTAL:	4:45pm	Density Gauge:	1		
	1.5	Other:			

Reviewed: Darrell A. Gilman, CMT Manager
Date: 10/30/09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Tel: (207) 621.8334 Fax: (207) 626.9094

SUMMIT GEOENGINEERING FIELD DENSITY RESULTS SUMMARY

PROJECT NO: 14194
PROJECT NAME: Avesta Florence House
CLIENT: Avesta Florence House. LP
Proctor Information:

Test Number	Date	Location:	Lift Elevation	Moisture Content %	In-place Density DD PCF	Maximum Density PCF	Percent Compaction	Required Compaction	Remarks
1	3/27/2009	N-line at 8.2-line	Bottom of Footing	4.1	131.8	136.5	96.6	95	Pass
2	3/27/2009	N-line at 12-line	Bottom of Footing	3.4	132.1	136.5	96.8	95	Pass
3	3/27/2009	N-line at 16-line	Bottom of Footing	2.9	134.8	136.5	98.8	95	Pass
1	20-Apr	E-7	Bottom of Footing	4	126.5	129.5	97.7	95	Pass
2	20-Apr	A-1	Bottom of Footing	3.7	126.4	129.5	97.6	95	Pass
1	28-Apr	N-5.2 footing	Bottom of Footing	3.4	123.5	129.5	95.4	95	Pass
2	28-Apr	N-1	Bottom of Footing	3	127.9	129.5	98.7	95	Pass
3	28-Apr	L.8-1	Bottom of Footing	3.6	126.6	129.5	97.8	95	Pass
4	28-Apr	G-1	Bottom of Footing	4.6	123	129.5	95	95	Pass
1	8/3/2009	H-5	finished grade	3.4	127.1	129.5	98.1	95	Pass
2	8/3/2009		finished grade	3.6	127.5	129.5	98.4	95	Pass
3	8/3/2009		finished grade	3.8	127.6	129.5	98.5	95	Pass
4	8/3/2009		finished grade	2.5	130.9	129.5	101.1	95	Pass
5	8/3/2009		finished grade	2.6	125.4	129.5	96.9	95	Pass
1	8/28/2009		3' BTW	6.6	123.6	128	96.6	95	Pass
2	8/28/2009		3' BTW	5.3	127.4	128	99.5	95	Pass
3	8/28/2009		3' BTW	6.2	122.6	128	95.8	95	Pass
1	9/1/2009	Outside N line at 5.8	18' BTW	5.7	122.5	128	95.7	95	Pass
2	9/1/2009	Outside N line at 7.1	16.5' BTW	6.9	125.4	128	97.9	95	Pass
3	9/1/2009	Outside N line at 11	16' BTW	6.2	123.3	128	96.3	95	Pass
4	9/1/2009	Outside N line at 13	15'BTW	6.4	122.4	128	95.6	95	Pass
5	9/1/2009	Outside N line at 12	14'BTW	7.3	122.8	128	95.9	95	Pass
6	9/1/2009	Outside N line at 5.8	15'BTW	6.2	123.8	128	96.7	95	Pass
7	9/1/2009	Outside N line at 11.5	13' BTW	5.9	125.4	128	98	95	Pass
8	9/1/2009	Outside N line at 6.5	14' BTW	6.2	122.6	128	95.3	95	Pass
1	9/2/2009	Outside N line at 10	12' BTW	6.5	122.5	128	95.7	95	Pass
2	9/2/2009	Outside N line at 6.5	13' BTW	5.3	122.4	128	95.6	95	Pass
3	9/2/2009	Outside N line at 11	11' BTW	6.1	123	128	96.1	95	Pass
4	9/2/2009	Outside N line at 7	11' BTW	6.2	122.5	128	95.7	95	Pass
5	9/2/2009	Outside N line at 14	10' BTW	5.6	121.9	128	95.2	95	Pass
6	9/2/2009	Outside N line at 8	10' BTW	4.2	123.3	128	96.3	95	Pass
7	9/2/2009	Outside N line at 9	9' BTW	4.8	122.4	128	95.6	95	Pass
8	9/2/2009	Outside N line at 7	10' BTW	5.5	123.6	128	96.6	95	Pass
9	9/2/2009	Outside N line at 5.5	9' BTW	6.1	123.1	128	96.2	95	Pass

10	9/2/2009	Outside N line at 16	9' BTW		6.2	122.1	128	95.4	95	Pass
11	9/2/2009	Outside N line at 16	8' BTW		5.8	123	128	96.1	95	Pass
12	9/2/2009	Outside N line at 5	13' BTW		6.2	122	128	95.3	95	Pass
13	9/2/2009	Outside N line at 6	12' BTW		5.3	126	128	98.4	95	Pass
14	9/2/2009	Outside N line at 7	11' BTW		6.1	123.1	128	96.4	95	Pass
15	9/2/2009	Outside N line at 8	10' BTW		4.6	125.8	128	98.3	95	Pass
1	9/9/2009	Outside N line at 5.5	12' BTW		3	113.4	114.5	99	95	Pass
2	9/9/2009	Outside N line at 5	11' BTW		2.6	120.6	114.5	105.3	95	Pass
3	9/9/2009	Outside N line at 4.8	10' BTW		2.4	114.1	114.5	99.7	95	Pass
4	9/9/2009	Outside N line at 6.2	9' BTW		4.1	117	114.5	102.2	95	Pass
5	9/9/2009	Outside N line at 5.3	8' BTW		3.4	116.3	114.5	101.6	95	Pass
6	9/9/2009	Outside N line at 4.5	7' BTW		4.5	16.1	114.5	101.4	95	Pass
7	9/9/2009	Outside N line at 6	6' BTW		4.1	118.3	114.5	103.3	95	Pass
8	9/9/2009	Outside N line at 6.3	5' BTW		4.2	115.2	114.5	100.5	95	Pass
9	9/10/2009	Outside M.5 line at 4.5	7' BTW		3.1	118.6	114.5	103.5	95	Pass
1	10/21/09	Inside Retaining Wall - South Quarter		44.7	4.9	105.8	113	93.6	95	Fail
2	10/21/09	Inside Retaining Wall - 2nd to S Quarter		44.7	3.4	104.2	113	92.2	95	Fail
3	10/21/09	Inside Retaining Wall - South Quarter		45.4	8.4	108.6	113	96.1	95	Pass
4	10/21/09	Inside Retaining Wall - 2nd to S Quarter		45.4	9	109.7	113	97.1	95	Pass

EXHIBIT B

03300 Concrete Construction

Project: Florence House
Date Prepared: September 2, 2008

Structural Schedule of Special Inspections
CONCRETE CONSTRUCTION

VERIFICATION AND INSPECTION IBC Section 1704.4	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS	AGENT	AGENT QUALIFICATION	TASK COMPLETED
1. Inspection of reinforcing steel, including prestressing tendons, and placement	Y	P	ACI 318: 3.5, 7.1-7.7	SI1	PE/SE or EIT	JCF
2. Inspection of reinforcing steel welding in accordance with Table 1704.3, Item 5B	N		Welding of Reinf Not Allowed	TA2	AWS-CWI	N/A
3. Inspect bolts to be installed in concrete prior to and during placement of concrete where allowable loads have been increased	Y	C	IBC 1912.5	SI1	PE/SE or EIT	JCF
4. Verifying use of required design mix	Y	P	ACI 318: Ch 4, 5.2-5.4	SI1	PE/SE or EIT	JCF
5. At time fresh concrete is sampled to fabricate specimens for strength test, perform slump and air content test and temperature	Y	C	ASTM C 172 ASTM C 31 ACI 318: 5.6, 5.8	TA1	ACI-CFTT or ACI-STT	See included
6. Inspection of concrete and shotcrete placement for proper application techniques	N/A	C	ACI 318: 5.9, 5.10		PE/SE or EIT	
7. Inspection for maintenance of specified curing temperature and techniques	Y	P	ACI 318: 5.11-5.13	SI1	PE/SE or EIT	JCF
8. Inspection of Prestressed Concrete						
a. Application of prestressing force.	N/A	C	ACI 318: 18.20		PE/SE or EIT	
b. Grouting of bonded prestressing tendons in seismic force resisting system	N/A	C	ACI 318: 18.18.4		PE/SE or EIT	
9. Erection of precast concrete members	N/A	P	ACI 318: Ch 16		PE/SE or EIT	
10. Verification of in-situ concrete strength, prior to stressing of tendons in post-tensioned concrete and prior to removal of shores and forms beans and structural slabs	N/A	P	ACI 318: 6.2		ACI-STT	

BECKER
structural engineers, inc.

03300

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	1858

OBSERVATION REPORT
Cast in Place Concrete

Date:	04/01/09
Time:	10:00 to 10:30 AM
Temp:	Mid 40's
Weather:	Sunny

Observation Location: Footings Line N from 8.2 to 16

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Reinforcement Size	☒	☐	☐	☐	
Quantity	☒	☐	☐	☐	
Condition	☒	☐	☐	☐	
Placement	☒	☐	☐	☐	
Embed/Anchors	☒	☐	☐	☐	
Lap Splices	☒	☐	☐	☐	
Hot Weather	☐	☐	☐	☒	
Cold Weather	☐	☐	☐	☒	
Bond Beams	☐	☐	☐	☒	
Additional Items	☐	☐	☐	☐	
Additional Items	☐	☐	☐	☐	

Notes:

Signed: Matthew J. Miller, P.E.

75 York Street, Portland, ME 04101-4701 ■ Tel 207-879-1838 ■ Fax 207-879-1822

BECKER
structural engineers, inc.

03300

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	1858

OBSERVATION REPORT
Cast in Place Concrete

Date:	Friday, 04/10/09
Time:	12:45PM
Temp:	Low 50's
Weather:	Sunny

Observation Location: Wall reinforcing and formwork (Valley Street side) along N line, 16 line to 8.2 line and 16 line, from the N corner to approximately 12 feet toward St. John's Street

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Reinforcement Size	☒	☐	☐	☐	
Quantity	☒	☐	☐	☐	
Condition	☒	☐	☐	☐	
Placement	☒	☐	☐	☐	
Embed/Anchors	☐	☐	☒	☐	
Lap Splices	☒	☐	☐	☐	
Hot Weather	☐	☐	☐	☒	
Cold Weather	☐	☐	☐	☒	
Bond Beams	☐	☐	☐	☒	
Additional Items	☐	☐	☐	☐	
Additional Items	☐	☐	☐	☐	

Notes:
I met with Dan Beaulieu and Everett Stewart of Ganneston, and Mike Tindle of Newman concrete. Newman Concrete was seeking clarification of the pier and brick shelf conditions of the wall that was in place. Newman's crews were packing up for the weekend and it was agreed that we would meet again on Monday morning. The reinforcing that was in place at the time appeared to conform with the project documents.

Signed: Ethan A. Rhile, P.E.

75 York Street, Portland, ME 04101-4701 ■ Tel 207-879-1838 ■ Fax 207-879-1822

BECKER
structural engineers, inc.

03300

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	1858

OBSERVATION REPORT
Cast in Place Concrete

Date:	Monday, 04/13/09
Time:	7:55AM
Temp:	Low 30's
Weather:	Windy with sun, Chilly

Observation Location: Wall reinforcing and formwork (Valley Street side) along N line, 16 line to 8.2 line and 16 line, from the N corner to approximately 12 feet toward St. John's Street

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Reinforcement Size	☒	☐	☐	☐	
Quantity	☒	☐	☐	☐	
Condition	☒	☐	☐	☐	
Placement	☒	☐	☐	☐	
Embed/Anchors	☐	☐	☒	☐	
Lap Splices	☒	☐	☐	☐	
Hot Weather	☐	☐	☐	☒	
Cold Weather	☐	☐	☐	☒	
Bond Beams	☐	☐	☐	☒	
Additional Items	☐	☐	☐	☐	
Additional Items	☐	☐	☐	☐	

Notes:
At the time of my visit the project site was taking delivery of reinforcment. I met with Dan Beaulieu and Everett Stewart of Ganneston, and Mike Tindle of Newman concrete and presented draft sketches CSKS-01/02/03 to clarify the pier and brick shelf conditions. Field direction was given to: Raise 6 feet of brick shelf along N line from the corner up to 55'-3" to match the elevation along the 16 line, revise the location of the (5) number 8 reinforcing bars between 14 & 13 to be below the pier tops in these locations and to add (2)#5x6'-0" below the brick shelfs where the pier bond outs extended above the shelf.

Signed: Ethan A. Rhile, P.E.

75 York Street, Portland, ME 04101-4701 ■ Tel 207-879-1838 ■ Fax 207-879-1822

BECKER
structural engineers, inc.

03300

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	1858

OBSERVATION REPORT
Cast in Place Concrete

Date:	Monday, 04/13/09
Time:	2:30PM
Temp:	40's
Weather:	Windy with sun, Chilly

Observation Location: Wall reinforcing and formwork (Valley Street side) along N line, 16 line to 8.2 line and 16 line, from the N corner to approximately 12 feet toward St. John's Street

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Reinforcement Size	☒	☐	☐	☐	
Quantity	☒	☐	☐	☐	
Condition	☒	☐	☐	☐	
Placement	☒	☐	☐	☐	
Embed/Anchors	☐	☐	☒	☐	
Lap Splices	☒	☐	☐	☐	
Hot Weather	☐	☐	☐	☒	
Cold Weather	☐	☐	☐	☒	
Bond Beams	☐	☐	☐	☒	
Additional Items	☐	☐	☐	☐	
Additional Items	☐	☐	☐	☐	

Notes:
I returned to review the work discussed in the morning. A majority of the work discussed in the morning was completed. It was requested that Newman install (2) #6 in place of one of the #8 reinforcing bars between 13 & 14 that had been accidentially been cut to create the column bond outs.

Signed: Ethan A. Rhile, P.E.

75 York Street, Portland, ME 04101-4701 ■ Tel 207-879-1838 ■ Fax 207-879-1822

BECKER
structural engineers, inc.

03300

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI 2058

OBSERVATION REPORT
Cast in Place Concrete

Date:	Wednesday, 04/22/09
Time:	3:30PM - 4:30 PM
Temp:	Low 40's
Weather:	Cloudy with Rain Showers

Observation Location: Strip footing along Line A and along Line 7 from Line A to west of Line F where the footing was already in place.

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Reinforcement Size	☒	☐	☐	☐	
Quantity	☒	☐	☐	☐	
Condition	☒	☐	☐	☐	
Placement	☒	☐	☐	☐	
Embed/Anchors	☒	☐	☐	☐	
Lap Splices	☒	☐	☐	☐	
Hot Weather	☐	☐	☐	☒	
Cold Weather	☐	☐	☐	☒	
Bond Beams	☐	☐	☐	☒	
Additional Items	☐	☐	☐	☒	
Additional Items	☐	☐	☐	☒	

Notes:

Work was found to be in general conformance with the construction documents. Reinforcement was located adequately and footing forms appear to be located to properly size the footing. The concrete sub-contractor was still working along Line 7 during this visit, and not all reinforcement had been placed. The reinforcement needed in this area was discussed with the concrete superintendent, and this work will be completed tomorrow prior to the concrete placement.

Please contact me with any questions or comments. Thank you -

Signed: James Fortin, P.E.

75 York Street, Portland, ME 04101-4701 ■ Tel 207-879-1838 ■ Fax 207-879-1822

BECKER
structural engineers, inc.

03300

OBSERVATION REPORT
Cast in Place Concrete

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI 2058

Date:	Friday, 05/01/09
Time:	7:30AM - 9:00 AM
Temp:	Low 40's
Weather:	Cloudy with Rain Showers

Observation Location: Foundation Wall along F-Line and remainder of 16-Line, elevator pit mat with adjacent F4 footings at Grid M/5 & M/5.8.

Note: A preliminary review of the major foundation wall reinforcement was completed on Tuesday, April 28th, prior to standing the interior wall forms.

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Reinforcement Size	☒	☐	☐	☐	
Quantity	☒	☐	☐	☐	
Condition	☒	☐	☐	☐	
Placement	☒	☐	☒	☐	
Embed/Anchors	☒	☐	☐	☐	
Lap Splices	☒	☐	☐	☐	
Hot Weather	☐	☐	☐	☒	
Cold Weather	☐	☐	☐	☒	
Bond Beams	☐	☐	☐	☒	
Additional Items	☐	☐	☐	☒	
Additional Items	☐	☐	☐	☒	

Notes:

Review of the areas referenced above found that rebar and embedments layout appeared in general conformance with the construction documents and supplemental sketches. Upon arrival, it was noted that the U-bars and slab dowels identified on sketch CSKS-5 were not in place. The concrete sub-contractor was aware of this, and had begun placing the u-bars prior to my departure. I discussed this with Everett, and he stated that he would confirm that the missing bars are in place prior to pouring the wall.

Please contact me with any questions or comments. Thank you -

Signed: James Fortin, P.E.

75 York Street, Portland, ME 04101-4701 ■ Tel 207-879-1838 ■ Fax 207-879-1822

BECKER
structural engineers, inc.

03300

OBSERVATION REPORT
Cast in Place Concrete

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI 2058

Date:	Wednesday, 05/06/09
Time:	11:30AM - 1:00 PM
Temp:	Low 50's
Weather:	Cloudy

Observation Location: Foundation Walls along the remainder of 7-Line & A-Line, elevator pit walls, strip footing along Line 1 from Line L.8 to H, and site retaining wall footing off Line 1. Also spread footings at Grid J/14, L/14, J/13, L/13, L/12, L/11, J/10 & L/9 and CMU footings along Lines 13, 14, & L.

Note: A preliminary review of the major foundation wall reinforcement was completed on Monday, May 4th, prior to standing the interior wall forms.

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Reinforcement Size	☒	☐	☐	☐	
Quantity	☒	☐	☒	☐	
Condition	☒	☐	☐	☐	
Placement	☒	☐	☒	☐	
Embed/Anchors	☒	☐	☒	☐	
Lap Splices	☒	☐	☐	☐	
Hot Weather	☐	☐	☐	☒	
Cold Weather	☐	☐	☐	☒	
Bond Beams	☐	☐	☐	☒	
Additional Items	☐	☐	☐	☒	
Additional Items	☐	☐	☐	☒	

Notes:

Review of the areas referenced above found that rebar and embedments layout appeared in general conformance with the construction documents and supplemental sketches. A majority of the wall rebar was in placed and appeared adequate, but work was ongoing and the following items were noted that will need to be addressed prior to pouring. We understand that a concrete sub-contractor is likely aware of a majority of these items, but provide them for reference to ensure they are not missed:

1. Per CSKS10, it is specified to pour the (2) interior footings at Line 13 & 14 integrally. These footings are currently formed separately. Although preferable to place as one integral footing, it is adequate to fill space between footings with concrete as a separate placement.

2. Reinforcement in many of the interior footings needs to be straightened out and lifted off the ground to maintain rebar cover requirements.
3. Reinforcement in the CMU wall strip footings was in contact with the earth and needs to be placed on chairs to obtain proper rebar cover.
4. Z-bars specified on CSKS-10 are not yet in place.
5. Dowels for CMU walls shall be placed and secured prior to the concrete placement.
6. At footing F6A at Grid L/9, rebar grid is required at both top and bottom of footing (see footing schedule)
7. At high wall along Line 7, joist seats are not yet in place, as well as U-bars, and dowels per CSKS-5.
8. The brick shelf transition along Line 7 does not appear to be in the proper location. This was confirmed with the GC, and the area will be corrected. The step in the stem wall is not at the same location as the step in the brick shelf.
9. Anchor bolts are not yet in place at column locations.
10. Along Line 1, between L.8 & H, a few vertical wall dowels are not yet in place per Section 1/S2.2.

The General Contractor was informed of these items prior to my departure. The General Contractor is requested to review these items with the concrete sub-contractor, and review completion of these items prior to the concrete placement.

Please contact me with any questions or comments. Thank you -

Signed: James Fortin, P.E.

75 York Street, Portland, ME 04101-4701 ■ Tel 207-879-1838 ■ Fax 207-879-1822

BECKER
structural engineers, inc.

03300

OBSERVATION REPORT
Cast in Place Concrete

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI 2058

Date:	Wednesday, 05/13/09
Time:	2:00PM - 3:00 PM
Temp:	Low 60's
Weather:	Sunny

Observation Location: Foundation footings along Line 1 from Line A to between Line C & E, including the footings for the entrance slab.

Note: A pre-steel and pre-wood framing meeting was also conducted during this visit.

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Reinforcement Size	☒	☐	☐	☐	
Quantity	☒	☐	☒	☐	
Condition	☒	☐	☐	☐	
Placement	☒	☐	☐	☐	
Embed/Anchors	☒	☐	☐	☐	
Lap Splices	☒	☐	☐	☐	
Hot Weather	☐	☐	☐	☒	
Cold Weather	☐	☐	☐	☒	
Bond Beams	☐	☐	☐	☒	
Additional Items	☐	☐	☐	☒	
Additional Items	☐	☐	☐	☒	

Notes:

Review of the areas referenced above found that rebar layout appeared in general conformance with the construction documents

Please contact me with any questions or comments. Thank you -

Signed: James Fortin, P.E.

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BECKER
structural engineers, inc.

03300

OBSERVATION REPORT
Cast in Place Concrete

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI 2058

Date:	Tuesday, 05/15/09
Time:	11:00 AM - 12:00 PM
Temp:	Low 70's
Weather:	Sunny

Observation Location: Foundation Walls along Line 1 from Line A to between Lines C & E, including the entrance slabs walls.

Note: A preliminary review of the major foundation wall reinforcement along Line N and 1 was also completed during this visit.

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Reinforcement Size	☒	☐	☐	☐	
Quantity	☒	☐	☐	☐	
Condition	☒	☐	☐	☐	
Placement	☒	☐	☐	☐	
Embed/Anchors	☒	☐	☐	☐	
Lap Splices	☒	☐	☐	☐	
Hot Weather	☐	☐	☐	☒	
Cold Weather	☐	☐	☐	☒	
Bond Beams	☐	☐	☐	☒	
Additional Items	☐	☐	☐	☒	
Additional Items	☐	☐	☐	☒	

Notes:

Review of the areas referenced above found that rebar layout appeared in general conformance with the construction documents. There were no comments provided, and the area was determined ready for concrete placement.

Please contact me with any questions or comments. Thank you -

Signed: James Fortin, P.E.

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BECKER
structural engineers, inc.

03300

OBSERVATION REPORT
Cast in Place Concrete

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI 2058

Date:	Tuesday, 05/19/09
Time:	3:00 PM - 4:00 PM
Temp:	60's
Weather:	Sunny

Observation Location: Foundation Walls along the remainder of N-Line & 1-Line to the intersection of the site retaining wall, and site retaining wall off Line 1.

Note: A preliminary review of the major foundation wall reinforcement was completed on Friday, May 15th, prior to standing the interior wall forms.

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Reinforcement Size	☒	☐	☐	☐	
Quantity	☒	☐	☐	☐	
Condition	☒	☐	☐	☐	
Placement	☐	☐	☒	☐	
Embed/Anchors	☐	☐	☒	☐	
Lap Splices	☒	☐	☐	☐	
Hot Weather	☐	☐	☐	☒	
Cold Weather	☐	☐	☐	☒	
Bond Beams	☐	☐	☐	☒	
Additional Items	☐	☐	☐	☒	
Additional Items	☐	☐	☐	☒	

Notes:

Review of the areas referenced above found that rebar layout appeared in general conformance with the construction documents and supplemental sketches. A majority of the wall rebar was in placed and appeared adequate, but work was ongoing and the following items were noted that will need to be completed prior to pouring. We understand that a concrete sub-contractor is likely aware of a majority of these items, but provide them for reference to ensure they are not missed. The General Contractor has agreed to take pictures as necessary to confirm.

1. Per CSKS-5, U-bars are necessary between each joist pocket. This reinforcement was on site but had not yet been placed.
2. The bond-outs and joist angle seat embedments had not been placed within the forms.

3. Per CSKS-5, the #5 dowels extending from the top of the concrete wall into the elevated slab were not in place.

During the preliminary review on May 15th, it was noted that the additional #5 bars specified on Section 8/S2.3 were not in place. These bars were found to be in place during this visit.

The General Contractor was informed of these items prior to my departure. The General Contractor is requested to review these items with the concrete sub-contractor, and review completion of these items prior to the concrete placement.

Please contact me with any questions or comments. Thank you -

Signed: James Fortin, P.E.

BECKER
structural engineers, inc.

03300

OBSERVATION REPORT
Cast in Place Concrete

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI 2058

Date:	Tuesday, 05/27/09
Time:	9:00 AM - 10:00 AM
Temp:	50's
Weather:	Rain Showers

Observation Location: Remainder of interior spread footings between Line 1 and Line 7 as well as foundation wall along Line 1, from approx. Line D to Line H. Note: The spread footing at Grid 1/N was also placed, but the rebar at this area was not in place during this visit.

Note: A preliminary review of this reinforcement was completed on Friday, May 22nd.

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Reinforcement Size	☒	☐	☐	☐	
Quantity	☒	☐	☐	☐	
Condition	☒	☐	☐	☐	
Placement	☒	☐	☐	☐	
Embed/Anchors	☒	☐	☐	☐	
Lap Splices	☒	☐	☐	☐	
Hot Weather	☐	☐	☐	☒	
Cold Weather	☐	☐	☐	☒	
Bond Beams	☐	☐	☐	☒	
Additional Items	☐	☐	☐	☒	
Additional Items	☐	☐	☐	☒	

Notes:

Review of the areas referenced above found that rebar layout appeared in general conformance with the construction documents.

Please contact me with any questions or comments. Thank you -

Signed: James Fortin, P.E.

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BECKER
structural engineers, inc.

03300

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI2058

OBSERVATION REPORT
Cast in Place Concrete

Date:	07-06-09
Time:	7:30 am - 9:00 am
Temp:	60's
Weather:	Sunny

Observation Location: Slab Reinforcement at the First Floor.

Note: An initial visit to review first floor slab reinforcement was completed on July 2nd. This is a follow-up visit to confirm that incomplete areas found on July 2nd had been completed and the area was ready for slab palcement.

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Reinforcement Size	☒	☐	☐	☐	
Quantity	☒	☐	☐	☐	
Condition	☒	☐	☐	☐	
Placement	☒	☐	☐	☐	
Embed/Anchors	☒	☐	☐	☐	
Lap Splices	☒	☐	☐	☐	
Hot Weather	☐	☐	☐	☒	
Cold Weather	☐	☐	☐	☒	
Bond Beams	☐	☐	☐	☒	
Additional Items	☐	☐	☐	☒	
Additional Items	☐	☐	☐	☒	

Notes:

This site observation was conducted to review reinforcement to be placed in the first floor concrete slab on metal deck. Reinforcement was in place and was in general conformance with the construction documents and supplemental sketches (CSKS's) provided.

Signed: James Fortin, P.E.

BECKER
structural engineers, inc.

03300

OBSERVATION REPORT
Cast in Place Concrete

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI 2058

Date:	Wednesday, 08/05/09
Time:	1:30PM - 2:30PM
Temp:	80's
Weather:	Sunny

Observation Location: Slab-on-grade reinforcement and sub-grade

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Reinforcement Size	☒	☐	☐	☐	
Quantity	☐	☒	☐	☐	
Condition	☐	☐	☐	☐	
Placement	☐	☒	☐	☐	
Embed/Anchors	☐	☒	☐	☐	
Lap Splices	☐	☐	☒	☐	
Hot Weather	☐	☐	☐	☒	
Cold Weather	☐	☐	☐	☒	
Bond Beams	☐	☐	☐	☒	
Additional Items	☐	☐	☐	☒	
Additional Items	☐	☐	☐	☒	

Notes:
This visit was completed to review the placement of reinforcement for the concrete slab-on-grade. Information provided on Drawing S1.1 and CSKS-10 was reviewed. The following items were noted that will require attention by the General Contractor prior to slab placement. The GC is requested to take photographs of these items when completed and forward to BSE for record.

1. The (3) #5 dowels shown in Section B to be drilled and grouted into the existing foundation strip footing have not yet been placed.
2. The #4 bars around the perimeter of the slab and along Line M shall be continuous. They are currently broken at each column slab bout-out. Pull the bars away from the wall to allow for continuous bar placement. Provide 2'-0" lap splices between bars.
3. The #4 slab bars shall be located at the center of the slab depth. Provide taller chairs for bar placement.

Please contact me with any questions or comments. Thank you -

Signed: James Fortin, P.E.

75 York Street, Portland, ME 04101-4701 ■ Tel 207-879-1838 ■ Fax 207-879-1822

AUBURN CONCRETE

82 Goldthwaite Road • P.O. Box 1747 • Auburn, Maine 04210
93 Scott Drive • Westbrook, Maine 04092
50 Arthur Reno Road • West Bath, Maine 04530

Phone: (207) 777-7100 • Fax: (207) 777-7171
Phone: (207) 780-0523 • Fax: (207) 780-1521
Phone: (207) 386-5100 • Fax: (207) 386-5151

GANNESTON CONSTRUCTION CORP.

ATTN: MIKE ADAMS
3025 NORTH BELFAST AVE.
P.O. BOX 27
AUGUSTA, ME 04332
PH: (207) 621-8505
FX: (207) 621-8508

GANNESTON CONSTRUCTION CORP.
P.O. BOX 27 AUGUSTA, ME 04332

REVIEWED	REVIEWED WITH NOTES NO RESUBMISSION	CHECK MARK INDICATES ACTION TAKEN
		REVIEWED WITH NOTES RESUBMISSION REQUESTED
		NOT ACCEPTABLE

CHECKED BY _____ DATE 2/6/07

Mix Design Submittals for:

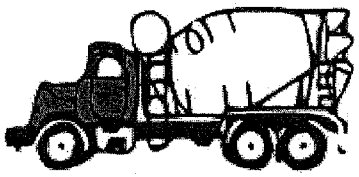
FLORENCE HOUSE

390 VALLEY ST. - PORTLAND, ME

GANNESTON CONSTRUCTION CORP - PROJECT # 677

As prepared by:
AUBURN CONCRETE
Remi Delcourt, Sales & Quality Control
P.O. Box 1747 - 82 Goldthwaite Road
Auburn, Maine 04210
Office: (207) 777-7100
Facsimile: (207) 777-7171
E-Mail: remi@auburnconcrete.com

AUBURN CONCRETE



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☐ Reviewed ☒ Furnish as Corrected
☐ Rejected ☐ Revise and Resubmit
☐ Submit Specific Item:

This review is only for general conformance with the design concept of the project and general compliance with the information given in the Contract Documents. Corrections or comments made on the shop drawings during this review do not relieve contractor from compliance with the requirements of the plans and specifications. Approval of a specific item shall not include approval of an assembly of which the item is a component. Contractor is responsible for: dimensions to be confirmed and correlated at the jobsite: information that pertains solely to the fabrication processes or to the means, methods, techniques, sequences and procedures of construction: coordination of his or her Work with that of all other trades: and for performing all work in a safe and satisfactory manner.

Becker Structural Engineers, Inc.

Date 2-18-09 By MJM

AUBURN CONCRETE

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Ganneston Construction Corp.

FLORENCE HOUSE
390 VALLEY ST. - PORTLAND, ME

3000PSI - Air Entrained, ¾" Crushed Stone

Mix Design Submittal (Footings)

1/30/2009 — 3034SA

		Weight-SSD (lbs)	Volume (Cu.Ft.)	Sources
CEMENT, T I/II	ASTM C-150	517	2.63	DRAGON PRODUCTS COMPANY
COARSE AGG	ASTM C-33: #57/#67	1700	10.28	PIKE INDUSTRIES
FINE AGGREGATE	ASTM C-33	1330	8.17	PORTLAND SAND & GRAVEL
WATER U.S. GAL/CY:	33.0	275	4.41	CITY OF WESTBROOK
	AIR CONTENT (%):	6.0 +/- 1.5%	1.63	
	WATER/CEMENT RATIO:	~~0.53~~		
	SLUMP (Inches):	4.00 ± 1.00"		4" MAX PER SPECS. PRIOR TO WATER REDUCER
	YIELD:	141.0 PCF	27.1 Cu.Ft.	

GLENIUM 7500	ASTM C494, TYPE A,F	3.00 oz/cwt	15.5	US oz/CY	BASF/MASTER BUILDERS
MICROAIR	ASTM C-260	0.15 oz/cwt	0.8	US oz/CY	BASF/MASTER BUILDERS
OPTIONAL:					
POZZOLITH 100XR	ASTM C-494, Type B,D	2.00 oz/cwt	10.3	US oz/CY	BASF/MASTER BUILDERS
POZZUTEC 20+	ASTM C-494, Type C,E	10.00 oz/cwt	51.7	US oz/CY	BASF/MASTER BUILDERS

* GLENIUM 7500 dose is for **MIDRANGE** applications.

* GLENIUM 7500 meets the requirements of ASTM C494 for Type A (water-reducing) and Type F (high-range water -reducing). Plant of site addition of 2 - 3 fl.oz./cwt (10.3 - 15.5 fl.oz./cy) will be required to achieve end slump indicated.

MIX IDENTIFICATION #: 3034SA
DESIGN STRENGTH: 3000 PSI @ 28 DAYS

AUBURN CONCRETE

PROJECT:
CONTRACTOR:

W/C: 0.53
MAX. C.A.: 3/4"

1/30/2009
3:11 PM

DATE	PROJECT	ID#	SLUMP	% AIR	TEMPERATURE CONC	TEMPERATURE AIR	7 DAY	28 DAY	MOVING AVG. OF 3
1/10/2008	TURNER-LEEDS BRIDGES	14039-6					2950	3120	
1/12/2008	TURNER-LEEDS BRIDGES	14039-12	7.50	6.4	60		2785	3980	
3/6/2008	TURNER-LEEDS BRIDGES	14039-13	6.50	6.5	52		2595	3435	3512
3/6/2008	TURNER-LEEDS BRIDGES	14039-14	6.50	7.0	51		3010	3910	3775
4/5/2008	TURNER-LEEDS BRIDGES	14039-17		4.5	62		3410	4055	3800
4/5/2008	TURNER-LEEDS BRIDGES	14039-19		6.5	61		2620	3315	3760
4/5/2008	TURNER-LEEDS BRIDGES	14039-20		6.5	61		3390	4420	3930
4/5/2008	TURNER-LEEDS BRIDGES	14039-21		7.1	63		3580	4070	3935
4/9/2008	RESIDENCE INN-AUBURN	14064-1	3.50	5.3	63		2830	4160	4213
4/10/2008	RESIDENCE INN-AUBURN	14064-2	3.75	5.0	63		3380	4470	4230
4/11/2008	RESIDENCE INN-AUBURN	14064-3	4.00	5.4	63		3010	4470	4363
4/15/2008	RESIDENCE INN-AUBURN	14064-4	5.25	5.2	61		3140	3885	4275
4/17/2008	RESIDENCE INN-AUBURN	14064-5	5.00	5.8	65		3290	4090	4148
4/22/2008	RESIDENCE INN-AUBURN	14064-6	5.75	5.6	69		2710	3515	3830
4/25/2008	RESIDENCE INN-AUBURN	14064-7	5.00	6.5	65		2950	3770	3792
4/30/2008	RESIDENCE INN-AUBURN	14064-8	3.00	6.0	58		3350	4400	3895
5/5/2008	RESIDENCE INN-AUBURN	14064-9	5.50	4.5	64		3030	3540	3903
5/8/2008	RESIDENCE INN-AUBURN	14064-10	5.75	6.0	67		2960	3555	3832
5/13/2008	RESIDENCE INN-AUBURN	14064-11	3.50	6.8	64		3310	3970	3688
5/19/2008	RESIDENCE INN-AUBURN	14064-12	5.25	6.2	65		3230	3470	3665
5/29/2008	RESIDENCE INN-AUBURN	14064-13	4.25	5.5	68		3900	4550	3997
7/2/2008	TURNER-LEEDS BRIDGES	14039-33	6.00	6.0	77		2850	3640	3887
7/2/2008	TURNER-LEEDS BRIDGES	14039-34	6.50	6.1	81		3100	3850	4013
7/2/2008	TURNER-LEEDS BRIDGES	14039-35	6.50	8.0	81		2940	3640	3710
7/2/2008	TURNER-LEEDS BRIDGES	14039-36	6.00	8.2	84		2670	3355	3615
7/15/2008	GORHAM BY-PASS	889-10	6.00	6.8	81	83	2920	3200	3398
7/25/2008	St. MARYS ER ADDITION	14093-7	5.00	4.7	64		3080	3935	3497
7/29/2008	St. MARYS ER ADDITION	14093-8	4.00	5.4	67		3140	4095	3743
7/30/2008	St. MARYS ER ADDITION	14093-9	4.25	4.9	83		3390	4255	4095
8/1/2008	St. MARYS ER ADDITION	14093-10	5.00	5.5	82		3030	3735	4028
8/4/2008	St. MARYS ER ADDITION	14093-11	5.00	6.9	84		2610	3200	3730
8/7/2008	St. MARYS ER ADDITION	14093-12	5.50	5.9	71		3040	4195	3710
8/9/2008	St. MARYS ER ADDITION	14093-13	5.00	6.8	79		3270	3795	3730
8/15/2008	St. MARYS ER ADDITION	14093-14	4.00	6.9	86		2800	3190	3727
9/17/2008	ST. MARY'S MOB	14131-1	4.50	5.8	81		4550	4915	3967
9/18/2008	ST. MARY'S MOB	14131-2	7.25	6.0	76		3290	4110	4072
9/23/2008	ST. MARY'S MOB	14131-3	3.75	7.3	70		3310	4090	4372
9/23/2008	ST. MARY'S MOB	14131-4	5.00	6.9	78		3500	4115	4105
9/30/2008	ST. MARY'S MOB	14131-5	5.50	6.6	74		2990	4055	4087
10/2/2008	ST. MARY'S MOB	14131-6	5.50	7.2	70		2990	3935	4035
10/2/2008	BOWDOIN COL. FITNESS CTR	14138-1	5.50	8.0	72		2840	3890	3960
10/6/2008	ST. MARY'S MOB	14131-7	5.50	6.8	63		3320	3825	3883
10/7/2008	ST. MARY'S MOB	14131-8	4.50	7.3	61		3100	3920	3878
10/8/2008	ST. MARY'S MOB	14131-9	5.50	5.0	71		3680	4140	3962
10/13/2008	ST. MARY'S MOB	14131-10	5.25	5.9	73		3410	3910	3990
10/14/2008	BOWDOIN COL. FITNESS CTR	14138-2	5.50	5.6	70		3140	3905	3985
10/14/2008	BOWDOIN COL. FITNESS CTR	14138-3	5.50	6.5	68		3050	3775	3863
10/17/2008	ST. MARY'S MOB	14131-11	6.00	7.0	68		2740	3555	3745
10/21/2008	BOWDOIN COL. FITNESS CTR	14138-4	6.00	5.8	62		3010	4005	3776
10/22/2008	St. MARYS ER ADDITION	14093-18	5.00	5.8	60		3290	4100	3887
10/24/2008	St. MARYS ER ADDITION	14093-19	6.25	6.0	57		1980	3270	3792
11/3/2008	BOWDOIN COL. FITNESS CTR	14138-5	8.00	7.8	51		2510	3155	3508
11/3/2008	BOWDOIN COL. FITNESS CTR	14138-6	8.50	7.4	53		2830	3620	3348
11/14/2008	BOWDOIN COL. FITNESS CTR	14138-7	8.00	7.2	67		3130	3915	3563
11/14/2008	BOWDOIN COL. FITNESS CTR	14138-8	8.00	5.8	67		2880	3485	3673
11/25/2008	BOWDOIN COL. FITNESS CTR	14138-9	7.00	6.8	63		3150	3790	3730
COUNT:		56	51	55	55	2	56	56	50
RANGE:	LOW	3.00	4.5	51	83	1980	3120	3398	
	HIGH	8.50	8.5	87	83	4550	4915	4372	
AVERAGE OF ALL:		5.50	6.3	69	83	3061	3852	3886	
STANDARD DEVIATION:		1.2	0.9	9.4	0.0	405	384	213	
COEFFICIENT OF VARIATION:		22.4	14.8	13.7	0.0	13.2	10.0	5.5	

ACI 214 SUMMARY:

AVERAGE STRENGTH:	3852 PSI ✓	
AVERAGE STRENGTH BASED ON:	56 TESTS	
STANDARD DEVIATION:	384 PSI	CONTROL IS EXCELLENT
OVERALL COEFFICIENT OF VARIATION:	10.0 %	
WITHIN-TEST STANDARD DEVIATION:	114 PSI	
WITHIN-TEST COEFFICIENT OF VARIATION:	3.0 %	CONTROL IS EXCELLENT
BATCH-TO-BATCH STANDARD DEVIATION:	366 PSI	
RECOMMENDED STRENGTH:	3514 PSI ✓	

AUBURN CONCRETE

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Ganneston Construction Corp.

FLORENCE HOUSE
390 VALLEY ST. - PORTLAND, ME

3500PSI - Air Entrained, ¾" Crushed Stone

Mix Design Submittal (Foundation Walls)

1/30/2009 — 3534SA

		Weight-SSD (lbs)	Volume (Cu.Ft.)	Sources
CEMENT, T I/II	ASTM C-150	564	2.87	DRAGON PRODUCTS COMPANY
COARSE AGG	ASTM C-33: #57/#67	1700	10.28	PIKE INDUSTRIES
FINE AGGREGATE	ASTM C-33	1270	7.80	PORTLAND SAND & GRAVEL
WATER U.S. GAL/CY:	33.6	280	4.49	CITY OF WESTBROOK
	AIR CONTENT (%):	6.0 +/- 1.5%	1.63	
	WATER/CEMENT RATIO:	0.50		
	SLUMP (Inches):	4.00 ± 1.00"	← 4" MAX PRIOR TO WR ADMIXTURE	
	YIELD:	140.9 PCF	27.1 Cu.Ft.	

GLENIUM 7500	ASTM C494, TYPE A,F	3.00 oz/cwt	16.9	US oz/CY	BASF/MASTER BUILDERS
MICROAIR	ASTM C-260	0.2 oz/cwt	1.1	US oz/CY	BASF/MASTER BUILDERS
OPTIONAL:					
POZZOLITH 100XR	ASTM C-494, Type B,D	2.00 oz/cwt	11.3	US oz/CY	BASF/MASTER BUILDERS
POZZUTEC 20+	ASTM C-494, Type C,E	10.00 oz/cwt	56.4	US oz/CY	BASF/MASTER BUILDERS

** GLENIUM 7500 dose is for MIDRANGE applications.

* GLENIUM 7500 meets the requirements of ASTM C494 for Type A (water-reducing) and Type F (high-range water -reducing). Plant of site addition of 2 - 3 fl.oz./cwt (11.3 - 16.9 fl.oz./cy) will be required to achieve end slump indicated.

MIX IDENTIFICATION #: 3534SA
DESIGN STRENGTH: 3500 PSI @ 28 DAYS

AUBURN CONCRETE

PROJECT:
CONTRACTOR:

DATE	PROJECT	ID#	SLUMP	% AIR	TEMPERATURE CONC	TEMPERATURE AIR	7 DAY	28 DAY	MOVING AVG. OF 3
9/28/2006	GASWORKS	687-1	6.50	6.1	71	65	3240	4545	
3/13/2007	Davis Island Townhouses	4896-2	5.50	5.2	70		3230	4530	
3/15/2007	Davis Island Townhouses	4896-3	7.75	5.8	61		4160	5005	4693
3/19/2007	Davis Island Townhouses	4896-4	8.00	6.5	68		3990	4750	4762
3/23/2007	Davis Island Townhouses	4896-6	4.00	5.6	66		3120	3850	4535
4/3/2007	Davis Island Townhouses	4896-12	4.00	4.7	56		2990	3700	4100
4/12/2007	Davis Island Townhouses	4896-15	5.50	5.2	60		3080	3990	3847
4/24/2007	Davis Island Townhouses	4896-19	4.75	5.9	70		2930	3660	3783
4/26/2007	Davis Island Townhouses	4896-20	5.75	4.7	59		2430	3420	3690
6/12/2007	Davis Island Townhouses	4896-25	5.25	5.4	78		3200	3980	3687
12/7/2008	Wal-Mart Scarborough	1788-13	6.50	5.0	61	20	3550	4690	4030
12/7/2008	Wal-Mart Scarborough	1788-14	4.00	5	65	28	3730	4685	4452
12/27/2008	Wal-Mart Scarborough	1788-25	5.50	6.6	60	31	3830	4540	4638
COUNT:		13	13	13	13	4	13	13	11
RANGE:		LOW	4.00	4.7	56	20	2430	3420	3687
		HIGH	8.00	6.6	78	65	4160	5005	4762
AVERAGE OF ALL:			5.62	5.5	65	36	3345	4257	4202
STANDARD DEVIATION:			1.3	0.6	6.0	17.2	465	488	403
COEFFICIENT OF VARIATION:			22.4	11.0	9.2	47.8	13.9	11.5	9.6

ACI 214 SUMMARY:

AVERAGE STRENGTH:	4257 PSI ✓	
AVERAGE STRENGTH BASED ON:	13 TESTS	
STANDARD DEVIATION:	488 PSI	CONTROL IS VERY GOOD
OVERALL COEFFICIENT OF VARIATION:	11.5 %	
WITHIN-TEST STANDARD DEVIATION:	51 PSI	
WITHIN-TEST COEFFICIENT OF VARIATION:	1.2 %	CONTROL IS EXCELLENT
BATCH-TO-BATCH STANDARD DEVIATION:	485 PSI	
RECOMMENDED STRENGTH:	Too few tests to determine recommended strength	

BASED ON 13 TEST

$f'_{cr} = f'_c + 1200 = 3500 + 1200 = 4700$

HOWEVER BASED ON TEST OF 3K CONCRETE

$S_s = 394 \quad \therefore f'_{cr} = 3500 + 1.3(394) = 4014$ ✓

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Ganneston Construction Corp.

FLORENCE HOUSE
390 VALLEY ST. - PORTLAND, ME

3000PSI - Non-Air Entrained, ¾" Crushed Stone

Mix Design Submittal (Interior S-O-G, elevated slabs)

1/30/2009 — 3034SNA

		Weight-SSD (lbs)	Volume (Cu.Ft.)	Sources
CEMENT, T I/II	ASTM C-150	517	2.63	DRAGON PRODUCTS COMPANY
COARSE AGG	ASTM C-33: #57/#67	1750	10.58	PIKE INDUSTRIES
FINE AGGREGATE	ASTM C-33	1440	8.84	PORTLAND SAND & GRAVEL
WATER U.S. GAL/CY:	33.6	280	4.49	CITY OF WESTBROOK
	AIR CONTENT (%):	2.0 +/- 1.5%	0.54	
	WATER/CEMENT RATIO:	0.54		
	SLUMP (Inches):	4.00 ± 1.00"	7.00" ± 1.00" (After Superplasticizer**)	
	YIELD:	147.2 PCF	27.1 Cu.Ft.	
GLENIUM 7500*	ASTM C494, TYPE A,F	3.50 oz/cwt	18.1 US oz/CY	BASF/MASTER BUILDERS
OPTIONAL:				
POLYMESH	ASTM C-1116 TYPE III	1.5 lbs/cy		O'DEA CONCRETE PRODUCTS
POZZUTEC 20+	ASTM C-494, Type C,E	10.00 oz/cwt	51.7 US oz/CY	BASF/MASTER BUILDERS

* GLENIUM 7500 dose is for **MIDRANGE** applications.

**GLENIUM 7500 meets the requirements of ASTM C494 for Type A (water-reducing) and Type F (high-range water -reducing).
Plant of site addition of 2 - 3 fl.oz./cwt (9.6 - 14.4 fl.oz./cy) will be required to achieve end slump indicated.

MIX IDENTIFICATION #: 3034SNA
DESIGN STRENGTH: 3000 PSI @ 28 DAYS

AUBURN CONCRETE

PROJECT:
CONTRACTOR:

W/C: 0.54
MAX. C.A.: 3/4"

DATE	PROJECT	ID#	SLUMP	% AIR	TEMPERATURE CONC	TEMPERATURE AIR	7 DAY	28 DAY	MOVING AVG. OF 3
1/24/08	84 MARGINAL WAY	765-65	5.50	2.6	68	25	3380	3855	
1/24/08	84 MARGINAL WAY	765-66	6.50		63	25	2740	3980	
1/24/08	84 MARGINAL WAY	765-67	6.50	2.1	60	25	3400	4360	4065
1/24/08	84 MARGINAL WAY	765-68	5.00	2.5	60	28	3520	4105	4148
1/24/08	84 MARGINAL WAY	765-69	6.75	3.3	50	29	3480	4105	4190
1/24/08	84 MARGINAL WAY	765-70	6.75	2.9	50	28	3450	4640	4283
1/31/08	84 MARGINAL WAY	765-71	6.25	2.0	77	20	3660	3810	4185
1/31/08	84 MARGINAL WAY	765-72	6.50	2.0	65	23	2830	3260	3903
1/31/08	84 MARGINAL WAY	765-73	6.75	2.6	61	25	3710	4000	3690
1/31/08	84 MARGINAL WAY	765-74	6.50	2.2	61	31	3290	3840	3700
1/31/08	84 MARGINAL WAY	765-75	6.75	2.2	55	33	3040	4000	3947
2/8/08	84 MARGINAL WAY	765-76	7.00	2.7	58	23	2920	3560	3800
2/8/08	84 MARGINAL WAY	765-77	7.00	2.5	52	20	2580	3495	3685
2/8/08	84 MARGINAL WAY	765-78	6.50	2.5	48	26	3360	3900	3652
2/8/08	84 MARGINAL WAY	765-79	7.00	2.5	47	22	2600	3505	3633
2/8/08	84 MARGINAL WAY	765-80	6.50	2.5	48	22	3470	4095	3833
2/29/08	84 MARGINAL WAY	765-84	6.00	2.5	72	4	3460	4095	3898
2/29/08	84 MARGINAL WAY	765-85	6.75	2.6	48	11	2900	3885	4025
2/29/08	84 MARGINAL WAY	765-86	5.00	2.4	52	25	3290	4165	4048
6/13/08	RESIDENCE INN-AUBURN	14064-17	6.50	3.8	72		2410	3395	3815
6/13/08	RESIDENCE INN-AUBURN	14064-18	6.00	3.9	71		2530	3035	3532
6/13/08	RESIDENCE INN-AUBURN	14064-19	5.75	4.1	73		2820	3385	3272
6/25/08	RESIDENCE INN-AUBURN	14064-23	5.00	4.5	78		3110	3980	3467
6/25/08	RESIDENCE INN-AUBURN	14064-24	5.00	4.8	78		2950	3425	3597
6/25/08	RESIDENCE INN-AUBURN	14064-25	6.00	4.5	75		2940	3830	3745
6/25/08	RESIDENCE INN-AUBURN	14064-26	5.50	4.9	75		3000	3635	3630
6/25/08	RESIDENCE INN-AUBURN	14064-27	5.75	3.9	77		2920	3700	3722
6/25/08	RESIDENCE INN-AUBURN	14064-28	5.50	4.0	75		2680	3495	3610
9/23/08	St. MARYS ER ADDITION	14093-15	5.25	1.9	68		3690	4160	3785
9/23/08	St. MARYS ER ADDITION	14093-16	6.00	1.8	67		3540	4180	3945
9/23/08	St. MARYS ER ADDITION	14093-17	5.25	1.8	69		3770	4445	4262
10/30/08	ST. MARY'S MOB	14131-13	5.25	2.2	61		2980	4065	4230
10/30/08	ST. MARY'S MOB	14131-14	5.00	2.4	57		3290	3935	4148
10/30/08	ST. MARY'S MOB	14131-15	6.00	2.0	55		3190	4480	4160
12/3/08	BOWDOIN COL FITNESS CTR	14138-10	5.50	3.2	67		3940	4700	4372
12/30/08	ST. MARY'S MOB	14131-16	6.00	2.4	63		3320		
12/30/08	ST. MARY'S MOB	14131-17	5.50	2.0	59		3190		
COUNT:		39	39	38	39	19	39	35	33
RANGE:	LOW	5.00	1.8	47	4	2410	3035	3272	
	HIGH	7.00	4.9	78	33	3940	4700	4372	
AVERAGE OF ALL:		6.04	2.9	63	23	3190	3900	3878	
STANDARD DEVIATION:		0.6	0.9	9.7	6.5	377	382	266	
COEFFICIENT OF VARIATION:		10.8	30.7	15.4	27.7	11.8	9.8	6.9	

ACI 214 SUMMARY:

AVERAGE STRENGTH:	3900 PSI	
AVERAGE STRENGTH BASED ON:	35 TESTS	
STANDARD DEVIATION:	382 PSI	CONTROL IS EXCELLENT
OVERALL COEFFICIENT OF VARIATION:	9.8 %	
WITHIN-TEST STANDARD DEVIATION:	121 PSI	
WITHIN-TEST COEFFICIENT OF VARIATION:	3.1 %	CONTROL IS VERY GOOD
BATCH-TO-BATCH STANDARD DEVIATION:	362 PSI	
RECOMMENDED STRENGTH:	3512 PSI	

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Ganneston Construction Corp.

FLORENCE HOUSE
390 VALLEY ST. - PORTLAND, ME

4500PSI - Air Entrained, ¾" Crushed Stone

Mix Design Submittal (Exterior Slabs, exposed site concrete)

1/30/2009 — 4534SA

		Weight-SSD (lbs)	Volume (Cu.Ft.)	Sources
CEMENT, T I/II	ASTM C-150	658	3.35	DRAGON PRODUCTS COMPANY
COARSE AGG	ASTM C-33: #57/#67	1800	10.89	K & K EXCAVATION
FINE AGGREGATE	ASTM C-33	1100	6.75	PORTLAND SAND & GRAVEL
WATER U.S. GAL/CY:	33.6	280	4.49	CITY OF WESTBROOK
AIR CONTENT (%):		6.0 +/- 1.5%	1.63	
WATER/CEMENT RATIO:		0.43		
SLUMP (Inches):		3.00	± 1.00"	7.00" ± 1.00" (After Superplasticizer**)
YIELD:		141.6 PCF	27.1 Cu.Ft.	

GLENIUM 7500*	ASTM C494, TYPE A,F	3.00 oz/cwt	19.7	US oz/CY	BASF/MASTER BUILDERS
MICROAIR	ASTM C-260	0.35 oz/cwt	2.3	US oz/CY	BASF/MASTER BUILDERS
OPTIONAL:					
POZZOLITH 100XR	ASTM C-494, Type B,D	2.00 oz/cwt	13.2	US oz/CY	BASF/MASTER BUILDERS
POZZUTEC 20+	ASTM C-494, Type C,E	10.00 oz/cwt	65.8	US oz/CY	BASF/MASTER BUILDERS

* GLENIUM 7500 dose is for **MIDRANGE** applications.

* GLENIUM 7500 meets the requirements of ASTM C494 for Type A (water-reducing) and Type F (high-range water-reducing).
Plant of site addition of 2 - 3 fl.oz./cwt (13.2 - 19.7 fl.oz./cy) will be required to achieve end slump indicated.

DRAGON PRODUCTS COMPANY
P.O. Box 191, U.S. Route 1 • Thomaston, Maine 04861 • 207-594-5555

MILL TEST RESULTS
Laboratory at Thomaston, Maine

Date: January, 2008
Cement Type: I/II

CHEMICAL DATA	Percent
Silicon Dioxide	20.0
Aluminum Dioxide	4.2
Ferric Oxide	3.0
Calcium Oxide	61.5
Magnesium Oxide	3.0
Sulphur Trioxide	3.6
Loss on Ignition	1.4
Insoluble Residue	0.4
Tricalcium Silicate	55
Dicalcium Silicate	15
Tricalcium Aluminate	6
Sodium Oxide	0.5
Potassium Oxide	1.1
Equivalent Alkalies	1.2

(Chemical Analysis all per ASTM C 114)

PHYSICAL DATA	
Specific Surface, Blaine (sq m /kg) *(Per ASTM C 204)*	373
Percent Passing 325 Mesh. *(Per ASTM C 430)*	97.3
Compressive Strength (psi) *(Per ASTM C 109)*	
1 day	2300
3 day	4040
7 day	5030
28 day	
Vicat Setting Time *(Per ASTM C 191)*	
Initial (min.)	125
Final (min.)	230
Air Content (%) *(Per ASTM C 185)*	6.8
Autoclave Expansion (%) *(Per ASTM C 151)*	0.05
Expansion in water (%) *(Per ASTM C 1038)*	0.012

Certified by:

Jennifer K. Colburn

Jennifer K. Colburn

We hereby certify that this cement complies with current ASTM C 150, AASHTO M-85 and CSA-A3000 - 03 Type GU specifications.

Testing was completed by Brian Secord and/or Marie-Michele Bouchard.
This mill test report is generated for silos produced in the calendar month prior to the date upon this report.

AUBURN CONCRETE

- - - 2008 - 2009 GRADATION SUMMARY - - -

3/4" Quarry Stone (# 67)

Source: K & K Excavation - Christian Hill Quarry, Auburn, Maine

Specific Gravity: 2.67 Absorption: 0.40% Tested by: Summit Labs 4/24/08

		2"	1½"	1"	¾"	½"	⅜"	#4	#8	#200
4/1/2008	K&K		100	100	99	69	49	10	3	0.6
4/4/2008	RD-CHQ		100	100	99	57	29	7	3	0.5
4/9/2008	K&K		100	100	99	58	30	7	2	0.5
4/11/2008	RD-A		100	100	98	62	31	8	4	0.5
4/16/2008	K&K		100	100	98	60	33	9	3	0.5
4/25/2008	K&K		100	100	98	53	29	9	3	0.5
5/5/2008	RD-CHQ		100	100	97	53	27	8	4	0.5
5/7/2008	K&K		100	100	97	51	27	7	3	0.5
5/9/2008	K&K		100	100	94	50	24	6	3	0.5
5/13/2008	K&K		100	100	96	50	24	8	3	0.4
5/16/2008	RD-CHQ		100	100	95	52	22	4	3	0.8
5/28/2008	K&K		100	100	97	51	25	6	2	0.5
5/30/2008	K&K		100	100	94	48	20	5	2	0.4
6/2/2008	K&K		100	100	95	55	27	7	3	0.5
6/5/2008	RD-W		100	100	96	48	21	6	3	0.5
6/16/2008	K&K		100	100	99	61	32	8	3	0.7
6/18/2008	K&K		100	100	96	45	23	5	2	0.6
6/19/2008	RD-A		100	100	97	55	27	6	3	0.5
6/19/2008	RD-CHQ		100	100	98	58	32	9	4	0.9
6/30/2008	K&K		100	100	99	53	27	9	3	0.5
7/1/2008	RD-CHQ		100	100	95	43	20	5	2	0.7
7/1/2008	K&K		100	100	99	54	24	7	3	0.4
7/7/2008	K&K		100	100	97	60	28	7	3	0.7
7/10/2008	RD-W		100	100	93	60	32	8	3	0.5
7/16/2008	RD-CHQ		100	100	97	55	26	7	3	0.6
7/21/2008	RD-A		100	100	98	59	31	9	4	0.9
7/25/2008	K&K		100	100	98	59	27	6	2	0.4
7/28/2008	K&K		100	100	97	50	21	5	3	0.5
8/1/2008	RD-CHQ		100	100	99	46	29	8	4	0.8
8/12/2008	K&K		100	100	97	46	22	6	3	0.6
8/13/2008	RD-A		100	100	94	42	21	8	3	0.7
8/19/2008	K&K		100	100	96	47	24	5	2	0.3
8/20/2008	RD-WB		100	100	97	55	32	8	3	0.4
8/22/2008	K&K		100	100	92	51	32	8	3	0.7
8/25/2008	K&K		100	100	94	53	27	6	3	0.7
9/3/2008	RD-W		100	100	94	54	31	9	4	0.7
9/25/2008	RD-A		100	100	96	53	32	9	4	0.3
9/26/2008	K&K		100	100	94	44	22	5	2	0.6
9/29/2008	RD-A		100	100	93	44	23	8	4	0.6
9/29/2008	K&K		100	100	98	48	27	6	2	0.4
10/1/2008	K&K		100	100	94	57	29	8	3	0.5
10/9/2008	RD-WB		100	100	91	42	22	6	3	0.6
10/15/2008	K&K		100	100	92	48	22	4	2	0.4
11/6/2008	K&K		100	100	91	52	31	8	3	0.6
11/7/2008	RD-WB		100	100	90	45	24	6	3	1.0
12/8/2008	RD-WB		100	100	90	49	30	10	5	0.4
1/6/2009	RD-A		100	100	91	46	22	4	2	0.3
AVERAGE			**100.0**	**100.0**	**95.7**	**52.1**	**27.0**	**7.0**	**3.0**	**0.6**
SPECIFICATION:		**#57**	**100**	**95-100**	**----**	**25-60**	**----**	**0-10**	**0-5**	**<1.5**
ASTM C33		**#67**		**100**	**90-100**	**----**	**20-55**	**0-10**	**0-5**	**<1.5**

- - - 2007 - 2008 - 2009 GRADATION SUMMARY - - -

Concrete Sand

Source: Portland Sand & Gravel - Gray, Maine

Specific Gravity: 2.63 Absorption: 0.67% Tested by: Summit Labs 4/24/08

		FM	Color	3/8"	#4	#8	#16	#30	#50	#100	#200
9/6/2007	RD-A	2.54	<1	100	97	91	77	53	22	5	1.2
9/7/2007	RD-W	2.53		100	97	91	76	53	24	6	1.4
9/19/2007	RD-PSG	2.43	<1	100	98	93	81	57	23	5	1.0
10/4/2007	RD-A	2.49		100	98	92	77	52	25	8	1.8
11/15/2007	RD-A	2.58	<1	100	98	91	75	51	22	6	1.2
11/21/2007	RD-A	2.59		100	97	90	75	51	22	6	1.4
12/12/2007	RD-W	2.60		100	98	90	75	50	22	6	1.3
12/17/2007	RD-A	2.53	<1	100	98	92	76	52	23	6	1.3
12/28/2007	RD-A	2.53		100	97	91	76	52	24	7	1.6
1/11/2008	RD-W	2.60		100	98	90	75	50	22	6	1.3
1/31/2008	RD-PSG	2.60	<1	100	98	91	74	49	22	6	1.3
2/12/2008	RD-PSG	2.59		100	98	90	75	50	23	6	1.4
2/15/2008	RD-PSG	2.63		100	98	90	73	48	22	6	1.2
2/25/2008	RD-PSG	2.68		100	98	89	72	47	21	5	1.2
2/28/2008	RD-W	2.65		100	98	89	73	48	22	6	1.2
3/10/2008	JC-W	2.63		100	98	89	73	48	22	6	1.3
3/13/2008	RD-A	2.70	<1	100	98	89	71	46	20	6	1.2
3/28/2008	RD-PSG	2.61		100	98	90	74	49	22	5	1.2
4/4/2008	RD-PSG	2.57		100	98	91	75	50	23	6	1.4
4/11/2008	RD-A	2.60		100	98	91	75	49	22	6	1.3
5/5/2008	RD-PSG	2.64		100	98	89	73	49	22	6	1.3
5/19/2008	RD-PSG	2.44		100	98	92	80	57	23	5	1.2
5/27/2008	RD-A	2.62		100	97	91	76	50	18	4	1.0
6/5/2008	RD-W	2.68		100	97	91	75	48	17	3	0.8
6/17/2008	RD-A	2.54	<1	100	97	91	77	54	23	5	1.2
6/20/2008	RD-PSG	2.52		100	97	91	77	53	23	6	1.3
7/1/2008	RD-PSG	2.49		100	98	92	79	56	22	5	1.1
7/10/2008	RD-W	2.71	<1	100	97	90	72	46	19	5	1.2
7/16/2008	RD-PSG	2.71		100	97	90	75	46	17	4	0.9
7/21/2008	RD-A	2.54	<1	100	98	92	77	53	21	5	1.1
7/30/2008	RD-A	2.45		100	98	93	80	56	23	5	1.1
7/31/2008	JC-W	2.70		100	97	91	76	48	15	4	0.8
8/13/2008	RD-A	2.57		100	97	89	74	53	24	6	1.3
8/21/2008	RD-WB	2.79		100	97	89	72	44	15	4	0.9
9/3/2008	RD-W	2.64	<1	100	96	90	75	50	20	5	1.2
9/25/2008	RD-A	2.65	<1	100	98	92	76	48	18	4	0.9
9/29/2008	RD-A	2.65		100	98	93	77	49	15	3	0.8
10/9/2008	RD-WB	2.75		100	97	90	74	46	15	3	0.7
11/7/2008	RD-WB	2.83	< 1	100	98	93	75	40	10	1	0.3
11/13/2008	RD-WB	2.72		100	97	91	75	47	15	3	0.5
12/8/2008	RD-WB	2.74		100	98	93	77	45	13	2	0.4
1/6/2009	RD-PSG	2.63	< 1	100	99	90	73	49	21	5	0.8
AVERAGE		**2.61**		**100.0**	**97.6**	**90.8**	**75.3**	**49.8**	**20.4**	**5.0**	**1.1**
SPECIFICATION: ASTM C33		**2.3 - 3.1**		**100**	**95-100**	**80-100**	**50-85**	**25-60**	**10-30**	**2-10**	**< 5**

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: 626-9094

DAILY FIELD REPORT

Date: 4/1/09

Project: Avesta Florence House

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Concrete Testing Services

Work Activities: A crew from Newman Concrete placed a total of 16 cubic yards of 3/4" aggregate, 3000 psi concrete for footings on N-line from 16-line to 7.1-line. All concrete was supplied by Auburn Concrete, placed via truck chute, and contained Micro Air (Air entraining admixture) and Glenium 7500 (mid-range water reducer.) One set of four concrete test cylinders were cast and will be picked up at a later date for controlled storage and compressive strength testing.

Test Results:
Slump: 4-3/4" to 5"
Air%: 7.0%
Concrete Temp: 61 to 62 deg.F
Air Temp: 42 deg.F

Portal to Portal		**Expenses**		**Signed:**	Neil Davis
Leave:	11:15am	Mileage:	11	**cc:**	
Return:	1:45pm	Density Gauge:			
TOTAL:	2.5	Other:			

Reviewed: Darrell A. Gilman, CMT Manager
Sent: 4/1/09

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Phone: (207) 621-8334 Fax: 626-9094

DAILY FIELD REPORT

Date: 4/16/09

Project: Avesta Florence House

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Concrete Testing Services

Work Activities: A crew from Newman Concrete placed a total of 24 cubic yards for the footings on line 16 from F to M and line F from 7 to 16. All concrete was supplied by Auburn Concrete with a mix design of 3/4" aggregate and a design strength of 3000psi., with a mid-range water reducer (glenium 7500) added at the batch plant. One set (# 2) of four concrete test cylinders was cast and will be retrieved at a later date for controlled storage and compressive strength testing.

Test Results:
Slump: 5-1/4" to 5-3/4"
Air%: 5.5 to 5.8%
Concrete Temp: 47 to 49 deg.F
Air Temp: 53 deg.F

Portal to Portal		**Expenses**		**Signed:**	Frank Clark
Leave:	9:30am	Mileage:	11	**cc:**	
Return:	12:15pm	Density Gauge:			
TOTAL:	2.75	Other:	2.75 tolls		

Reviewed: Darrell A. Gilman, CMT Manager
Sent: 4/17/09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: 626-9094

DAILY FIELD REPORT

Date: 4/17/09

Project: Avesta Florence House - Portland, Maine

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Concrete testing services.

Work Activities: A crew from Newman Concrete placed a total of 65cy of 3/4" aggregate, 3500 psi concrete for the foundation wall on line N from line 16 to 7.1, and line 16 from N to N-10'. A high range water reducer (HRWR) Glenium 7500 was added to the concrete at the batch plant (19.7oz/cy) for workability. The concrete was supplied by Auburn Concrete and placed via pump truck.
Soil compaction tests were performed on the structural gravel below the foundation footing along line 7 at D-15'.

Test Results:
Concrete:
Slump range: 5 1/2" to 7 1/2"
Air Content range: 5.4% to 5.7%
Concrete Temp.: 56 to 58 deg. F
Air Temp.: 68 deg. F
W/C Ratios: 0.49 to 0.50
Soil:
% Compaction: 98.1%

Remarks:
Two sets of four cylinders were cast for compressive strength testing.

Portal to Portal		**Expenses**		**Signed:**	Darrell Gilman
Leave:	9:30	Mileage:	13	**cc:**	
Return:	3:00	Density Gauge:	1		
TOTAL:	5.5	Other:			

Reviewed: Darrell A. Gilman, CMT Manager
Sent: 4/20/09

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DAILY FIELD REPORT

Date: 4/23/09

Project: Avesta Florence House - Portland, Maine

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Concrete Testing Services.

Work Activities: A crew from Newman Concrete placed a total of 13.5cy of 3/4" aggregate, 3000 psi concrete for the footing on line 7 from line D-A and line A from 1-7. The concrete was supplied by Auburn Concrete and placed via tailgate.
One set of four test cylinders was cast and will be picked up at a later date.

Test Results:
Concrete:
Slump range: 4 1/2" to 5"
Air Content range: 6.2%
Concrete Temp.: 60 deg. F
Air Temp.: 45 deg. F
W/C Ratios: 0.49

Remarks:

Portal to Portal		**Expenses**		**Signed:**	Mike Sullivan
Leave:	9:30	Mileage:	13	**cc:**	
Return:	11:30	Density Gauge:			
TOTAL:	2	Other:			

Reviewed: Darrell A. Gilman, CMT Manager
Sent: 4/23/09

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DAILY FIELD REPORT

Date: 5/1/09

Project: Avesta Florence House - Portland, Maine

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Concrete Testing Services.

Work Activities: A crew from Newman Concrete placed a total of 60cy of 3/4" aggregate, 3000 psi concrete for the walls on 16-line from A to F, and F-line from 16 to the south end. The concrete was supplied by Auburn Concrete and placed via pump truck. Two sets of four test cylinders were cast and will be picked up at a later date.

Test Results:
Slump Range: 4 3/4" to 6"
Air Content range: 6.2% to 6.3%
Concrete Temp.: 64 to 67 deg. F
Air Temp.: 70 to 71 deg. F

Remarks:

Portal to Portal		**Expenses**		**Signed:**	Neil Davis
Leave:	9:15am	Mileage:	13	**cc:**	
Return:	1:45pm	Density Gauge:			
TOTAL:	4.5	Other:			

Reviewed: Darrell A. Gilman, CMT Manager
Sent: 5/4/09

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DAILY FIELD REPORT

Date: 5/4/09

Project: Avesta Florence House - Portland, Maine

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Concrete Testing Services and Cylinder Pick Up.

Work Activities: A crew from Newman Concrete placed a total of 17cy of 3/4" aggregate, 3000 psi concrete for the footings on N-line from 8.2 to the elevator base slab to L8 on 1-line. The concrete was supplied by Auburn Concrete and placed via truck chute.
One set of four test cylinders were cast and will be picked up at a later date.
Also, retrieved two sets of four concrete test specimens for controlled storage and compressive strength testing.

Test Results:
Slump Range: 4 1/2" to 5"
Air Content: 6.2%
Concrete Temp.: 66 to 67 deg. F
Air Temp.: 60 deg. F

Remarks:

Portal to Portal		**Expenses**		**Signed:**	Neil Davis
Leave:	12:15pm	Mileage:	13	**cc:**	
Return:	3:15pm	Density Gauge:			
TOTAL:	3	Other:			

Reviewed: Darrell A. Gilman, CMT Manager
Sent: 5/5/09

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DAILY FIELD REPORT

Date: 5/8/09

Project: Avesta Florence House - Portland, Maine

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Concrete Testing Services.

Work Activities: A crew from Newman Concrete placed a total of 38cy of 3/4" aggregate, 3000 psi concrete for the following:
-footings on lines 13 and 14 from N to J
-column piers J13 and L13
-four pads south of 13-line
-retaining wall footing
-footings on 1-line from A.5 to D and south of 1-line from A.5 to D. Newman Concrete also placed a total of 33 cubic yards of 3/4" aggregate, 3500 psi concrete for elevator shaft walls on 5 and 5.8-line from M to N, M-line from 5 to 5.8, A-line from 1 to 7, and 7-line from A to F. The concrete was supplied by Auburn Concrete contained a mid-range water reducer (Glenium 7500), air-entraining admixture (Micro-Air), and was placed via pump truck.

Test Results:
Slump Range: 5-1/2" to 6"
Air Content range: 6.0% to 6.2%
Concrete Temp.: 64 to 66 deg. F
Air Temp.: 70 to 71 deg. F

Remarks: Two sets of four test specimen were cast for compressive strength testing.

Portal to Portal		**Expenses**		**Signed:**	Neil Davis
Leave:	10:30am	Mileage:	13	**cc:**	
Return:	3:30pm	Density Gauge:			
TOTAL:	5	Other:			

Reviewed: Darrell A. Gilman, CMT Manager
Sent: 5/11/09

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DAILY FIELD REPORT

Date: 5/15/09

Project: Avesta Florence House - Portland, Maine

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Concrete Testing Services.

Work Activities: A crew from Newman Concrete placed a total of 7cy of 3/4" aggregate, 3500 psi concrete for the foundation wall at 1line A to D. The concrete was supplied by Auburn Concrete and contained a mid-range water reducer (Glenium 7500), and was placed via tail gate.

Test Results:
Slump Range: 6 1/4" to 6 3/4"
Air Content range: 7.0% to 7.2%
Concrete Temp.: No thermometer was available
Air Temp.: 70 deg. F

Remarks: One set of four test specimen was cast for compressive strength testing.

Portal to Portal		**Expenses**		**Signed:**	Justin Rouillard
Leave:	8:30 AM	Mileage:	15	**cc:**	
Return:	10:30 PM	Density Gauge:			
TOTAL:	2	Other:			
				Reviewed:	Darrell A. Gilman, CMT Manager
				Sent:	5/21/09

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DAILY FIELD REPORT

Date: 5/20/09

Project: Avesta Florence House - Portland, Maine

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Concrete Testing Services.

Work Activities: A crew from Newman Concrete placed a total of 54cy of 3/4" aggregate, 3500 psi concrete for the foundation wall at 1line H to M, along M to 1.75, 2 line M to N, and along N to 7.1. The concrete was supplied by Auburn Concrete and contained a mid-range water reducer (Glenium 7500), and was placed via pump truck.

Test Results:
Slump Range: 6 1/4" to 6 3/4"
Air Content range: 7.0% to 7.2%
Concrete Temp.: No thermometer was available
Air Temp.: 70 deg. F

Remarks: Two sets of four test specimen were cast for compressive strength testing.

Portal to Portal		**Expenses**		**Signed:**	Adam Lyons
Leave:	11:00 AM	Mileage:	115	**cc:**	
Return:	5:15 PM	Density Gauge:			
TOTAL:	6.25	Other:			

Reviewed: Darrell A. Gilman, CMT Manager
Sent: 5/21/09

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DAILY FIELD REPORT

Date: 5/22/09

Project: Avesta Florence House - Portland, Maine

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Concrete Testing Services.

Work Activities: A crew from Newman Concrete placed a total of 5cy of 3/4" aggregate, 3500 psi concrete for the foundation footing at 1line H to D.2. The concrete was supplied by Auburn Concrete and contained a mid-range water reducer (Glenium 7500), and was placed via tail gate.

Test Results:
Slump Range: 3 1/2" to 4"
Air Content range: 6.0% to 6.5%
Concrete Temp.: 73 deg. F
Air Temp.: 80 deg. F

Remarks: One set of four test specimen was cast for compressive strength testing.

Portal to Portal		**Expenses**		**Signed:**	Justin Rouillard
Leave:	12:00 AM	Mileage:	25	**cc:**	
Return:	3:00 PM	Density Gauge:			
TOTAL:	3	Other:			

Reviewed: Darrell A. Gilman, CMT Manager
Sent: 5/27/09

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DAILY FIELD REPORT

Date: 5/27/09

Project: Avesta Florence House - Portland, Maine

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Concrete Testing Services.

Work Activities: A crew from Newman Concrete placed a total of 80cy of 3/4" aggregate, 3000 psi concrete for stairwell 1 and 3 footings, and all interior footings between line 1 and 5.8. The crew also placed 11cy of 3/4" aggregate, 3500psi concrete for the foundation wall along line 1 from "D" to "H". The concrete was supplied by Auburn Concrete and contained a mid-range water reducer (Glenium 7500), Micro Air, and was placed via pump truck.

Test Results:
Slump Range: 5 1/2" to 6 3/4"
Air Content range: 7.0% to 7.6%
Concrete Temp.: No thermometer was available
Air Temp.: 53 deg. F

Remarks: Three sets of four test specimen were cast for compressive strength testing. (Sets 15, 16, and 17)

Portal to Portal		**Expenses**		**Signed:**	Adam Lyons
Leave:	11:30 AM	Mileage:	113	**cc:**	
Return:	5:45 PM	Density Gauge:			
TOTAL:	6.25	Other:			

Reviewed: Darrell A. Gilman, CMT Manager
Sent: 5/29/09

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DAILY FIELD REPORT

Date: 6/18/09

Project: Avesta Florence House - Portland, Maine

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Concrete testing services.

Work Activities: A crew from Newman Concrete placed a total of 18cy of 3/4" aggregate, 3,500psi concrete for the retaining wall. Concrete was supplied by Auburn Concrete and was placed via pump truck supplied bby Northeast Pumping Services.

Test Results:

Slump range: 4 1/2" to 4 3/4"
Air Content: 5.2%
Concrete Temp.: 68 deg. F
Air Temp.: ~65 deg. F
One set of four cylinders was cast for compressive strength testing.

Remarks:

Portal to Portal		**Expenses**		**Signed:**	Darrell A. Gilman
Leave:	11:45	Mileage:	15	**cc:**	
Return:	2:15	Density Gauge:			
TOTAL:	2.5	Other:			

Reviewed: Darrell A. Gilman, CMT Manager
Sent: 6/19/09

SUMMIT GEOENGINEERING SERVICES
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DAILY FIELD REPORT

Date: 7/6/09

Project: Avesta Florence House - Portland, Maine

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Concrete testing services.

Work Activities: A crew from Pride Concrete placed a total of 80cy of 3/4" aggregate, 3,000psi, no air concrete for the first floor slab. Concrete was supplied by Auburn Concrete and was placed via pump truck supplied by Northeast Pumping Services.

Test Results:

Slump range: 5-1/2 to 7-1/2"
Air Content: 2.0 to 2.5%
Concrete Temp.: 67 to 75 deg. F
Air Temp.: 62 to 75 deg. F
Two sets (#19,20) of four cylinders was cast for compressive strength testing.

Remarks: Mike Adams was notified of all testing results.

Portal to Portal		**Expenses**		**Signed:**	Kris Bennett
Leave:	6:45	Mileage:	11	**cc:**	
Return:	11:15	Density Gauge:			
TOTAL:	4.5	Other:			

Reviewed: Darrell A. Gilman, CMT Manager
Sent: 7/8/09

SUMMIT GEOENGINEERING SERVICES
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DAILY FIELD REPORT

Date: 7/10/09

Project: Avesta Florence House - Portland, Maine

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Concrete Testing Services.

Work Activities: A crew from Pride Concrete placed a total of 80cy of 3/4" aggregate, 3,000psi, no air concrete for the second floor slab. Concrete was supplied by Auburn Concrete and was placed via pump truck supplied by Northeast Pumping Services.

Test Results:

Slump range: 5-1/2 to 7-1/2"
Air Content: 2.0 to 2.4%
Concrete Temp.: 69 to 75 deg. F
Air Temp.: 56 to 72 deg. F
Two sets (#21,22) of four cylinders was cast for compressive strength testing.

Remarks: Mike Adams was notified of all testing results.

Portal to Portal		**Expenses**		**Signed:**	Kris Bennett
Leave:	6:30	Mileage:	11	**cc:**	
Return:	11:45	Density Gauge:			
TOTAL:	5.25	Other:			

Reviewed: Darrell A. Gilman, CMT Manager
Sent: 7/8/09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
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DAILY FIELD REPORT

Date: 8/6/09

Project: Avesta Florence House - Portland, Maine

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Concrete Testing

Work Activities: A crew from pride floors place a total of 126cy of concrete for the first floor slab. The first 70cy of concrete was a 3000psi mix design with a mid range water reducer (glenium 7500) added during the batching process. The remaining 56cy of concrete was also a 3000psi mix design with a mid range water reducer but in addition reinforcing fiber mesh was added during the batching process as well. All was going well until the crew began to slow down because the slab on the south half of the building was only approximately 3" thick. This caused the 10th truck to exceed it 90 min time limit while it was unloading. I informed Everett and he said the he was going to keep the truck and let it finish unloading. However, he rejected the 11th truck because it had been batched for 120 mins. The wait for another truck to arrive caused a "cold joint" to occur.

Test Results:

Slump Range:5.5 to 6.5
Air Content: 2.5 to 4.0
Concrete Temp:
Air Temp: 80degrees

Remarks:

Portal to Portal		**Expenses**		**Signed:**	Justin Rouillard
Leave:	6:00	Mileage:	11	**cc:**	
Return:	3:00	Density Gauge:			
TOTAL:	9	Other:			

Reviewed: Darrell A. Gilman, CMT Manager
Sent: 8/10/09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: (207) 626-9094

CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No:	14194
Project:	Avesta Florence House
Client:	Avesta Florence House, LP and Florence House 307 Cumberland Avenue Portland, ME 04101

Field Test Data

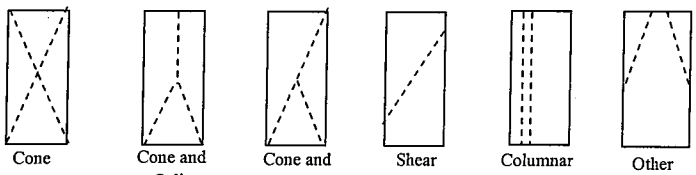
Set No.:	C1
Placement Date:	1-Apr-09
Lab Rec'd Date:	2-Apr-09
Location:	Footings on N-Line 16 to 12.5
Technician:	N. Davis
Supplier:	Auburn Concrete
Mix Designation:	3/4" Aggregate
Design Strength:	3000psi

Slump (initial)		in.
Slump (placed)	5	in.
Air Content	7.0	%
Conc Temp.	62.0	°F
Air Temp.	42.0	°F
Volume (yds)	8.0	of 16.0
Admixture:	Micro Air, Glenium 7500 (Mid-Range Water)	

Laboratory Test Data

Sample No.	Test Date	Age	Type	Unit Wt.	Area (in^2)	Load (K)	Strength (psi)
C1a	8-Apr-09	7	2	144.3	28.27	102.7	3630
C1b	29-Apr-09	28	3	144.3	28.27	124.1	4390
C1c	29-Apr-09	28	2	144.9	28.27	123.8	4380
C1d							

Average 28 Day (psi): 4385



Remarks: _____

Reviewed: Darrell Gilman, CMT Manager
Sent:04-29-09

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Phone: (207) 621-8334 Fax: (207) 626-9094

CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No: 14194
Project: Avesta Florence House
Client: Avesta Florence House, LP and Florence House
307 Cumberland Avenue
Portland, ME 04101

Field Test Data

Set No.: C2
Placement Date: 16-Apr-09
Lab Rec'd Date: 17-Apr-09
Location: Footing; Line F from 7 to 16, Line 16 from F to M

Technician: F. Clark
Supplier: Auburn Concrete
Mix Designation: 3/4" Aggregate
Design Strength: 3000psi

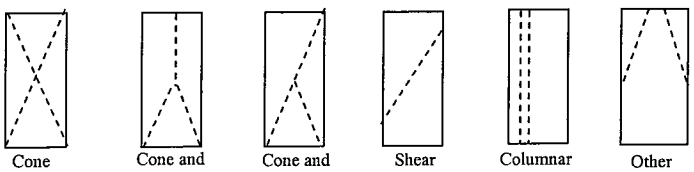
Slump (initial)		in.
Slump (placed)	5 1/4	in.
Air Content	5.5	%
Conc Temp.	49.0	°F
Air Temp.	53.0	°F
Volume (yds)	16.0	of 24.5

Admixture: Glenium 7500 (Mid-Range Water Reducer)

Laboratory Test Data

Sample No.	Test Date	Age	Type	Unit Wt.	Area (in^2)	Load (K)	Strength (psi)
C2a	23-Apr-09	7	6	147.5	28.27	103.7	3670
C2b	14-May-09	28	3	149.1	28.27	127.4	4510
C2c	14-May-09	28	3	148.2	28.27	123.7	4370
C2d							

Average 28 Day (psi): 4440



Remarks: _____

Reviewed: Darrell Gilman, CMT Manager
Sent: 5-14-09

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CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No: 14194
Project: Avesta Florence House
Client: Avesta Florence House, LP and Florence House
307 Cumberland Avenue
Portland, ME 04101

Field Test Data

Set No.: C3
Placement Date: 17-Apr-09
Lab Rec'd Date: 18-Apr-09
Location: Foundation Wall Line N from 16 to 7.1
Wall Line 16 from N to N-10'
Technician: D. Gilman
Supplier: Auburn Concrete
Mix Designation: 3/4" Aggregate
Design Strength: 3500psi

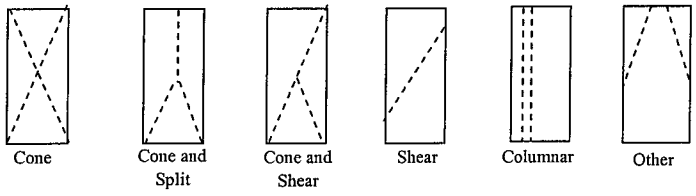
Slump (initial)		in.
Slump (placed)	7 1/2	in.
Air Content	5.4	%
Conc Temp.	57.0	°F
Air Temp.	68.0	°F
Volume (yds)	20.0	of 65.0

Admixture: Glenium 7500 (Mid-Range Water Reducer)

Laboratory Test Data

Sample No.	Test Date	Age	Type	Unit Wt.	Area (in^2)	Load (K)	Strength (psi)
C3a	24-Apr-09	7	2	146.0	28.27	91.5	3240
C3b	15-May-09	28	3	146.8	28.27	109.4	3870
C3c	15-May-09	28	3	147.1	28.32	114.9	4060
C3d							

Average 28 Day (psi): 3965



Remarks: _____

Reviewed: Darrell Gilman, CMT Manager
Sent: 5-14-09

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CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No: 14194
Project: Avesta Florence House
Client: Avesta Florence House, LP and Florence House
307 Cumberland Avenue
Portland, ME 04101

Field Test Data

Set No.: C4
Placement Date: 17-Apr-09
Lab Rec'd Date: 18-Apr-09
Location: Foundation Wall Line N from 16 to 7.1
Line 16 from N to N-10'
Technician: D. Gilman
Supplier: Auburn Concrete
Mix Designation: 3/4" Aggregate
Design Strength: 3500psi

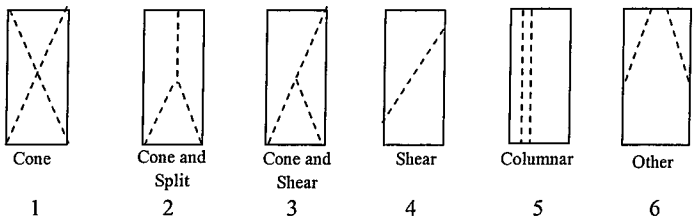
Slump (initial)		in.
Slump (placed)	7 1/2	in.
Air Content	5.7	%
Conc Temp.,	58.0	°F
Air Temp.	68.0	°F
Volume (yds)	50.0	of 65.0

Admixture: Glenium 7500 (Mid-Range Water Reducer)

Laboratory Test Data

Sample No.	Test Date	Age	Type	Unit Wt.	Area (in^2)	Load (K)	Strength (psi)
C4a	24-Apr-09	7	2	144.5	28.27	92.9	3290
C4b	15-May-09	28	2	145.5	28.27	109.3	3870
C4c	15-May-09	28	2	145.9	28.27	107.1	3790
C4d							

Average 28 Day (psi): 3830



Remarks: _____

Reviewed: Darrell Gilman, CMT Manager
Sent: 5-14-09

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CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No: 14194
Project: Avesta Florence House
Client: Avesta Florence House, LP and Florence House
307 Cumberland Avenue
Portland, ME 04101

Field Test Data

Set No.: C5
Placement Date: 23-Apr-09
Lab Rec'd Date: 24-Apr-09
Location: Line 7 From Line D to A, Line A From 1 to 7 Footing

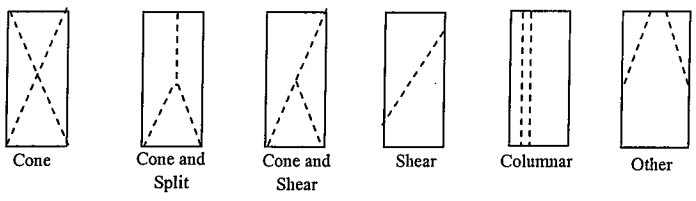
Technician: M. Sullivan
Supplier: Auburn Concrete
Mix Designation: 3/4" Aggregate
Design Strength: 3000psi

Slump (initial)		in.	
Slump (placed)	5	in.	
Air Content	6.2	%	
Conc Temp.	60.0	°F	
Air Temp.	45.0	°F	
Volume (yds)	13.5	of	13.5
Admixture:			

Laboratory Test Data

Sample No.	Test Date	Age	Type	Unit Wt.	Area (in^2)	Load (K)	Strength (psi)
C5a	30-Apr-09	7	2	145.5	28.27	101.1	3570
C5b	21-May-09	28	6	147.5	27.91	126.0	4510
C5c	21-May-09	28	3	147.7	27.97	121.1	4330
C5d							

Average 28 Day (psi): 4420



Remarks:

Reviewed: Darrell Gilman, CMT Manager
Sent:5/21/09

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CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No:	14194
Project:	Avesta Florence House
Client:	Avesta Florence House, LP and Florence House 307 Cumberland Avenue Portland, ME 04101

Field Test Data

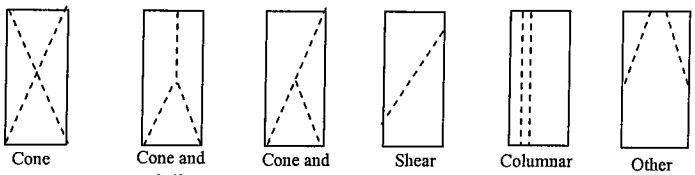
Set No.:	C6
Placement Date:	1-May-09
Lab Rec'd Date:	2-May-09
Location:	Walls on 16-Line from A to F (Lower Half)
Technician:	N. Davis
Supplier:	Auburn Concrete
Mix Designation:	3/4" Aggregate
Design Strength:	3500 psi

Slump (initial)		in.
Slump (placed)	6	in.
Air Content	6.2	%
Conc Temp.	65.0	°F
Air Temp.	71.0	°F
Volume (yds)	30.0	of 60.0
Admixture:	Micro Air, Glenium 7500 (Mid-Range Water Reducer)	

Laboratory Test Data

Sample No.	Test Date	Age	Type	Unit Wt.	Area (in^2)	Load (K)	Strength (psi)
C6a	8-May-09	7	2	143.9	28.27	97.2	3440
C6b	29-May-09	28	6	145.3	28.18	120.9	4290
C6c	29-May-09	28	3	143.3	28.46	117.7	4140
C6d							

Average 28 Day (psi): 4215



Remarks:

Reviewed: Darrell Gilman, CMT Manager
Date: 5-29-09

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CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No: 14194
Project: Avesta Florence House
Client: Avesta Florence House, LP and Florence House
307 Cumberland Avenue
Portland, ME 04101

Field Test Data

Set No.: C7
Placement Date: 1-May-09
Lab Rec'd Date: 2-May-09
Location: Walls on 16-Line from A to F (Upper Half)

Technician: N. Davis
Supplier: Auburn Concrete
Mix Designation: 3/4" Aggregate
Design Strength: 3500 psi

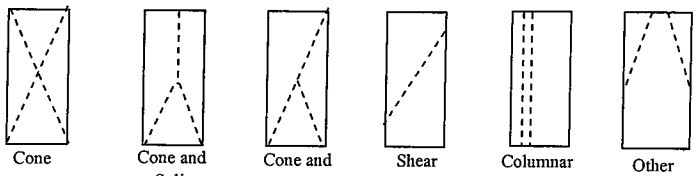
Slump (initial)		in.
Slump (placed)	5 1/2	in.
Air Content	6.3	%
Conc Temp.	64.0	°F
Air Temp.	70.0	°F
Volume (yds)	60.0	of 60.0

Admixture: Micro Air, Glenium 7500 (Mid-Range Water Reducer)

Laboratory Test Data

Sample No.	Test Date	Age	Type	Unit Wt.	Area (in^2)	Load (K)	Strength (psi)
C7a	8-May-09	7	2	143.8	28.27	91.1	3220
C7b	29-May-09	28	6	146.3	28.18	108.6	3850
C7c	29-May-09	28	1	144.5	28.46	111.2	3910
C7d							

Average 28 Day (psi): 3880



Remarks:

Reviewed: Darrell Gilman, CMT Manager
Date: 5-29-09

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CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No: 14194
Project: Avesta Florence House
Client: Avesta Florence House, LP and Florence House
307 Cumberland Avenue
Portland, ME 04101

Field Test Data

Set No.: C8
Placement Date: 4-May-09
Lab Rec'd Date: 5-May-09
Location: Footings on N-Line from 8.2 to Elevator Base Slab

Technician: N. Davis
Supplier: Auburn Concrete
Mix Designation: 3/4" Aggregate
Design Strength: 3000psi

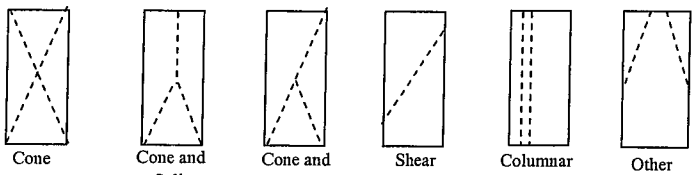
Slump (initial)		in.
Slump (placed)	4 1/2	in.
Air Content	6.2	%
Conc Temp.	66.0	°F
Air Temp.	60.0	°F
Volume (yds)	8.5	of 17.0

Admixture: Micro Air, Glenium 7500 (Mid-Range Water Reducer)

Laboratory Test Data

Sample No.	Test Date	Age	Type	Unit Wt.	Area (in^2)	Load (K)	Strength (psi)
C8a	11-May-09	7	MS	147.1	28.27	103.0	3640
C8b	1-Jun-09	28	AL	147.6	28.27	130.6	4620
C8c	1-Jun-09	28	AL	146.9	28.27	122.2	4320
C8d							

Average 28 Day (psi): 4470



Remarks:

Reviewed: Darrell Gilman, CMT Manager
Date: 6-2-09

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CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No:	14194
Project:	Avesta Florence House
Client:	Avesta Florence House, LP and Florence House 307 Cumberland Avenue Portland, ME 04101

Field Test Data

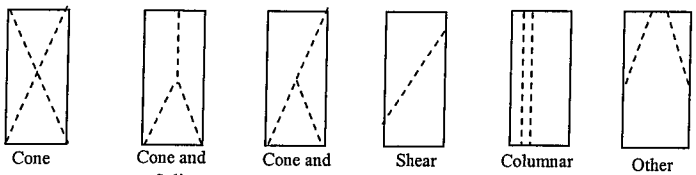
Set No.:	C9
Placement Date:	8-May-09
Lab Rec'd Date:	9-May-09
Location:	Foundation Footings on 13-Line from N-Line to J-Line Including Pads L13 and J13
Technician:	N. Davis
Supplier:	Auburn Concrete
Mix Designation:	3/4" Aggregate
Design Strength:	3000psi

Slump (initial)		in.
Slump (placed)	5 1/4	in.
Air Content	6.0	%
Conc Temp.	64.0	°F
Air Temp.	70.0	°F
Volume (yds)	19.0	of 71.0
Admixture:	Micro Air, Glenium 7500 (Mid-Range Water Reducer)	

Laboratory Test Data

Sample No.	Test Date	Age	Type	Unit Wt.	Area (in^2)	Load (K)	Strength (psi)
C9a	15-May-09	7	2	146.2	28.27	101.1	3570
C9b	5-Jun-09	28	3	146.0	28.27	114.1	4040
C9c	5-Jun-09	28	3	146.9	28.27	114.5	4050
C9d							

Average 28 Day (psi): 4045



Remarks: _____

Reviewed: Darrell Gilman, CMT Manager
Date: 6-5-09

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CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No:	14194
Project:	Avesta Florence House
Client:	Avesta Florence House, LP and Florence House 307 Cumberland Avenue Portland, ME 04101

Field Test Data

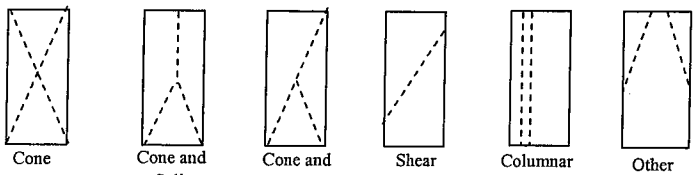
Set No.:	C10
Placement Date:	8-May-09
Lab Rec'd Date:	9-May-09
Location:	Foundation Walls; Elevator Shaft Walls on 5 and 5.8 Lines from N to M and M-Line from 5 to 5.8
Technician:	N. Davis
Supplier:	Auburn Concrete
Mix Designation:	3/4" Aggregate
Design Strength:	3500psi

Slump (initial)		in.
Slump (placed)	5 3/4	in.
Air Content	6.1	%
Conc Temp.	65.0	°F
Air Temp.	70.0	°F
Volume (yds)		of
Admixture:	Micro Air, Glenium 7500 (Mid-Range Water Reducer)	

Laboratory Test Data

Sample No.	Test Date	Age	Type	Unit Wt.	Area (in^2)	Load (K)	Strength (psi)
C10a	15-May-09	7	3	143.1	28.27	93.2	3300
C10b	5-Jun-09	28	2	143.6	28.27	109.1	3860
C10c	5-Jun-09	28	2	143.7	28.27	110.0	3890
C10d							

Average 28 Day (psi): 3875



Remarks:

Reviewed: Darrell Gilman, CMT Manager
Sent:6-5-09

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CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No: 14194
Project: Avesta Florence House
Client: Avesta Florence House, LP and Florence House
307 Cumberland Avenue
Portland, ME 04101

Field Test Data

Set No.: C11
Placement Date: 15-May-09
Lab Rec'd Date: 18-May-09
Location: Foundation Wall A1 - D1

Technician: J. Rouillard
Supplier: Auburn Concrete
Mix Designation: 3/4" Aggregate
Design Strength: 3500psi

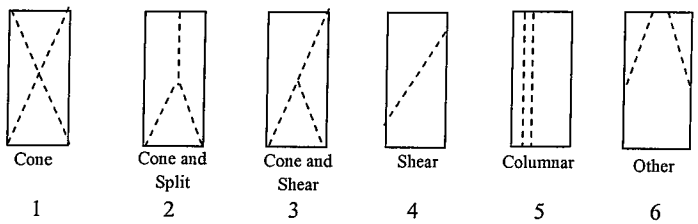
Slump (initial)		in.
Slump (placed)	3 1/2	in.
Air Content	7.0	%
Conc Temp.	72.5	°F
Air Temp.	68.0	°F
Volume (yds)	7.0	of 7.0

Admixture: Glenium 7500 (Mid-Range Water Reducer), Micro Air

Laboratory Test Data

Sample No.	Test Date	Age	Type	Unit Wt.	Area (in^2)	Load (K)	Strength (psi)
C11a	22-May-09	7	6	143.7	28.27	75.5	2670
C11b	12-Jun-09	28	6	144.2	28.27	127.8	4520
C11c	12-Jun-09	28	6	147.9	28.27	118.0	4170
C11d							

Average 28 Day (psi): 4345



Remarks:

Reviewed: Darrell Gilman, CMT Manager
Date: 6-15-09

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CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No: 14194
Project: Avesta Florence House
Client: Avesta Florence House, LP and Florence House
307 Cumberland Avenue
Portland, ME 04101

Field Test Data

Set No.: C12
Placement Date: 20-May-09
Lab Rec'd Date: 21-May-09
Location: Along M to 1.75 Footing

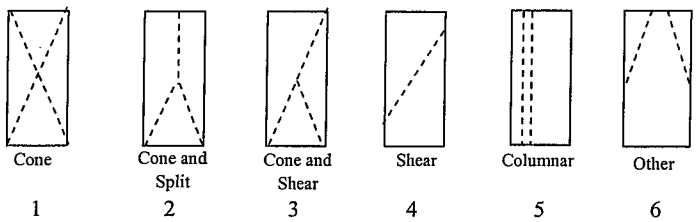
Technician: A. Lyons
Supplier: Auburn Concrete
Mix Designation: 3/4" Aggregate
Design Strength: 3500psi

Slump (initial)		in.
Slump (placed)	6 1/4	in.
Air Content	7.0	%
Conc Temp.		°F
Air Temp.	70.0	°F
Volume (yds)	11.0	of 54.0
Admixture:	Mid-Range Water Reducer	

Laboratory Test Data

Sample No.	Test Date	Age	Type	Unit Wt.	Area (in^2)	Load (K)	Strength (psi)
C12a	27-May-09	7	2	145.5	28.27	111.6	3950
C12b	17-Jun-09	28	3	144.3	28.27	122.5	4330
C12c	17-Jun-09	28	3	144.2	28.27	120.5	4260
C12d							

Average 28 Day (psi): 4295



Remarks: _____

Reviewed: Darrell Gilman, CMT Manager
Date: 5-28-09

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CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No:	14194
Project:	Avesta Florence House
Client:	Avesta Florence House, LP and Florence House
	307 Cumberland Avenue
	Portland, ME 04101

Field Test Data

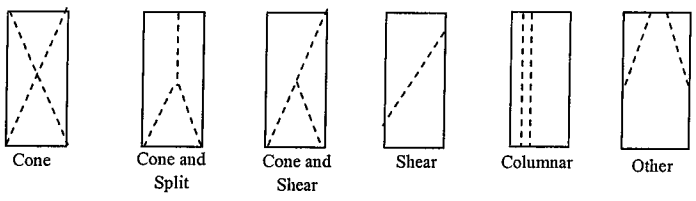
Set No.:	C13
Placement Date:	20-May-09
Lab Rec'd Date:	21-May-09
Location:	Along N Line Approximately at 4 Footing
Technician:	A. Lyons
Supplier:	Auburn Concrete
Mix Designation:	3/4" Aggregate
Design Strength:	3500psi

Slump (initial)		in.
Slump (placed)	6 3/4	in.
Air Content	7.2	%
Conc Temp.		°F
Air Temp.	70.0	°F
Volume (yds)	33.0	of 54.0
Admixture:	Mid-Range Water Reducer	

Laboratory Test Data

Sample No.	Test Date	Age	Type	Unit Wt.	Area (in^2)	Load (K)	Strength (psi)
C13a	27-May-09	7	4	141.4	28.27	86.7	3070
C13b	17-Jun-09	28	6	141.8	28.27	108.1	3820
C13c	17-Jun-09	28	3	142.2	28.27	107.8	3810
C13d							

Average 28 Day (psi): 3815



Remarks:

Reviewed: Darrell Gilman, CMT Manager
Date: 6-17-09

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CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No: 14194
Project: Avesta Florence House
Client: Avesta Florence House, LP and Florence House
307 Cumberland Avenue
Portland, ME 04101

Field Test Data

Set No.: C14
Placement Date: 22-May-09
Lab Rec'd Date: 23-May-09
Location: Along Line 1 from H - D.2 Wall Footing

Technician: J. Rouillard
Supplier: Auburn Concrete
Mix Designation: 3/4" Aggregate
Design Strength: 3500psi

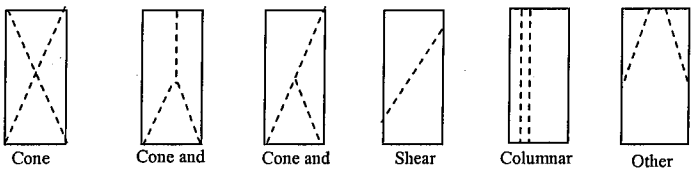
Slump (initial)		in.
Slump (placed)	3 1/2	in.
Air Content	6.0	%
Conc Temp.	73.0	°F
Air Temp.	80.0	°F
Volume (yds)	5.0	of 5.0

Admixture: Glenium 7500 (Mid-Range Water Reducer)

Laboratory Test Data

Sample No.	Test Date	Age	Type	Unit Wt.	Area (in^2)	Load (K)	Strength (psi)
C14a	29-May-09	7	4	147.7	28.27	91.2	3230
C14b	19-Jun-09	28	4	146.9	28.27	117.4	4150
C14c	19-Jun-09	28	4	146.6	28.27	119.4	4220
C14d							

Average 28 Day (psi): 4185



Remarks:

Reviewed: Darrell Gilman, CMT Manager
Date: 6-19-09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: (207) 626-9094

CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No: 14194
Project: Avesta Florence House
Client: Avesta Florence House, LP and Florence House
307 Cumberland Avenue
Portland, ME 04101

Field Test Data

Set No.: C15
Placement Date: 27-May-09
Lab Rec'd Date: 28-Feb-09
Location: Stair 1 Footings

Technician: A. Lyons
Supplier: Auburn Concrete
Mix Designation: 3/4" Aggregate
Design Strength: 3000psi

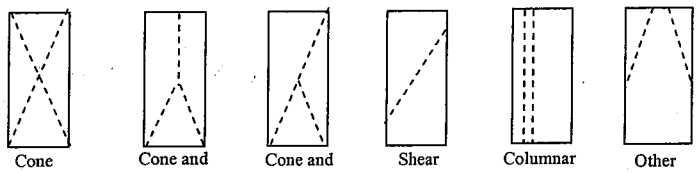
Slump (initial)		in.
Slump (placed)	6	in.
Air Content	7.6	%
Conc Temp.		°F
Air Temp.	53.0	°F
Volume (yds)	10.0	of 91.0

Admixture: Glenium 7500 (Mid-Range Water Reducer), Micro Air

Laboratory Test Data

Sample No.	Test Date	Age	Type	Unit Wt.	Area (in^2)	Load (K)	Strength (psi)
C15a	3-Jun-09	7	3	143.1	28.27	100.4	3550
C15b	24-Jun-09	28	3	138.3	28.27	126.7	4480
C15c	24-Jun-09	28	3	143.7	28.27	124.8	4420
C15d							

Average 28 Day (psi): 4450



Remarks: _____

Reviewed: Darrell Gilman, CMT Manager
Date: 6-24-09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: (207) 626-9094

CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No:	14194
Project:	Avesta Florence House
Client:	Avesta Florence House, LP and Florence House 307 Cumberland Avenue Portland, ME 04101

Field Test Data

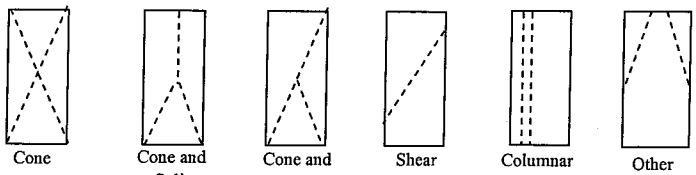
Set No.:	C16
Placement Date:	27-May-09
Lab Rec'd Date:	28-May-09
Location:	Large Interior Footing BJ5
Technician:	A. Lyons
Supplier:	Auburn Concrete
Mix Designation:	3/4" Aggregate
Design Strength:	3000psi

Slump (initial)		in.
Slump (placed)	6	in.
Air Content	7.2	%
Conc Temp.		°F
Air Temp.	53.0	°F
Volume (yds)	50.0	of 91.0
Admixture:	Glenium 7500 (Mid-Range Water Reducer), Micro Air	

Laboratory Test Data

Sample No.	Test Date	Age	Type	Unit Wt.	Area (in^2)	Load (K)	Strength (psi)
C16a	3-Jun-09	7	6	139.3	28.27	71.0	2510
C16b	24-Jun-09	28	4	139.2	28.27	96.6	3420
C16c	24-Jun-09	28	4	138.8	28.27	93.3	3300
C16d							

Average 28 Day (psi): 3360



Remarks:

Reviewed: Darrell Gilman, CMT Manager
Date: 6-24-09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: (207) 626-9094

CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No: 14194
Project: Avesta Florence House
Client: Avesta Florence House, LP and Florence House
307 Cumberland Avenue
Portland, ME 04101

Field Test Data

Set No.: C17
Placement Date: 27-May-09
Lab Rec'd Date: 28-May-09
Location: Foundation Walls Line 1 "D" to "H"

Technician: A. Lyons
Supplier: Auburn Concrete
Mix Designation: 3/4" Aggregate
Design Strength: 3500psi

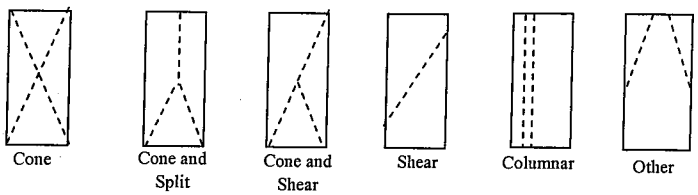
Slump (initial)		in.
Slump (placed)	6 3/4	in.
Air Content	7.0	%
Conc Temp.		°F
Air Temp.	53.0	°F
Volume (yds)	91.0	of 91.0

Admixture: Glenium 7500 (Mid-Range Water Reducer), Micro Air

Laboratory Test Data

Sample No.	Test Date	Age	Type	Unit Wt.	Area (in^2)	Load (K)	Strength (psi)
C17a	3-Jun-09	7	4	143.8	28.27	106.3	3760
C17b	24-Jun-09	28	4	144.4	28.27	142.5	5040
C17c	24-Jun-09	28	4	144.4	28.27	139.9	4950
C17d							

Average 28 Day (psi): 4995



Remarks:

Reviewed: Darrell Gilman, CMT Manager
Date: 6-24-09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: (207) 626-9094

CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No: 14194
Project: Avesta Florence House
Client: Avesta Florence House, LP and Florence House
307 Cumberland Avenue
Portland, ME 04101

Field Test Data

Set No.: C18
Placement Date: 18-Jun-09
Lab Rec'd Date: 19-Jun-09
Location: Retaining Wall

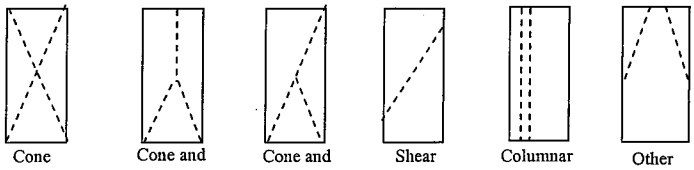
Technician: D. Gilman
Supplier: Auburn Concrete
Mix Designation: 3/4 Aggregate
Design Strength: 3500psi

Slump (initial)		in.	
Slump (placed)	4 3/4	in.	
Air Content	5.2	%	
Conc Temp.	68.0	°F	
Air Temp.	85.0	°F	
Volume (yds)	8.5	of	18.0
Admixture:			

Laboratory Test Data

Sample No.	Test Date	Age	Type	Unit Wt.	Area (in^2)	Load (K)	Strength (psi)
C18a	25-Jun-09	7	4	147.4	28.27	101.7	3600
C18b	16-Jul-09	28	6	146.9	28.27	116.2	4110
C18c	16-Jul-09	28	6	146.9	28.27	111.4	3940
C18d							

Average 28 Day (psi): 4025



Remarks: _____

Reviewed: Darrell Gilman, CMT Manager
Date: 7-16-09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: (207) 626-9094

CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No: 14194
Project: Avesta Florence House
Client: Avesta Florence House, LP and Florence House
307 Cumberland Avenue
Portland, ME 04101

Field Test Data

Set No.: C19
Placement Date: 6-Jul-09
Lab Rec'd Date: 7-Jul-09
Location: First Floor Slab

Technician: K. Bennett
Supplier: Auburn Concrete
Mix Designation: 3/4" Aggregate
Design Strength: 3000psi

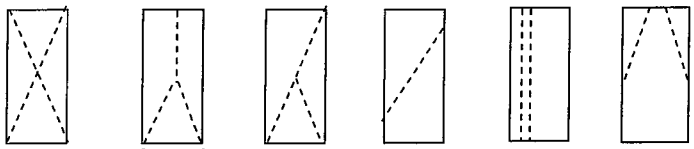
Slump (initial)		in.
Slump (placed)	7 1/2	in.
Air Content	2.5	%
Conc Temp.	71.0	°F
Air Temp.		°F
Volume (yds)	25.0	of 80.0

Admixture: Glenium 7500 (Mid-Range Water Reducer)

Laboratory Test Data

Sample No.	Test Date	Age	Type	Unit Wt.	Area (in^2)	Load (K)	Strength (psi)
C19a	13-Jul-09	7	6	147.7	28.27	68.7	2430
C19b	3-Aug-09	28	6	148.3	28.27	90.9	3210
C19c	3-Aug-09	28	6	148.0	28.27	89.2	3150
C19d							

Average 28 Day (psi): 3180



Remarks: _____

Reviewed: Darrell Gilman, CMT Manager
Date: 8-04-09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: (207) 626-9094

CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No: 14194
Project: Avesta Florence House
Client: Avesta Florence House, LP and Florence House
307 Cumberland Avenue
Portland, ME 04101

Field Test Data

Set No.: C20
Placement Date: 6-Jul-09
Lab Rec'd Date: 7-Jul-09
Location: Second Floor Slab

Technician: K. Bennett
Supplier: Auburn Concrete
Mix Designation: 3/4" Aggregate
Design Strength: 3000psi

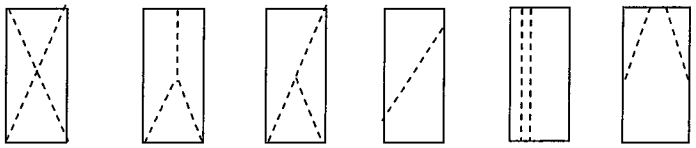
Slump (initial)		in.
Slump (placed)	7	in.
Air Content	2.4	%
Conc Temp.	73.0	°F
Air Temp.	75.0	°F
Volume (yds)	65.0	of 80.0

Admixture: Glenium 7500 (Mid-Range Water Reducer)

Laboratory Test Data

Sample No.	Test Date	Age	Type	Unit Wt.	Area (in^2)	Load (K)	Strength (psi)
C20a	13-Jul-09	7	6	147.5	28.27	70.8	2510
C20b	3-Aug-09	28	6	149.3	28.27	90.0	3180
C20c	3-Aug-09	28	6	150.0	28.27	88.0	3110
C20d							

Average 28 Day (psi): 3145



Remarks:

Reviewed: Darrell Gilman, CMT Manager
Date: 8-04-09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: (207) 626-9094

CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No: 14194
Project: Avesta Florence House
Client: Avesta Florence House, LP and Florence House
307 Cumberland Avenue
Portland, ME 04101

Field Test Data

Set No.: C23
Placement Date: 6-Aug-09
Lab Rec'd Date: 7-Aug-09
Location: First Floor Slab

Technician: J. Rouillard
Supplier: Auburn Concrete
Mix Designation: 3/4" Aggregate
Design Strength: 3000psi

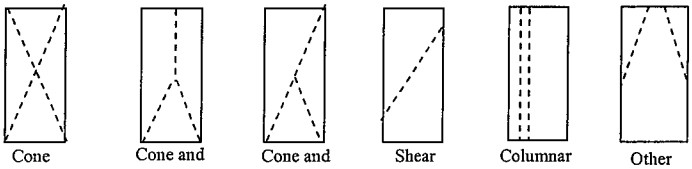
Slump (initial)		in.
Slump (placed)	6 1/2	in.
Air Content	2.5	%
Conc Temp.		°F
Air Temp.	80.0	°F
Volume (yds)	10.0	of 126.0

Admixture: Glenium 7500 (Mid Range Water Reducer

Laboratory Test Data

Sample No.	Test Date	Age	Type	Unit Wt.	Area (in^2)	Load (K)	Strength (psi)
C23a	13-Aug-09	7	6	148.3	28.27	93.3	3300
C23b	3-Sep-09	28	6	149.2	28.27	104.9	3710
C23c	3-Sep-09	28	6	149.0	28.27	109.9	3890
C23d							

Average 28 Day (psi): 3800



Remarks: _____

Reviewed: Darrell Gilman, CMT Manager
Date: 9-7-09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: (207) 626-9094

CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No: 14194
Project: Avesta Florence House
Client: Avesta Florence House, LP and Florence House
307 Cumberland Avenue
Portland, ME 04101

Field Test Data

Set No.: C24
Placement Date: 6-Aug-09
Lab Rec'd Date: 7-Aug-09
Location: First Floor Slab

Technician: J. Rouillard
Supplier: Auburn Concrete
Mix Designation: 3/4" Aggregate
Design Strength: 3000psi

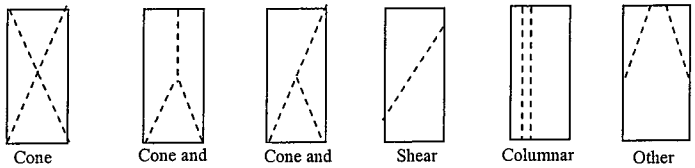
Slump (initial)		in.
Slump (placed)	6	in.
Air Content	3.0	%
Conc Temp.		°F
Air Temp.	80.0	°F
Volume (yds)	70.0	of 126.0

Admixture: Glemium 7500 (Mid-Range Water Reducer)

Laboratory Test Data

Sample No.	Test Date	Age	Type	Unit Wt.	Area (in^2)	Load (K)	Strength (psi)
C24a	13-Aug-09	7	6	149.5	28.27	88.9	3150
C24b	3-Sep-09	28	1	149.2	28.27	108.6	3840
C24c	3-Sep-09	28	3	149.0	28.27	106.6	3770
C24d							

Average 28 Day (psi): 3805



Remarks: _____

Reviewed: Darrell Gilman, CMT Manager
Date: 9-8-09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: (207) 626-9094

CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No: 14194
Project: Avesta Florence House
Client: Avesta Florence House, LP and Florence House
307 Cumberland Avenue
Portland, ME 04101

Field Test Data

Set No.: C25
Placement Date: 6-Aug-09
Lab Rec'd Date: 7-Aug-09
Location: First Floor Slab

Technician: J. Rouillard
Supplier: Auburn Concrete
Mix Designation: 3/4 Aggregate
Design Strength: 3000psi

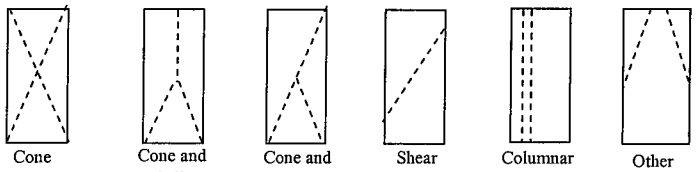
Slump (initial)		in.
Slump (placed)	5 1/2	in.
Air Content	4.0	%
Conc Temp.		°F
Air Temp.	80.0	°F
Volume (yds)	120.0	of 126.0

Admixture: Glenium 7500 (Mid-Range Water Reducer)

Laboratory Test Data

Sample No.	Test Date	Age	Type	Unit Wt.	Area (in^2)	Load (K)	Strength (psi)
C25a	13-Aug-09	7	6	148.7	28.27	88.9	3140
C25b	3-Sep-09	28	6	146.4	28.27	101.5	3590
C25c	3-Sep-09	28	3	147.8	28.27	100.3	3550
C25d							

Average 28 Day (psi): 3570



Remarks: _____

Reviewed: Darrell Gilman, CMT Manager
Date: 9-8-09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: (207) 626-9094

CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No: 14194
Project: Avesta Florence House
Client: Avesta Florence House, LP and Florence House
307 Cumberland Avenue
Portland, ME 04101

Field Test Data

Set No.: C26
Placement Date: 28-Aug-09
Lab Rec'd Date: 31-Aug-09
Location: Stairwell Slab 1 to 3 Basement Level

Technician: C. Young
Supplier: Auburn Concrete
Mix Designation: 3/4" Aggregate
Design Strength: 3000psi

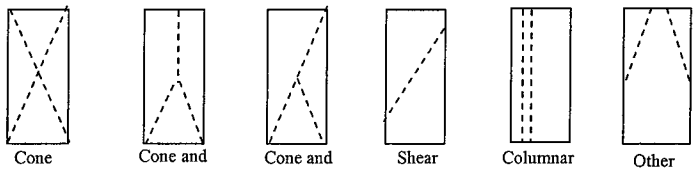
Slump (initial)		in.
Slump (placed)	5	in.
Air Content	3.0	%
Conc Temp.	72.0	°F
Air Temp.	54.0	°F
Volume (yds)	7.0	of 15.0

Admixture: Glenium 7500 (Mid-Range Water Reducer), 1%-Pozzutec 20%

Laboratory Test Data

Sample No.	Test Date	Age	Type	Unit Wt.	Area (in^2)	Load (K)	Strength (psi)
C26a	4-Sep-09	7	2	145.4	28.27	76.0	2690
C26b	25-Sep-09	28	4	146.5	28.27	95.5	3380
C26c	25-Sep-09	28	4	146.4	28.27	109.8	3880
C26d							

Average 28 Day (psi): 3630



Remarks: _____

Reviewed: Darrell Gilman, CMT Manager
Date: 9-28-09

EXHIBIT B

04200 CMU Construction

Project: Florence House
Date Prepared: September 2, 2008

Structural Schedule of Special Inspections
MASONRY CONSTRUCTION – LEVEL 1 (NON-ESSENTIAL FACILITY)

VERIFICATION AND INSPECTION IBC Section 1704.5	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS	AGENT	AGENT QUALIFICATION	TASK COMPLETED
1. As masonry construction begins, the following shall be verified to ensure compliance:						
a. Proportions of site-prepared mortar.	Y	P	ACI530.1, 2.6A	SI1	PE/SE or EIT	TESTING AGENCY
b. Construction of mortar joints.	Y	P	ACI530.1, 3.3B	SI1	PE/SE or EIT	JLF
c. Location of reinforcement and connectors.	Y	P	ACI530.1, 3.4, 3.6A	SI1	PE/SE or EIT	JLF
d. Prestressing technique.	N/A	P	ACI530.1, 3.6B		PE/SE or EIT	
e. Grade and size of prestressing tendons and anchorages.	N/A	P	ACI530.1, 2.4B, 2.4H		PE/SE or EIT	
2. The inspection program shall verify:						
a. Size and location of structural elements.	Y	P	ACI530.1, 3.3G	SI1	PE/SE or EIT	JLF
b. Type, size and location of anchors, including other details of anchorage of masonry to structural members, frames or other construction.	Y	P	ACI530, 1.2.2(e), 2.1.4, 3.1.6	SI1	PE/SE or EIT	JLF
c. Specified size, grade and type of reinforcement.	Y	P	ACI530, 1.12, ACI530.1, 2.4, 3.4	SI1	PE/SE or EIT	JLF
d. Welding of reinforcing bars.	Y	C	AC530, 2.1.10.6.2, 3.2..4 (b)	TA2	AWS-CWI	—
e. Protection of masonry during cold weather (temperature below 40°F) or hot weather (temperature above 90°F).	Y	P	IBC 2104.3, 2104.4; ACI530.1, 1.8C, 1.8D	SI1	PE/SE or EIT	✓
f. Application and measurement of prestressing force.	N/A	P	ACI530.1, 3.6B		PE/SE or EIT	
3. Prior to grouting, the following shall be verified to ensure compliance:						
a. Grout space is clean.	Y	P	ACI530.1, 3.2D	SI1/TM	PE/SE or EIT	JLF
b. Placement of reinforcement and connectors and prestressing tendons and anchorages.	Y	P	ACI530, 1.12, ACI530.1, 3.4	SI1	PE/SE or EIT	JLF
c. Proportions of site-prepared grout and prestressing grout for bonded tendons.	Y	P	ACI530.1, 2.6B	SI1	PE/SE or EIT	JLF
d. Construction of mortar joints.	Y	P	ACI530.1, 3.3B	SI1	PE/SE or EIT	JLF
4. Grout placement shall be verified to ensure compliance with code and construction document provisions.	Y	C	ACI530.1, 3.5	SI1	PE/SE or EIT	JLF
a. Grouting of prestressing bonded tendons.	N/A	C	ACI530.1, 3.6C		PE/SE or EIT	
5. Preparation of any required grout specimens, mortar specimens and/or prisms shall be observed.	Y	C	IBC 2105.2.2, 2105.3; ACI530.1, 1.4	SI1	PE/SE or EIT	TESTING AGENCY
6. Compliance with required inspection provisions of the construction documents and the approved submittals shall be verified.	Y	P	ACI530.1, 1.5	SI1	PE/SE or EIT	✓

BECKER
structural engineers, inc.

04230

OBSERVATION REPORT
CMU

Date:	June 30, 2009
Time:	2:30pm - 3:30 pm
Temp:	60's
Weather:	Cloudy

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI 2058

Observation Location: CMU walls at Elevator and Stairs adjacent to Line N

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Reinforcement Size	☒	☐	☐	☐	
Quantity	☒	☒	☐	☐	See note below
Condition	☒	☐	☐	☐	
Placement	☒	☐	☐	☐	Rebar positioners shall be used to ensure proper bar location,
Embed/Anchors	☐	☐	☐	☒	
Lap Splices	☐	☐	☐	☒	
Hot Weather	☐	☐	☐	☒	
Cold Weather	☐	☐	☐	☒	
CMU Size	☒	☐	☐	☐	
Layout/Fit-up/Plumbness	☒	☐	☐	☐	
Mortar/Grouting Procedure	☒	☐	☐	☐	At cells to receive grout, keep cavities free of mortar fins and droppings.
Lift Height	☐	☐	☐	☒	
Clean Outs	☐	☐	☐	☒	
Bond Beams	☐	☐	☐	☒	
Additional Items	☐	☐	☐	☒	

Notes:

This visit was conducted to review the start of CMU construction at the areas noted above. Work appears to be progressing satisfactorily, and was found to be in general conformance with the contract documents. One item was noted that will require attention by the General Contractor and masonry sub-contractor.

1. Vertical bars are required at the ends of CMU walls terminating against the concrete foundation wall, as shown in CSKS-11 and Dwg. S2.1.

Thank you -

Signed: James Fortin, P.E.

75 York Street, Portland, ME 04101-4701 ■ Tel 207-879-1838 ■ Fax 207-879-1822

Gawron Turgeon
ARCHITECTS

Transmittal

To: Jim Fortin
Becker Structural Engineers, Inc
75 York Street
Portland, Me 04101
From: Wendi Holden
cc: file
Date: 7-14-09
Re: Florence House Submittals

Gawron Turgeon Architects is sending you:

- ☐ Prints
- ☐ Layouts
- ☐ Estimates
- ☐ Copy of Letter
- ☐ Specifications
- ☐ Plans
- ☐ Photography
- ☐ Samples
- ☒ Shop Drawings
- ☐ Presentation
- ☐ Change Order
- ☐ Other

Copies	Date	Description
8		Masonry reinforcing certification of Compliance

- ☐ Returned for corrections
- ☐ Approved & noted
- ☐ Return _____ corrected prints
- ☐ Submit _____ copies for _____
- ☐ Resubmit _____ copies for _____
- ☐ Returned after loaned to us
- ☐ Construction approval
- ☒ For review & comment
- ☐ For your use
- ☐ As requested
- ☐ For approval
- ☐ For bids due

Remarks:

BECKER STRUCTURAL ENGINEERS, INC.
JUL 15

architecture
interior design
master planning
landscape architecture

29 Black Point Road • Scarborough, Maine 04074 • 207-883-6307 *tel* • 207-883-0361 *fax* • www.gawronturgeon.com

Ganneston Construction Corp
PO Box 27, North Belfast Avenue
Augusta, ME 04332-0027

Tel: (207) 621-8505
Fax: (207) 621-8508

LETTER OF TRANSMITTAL

GCC PROJECT NUMBER
GCC-80677

To:
GAWRON TURGEON ARCHITECTS
29 BLACK POINT ROAD
SCARBOROUGH, MAINE 04074

Date: July 13, 2009
Attention: WENDI
RE: FLORENCE HOUSE PROJECT

WE ARE SENDING YOU X ATTACHED _____ UNDER SEPARATE COVER

COPIES	NO.	DESCRIPTION
8		MASONRY REINVORCING CERTIFICATE OF COMPLIANCE
		FOR APPROVAL
		PLEASE RETURN 3 SETS TO GCC

RECEIVED JUL 14 2009

REMARKS

Signed: *Michael L. Adams* Date: 13-Jul-09
Sr. Project Manager

BARKER STEEL LLC
a Harris company
55 Sumner Street - Milford, MA 01757
TEL 508 473-8484 FAX 508 473-8512

CERTIFICATE OF COMPLIANCE

Barker Contract#: **10031509**
Project: Florence House Masonry - Portland

Sold To: AUDETTE & SCE INC
216 LEWISTON RD
GRAY, ME 04039

Ship To: 190 Valley St
Portland, ME 04102

Shipped From: Deerfield BOL:176398-DEE
Shipped Date: 06/29/2009 Ship Via: Barker Trucking

Order	Description	Weight
00001F	CMU Shafts	22715 Lbs.

Size	Weight	Grade	Supplier	Heat#	Yield	Tensile	Elng%	Bend	C	Mn	P	S	Coater	Batch	Cntrl#
5	265	60	NucorSteel	AU0910107801	68100	106400	13.7	OK	.42	1.10	.018	.062			D9392
5	265	60	NucorSteel	AU0910107901	73700	114300	14.5	OK	.41	1.20	.043	.064			D9392
5	265	60	NucorSteel	AU0910108001	66500	104700	13.2	OK	.42	1.15	.031	.064			D9392
5	265	60	NucorSteel	AU0910108301	66900	104000	16.6	OK	.41	1.13	.021	.049			D9392
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6	7041	60	NucorSteel	E4180	66800	104700	13.7	OK	.43	1.13	.029	.054			D9219
6	7041	60	NucorSteel	E4183	67400	106800	13.2	OK	.43	1.10	.030	.054			D9219

BARKER STEEL certifies that the above material conforms to all current job plans and specifications. We certify that all manufacturing processes for this steel have occurred in the United States.

BY: Mark E. Webb

GANNESTON CONSTRUCTION CORP.
P.O. BOX 27 AUGUSTA, ME 04332

X REVIEWED	REVIEWED WITH NOTES RESUBMISSION REQUESTED
REVIEWED WITH NOTES NO RESUBMISSION	NOT ACCEPTABLE

CHECK MARK INDICATES ACTION TAKEN
CHECKED BY [signature] DATE 7/13/09

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a Harris company
55 Sumner Street - Milford, MA 01757
TEL 508 473-8484 FAX 508 473-8512

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BY Mark E Wall

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BY: Mark E Gobeil

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a Harris company
55 Sumner Street - Milford, MA 01757
TEL 508 473-8484 FAX 508 473-8512

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BY [signature]

Audette &
S.C.E., INC.
Since 1973
216 Lewiston Road
Gray, Maine 04039

(207) 657-3313 Fax (207) 657-4113

LETTER OF TRANSMITTAL

DATE	JOB NO.
July 8 2009	80627

ATTENTION Mike Adams

RE: Florence House

Masonry Submittal Spec Section 04810

TO Ganneston Construction Corp.
PO Box 27 - North Belfast Avenue
Augusta, ME 04332-0027

RECEIVED
JUL 9 2009
GANNESTON CONSTRUCTION CORP.

WE ARE SENDING YOU ☒ Attached ☐ Under separate cover via _____ the following items:

☐ Shop drawings ☐ Prints ☐ Plans ☐ Samples ☐ Specifications
☐ Copy of letter ☐ Change order ☒ certificate of Compliance

COPIES	DATE	NO.	DESCRIPTION
8	7-8-09	1	Reinforcing Bars - Certificate of Compliance - Spec Section 04810 - 1.04 - F.1 4.

THESE ARE TRANSMITTED as checked below:

☒ For approval ☐ Approved as submitted ☐ Resubmit _____ copies for approval
☒ For your use ☐ Approved as noted ☐ Submit _____ copies for distribution
☐ As requested ☐ Returned for corrections ☐ Return _____ corrected prints
☐ For review and comment ☐ _____
☐ FOR BIDS DUE _____ ☐ PRINTS RETURNED AFTER LOAN TO US

REMARKS _____

COPY TO _____

SIGNED: Linda Traino

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: 626-9094

DAILY FIELD REPORT

Date: 7/2/09

Project: Avesta Florence House - Portland, Maine

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Grout Testing

Work Activities: A crew from Audette & S C E Inc. placed grout for the first five feet of stairwell one and two. One set (#1) of four grout cubes were cast and will be retrieved at a later date for controlled storage and compressive strength testing.

Test Results:

Grout Test Results:
Grout Temperature: 65 deg. F
Air Temperature: 60 deg F
Slump: 10"

Remarks:

Portal to Portal		**Expenses**		**Signed:**	Kris Bennett
Leave:	9:45 AM	Mileage:	15	**cc:**	
Return:	10:45 AM	Density Gauge:			
TOTAL:	1	Other:			
				Reviewed:	Darrell A. Gilman, CMT Manager
				Sent:	7/9/09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: 626-9094

DAILY FIELD REPORT

Date: 7/20/09

Project: Avesta Florence House - Portland, Maine

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Mortar & Grout testing

Work Activities: When I arrived I asked the mason if they were mixing mortar and he replied they were done with mortar and were now mixing grout. I talked with Mike Adams about what he would like me to do and he asked if I would put together a grout box and take a sample of the grout. I cast one set of 4 grout cubes to be picked up later for compressive strength testing.

Test Results:

Remarks:

Portal to Portal		**Expenses**		**Signed:**	Justin Rouillard
Leave:	10:00 AM	Mileage:	11	**cc:**	
Return:	11:00 AM	Density Gauge:			
TOTAL:	1	Other:			
				Reviewed:	Darrell A. Gilman, CMT Manager
				Sent:	7/21/09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: 626-9094

DAILY FIELD REPORT

Date: 7/21/09

Project: Avesta Florence House - Portland, Maine

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Mortar Testing

Work Activities: Retrieved one grout box and brought it back for laboratory testing. Also cast one set of 3 mortar cubes for compressive strength testing. Sample was taken directly out of the mixer and at the time the sample was taken the mason's were working on the interior stairwell.

Test Results:

Remarks:

Portal to Portal		**Expenses**		**Signed:**	Justin Rouillard
Leave:	10:00 AM	Mileage:	11	**cc:**	
Return:	11:30 AM	Density Gauge:			
TOTAL:	1.5	Other:			
				Reviewed:	Darrell A. Gilman, CMT Manager
				Sent:	7/24/09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: 626-9094

DAILY FIELD REPORT

Date: 8/20/09

Project: Avesta Florence House - Portland, Maine

Project #: 14194

Site Contacts: Mike Adams, Ganneston Construction

Purpose of Visit: Grout testing

Work Activities: When I arrived the contractor was mixing up a batch of grout, when the batch was thoroughly mixed I cast one set of 4 grout cubes to be picked up later for compressive strength testing.

Test Results:

Remarks: Grout was being placed at the 3rd level of the stairwell.

Portal to Portal		**Expenses**		**Signed:**	Frank Clark
Leave:	9:00am	Mileage:	11	**cc:**	
Return:	10:15am	Density Gauge:			
TOTAL:	1.25	Other:	2.00 tolls		

Reviewed: Darrell A. Gilman, CMT Manager
Sent: 8/21/09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621.8334 Fax: (207) 626.9094

GROUT COMPRESSIVE STRENGTH TEST RESULTS - ASTM C1019

Project No: 14194
Project: Avesta Florence House
Client: Avesta Florence House, LP and Florence House
307 Cumberland Avenue
Portland, ME 04101

Field Test Data

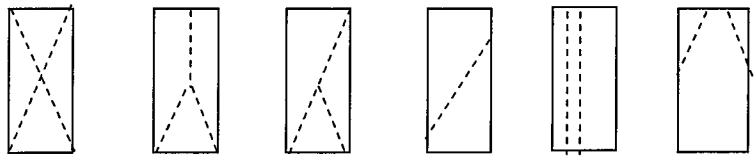
Set No.: G1
Placement Date: 2-Jul-09
Lab Rec'd Date: 3-Jul-09
Location: Stair Well 1 & 2, 5'
Technician: K. Bennett
Supplier: Audette & SCE
Mix Designation: CMU Block Grout
Design Strength: 3000psi

Slump (inches) 10
Grout Temp. (^{0}F) 65.0
Air Temp. (^{0}F) 62.0

Laboratory Test Data

Sample No.	Test Date	Age	Type	Wt. (lb.)	Unit Wt.	Area (in^2)	Load (K)	Strength (psi)
C1a	9-Jul-09	7	3	4.78	120.6	11.17	31.3	2800
C1b	30-Jul-09	28	3	4.88	119.1	11.29	44.1	3910
C1c	30-Jul-09	28	3	4.82	125.4	11.22	46.5	4150
C1d								

Average 28 Day (psi): _____



Remarks: _____

Reviewed: Darrell Gilman, CMT Manager
Date: 7-31-09

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Phone: (207) 621.8334 Fax: (207) 626.9094

GROUT COMPRESSIVE STRENGTH TEST RESULTS - ASTM C1019

Project No: 14194
Project: Avesta Florence House
Client: Avesta Florence House, LP and Florence House
307 Cumberland Avenue
Portland, ME 04101

Field Test Data

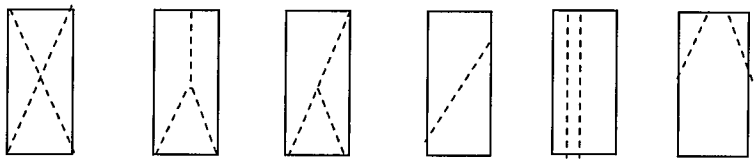
Set No.: G2
Placement Date: 20-Jul-09
Lab Rec'd Date: 22-Jul-09
Location: Block Wall at the Intersection of Line M and Line 16
Technician: J. Rouillard
Supplier: Audette & SCE
Mix Designation: CMU Grout
Design Strength: 3000psi

Slump (inches) 11
Grout Temp. (^{0}F) 62.0
Air Temp. (^{0}F) 75.0

Laboratory Test Data

Sample No.	Test Date	Age	Type	Wt. (lb.)	Unit Wt.	Area (in^2)	Load (K)	Strength (psi)
G2a	27-Jul-09	7	3	4.86	123.0	11.27	19.7	1740
G2b	17-Aug-09	28	6	4.62	124.6	10.83	25.5	2350
G2c	17-Aug-09	28	3	4.80	123.6	10.96	27.6	2510
G2d	14-Sep-09	56	6	4.70	124.5	11.01	28.2	2560

Average 28 Day (psi): 2430



Remarks:

Reviewed: Darrell Gilman, CMT Manager
Date: 9-14-09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621.8334 Fax: (207) 626.9094

GROUT COMPRESSIVE STRENGTH TEST RESULTS - ASTM C1019

Project No: 14194
Project: Avesta Florence House
Client: Avesta Florence House, LP and Florence House
307 Cumberland Avenue
Portland, ME 04101

Field Test Data

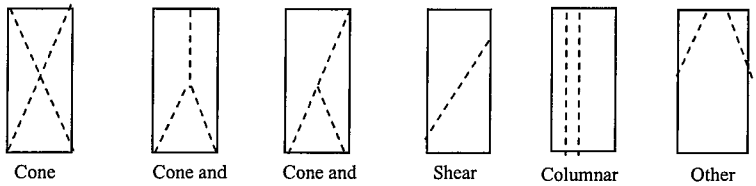
Set No.: G3
Placement Date: 20-Aug-09
Lab Rec'd Date: 21-Aug-09
Location: Third Floor Level Stairwell
Technician: F. Clark
Supplier: Audette & SCE
Mix Designation: CMU Block Grout
Design Strength: 3000psi

Slump (inches) 11
Grout Temp. (^{0}F) 64.0
Air Temp. (^{0}F) 87.0

Laboratory Test Data

Sample No.	Test Date	Age	Type	Wt. (lb.)	Unit Wt.	Area (in^2)	Load (K)	Strength (psi)
G3a	27-Aug-09	7	6	4.84	116.2	12.07	25.0	2070
G3b	17-Sep-09	28	6	4.58	115.7	11.59	24.5	2110
G3c	17-Sep-09	28	6	4.62	114.6	11.73	29.5	2510
G3d	15-Oct-09	56	3	4.73	113.9	12.04	35.5	2950

Average 28 Day (psi): 2310



Remarks: _____

Reviewed: Darrell Gilman, CMT Manager
Date: 10-15-09

SUMMIT GEOENGINEERING SERVICES
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621.8334 Fax: (207) 626.9094

SUMMIT ENVIRONMENTAL CONSULTANTS INC.

MORTAR COMPRESSIVE STRENGTH TEST RESULTS

Project No: 14194
Project: Avesta Florence House
Client: Avesta Florence House, LP
307 Cumberland Avenue
Portland, ME 04101

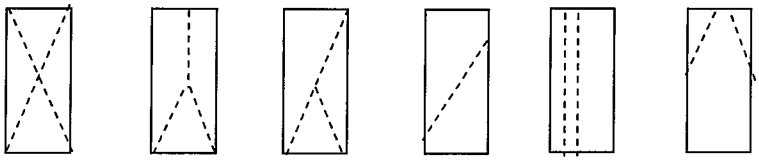
Field Test Data

Set No.: M1
Placement Date: 21-Jul-09
Lab Rec'd Date: 23-Jul-09
Location: Stairwell Wall
Techician: J. Rouillard
Supplier: Audette SCE
Mix Designation: Type "S"
Design Strength: 1800psi

Laboratory Test Data

Sample No.	Test Date	Age	Type	Wt. (lb.)	Unit Wt.	Area (in^2)	Load (K)	Strength (psi)
M1a	28-Jul-09	7	1	0.60	127.8	4.03	12.1	3000
M1b	18-Aug-09	28	1	0.60	125.4	4.03	14.4	3560
M1c	18-Aug-09	28	6	0.62	132.1	4.04	10.6	2620

Average 28 Day (psi): 3090



Cone 1 | Cone and Split 2 | Cone and Shear 3 | Shear 4 | Columnar 5 | Other 6

Remarks: _____

Reviewed: Darrell Gilman, CMT Manager
Date:8-18-09

EXHIBIT B

05120 Steel Construction

Project: Florence House
Date Prepared: September 2, 2008

Structural Schedule of Special Inspections - STEEL CONSTRUCTION

VERIFICATION AND INSPECTION IBC Section 1704.3	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS	AGENT	AGENT QUALIFICATION	TASK COMPLETED
1. Material verification of high-strength bolts, nuts and washers:						
a. Identification markings to conform to ASTM standards specified in the approved construction documents.	Y	S	Applicable ASTM material specifications; AISC 335, Section A3.4; AISC LRFD, Section A3.3	SI1	PE/SE or EIT	JLF
b. Manufacturer's certificate of compliance required.	Y	S		SI1	PE/SE or EIT	JLF
2. Inspection of high-strength bolting						
a. Bearing-type connections.	Y	P	AISC LRFD Section M2.5 IBC Sect 1704.3.3	TA2	AWS/AISC-SSI	SEE ATTACHED
b. Slip-critical connections.	Y	C or P (method dependent)		TA2	AWS/AISC-SSI	SEE ATTACHED
3. Material verification of structural steel (IBC Sect 1708.4):						
a. Identification markings to conform to ASTM standards specified in the approved construction documents.	Y	S	ASTM A 6 or ASTM A 568 IBC Sect 1708.4	SI1	PE/SE or EIT	JLF
b. Manufacturers' certified mill test reports.	Y	S	ASTM A 6 or ASTM A 568 IBC Sect 1708.4	SI1	PE/SE or EIT	SUBMITTED
4. Material verification of weld filler materials:						
a. Identification markings to conform to AWS specification in the approved construction documents.	Y	S	AISC, ASD, Section A3.6; AISC LRFD, Section A3.5	SI1	PE/SE or EIT	JLF
b. Manufacturer's certificate of compliance required.	Y	S		SI1	PE/SE or EIT	X
5. Submit current AWS D1.1 welder certificate for all field welders who will be welding on this project.	Y	S	AWS D1.1	SI1	PE/SE or EIT	SEE ATTACHED
6. Inspection of welding (IBC 1704.3.1): a. Structural steel:						
1) Complete and partial penetration groove welds.	Y	C	AWS D1.1	TA2	AWS-CWI	JLF
2) Multipass fillet welds.	Y	C		TA2	AWS-CWI	N/A
3) Single-pass fillet welds> 5/16"	Y	C		TA2	AWS-CWI	N/A
4) Single-pass fillet welds< 5/16"	Y	P		TA2	AWS-CWI	JLF
5) Floor and deck welds.	Y	P	AWS D1.3	TA2	AWS-CWI	JLF
b. Reinforcing steel (IBC Sect 1903.5.2):						
1) Verification of weldability of reinforcing steel other than ASTM A706.	N/A	C	NOT PERMITTED			
2) Reinforcing steel-resisting flexural and axial forces in intermediate and special moment frames, and boundary elements of special reinforced concrete shear walls and shear reinforcement.	N/A	C	AWS D1.4 ACI 318: 3.5.2		AWS-CWI	
3) Shear reinforcement.	N/A	C			AWS-CWI	
4) Other reinforcing steel.	Y	P		TA2	AWS-CWI	N/A
7. Inspection of steel frame joint details for compliance (IBC Sect 1704.3.2) with approved construction documents:						
a. Details such as bracing and stiffening.	Y	P		SI1	PE/SE or EIT	JLF
b. Member locations.	Y	P		SI1	PE/SE or EIT	JLF
c. Application of joint details at each connection.	Y	P		SI1	PE/SE or EIT	JLF

Project: Florence House
Date Prepared: September 2, 2008

Structural Schedule of Special Inspection Services

FABRICATION AND IMPLEMENTATION PROCEDURES – STRUCTURAL STEEL

VERIFICATION AND INSPECTION IBC Section 1704.2	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS	AGENT	AGENT QUALIFICATION	TASK COMPLETED
1. Fabrications Procedures: Review of fabricator's written procedural and quality control manuals and periodic auditing of fabrication practices by an approved special inspection agency. At the completion of fabrication, the approved fabricator shall submit a certificate of compliance to the building code official stating that the work was performed in accordance with the approved construction documents. -OR- 2. AISC Certification	Y	S	Fabricator shall submit one of the two qualifications	SI1	PE/SE or EIT	TJF AISC CERTIFICATION
3. At completion of fabrication, the approved fabricator shall submit a certificate of compliance to the building code official stating that the work was performed in accordance with the approved construction documents.	Y	S	IBC 1704.2.2	SI1	PE/SE or EIT	OK

American Institute of Steel Construction

is proud to recognize

Precision Welding & Fabrication, Inc.

Westbrook, ME

for successfully meeting the quality certification requirements for

Standard for Steel Building Structures

Roger E. Ferch

Certification valid through October 2009

GANNESTON CONSTRUCTION CORP.
P.O. BOX 27 AUGUSTA, ME 04332

X	REVIEWED		REVIEWED WITH NOTES RESUBMISSION REQUESTED
	REVIEWED WITH NOTES NO RESUBMISSION		NOT ACCEPTABLE

CHECK MARK INDICATES ACTION TAKEN
CHECKED BY ____ DATE 2/23/09

Z187-1704.2

RECEIVED
... 2009
GANNESTON CONSTRUCTION CORP.

[X] Reviewed [ ] Furnish as Corrected
[ ] Rejected [ ] Revise and Resubmit
[ ] Submit Specific Item:

This review is only for general conformance with the design concept of the project and general compliance with the information given in the Contract Documents. Corrections or comments made on the shop drawings during this review do not relieve contractor from compliance with the requirements of the plans and specifications. Approval of a specific item shall not include approval of an assembly of which the item is a component. Contractor is responsible for: dimensions to be confirmed and correlated at the jobsite: information that pertains solely to the fabrication processes or to the means, methods, techniques, sequences and procedures of construction: coordination of his or her Work with that of all other trades: and for performing all work in a safe and satisfactory manner.

Becker Structural Engineers, Inc.

Date 3·3·09 By MJM

SECTION 05200 - PART 1, 1.04, H, 2.3.

P.O.#16166
#80677-5100
FLORENCE HOUSE

Vulcraft Sales Corporation

March 3, 2009

Larry Cunha
Larene Steel
5 Mills Point
Middleton, MA 01949-2506
Fax: 978-646-0090

Vulcraft File #: 032-08-6527

Vulcraft of New York, Inc inspects joists according to the SJI specifications 5.12, 104.13 and 1004.10 as stated in the Code of Standard Practice, section 4. In order to maintain SJI plant certification, inspection must be performed at the plant. The SJI compliance letter should cover any inspection requirements, if there is something more specific they need addressed please contact me.

Sincerely,

Jason M Thornton

Jason M. Thornton
District Sales Manager
Vulcraft Sales Corporation
Salem NH Office

22

GANNESTON CONSTRUCTION CORP.
P.O. BOX 27 AUGUSTA, ME 04332

X	REVIEWED		REVIEWED WITH NOTES RESUBMISSION REQUESTED
	REVIEWED WITH NOTES NO RESUBMISSION		NOT ACCEPTABLE

CHECK MARK INDICATES ACTION TAKEN
CHECKED BY [illegible] DATE 3/6/09

1

SECTION 05200 - PART 1 - 1.04, H, 3

P.O.# 16166
FLORENCE HOUSE
0677-5100

Vulcraft Sales Corporation

March 3, 2009

23

GANNESTON CONSTRUCTION CORP.
P.O. BOX 27 AUGUSTA, ME 04332

X	REVIEWED		REVIEWED WITH NOTES RESUBMISSION REQUIRED
	REVIEWED WITH NOTES NO RESUBMISSION		NOT ACCEPTABLE

JMT 3/6/09

Larry Cunha
Larene Steel
5 Mills Point
Middleton, MA 01949-2506
Fax: 978-646-0090

Vulcraft File #: 032-08-6527

Vulcraft performs visual inspection of all joist and joist girder products during their production. We do not generate inspection reports for individual projects. All products are deemed acceptable for shipment unless they are set off by our inspectors. Any corrective action is also inspected prior to the product being released for shipment.

In lieu of inspection reports, you may choose either in-house inspection performed by an inspection agency of your choosing, or we can perform a full-scale load test. For in-house inspection by an outside agency, we will provide the day and time of production to you. The chosen agency should be at our facility at that time in order to witness the fabrication process.

For a full scale load test, the member to be tested can be chosen in-house by our QA Supervisor, or by the Engineer-of-Record. Full scale load test includes verifying the member load capacity and deflection using a series of concentrated loads that simulate the loading from the SJI Load Table. The tested member must safely support 1.65 x theoretical design capacity of critical components. The member tested will not be utilized within the project. Cost for a full-scale test is $750 per joist. If the member to be tested is chosen by the Engineer-of-Record, please indicate on your approved bill of materials the joist to be tested.

Sincerely,

Jason M. Thornton
District Sales Manager
Vulcraft Sales Corporation
Salem NH Office

☒ Reviewed ☐ Furnish as Corrected
☐ Rejected ☐ Revise and Resubmit
☐ Submit Specific Item:

This review is only for general conformance with the design concept of the project and general compliance with the information given in the Contract Documents. Corrections or comments made on the shop drawings during this review do not relieve contractor from compliance with the requirements of the plans and specifications. Approval of a specific item shall not include approval of an assembly of which the item is a component. Contractor is responsible for: dimensions to be confirmed and correlated at the jobsite; information that pertains solely to the fabrication processes or to the means, methods, techniques, sequences and procedures of construction; coordination of his or her Work with that of all other trades; and for performing all work in a safe and satisfactory manner.

Becker Structural Engineers, Inc.
Date 3-16-09 By MJM

SECTION 05200 - PART 1, 1.04, G

P.O.# 16166

80677-510

NUCOR
VULCRAFT GROUP

VULCRAFT SALES CORPORATION

☒ Reviewed ☐ Furnish as Corrected
☐ Rejected ☐ Revise and Resubmit
☐ Submit Specific Item:

This review is only for general conformance with the design concept of the project and general compliance with the information given in the Contract Documents. Corrections or comments made on the shop drawings during this review do not relieve contractor from compliance with the requirements of the plans and specifications. Approval of a specific item shall not include approval of an assembly of which the item is a component. Contractor is responsible for: dimensions to be confirmed and correlated at the jobsite; information that pertains solely to the fabrication processes or to the means, methods, techniques, sequences and procedures of construction; coordination of his or her Work with that of all other trades; and for performing all work in a safe and satisfactory manner.

Becker Structural Engineers, Inc.

Date 3-16-09 By MJM

February 26, 2009

Contact LARRY CUNHA
Company Name: LARENE STEEL PRODUCTS
Address 5 MILLS POINT
City: MIDDLETOWN ST: MA Zip Code 01949

Reference: FLORENCE HOUSE / PORTLAND , ME
Purchase Order Number: 810-25
Vulcraft Number: 032-08-6527

Gentlemen:

Vulcraft Division, Nucor Corporation, hereby certifies that we are a member of the Steel Joist Institute. Vulcraft open web steel joists are designed and manufactured in accordance with the standard joist specifications of the Steel Joist Institute.

These joists will safely support a uniformly distributed load as designated in the applicable Steel Joist Institute load table, for the particular type and span, when field applications are in accordance with these specifications.

Cordially,

Linda R Chappell

Linda R Chappell
DISTRICT SALES CLERK

#328

GANNESTON CONSTRUCTION CORP.
P.O. BOX 27 AUGUSTA, ME 04332

X	REVIEWED		REVIEWED WITH NOTES RESUBMISSION REQUESTED
	REVIEWED WITH NOTES NO RESUBMISSION		NOT ACCEPTABLE

CHECK MARK INDICATES ACTION TAKEN
CHECKED BY MM DATE 3/6/09

5 MANOR PARKWAY SALEM, NH 03079 BUS: 603.894.1146 FAX: 603.894.1149

AWS Certified Welder

Solomon R. Gay

Cert # **0702070W** SSN # **XXX-XX-9877**

1-800-443-9353

Information relating to identification and certification of the bearer of this card may be verified by calling or writing:
CERTIFICATION DEPARTMENT OF THE AMERICAN WELDING SOCIETY
550 N.W. LeJeune Road, Miami, FL 33126

AMERICAN WELDING SOCIETY

VALID ONLY IF ACCOMPANIED BY PHOTO ID

This Card is the property of AWS and shall be returned on demand.

☐ Reviewed ☐ Furnish as Corrected
☐ Rejected ☐ Revise and Resubmit
☐ Submit Specific Item:

This review is only for general conformance with the design concept of the project and general compliance with the information given in the Contract Documents. Corrections or comments made on the shop drawings during this review do not relieve contractor from compliance with the requirements of the plans and specifications. Approval of a specific item shall not include approval of an assembly of which the item is a component. Contractor is responsible for: dimensions to be confirmed and correlated at the jobsite: information that pertains solely to the fabrication processes or to the means, methods, techniques, sequences and procedures of construction: coordination of his or her Work with that of all other trades: and for performing all work in a safe and satisfactory manner.

Becker Structural Engineers, Inc.

Date _____ By _____

RECEIVED
[illegible] 2009
GANNESTON
CONSTRUCTION CORP.

05729-105

GANNESTON CONSTRUCTION CORP.
P.O. BOX 27 AUGUSTA, ME 04332

REVIEWED	REVIEWED WITH NOTES RESUBMISSION REQUESTED
REVIEWED WITH NOTES NO RESUBMISSION	NOT ACCEPTABLE

CHECK MARK INDICATES ACTION TAKEN
CHECKED BY MB DATE 2/23/09

Solomon R. Gay

#	Test Date	Sup	Code	Process	Gas	Filler Metal	Base Metal	Position	Thickness	Expires
1	08/02/96	G	D1.1	SMAW	N/A	F4	P	A	L	07/11/09

AWS Certified Welder

Welder's Brief Title and Operation

Ronald H. Moody

Cert # **0801130W** SSN # **XXX-XX-7265**

1-800-443-9353

Information relating to identification and certification of the bearer of this card may be verified by calling or writing:
CERTIFICATION DEPARTMENT OF THE AMERICAN WELDING SOCIETY
550 N.W. LeJeune Road, Miami, FL 33126

AMERICAN WELDING SOCIETY

VALID ONLY IF ACCOMPANIED BY PHOTO ID

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Ronald H. Moody

#	Test Date	Sup	Code	Process	Gas	Filler	Metal	Base Metal	Position	Thickness	Expires
1	06/10/96	G	D1.1	SMAW	N/A	F4		P	A	L	06/25/09

AWS Certified Welder

Edward P Nadeau

Cert # 0009030W SSN # XXX-XX-2162

1-800-443-9353

Information relating to certification of the bearer of this card may be obtained by calling or writing the Certification Department of the American Welding Society

AMERICAN WELDING SOCIETY

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This Card is the property of AWS and shall be returned on demand.

Edward P Nadeau

#	Test Date	Sup	Code	Process	Gas	Filler Metal	Base Metal	Position	Thickness	Expires
1	08/01/00	G	D1.1	SMAW	--	F4	P1	A		01/20/06

VERIFY CERTIFICATE HAS BEEN UPDATED.

54c

AWS Certified Welder
Welders, Fitters and Operators

Scott J Flaherty

Cert # **0510038W** SSN # **XXX-XX-5584**

1-800-443-9353

Information relating to identification and certification of the bearer of this card may be verified by calling or writing:
CERTIFICATION DEPARTMENT OF THE AMERICAN WELDING SOCIETY
550 N.W. LeJeune Road, Miami, FL 33126

AMERICAN WELDING SOCIETY

VALID ONLY IF ACCOMPANIED BY PHOTO ID

This Card is the property of AWS and shall be returned on demand.

Scott J Flaherty

#	Test Date	Sup	Code	Process	Gas	Filler Metal	Base Metal	Position	Thickness	Expires
1	10/13/05	G	D1.1	SMAW	N/A	E7018	SA 36	ALL	UNLIMITED	03/09/09
2	10/13/05	G	D1.5	FCAW	75/25	E71T-1	SA 36	ALL	LIMITED	03/09/09

AWS QC-3

PERFORMANCE QUALIFICATION TEST RECORD

Eye correction required Yes ☐ No ☐ Type of Eye Correction: Eye glasses ☐ Contact lenses ☐ Magnifiers ☐

Name MOODY, RONALD H. Social Security # [redacted]-7265

Welder ☒ Operator ☐

Qualified with AWS WPS No. 1-QC-W2 Supplement No. G Test No. D1-SM-F4-P-A-L

Process(es) SMAW Manual ☒ Semi-Automatic ☐ Automatic ☐ Machine ☐

Test base metal specification SA 36 To SA 36

Material number (M or P Number) P1 To P1

Shielding Gas NA Flow Rate NA

AWS Filler metal classification E7018 F no 4 Size 2/32" & 1/8"

Backing Yes ☒ No ☐ Consumable Insert Yes ☐ No ☒

Double Welded ☐ or Single Welded ☒ Short Circuiting arc (GMAW) Yes ☐ No ☒

Current AC ☐ DC ☒ Back Purging Yes ☐ No ☒

Test results

Visual test results Pass ☒ Fail ☐ Radiographic test results NA ☒ Pass ☐ Fail ☐

Bend test results NA ☐ Pass ☒ Fail ☐

PROCESS(es) QUALIFIED FOR SMAW

POSITION(s) QUALIFIED FOR:

Groove:

Pipe 1G ☒ 2G ☒ 5G ☒ 6G ☒ 6GR ☐ (T)Min 1/8" Max 3/4" Diameter 24" Range 24" and greater

Plate 1G ☒ 2G ☒ 3G ☒ 4G ☒ (T)Min 1/8" Max 3/4"

Consumable Insert ☐ Backing type ☒

Fillet:

Pipe 1F ☒ 2F ☒ 4F ☒ 5F ☒ (T)Min 1/8" Max Unlimited

Plate 1F ☒ 2F ☒ 3F ☒ 4F ☒ (T)Min 1/8" Max Unlimited

Vertical Up ☒ Down ☐ Weld Deposit Min 1/8" Max 3/4"

Single Side ☒ Double Side ☐

The above named person qualified for the welding process(es) used in this test within the limits of essential variables shown above, including materials and filler metal variables of the AWS Standard for welder certification and ANSI/AWS D1.1 Code or Standard. I hereby certify that I was not involved in the training of the above named individual as a welder:

Date Tested 6/10/96 Signed by Thomas E. Giles
Test Supervisor

AWS CWI No. 88070281

Signed by Thomas E. Giles Coordinator-WELDING CENTER, Eastern Maine Technical College
Corporate Representative Title

Form QC-WP1-- Performance Qualification Test Record

56

AWS QC-3

PERFORMANCE QUALIFICATION TEST RECORD

Eye correction required Yes ☐ No ☐ Type of Eye Correction: Eye glasses ☐ Contact lenses ☐ Magnifiers ☐

Name GAY, SOLOMON R. Social Security # [redacted]-9877

Welder ■ Operator ☐

Qualified with AWS WPS No. 1-QC-W2 Supplement No. G Test No. D1-SM-F4-P-A-L

Process(es) SMAW Manual ■ Semi-Automatic ☐ Automatic ☐ Machine ☐

Test base metal specification SA 36 To SA 36

Material number (M or P Number) P1 To P1

Shielding Gas NA Flow Rate NA

AWS Filler metal classification E7018 F no 4 Size 2/32" & 1/8"

Backing Yes ■ No ☐ Consumable insert Yes ☐ No ■

Double Welded ☐ or Single Welded ■ Short Circuiting arc (GMAW) Yes ☐ No ■

Current AC ☐ DC ■ Back Purging Yes ☐ No ■

Test results

Visual test results Pass ■ Fail ☐ Radiographic test results NA ■ Pass ☐ Fail ☐

Bend test results NA ☐ Pass ■ Fail ☐

PROCESS(es) QUALIFIED FOR SMAW

POSITION(s) QUALIFIED FOR:

Groove:

Pipe 1G ■ 2G ■ 5G ■ 6G ■ 6GR ☐ (T)Min 1/8" Max 3/4" Diameter 24" Range 24" and greater

Plate 1G ■ 2G ■ 3G ■ 4G ■ (T)Min 1/8" Max 3/4"

Consumable Insert ☐ Backing type ■

Fillet:

Pipe 1F ■ 2F ■ 4F ■ 5F ■ (T)Min 1/8" Max Unlimited

Plate 1F ■ 2F ■ 3F ■ 4F ■ (T)Min 1/8" Max Unlimited

Vertical Up ■ Down ☐ Weld Deposit Min 1/8" Max 3/4"

Single Side ■ Double Side ☐

The above named person qualified for the welding process(es) used in this test within the limits of essential variables shown above, including materials and filler metal variables of the AWS Standard for welder certification and ANSI/AWS D1.1 Code or Standard. I hereby certify that I was not involved in the training of the above named individual as a welder:

Date Tested 8/2/96 Signed by Thomas E. Giles Test Supervisor

AWS CWI No. 88070281

Signed by Thomas E. Giles Coordinator-WELDING CENTER, Eastern Maine Technical College

Corporate Representative Title

Form QC WF1-- Performance Qualification Test Record

WELDING TEST CENTER / Eastern Maine Community College
354 Hogan Road, Bangor, ME 04401
(207) 974-4662, FAX (207) 974-4608
tgiles@emcc.edu

FAX TRANSMITTAL

For Immediate Delivery To:

Name: Sol

Organization: Precision Welding and Fabrication, Inc.

FAX Number: (207) 854-9694

Total number of pages, including this page: 4

From:

Name: Thomas Giles

Office: Welding Test Center

Date: 01/29/08

Message:

Attached please find the supporting WPS for the welder certifications you mentioned. This procedure is from the AWS QC 3-89 "Standard for AWS Certified Welders" WPS# 1-QC-W2 and is pre-qualified per AWS D1.1 Structural Welding Code - Steel.

If problems in transmission are encountered, please contact the sender at:
(207)974-4662 or FAX (207)974-4608.

WELDER, WELDING OPRATOR, OR TACK WELDER QUALIFICATION TEST RECORD

Name of Welder: Jason Joiner
Name: Precision Welding Identification No. 6847
Welding Procedure Specification No. 1 Rev. N/A Date Aug. 20, 2008

Variables	Record Actual Values Used in Qualification	Qualification Range
Process/Type [Table 4.10, Item (1)]	**SMAW**	
Electrode (single or multiple) [Table 4.10, Item (4)]	1/8 E7018	ALL
Current Polarity	115 A DC+	
Position [Table 4.10, Item (6)]	2F - Tack test	1F, 2F
Weld Progression [Table 4.10, Item (6)]	N/A	N/A
Backing (YES or NO) [Table 4.10 Item (7)]	NO	NO
Material/Spec.	Group 1 to Group 1	
Base Metal		
Thickness: (Plate)		
Groove	N/A	N/A
Fillet	1/2 inch	UNLIMITED
Thickness: (Pipe/Tube)		
Groove	N/A	N/A
Fillet	N/A	UNLIMITED
Diameter :(Pipe)PJP		
Groove	N/A	N/A
Fillet	N/A	N/A
Filler Metal [Table 4.10, Item (3)]		
Spec. No.	A5.1	
Class	E7018	
F-No. [Table 4.10, Item (2)]	F 4	F4, F3, F2, F1
Gas/Flux Type [Table 4.10item (3)]	N/A	
Other	N/A	N/A

VISUAL INSPECTION (4.8.1)

Acceptable YES or NO: YES

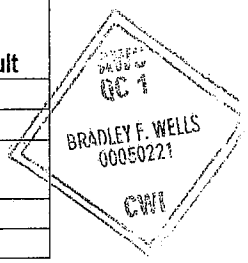
Guided Bend Test Results (4.30.5)

Type	Result	Type	Result

Fillet Test Results (4.30.2.3 and 4.30.4.1)

Appearance	Acceptable	Fillet Size	0.1875
Fracture Test Root Penetration	Acceptable	Macroetch	N/A

(Describe the location, nature, and size of any crack or tearing of the specimen.)



Inspected by Brad Wells CWI # 00050221 Test Number N/A
Organization Maine Oxy Date Aug 20, 2008

RADIOGRAPHIC TEST RESULTS (4.30.3.1)

Film Identification Number	Results	Remarks	Film Identification Number	Results	Remarks

Interpreted by _____ Test Number _____
Organization _____ Date _____

We, the undersigned, certify that the statements in this record are correct and that the test welds were prepared, welded, and tested in accordance with the requirements of section 4 of AWS D1.1, (_____) Structural Welding Code ---- Steel.

Manufacturer or Contractor _____ Authorized By _____
Date _____

Keywords— AWS Certified Welders
Welders
Welder Certification
Welder Qualification

AWS QC 3-89

Standard for AWS Certified Welders

Prepared by
AWS Qualification and Certification Committee

Under the Direction of
AWS Education and Certification Council

Approved by
AWS Board of Directors
October 31, 1989

Abstract

This Standard describes a program administrated by the American Welding Society (AWS) for the certification of welders. The AWS Certified Welder Program allows for the transference of welder qualification from employer to employer without retesting. The standard establishes the minimum requirements for welder participation in the program and the methods to be used by employers for adopting the program.

American Welding Society
550 N.W. LeJeune Road, Miami, Florida 33126

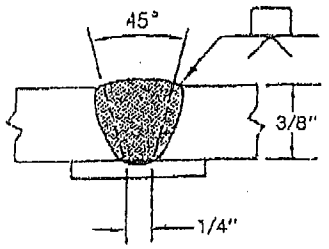
QC 3 PERFORMANCE WELDING

PROCEDURE SPECIFICATION

Material specification ASTM A-36
Welding process Shielded Metal Arc (SMAW)
Manual or machine Manual
Position of welding Vertical and Overhead
Filler metal specification AWS A5.1
Filler metal classification E7018
Flux N/A
Shielding gas N/A Flow rate N/A
Single or multiple pass Multiple pass
Single or multiple arc Single arc
Welding current Direct current
Polarity Electrode positive
Welding progression Vert-up weave, OH - stringer beads
Root treatment None
Preheat and interpass temperature 70° F min. - 500° max.
Postheat treatment None

WELDING PROCEDURE

Pass no.	Electrode size	Welding current		Travel speed	Joint detail
		Amperes *	Volts		
All	5/32" or 1/8"	75-120	19-23	Manual	
		110-180	19-23	Manual	
		*Consult Manufacturers recommendations for specific ranges.			



This procedure may vary due to fabrication sequence, fit-up, pass size, etc., within the limitation of variables given in 4B, C, or D and 5.1.2 of AWS D1.1, (1986 year) Structural Welding Code.

Procedure no. 1-QC-W2
Revision no. 0
Manufacturer or contractor American Welding Society
Authorized by R E Blackwell
Date May 16, 1989

S1-6

SUPPLEMENT 1

PERFORMANCE TEST DESCRIPTION
Test Number: D1-SM-F4-P-A-L

Welding Process:	Shielded Metal Arc Welding (SMAW)
Base Material:	ANSI/ASTM A-36, 3/8" thickness (rolling direction perpendicular to bevelled edge)
Material Form:	Plate
Filler Metal:	ANSI/AWS A5.1, Class E-7018 (Group: F4)
Weld Joint Detail:	ANSI/AWS D1.1, Para. 5.19
Backing:	Prequalified steel plate from ANSI/AWS D1.1
Welding Position:	3G & 4G (See ANSI/AWS A3.0-89 Figure 14)
Weld Progression (Vert):	up
WPS No.:	1-QC-W2
Test Required:	Visual plus one root bend and one face bend. Radiography may be used in lieu of bend tests.

LIMITS OF WELDER QUALIFICATION

Code:	ANSI/AWS D1.1
Weld Process:	SMAW
Base Material:	Prequalified steels from ANSI/AWS D1.1
Filler Metal:	Group F4 and lower
Thickness:	Groove: 1/8" thru 3/4"
Fillet Weld Size:	All
Positions:	All
Weld Progression:	Vertical, up
Backing:	Required for full penetration, single groove welds
Pipe/tubing Dia.:	Over 24"
Material Form:	Plate, pipe, shapes, strip, tubing as listed in ANSI/AWS D1.1

WELDER, WELDING OPRATOR, OR TACK WELDER QUALIFICATION TEST RECORD

Name of Welder: Ben Berry
Name: Precision Welding — Identification No. 8281
Welding Procedure Specification No. 2 — Rev. N/A — Date Nov. 7, 07

Variables	Record Actual Values Used in Qualification	Qualification Range
Process/Type [Table 4.10, Item (1)]	SMAW	ALL
Electrode (single or multiple) [Table 4.10, Item (1)]	1/8 E7018	
Current Polarity	110 A DC+	
Position [Table 4.10, Item (6)]	3G	1G, 2G, 3G
Weld Progression [Table 4.10, Item (6)]	UP	UP
Backing (YES or NO) [Table 4.10 Item (7)]	YES	YES
Material/Spec.	Group 1 to Group 1	
Base Metal		
Thickness: (Plate)	3/8 inch	1/8 " to 3/4 "
Groove	N/A	UNLIMITED
Fillet		
Thickness: (Pipe/Tube)	N/A	1/8 " to 3/4 "
Groove	N/A	UNLIMITED
Fillet		
Diameter :(Pipe)PJP	N/A	1/8" to 3/4 OVER 24" DIA.
Groove	N/A	OVER 24" DIA.
Fillet		
Filler Metal [Table 4.10, Item (3)]	A5.1	
Spec. No.	E7018	
Class	F 4	F4, F3, F2, F1
F-No. [Table 4.10, Item (2)]	N/A	
Gas/Flux Type [Table 4.10item (3)]	N/A	N/A
Other		

VISUAL INSPECTION (4.8.1)
Acceptable YES or NO: YES

Guided Bend Test Results (4.30.5)

Type	Result	Type	Result

Fillet Test Results (4.30.2.3 and 4.30.4.1)

Appearance	N/A	Fillet Size	N/A
Fracture Test Root Penetration	N/A	Macroetch	N/A

(Describe the location, nature, and size of any crack or tearing of the specimen.)

Inspected by Brad Wells CWI # 00050221 — Test Number N/A
Organization Maine Oxy — Date Nov. 7, 2007

RADIOGRAPHIC TEST RESULTS (4.30.3.1)

Film Identification Number	Results	Remarks	Film Identification Number	Results	Remarks
0-1	Accept	Por, Slag.			

Interpreted by Aaron Miller — Test Number
Organization OAL — Date 11/8/07

We, the undersigned, certify that the statements in this record are correct and that the test welds were prepared, welded, and tested in accordance with the requirements of section 4 of AWS D1.1, (_____) Structural Welding Code ---- Steel.

Manufacturer or Contractor _____ Authorized By _____
Date _____

ANNEX E

WELDER, WELDING OPERATOR, OR TACK WELDER QUALIFICATION TEST RECORD

Type of Welder: Precision Welding & Fabrication Inc. Identification No. [redacted] -21628
Name: Edward L. [illegible] Date: 4-13-04
Welding Procedure Specification No. 1 Rev n/a

Variables	Record Actual Values Used in Qualification	Qualification Range
Process/Type [Table 4.10, Item (1)]	FCAW	All
Electrode (single or multiple) [Table 4.10, Item (8)]	04: E71T-1	
Current/Polarity	DCT	
Position [Table 4.10, Item (4)]	2G	1G, 2G
Weld Progression [Table 4.10, Item (6)]		
Backing (YES or NO) [Table 4.10, Item (7)]	yes	yes
Material/Spec.	to	
Base Metal		
Thickness: (Plate)		
Groove	1"	unlimited
Fillet	n/a	unlimited
Thickness: (Pipe/tube)		
Groove	n/a	unlimited
Fillet	n/a	unlimited
Diameter: (Pipe)	n/a	
Groove	n/a	
Fillet	n/a	
Filler Metal [Table 4.10, Item (3)]		
Spec. No.	A5.20	
Class	E71T-1	
F-No. [Table 4.10, Item (2)]	F6	F6
Gas/Flux Type [Table 4.10, Item (3)]	75% argon 25% CO_2	n/a
Other	n/a	

VISUAL INSPECTION (4.8.1)

Acceptable YES or NO ____

Guided Bend Test Results (4.30.5)

Type	Result	Type	Result

Fillet Test Results (4.30.2.3 and 4.30.4.1)

Appearance ______ Fillet Size ______
Fracture Test Root Penetration ______ Macroetch ______
(Describe the location, nature, and size of any crack or tearing of the specimen.)

Inspected by ______ Test Number ______
Organization ______ Date ______

RADIOGRAPHIC TEST RESULTS (4.30.3.1)

Film Identification Number	Results	Remarks	Film Identification Number	Results	Remarks
Edward	Acceptable				

Interpreted by: David [illegible] Test Number ______
Organization: DAL Date: 5/5/04

We, the undersigned, certify that the statements in this record are correct and that the test welds were prepared, welded, and tested in accordance with the requirements of section 4 of AWS D1.1, (______) Structural Welding Code—Steel.
(year)

Manufacturer or Contractor ______ Authorized By ______
Date ______

Form E-4

04

ANNEX E

WELDING PROCEDURE SPECIFICATION (WPS) Yes ☑
PREQUALIFIED ✓ QUALIFIED BY TESTING ____
or PROCEDURE QUALIFICATION RECORDS (PQR) Yes ☐

Identification # 1
Revision N/A Date ____ By [illegible]
Authorized by ____ Date 4-13-04
Type—Manual ☐ Semi-Automatic ☑
Machine ☐ Automatic ☐

Edward P. MacLean
Company Name Precision Welding & Fabrication
Welding Process(es) FCAW
Supporting PQR No.(s) 1?? - Qualified

JOINT DESIGN USED
Type:
Single ☒ Double Weld ☐
Backing: Yes ☒ No ☐
Backing Material: ASTM A36
Root Opening 1/4" Root Face Dimension —
Groove Angle: 45° Radius (J–U) —
Back Gouging: Yes ☐ No ☐ Method —

BASE METALS
Material Spec. ASTM A36
Type or Grade —
Thickness: Groove 1" Fillet ____
Diameter (Pipe) ____

FILLER METALS
AWS Specification AWS 5.20
AWS Classification E71T-1

SHIELDING
Flux — Gas Argon / CO2
Composition 75%-25%
Electrode-Flux (Class) ____ Flow Rate 35-40 CFH
Gas Cup Size 5/8"

PREHEAT
Preheat Temp., Min PER AWS TABLE 3.2 (D1.1)
Interpass Temp., Min — Max —

POSITION
Position of Groove: 2G Fillet: ____
Vertical Progression: Up ☐ Down ☐

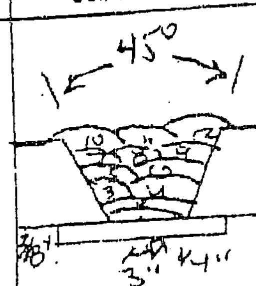
ELECTRICAL CHARACTERISTICS
Transfer Mode (GMAW) Short-Circuiting ☐
Globular ☐ Spray ☒
Current: AC ☐ DCEP ☒ DCEN ☐ Pulsed ☐
Other ____
Tungsten Electrode (GTAW)
Size: ____
Type: ____

TECHNIQUE
Stringer or Weave Bead: Stringer / Weave Root
Multi-pass or Single Pass (per side) Multi Pass
Number of Electrodes One
Electrode Spacing Longitudinal —
Lateral —
Angle —

Contact Tube to Work Distance 3/4"
Peening None
Interpass Cleaning: Hand wire brush, Chipping Hammer, Grinder on Spatter

POSTWELD HEAT TREATMENT
Temp. ____
Time ____

WELDING PROCEDURE

Pass or Weld Layer(s)	Process	Filler Metals: Class	Filler Metals: Diam.	Current: Type & Polarity	Current: Amps or Wire Feed Speed	Volts	Travel Speed	Joint Details
1-13	FCAW	E71T-1	.045	Vertical DCT	Variable 180-200 amp	26.5		

Form E-1 (Front)

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INSPECTION REPORT

PAGE 1 OF 1

CUSTOMER'S NAME: PRECISION WELDING & FABRICATION P.O. NO.:

RADIOGRAPHY REPORT NO.: QAL-04-280 PROCEDURE NO.: 0913 QUANTITY: 1

PART NO.: 1" WELDER TEST PLATE JOB NO.:

SOURCE: TYPE: Iridium 192 SIZE: .118 X .132 CURIES: 24.9 KV: MA: SID: 18"

SINGLE ☑ FRONT ☑

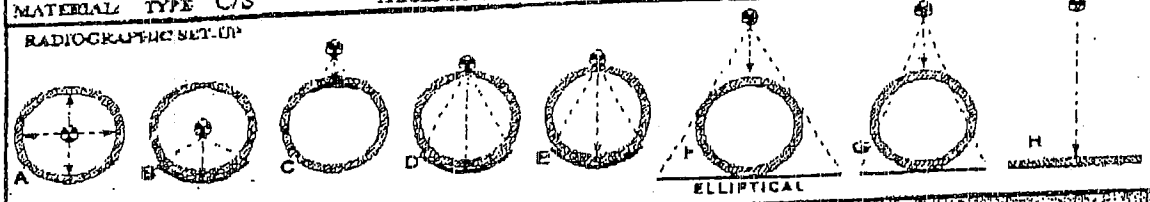
DOUBLE ☐ SIZE: 4½ x 10 SCREENS: 0.010" BACK ☑

FILM: TYPE II SPEED: 100 FILM SIDE ☐

IQI: SIZE ASTM "B" WIRE GROUP: I SENSITIVITY: .025 SHIM: N/A SOURCE SIDE ☑

MATERIAL: TYPE C/S THICKNESS: 1" ACCEPTANCE STANDARD: AWS D1.1

RADIOGRAPHIC SET-UP



OTHER

WELDER NUMBER	VIEW NUMBER	CONDITION OF PART	ACCEPT	REJECT
EDWARD	0-1	9	✓	

WELDER NUMBER	VIEW NUMBER	CONDITION OF PART	ACCEPT	REJECT

REMARKS:

METHOD(S), PROCESS(ES), PROCEDURE(S) MERCURY FREE

DEFINITIONS:
1. Crack
2. Porosity
3. Incomplete Fusion
4. Incomplete penetration
5. Slag
6. Inclusion
7. Gas Holes
8. Scratch
9. No Apparent Defects
10. Film Artifacts
11. RVLs
12. Surface
13. Undercut
14. Void
15. Industrial radiography

SIGNATURE: D. Pratt #2568772

DATE: M / D / Y 05/05/2004

LEVEL: II

(FORM XXXX REV 0)

Name of Welder: Josh Bragdon
Name: Precision Welding — Identification No. [redacted]-2257
Welding Procedure Specification No.: 1 — Rev.: N/A — Date: Nov. 07, 2007

Variables	Record Actual Values Used in Qualification	Qualification Range
Process/Type [Table 4.10, Item (1)]	FCAW	ALL
Electrode (single or multiple) [Table 4.10, Item (2)]	.045 E71T1-M-H8	
Current Polarity	215 A DC positive	
Position [Table 4.10, Item (6)]	1G	1G
Weld Progression [Table 4.10, Item (6)]		F
Backing (YES or NO) [Table 4.10 Item (7)]	YES	YES
Material/Spec.	Group 1 to Group 1	
Base Metal		
Thickness: (Plate)	1"	UNLIMITED
Groove	N/A	UNLIMITED
Fillet		
Thickness: (Pipe/Tube)	N/A	UNLIMITED
Groove	N/A	UNLIMITED
Fillet		
Diameter :(Pipe)	N/A	OVER 24" DIA.
Groove	N/A	OVER 24" DIA.
Fillet		
Filler Metal [Table 4.10, Item (3)]		
Spec. No.	A5.20	
Class	E71T1-1M-H8	
F-No. [Table 4.10, Item (2)]	F6	F6
Gas/Flux Type [Table 4.10item (3)]	N/A	
Other	N/A	N/A

VISUAL INSPECTION (4.8.1)

Acceptable YES or NO: YES

Guided Bend Test Results (4.30.5)

Type	Result	Type	Result
	N/A		N/A

Fillet Test Results (4.30.2.3 and 4.30.4.1)

Appearance: N/A — Fillet Size: N/A
Fracture Test Root Penetration: N/A — Macroetch: N/A
(Describe the location, nature, and size of any crack or tearing of the specimen.)

Inspected by: Brad Wells CWI # 00050221 — Test Number: N/A
Organization: Maine Oxy — Date: November 7, 2007

RADIOGRAPHIC TEST RESULTS (4.30.3.1)

Film Identification Number	Results	Remarks	Film Identification Number	Results	Remarks
0-1	Accept	Slag			

Interpreted by: Aaron Miller — Test Number:
Organization: QAL — Date: 11/8/07

We, the undersigned, certify that the statements in this record are correct and that the test welds were prepared, welded, and tested in accordance with the requirements of section 4 of AWS D1.1, (_____) Structural Welding Code ---- Steel.

Manufacturer or Contractor: _____ Authorized By: _____
Date: _____

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INSPECTION REPORT

CUSTOMER'S NAME: PRECISION WELDING | P.O. NO.: | PAGE 2 OF 2

RADIOGRAPHY REPORT NO.: QAL-07-2139 | PROCEDURE NO.: 1 | QUANTITY: 3

PART NO.: | JOB NO.:

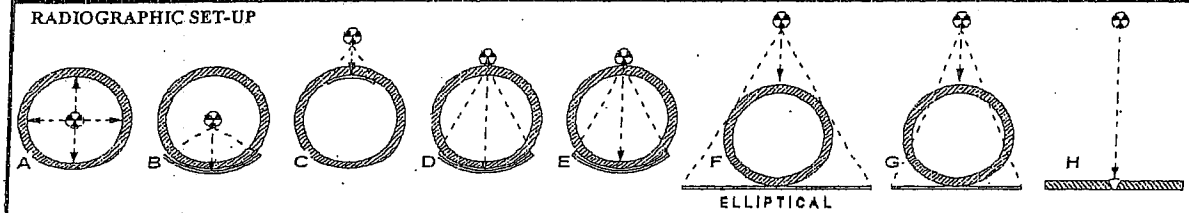
SOURCE: TYPE IR-192 | SIZE: .118" | CURIES: 14.1 | KV: | MA: | SFD: 16"

FILM: TYPE II | SPEED: 100 | SINGLE ☑ DOUBLE ☐ | SIZE: 41/2X10 | SCREENS: .010" | FRONT ☑ BACK ☑ FILM SIDE ☐ SOURCE SIDE ☑

IQI: SIZE B WIRE | GROUP: 1 | SENSITIVITY: .010" | SHIM:

MATERIAL: TYPE CS/SS | THICKNESS: 1.250"-3/8" | ACCEPTANCE STANDARD: AWSD1.1

RADIOGRAPHIC SET-UP



OTHER

SERIAL NUMBER	VIEW NUMBER	CONDITION OF PART (See Definitions)	ACCEPT	REJECT	SERIAL NUMBER	VIEW NUMBER	CONDITION OF PART (See Definitions)	ACCEPT	REJECT
J.Lunt									
1" 1G	0-1	5		✓					
Bragdon	0-1	5	✓						
1"									
3G									
Berry	0-1	2,5	✓						
3/8"									
3G									

REMARKS

DEFINITIONS:

1. Crack	6. Inclusions	11. Hi/Lo
2. Porosity	7. Gas Holes	12. Surface
3. Incomplete Fusion	8. Shrink	13. Undercut
4. Incomplete penetration	9. No Apparent Defects	14. Void
5. Slag	10. Film Artifacts	15. Internal concavity

SIGNATURE: Aaron Miller

DATE: M / D / Y 11/08/2007 LEVEL: II

(FORM XXXX REV 0)

WELDING PROCEDURE SPECIFICATION (WPS) Yes ☒

PREQUALIFIED ✓ QUALIFIED BY TESTING ____

Ben.B or PROCEDURE QUALIFICATION RECORDS (PQR) Yes ☐

Identification # 1

Company Name Precision Welding + Fabrication Revision N/A Date ____ Authorized by ____ By M. Schroeder Date ____

Welding Process(es) FCAW Type—Manual ☐ Machine ☐ Semi-Automatic ☒ Automatic ☐

Supporting PQR No.(s) PREQUALIFIED

JOINT DESIGN USED

Type:

Single ☒ Double Weld ☐

Backing: Yes ☒ No ☐

Backing Material: ASTM A36

Root Opening 1/4" Root Face Dimension —

Groove Angle: 45° Radius (J-U) —

Back Gouging: Yes ☐ No ☐ Method —

POSITION

Position of Groove: 2G Fillet: ____

Vertical Progression: Up ☐ Down ☐

ELECTRICAL CHARACTERISTICS

Transfer Mode (GMAW) Short-Circuiting ☐ Globular ☐ Spray ☒

Current: AC ☐ DCEP ☒ DCEN ☐ Pulsed ☐

Other ____

Tungsten Electrode (GTAW)

Size: ____

Type: ____

BASE METALS

Material Spec. ASTM A36

Type or Grade —

Thickness: Groove 1" Fillet ____

Diameter (Pipe) ____

FILLER METALS

AWS Specification AWS 5.20

AWS Classification E71T-1

TECHNIQUE

Stringer or Weave Bead: STRINGER / WEAVE Root

Multi-pass or Single Pass (per side) MULTIPASS

Number of Electrodes ONE

Electrode Spacing Longitudinal — Lateral — Angle —

SHIELDING

Flux —

Gas Argon / CO_2

Composition 75%-25%

Electrode-Flux (Class) ____

Flow Rate 35-40 CFH

Gas Cup Size 5/8"

Contact Tube to Work Distance 3/4"

Peening NONE

Interpass Cleaning: hand wire brush, Chipping hammer, grinder on spatter

PREHEAT

Preheat Temp., Min PER AWS TABLE 3.2 (D1.1)

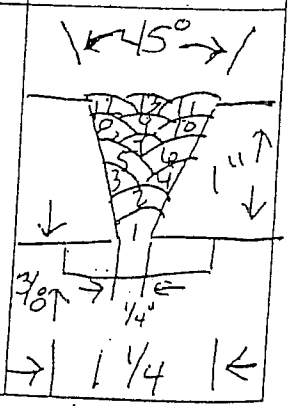
Interpass Temp., Min ____ Max —

POSTWELD HEAT TREATMENT

Temp. ____

Time ____

WELDING PROCEDURE

Pass or Weld Layer(s)	Process	Filler Metals Class	Filler Metals Diam.	Current Type & Polarity	Current Amps or Wire Feed Speed	Volts	Travel Speed	Joint Details
1-13	FCAW	E71T-1	.045	DC+ Vertical	200 AMPS.	26.5		

Form E-1 (Front)

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INSPECTION REPORT

CUSTOMER'S NAME: PRECISION WELDING | P.O. NO.: N/A | PAGE 1 OF 1

RADIOGRAPHY REPORT NO.: QAL-02-843 | PROCEDURE NO.: 0913 | QUANTITY: 1

PART NO.: TEST COUPON | JOB NO.: N/A

SOURCE: TYPE Iridium 192 | SIZE: .107 X .118 | CURIES: 30 | KV: N/A | MA: N/A | SFD: 20"

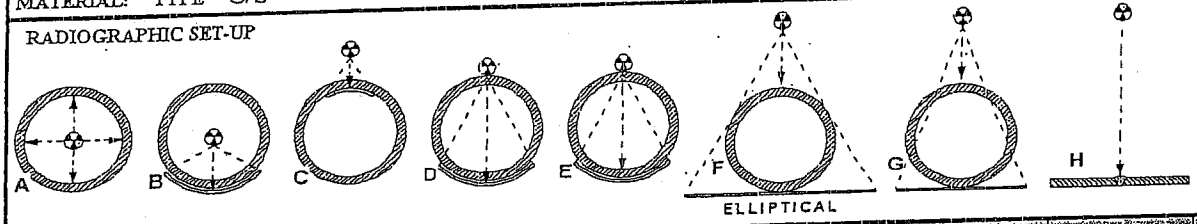
SINGLE ☑ | DOUBLE ☐ | FRONT ☑ | BACK ☑ | FILM SIDE ☐ | SOURCE SIDE ☑

FILM: TYPE II | SPEED: 100 | SIZE: 4½ x 10 | SCREENS: 0.010"

IQI: SIZE ASTM "B" WIRE | GROUP: I | SENSITIVITY: .020 | SHIM: N/A

MATERIAL: TYPE C/S | THICKNESS: 1" | ACCEPTANCE STANDARD: ASME SECTION IX

RADIOGRAPHIC SET-UP



OTHER

SERIAL NUMBER	VIEW NUMBER	CONDITION OF PART (See Definitions)	ACCEPT	REJECT
BEN B.	0 - 1	2,5	✓	
FCAW				

SERIAL NUMBER	VIEW NUMBER	CONDITION OF PART (See Definitions)	ACCEPT	REJECT

REMARKS

DEFINITIONS:
1. Crack
2. Porosity
3. Incomplete Fusion
4. Incomplete penetration
5. Slag
6. Inclusions
7. Gas Holes
8. Shrink
9. No Apparent Defects
10. Film Artifacts
11. Hi/Lo
12. Surface
13. Undercut
14. Void
15. Internal concavity

SIGNATURE: R. Russell #2687716 Ryan Russell

DATE: M / D / Y 12/09/2002 LEVEL: II

(FORM XXXX REV 0)

Name of Welder: Jeff Lunt
Name: Precision Welding — Identification No. 2258
Welding Procedure Specification No.: 1 — Rev.: N/A — Date: Jan 15, 2008

Variables	Record Actual Values Used in Qualification	Qualification Range
Process/Type [Table 4.10; Item (1)]	FCAW	ALL
Electrode (single or multiple) [Table 4.10, Item (2)]	.045 E71T1-M-H8	
Current Polarity	215 A DC Pos	
Position [Table 4.10, Item (6)]	3G	1G, 2G, 3G
Weld Progression [Table 4.10, Item (6)]	V-up	V-up
Backing (YES or NO) [Table 4.10 Item (7)]	YES	YES
Material/Spec.	Group 1 to Group 1	
Base Metal		
Thickness: (Plate)		
Groove	1 inch	UNLIMITED
Fillet	N/A	UNLIMITED
Thickness: (Pipe/Tube)		
Groove	N/A	UNLIMITED
Fillet	N/A	UNLIMITED
Diameter :(Pipe)PJP		
Groove	N/A	OVER 24" DIA.
Fillet	N/A	OVER 24" DIA.
Filler Metal [Table 4.10, Item (3)]		
Spec. No.	A5.18	
Class	E71T1-M-H8	
F-No. [Table 4.10, Item (2)]	F6	F6
Gas/Flux Type [Table 4.10item (3)]	C-25	
Other	N/A	N/A

VISUAL INSPECTION (4.8.1)

Acceptable YES or NO: YES

Guided Bend Test Results (4.30.5)

Type	Result	Type	Result

Fillet Test Results (4.30.2.3 and 4.30.4.1)

Appearance	N/A	Fillet Size	N/A
Fracture Test Root Penetration	N/A	Macroetch	N/A

(Describe the location, nature, and size of any crack or tearing of the specimen.)

Inspected by: Brad Wells CWI # 00050221 — Test Number: N/A
Organization: Maine Oxy — Date: Jan 15, 2008

RADIOGRAPHIC TEST RESULTS (4.30.3.1)

Film Identification Number	Results	Remarks	Film Identification Number	Results	Remarks
0-1	N.A.D	—			
	Accept				

Interpreted by: John Weese — Test Number: 08-0082
Organization: Q.A.L — Date: 1-16-08

We, the undersigned, certify that the statements in this record are correct and that the test welds were prepared, welded, and tested in accordance with the requirements of section 4 of AWS D1.1, (_____) Structural Welding Code — Steel.

Manufacturer or Contractor: _____ — Authorized By: _____
Date: _____

WELDER, WELDING OPERATOR, OR TACK WELDER QUALIFICATION TEST RECORD

Name of Welder: Solomon Gay
Name: Precision Welding Identification No. 9877
Welding Procedure Specification No. 1 Rev. N/A Date Jan 15, 2008

Variables	Record Actual Values Used in Qualification	Qualification Range
Process/Type [Table 4.10, Item (1)]	FCAW	
Electrode (single or multiple) [Table 4.10, Item (	045 E71T1-M-H8	ALL
Current Polarity	215 A DC Pos	
Position [Table 4.10, Item (6)]	3G	1G, 2G, 3G
Weld Progression [Table 4.10, Item (6)]	V-up	V-up
Backing (YES or NO) [Table 4.10 Item (7)]	YES	YES
Material/Spec.	Group 1 to Group 1	
Base Metal		
Thickness: (Plate)		
Groove	1 inch	UNLIMITED
Fillet	N/A	UNLIMITED
Thickness: (Pipe/Tube)		
Groove	N/A	UNLIMITED
Fillet	N/A	UNLIMITED
Diameter :(Pipe)PJP		
Groove	N/A	OVER 24" DIA.
Fillet	N/A	OVER 24" DIA.
Filler Metal [Table 4.10, Item (3)]		
Spec. No.	A5.18	
Class	E71T1-M-H8	
F-No. [Table 4.10, Item (2)]	F6	F6
Gas/Flux Type [Table 4.10item (3)]	C-25	
Other	N/A	N/A

VISUAL INSPECTION (4.8.1)

Acceptable YES or NO: YES

Guided Bend Test Results (4.30.5)

Type	Result	Type	Result

Fillet Test Results (4.30.2.3 and 4.30.4.1)

Appearance	N/A	Fillet Size	N/A
Fracture Test Root Penetration	N/A	Macroetch	N/A

(Describe the location, nature, and size of any crack or tearing of the specimen.)

Inspected by Brad Wells CWI # 00050221 Test Number N/A
Organization Maine Oxy Date Jan 15, 2008

RADIOGRAPHIC TEST RESULTS (4.30.3.1)

Film Identification Number	Results	Remarks	Film Identification Number	Results	Remarks
0-1	N.A.D	—			
	Accept				

Interpreted by John Weese Test Number 08-0082
Organization Q.A.L Date 1-16-08

We, the undersigned, certify that the statements in this record are correct and that the test welds were prepared, welded, and tested in accordance with the requirements of section 4 of AWS D1.1, (_____) Structural Welding Code ---- Steel.

Manufacturer or Contractor _____ Authorized By _____
Date _____

Quality Assurance Labs Inc.
NON-DESTRUCTIVE TESTING AND INSPECTION SERVICES
80 PLEASANT AVENUE • SOUTH PORTLAND, MAINE 04106 • TEL: (207) 799-8911 • FAX: (207) 799-7251

INSPECTION REPORT

CUSTOMER'S NAME: PRECISION WELDING | P.O. NO.: | PAGE 1 OF 1

RADIOGRAPHY REPORT NO.: QAL-08-0082 | PROCEDURE NO.: AWS D1.1 | QUANTITY: 2

PART NO.: SEE BELOW | JOB NO.: 2258/ 9877

SOURCE: TYPE Iridium 192 | SIZE: .118 | CURIES: 75 | KV: | MA: | SFD: 12"

SINGLE ☑ | FRONT ☑

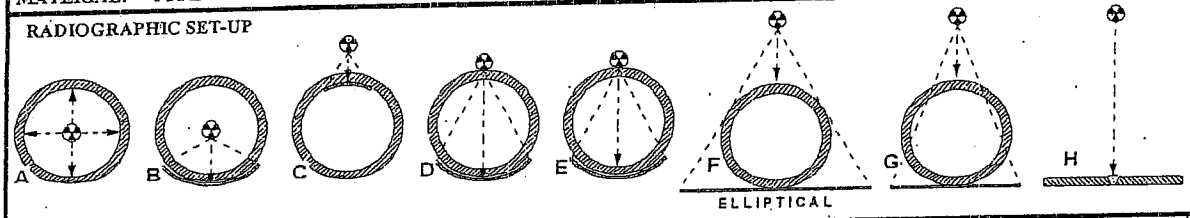
FILM: TYPE II | SPEED: 100 | DOUBLE ☐ | SIZE: 4½ x 10 | SCREENS: 0.005" | BACK ☑

FILM SIDE ☐

IQI: SIZE B | GROUP: 1 | SENSITIVITY: .025 | SHIM: | SOURCE SIDE ☑

MATERIAL: TYPE CS/SS | THICKNESS: 1" +1/4" BB | ACCEPTANCE STANDARD: AWS D1.1

RADIOGRAPHIC SET-UP



OTHER

SERIAL NUMBER	VIEW NUMBER	CONDITION OF PART (See Definitions)	ACCEPT	REJECT	SERIAL NUMBER	VIEW NUMBER	CONDITION OF PART (See Definitions)	ACCEPT	REJECT
JEFF					soloman				
LUNT	0-1	9	✓		gay	0-1	9	✓	
2258					9877				

REMARKS

DEFINITIONS:
1. Crack
2. Porosity
3. Incomplete Fusion
4. Incomplete penetration
5. Slag
6. Inclusions
7. Gas Holes
8. Shrink
9. No Apparent Defects
10. Film Artifacts
11. Hi/Lo
12. Surface
13. Undercut
14. Void
15. Internal concavity

SIGNATURE: John Weese

DATE: 01/16/2008 (M / D / Y)

LEVEL: II

(FORM XXXX REV 0)

AWS D1.1:2000

WELDING PROCEDURE SPECIFICATION (WPS) Yes ☑

PREQUALIFIED ✓ QUALIFIED BY TESTING ____

or PROCEDURE QUALIFICATION RECORDS (PQR) Yes ☐

Identification # 1

Revision N/A Date ____ By S. Gray

Authorized by ____ Date ____

Company Name Precision Welding & Fabrication Inc.

Welding Process(es) FCAW Type—Manual ☐ Semi-Automatic ☐

Machine ☐ Automatic ☐

Supporting PQR No.(s) Prequalified

JOINT DESIGN USED

Type:

Single ☒ Double Weld ☐

Backing: Yes ☒ No ☐

Backing Material:

Root Opening 1/4" Root Face Dimension -

Groove Angle: 45° Radius (J–U) -

Back Gouging: Yes ☐ No ☒ Method ____

POSITION

Position of Groove: 1G & 2G & 3G. Fillet: ____

Vertical Progression: Up ☐ Down ☐

ELECTRICAL CHARACTERISTICS

Transfer Mode (GMAW) Short-Circuiting ☐

Globular ☐ Spray ☒

Current: AC ☐ DCEP ☒ DCEN ☐ Pulsed ☐

Other ____

Tungsten Electrode (GTAW)

Size: ____

Type: ____

BASE METALS

Material Spec. ASTM A36

Type or Grade -

Thickness: Groove 1" Fillet ____

Diameter (Pipe) ____

TECHNIQUE

Stringer or Weave Bead: Stringer / Weave Both

Multi-pass or Single Pass (per side) Multi Pass

Number of Electrodes one

Electrode Spacing Longitudinal -

Lateral -

Angle -

FILLER METALS

AWS Specification AWS 5.20

AWS Classification A7IT-1

SHIELDING

Flux -

Gas Argon / CO2

Composition 75% - 25%

Electrode-Flux (Class) ____

Flow Rate 35-40 CFH

Gas Cup Size 5/8"

Contact Tube to Work Distance 3/4"

Peening None

Interpass Cleaning: hand wire Brush

Chipping Hammer, grinder on spatter

PREHEAT

Preheat Temp., Min per AWS Table 3.2 (D1.1)

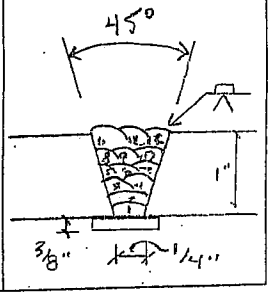
Interpass Temp., Min ____ Max ____

POSTWELD HEAT TREATMENT

Temp. ____

Time ____

WELDING PROCEDURE

Pass or Weld Layer(s)	Process	Filler Metals		Current		Volts	Travel Speed	Joint Details
		Class	Diam.	Type & Polarity	Amps or Wire Feed Speed			
1-13	FCAW	E71T-1	.045	Vertical DCT	200 amps	26.5		

Form E-1 (Front)

WELDING PROCEDURE SPECIFICATION (WPS) Yes ☑

PREQUALIFIED ✓ QUALIFIED BY TESTING ____

or PROCEDURE QUALIFICATION RECORDS (PQR) Yes ☐

Identification # 2

Revision N/A Date ____ By S. Cam

Company Name Precision Welding & Fabrication Inc. Authorized by ____ Date ____

Welding Process(es) SMAW Type—Manual ☐ Semi-Automatic ☐

Supporting PQR No.(s) Prequalified Machine ☒ Automatic ☐

JOINT DESIGN USED

Type:

Single ☒ Double Weld ☐

Backing: Yes ☐ No ☐

Backing Material:

Root Opening 1/4" Root Face Dimension –

Groove Angle: 45° Radius (J–U) –

Back Gouging: Yes ☐ No ☒ Method ____

BASE METALS

Material Spec. ASTM A36

Type or Grade —

Thickness: Groove 3/8" Fillet ____

Diameter (Pipe) ____

FILLER METALS

AWS Specification AWS A5.1

AWS Classification E7018

SHIELDING

Flux ____ Gas ____

Composition ____

Electrode-Flux (Class) ____ Flow Rate ____

Gas Cup Size ____

PREHEAT

Preheat Temp., Min PER AWS TABLE 3.2 (D1.1)

Interpass Temp., Min ____ Max ____

POSITION

Position of Groove: 3G Fillet: ____

Vertical Progression: Up ☐ Down ☐

ELECTRICAL CHARACTERISTICS

Transfer Mode (GMAW) Short-Circuiting ☐

Globular ☐ Spray ☐

Current: AC ☐ DCEP ☐ DCEN ☐ Pulsed ☐

Other ____

Tungsten Electrode (GTAW)

Size: –

Type: –

TECHNIQUE

Stringer or Weave Bead: Stringer

Multi-pass or Single Pass (per side) Multi Pass

Number of Electrodes ____

Electrode Spacing Longitudinal ____

Lateral ____

Angle ____

Contact Tube to Work Distance ____

Peening ____

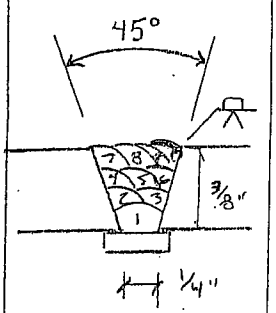
Interpass Cleaning: ____

POSTWELD HEAT TREATMENT

Temp. ____

Time ____

WELDING PROCEDURE

Pass or Weld Layer(s)	Process	Filler Metals		Current		Volts	Travel Speed	Joint Details
		Class	Diam.	Type & Polarity	Amps or Wire Feed Speed			
All			3/32" or 1/8"		75-120 110-180		manual manual	

Form E-1 (Front)

WELDING PROCEDURE SPECIFICATION (WPS) Yes ☒

PREQUALIFIED ✓ QUALIFIED BY TESTING ____
or PROCEDURE QUALIFICATION RECORDS (PQR) Yes ☐

Identification # PRECISION 702
Revision 1 Date 12/5/07 By BWELLS
Authorized by ____ Date ____

Company Name PRECISION WELDING
Welding Process(es) FCAW
Supporting PQR No.(s) BTC-P4-6F
prequalified

Type—Manual ☐ Semi-Automatic ☒
Machine ☐ Automatic ☐

JOINT DESIGN USED

Type: 6 GROOVE : PARTIAL
Single ☒ Double Weld ☐
Backing: Yes ☐ No ☒
Backing Material:
Root Opening 0 Root Face Dimension 1/8 min
Groove Angle: 45° Radius (J–U) ____
Back Gouging: Yes ☐ No ☒ Method ____

POSITION

Position of Groove: 1G Fillet: ____
Vertical Progression: Up ☐ Down ☐

ELECTRICAL CHARACTERISTICS

Transfer Mode (GMAW) Short-Circuiting ☐
Globular ☐ Spray ☐
Current: AC ☐ DCEP ☒ DCEN ☐ Pulsed ☐
Other ____
Tungsten Electrode (GTAW)
Size: ____
Type: ____

BASE METALS

Material Spec. GROUP I to GROUP II
Type or Grade A36 to A992
Thickness: Groove 3/16 Fillet ____
Diameter (Pipe) ____

FILLER METALS

AWS Specification A5.18
AWS Classification E71T-1C, 1M, 9C, 9M

TECHNIQUE

Stringer or Weave Bead: STRINGER
Multi-pass or Single Pass (per side) SINGLE
Number of Electrodes ONE
Electrode Spacing Longitudinal ____
Lateral ____
Angle ____

SHIELDING

Flux ____
Gas ARGON CO2
Composition 75%-25%
Electrode-Flux (Class) ____
Flow Rate 40 cfh
Gas Cup Size 5/8

Contact Tube to Work Distance 3/4"
Peening NONE
Interpass Cleaning: Slag hammer / wire brush

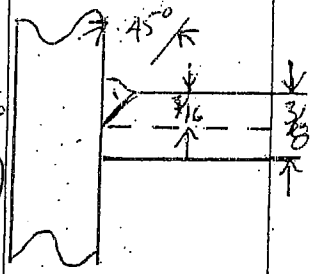
PREHEAT

Preheat Temp., Min TABLE 3.2-D1.1 PER AWS
Interpass Temp., Min SAME Max —

POSTWELD HEAT TREATMENT

Temp. ____
Time ____

WELDING PROCEDURE

Pass or Weld Layer(s)	Process	Filler Metals		Current		Volts	Travel Speed	Joint Details
		Class	Diam.	Type & Polarity	Amps or Wire Feed Speed			
1	FCAW	E71T1C 1M, 9C 9M	.045	DC+	550 IPM ± 10% (495-605)	29 ± 7% (27-31)	10 IPM ± 25% (7.5-13.5)	

Form E-1 (Front)

WELDING PROCEDURE SPECIFICATION (WPS) Yes ☒

PREQUALIFIED ✓ QUALIFIED BY TESTING ____

or PROCEDURE QUALIFICATION RECORDS (PQR) Yes ☐

Company Name PRECISION WELDING

Welding Process(es) FCAW

Supporting PQR No.(s) Prequalified - Fillet

Identification # 703

Revision 1 Date 12/5/07 By BWells

Authorized by ____ Date ____

Type—Manual ☐ Semi-Automatic ☒

Machine ☐ Automatic ☐

JOINT DESIGN USED

Type: Fillet - Skewed

Single ☐ Double Weld ☐

Backing: Yes ☐ No ☒

Backing Material:

Root Opening ____ Root Face Dimension ____

Groove Angle: ____ Radius (J–U) ____

Back Gouging: Yes ☐ No ☐ Method ____

POSITION

Position of Groove: ____ Fillet: 1F

Vertical Progression: Up ☐ Down ☐

ELECTRICAL CHARACTERISTICS

Transfer Mode (GMAW) Short-Circuiting ☐

Globular ☐ Spray ☐

Current: AC ☐ DCEP ☒ DCEN ☐ Pulsed ☐

Other ____

Tungsten Electrode (GTAW)

Size: ____

Type: ____

BASE METALS

Material Spec. Group I to Group II

Type or Grade A36 to A992

Thickness: Groove ____ Fillet 5/16 - 9/16

Diameter (Pipe) ____

FILLER METALS

AWS Specification A5.18

AWS Classification E71T-1C, 1M, 9C, 9M

TECHNIQUE

Stringer or Weave Bead: STRINGER

Multi-pass or Single Pass (per side) SINGLE / MULTI

Number of Electrodes ONE

Electrode Spacing Longitudinal ____

Lateral ____

Angle ____

SHIELDING

Flux ____

Electrode-Flux (Class) ____

Gas ARGON - CO_2

Composition 75%-25%

Flow Rate 40CFH

Gas Cup Size 5/8

Contact Tube to Work Distance 3/4 - 1"

Peening NONE

Interpass Cleaning: chipping hammer - wire brush

PREHEAT

Preheat Temp., Min Per Dwl Table 3.2 - AWS

Interpass Temp., Min SAME Max ____

POSTWELD HEAT TREATMENT

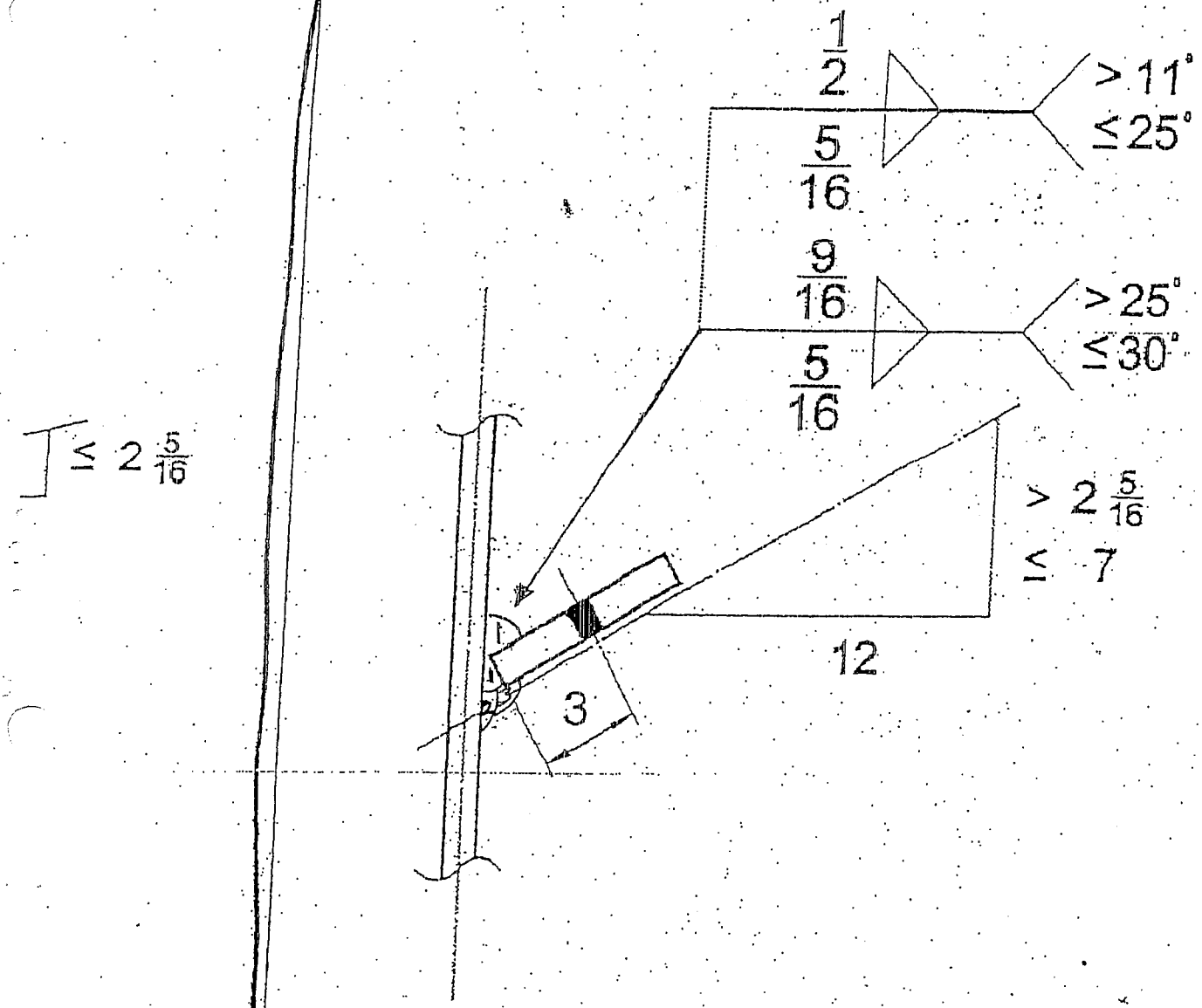
Temp. ____

Time ____

WELDING PROCEDURE

Pass or Weld Layer(s)	Process	Filler Metals		Current		Volts	Travel Speed	Joint Details
		Class	Diam.	Type & Polarity	Amps or Wire Feed Speed			
1-3	FCAW	E71T-1C 1M, 9C 9M	.045	DC+	550IPM ± 10% (495-605)	29 ± 7% (27-31)	10IPM ±25% (7.5-13.5)	SEE ATTACHED SHEET

Form E-1 (Front)



(b)

dicular

For $\theta = 45°$ from Perpendicular

WELDING PROCEDURE SPECIFICATION (WPS) Yes ☒

PREQUALIFIED ✓ QUALIFIED BY TESTING ____

or PROCEDURE QUALIFICATION RECORDS (PQR) Yes ☐

Identification # 704

Revision 1 Date 12/5/07 By B. Bell

Company Name PRECISION WELDING

Welding Process(es) FCAW

Supporting PQR No.(s) PREQUALIFIED

Authorized by ____ Date ____

Type—Manual ☐ Semi-Automatic ☒

Machine ☐ Automatic ☐

JOINT DESIGN USED

Type: FILLET

Single ☐ Double Weld ☐

Backing: Yes ☐ No ☒

Backing Material:

Root Opening ____ Root Face Dimension ____

Groove Angle: ____ Radius (J–U) ____

Back Gouging: Yes ☐ No ☐ Method ____

POSITION

Position of Groove: ____ Fillet: 2F

Vertical Progression: Up ☐ Down ☐

ELECTRICAL CHARACTERISTICS

Transfer Mode (GMAW) Short-Circuiting ☐

Globular ☐ Spray ☐

Current: AC ☐ DCEP ☒ DCEN ☐ Pulsed ☐

Other ____

Tungsten Electrode (GTAW)

Size: ____

Type: ____

BASE METALS

Material Spec. ASTM A36

Type or Grade ____

Thickness: Groove ____ Fillet ≤ 1/8

Diameter (Pipe) ____

FILLER METALS

AWS Specification A5.18

AWS Classification E71T-1C, 1M, 9C, 9M

TECHNIQUE

Stringer or Weave Bead: STRINGER

Multi-pass or Single Pass (per side) SINGLE

Number of Electrodes ONE

Electrode Spacing Longitudinal ____

Lateral ____

Angle ____

SHIELDING

Flux ____

Gas ARGON/CO_2

Composition 75-25%

Electrode-Flux (Class) ____

Flow Rate 40 CFH

Gas Cup Size 5/8

Contact Tube to Work Distance 3/4"-1"

Peening NONE

Interpass Cleaning: chipping hammer & wire brush

PREHEAT

Preheat Temp., Min Per Table 3.2 of D1.1 - AWS

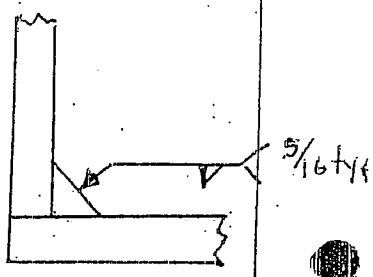
Interpass Temp., Min SAME Max ____

POSTWELD HEAT TREATMENT

Temp. ____

Time ____

WELDING PROCEDURE

Pass or Weld Layer(s)	Process	Filler Metals		Current		Volts	Travel Speed	Joint Details
		Class	Diam.	Type & Polarity	Amps or Wire Feed Speed			
1	FCAW	E71T-1C 1M, 9C 9M	.045	DC+	300 IPM ± 10% (270-330)	26 ± 7% (24.5-28)	8 IPM ±25% (6-10)	 5/16 t/t

Form E-1 (Front)

WELDING PROCEDURE SPECIFICATION (WPS) Yes ☒

PREQUALIFIED ✓ QUALIFIED BY TESTING _____

or PROCEDURE QUALIFICATION RECORDS (PQR) Yes ☐

Identification # Precision 705

Revision 1 Date 12/5/07 By BLAIS

Company Name PRECISION WELDING

Authorized by _____ Date _____

Welding Process(es) FCAW

Type—Manual ☐ Semi-Automatic ☒

Machine ☐ Automatic ☐

Supporting PQR No.(s) PREQUALIFIED

JOINT DESIGN USED

Type: FILLET

Single ☐ Double Weld ☐

Backing: Yes ☐ No ☒

Backing Material:

Root Opening _____ Root Face Dimension _____

Groove Angle: _____ Radius (J–U) _____

Back Gouging: Yes ☐ No ☒ Method _____

BASE METALS

Material Spec. ASTM A36

Type or Grade _____

Thickness: Groove _____ Fillet ≤ 1/8

Diameter (Pipe) _____

FILLER METALS

AWS Specification A5.18

AWS Classification E71T-1C, 1M, 9C, 9M

SHIELDING

Flux _____ Gas ARGON / CO_2

Composition 75%-25%

Electrode-Flux (Class) _____ Flow Rate 40 CFH

Gas Cup Size 5/8

PREHEAT

Preheat Temp., Min Per Table 3.2 of D1.1-AWS

Interpass Temp., Min SAME Max _____

POSITION

Position of Groove: _____ Fillet: 2F

Vertical Progression: Up ☐ Down ☐

ELECTRICAL CHARACTERISTICS

Transfer Mode (GMAW) Short-Circuiting ☐

Globular ☐ Spray ☐

Current: AC ☐ DCEP ☒ DCEN ☐ Pulsed ☐

Other _____

Tungsten Electrode (GTAW)

Size: _____

Type: _____

TECHNIQUE

Stringer or Weave Bead: STRINGER

Multi-pass or Single Pass (per side) SINGLE

Number of Electrodes ONE

Electrode Spacing Longitudinal _____

Lateral _____

Angle _____

Contact Tube to Work Distance 3/4 - 1"

Peening NONE

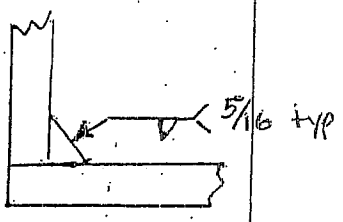
Interpass Cleaning: chipping hammer / wire brush

POSTWELD HEAT TREATMENT

Temp. _____

Time _____

WELDING PROCEDURE

Pass or Weld Layer(s)	Process	Filler Metals		Current		Volts	Travel Speed	Joint Details
		Class	Diam.	Type & Polarity	Amps or Wire Feed Speed			
1	FCAW	E71T-1C 1M, 9C 9M	.045	DC+	550 IPM ± 10% (495-605)	29 ± 7% (27-31)	10 IPM ± 25% (7.5-13.5)	

Form E-1 (Front)

WELDING PROCEDURE SPECIFICATION (WPS) Yes ☒

PREQUALIFIED ✓ QUALIFIED BY TESTING _____

or PROCEDURE QUALIFICATION RECORDS (PQR) Yes ☐

Company Name PRECISION WELDING

Welding Process(es) SMAW

Supporting PQR No.(s) PREQUALIFIED

Identification # S-2030

Revision 1 Date 12-5-07 By BLEVINS

Authorized by _____ Date _____

Type—Manual ☒ Semi-Automatic ☐

Machine ☐ Automatic ☐

JOINT DESIGN USED

Type: FILLET

Single ☐ Double Weld ☐

Backing: Yes ☐ No ☒

Backing Material:

Root Opening _____ Root Face Dimension _____

Groove Angle: _____ Radius (J–U) _____

Back Gouging: Yes ☐ No ☒ Method _____

POSITION

Position of Groove: _____ Fillet: 2F

Vertical Progression: Up ☐ Down ☐

ELECTRICAL CHARACTERISTICS

Transfer Mode (GMAW) Short-Circuiting ☐

Globular ☐ Spray ☐

Current: AC ☐ DCEP ☒ DCEN ☐ Pulsed ☐

Other _____

Tungsten Electrode (GTAW)

Size: _____

Type: _____

BASE METALS

Material Spec.: ASTM A36

Type or Grade _____

Thickness: Groove _____ Fillet ≤ 1/8

Diameter (Pipe) _____

FILLER METALS

AWS Specification A5.1B

AWS Classification E7018

TECHNIQUE

Stringer or Weave Bead: STRINGER

Multi-pass or Single Pass (per side) SINGLE

Number of Electrodes ONE

Electrode Spacing Longitudinal _____

Lateral _____

Angle _____

SHIELDING

Flux _____ Gas _____

Composition _____

Electrode-Flux (Class) _____ Flow Rate _____

Gas Cup Size _____

Contact Tube to Work Distance _____

Peening NONE

Interpass Cleaning: WIRE BRUSH, CHIPPING HAMMER

PREHEAT

Preheat Temp., Min PER TABLE 3.2 of D1.1 -AWS

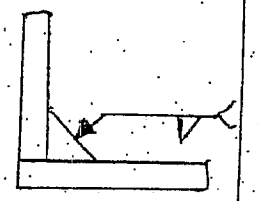
Interpass Temp.; Min SAME Max _____

POSTWELD HEAT TREATMENT

Temp. _____

Time _____

WELDING PROCEDURE

Pass or Weld Layer(s)	Process	Filler Metals		Current		Volts	Travel Speed	Joint Details
		Class	Diam.	Type & Polarity	Amps or Wire Feed Speed			
1	SMAW	E7018	1/8	DC+	115A		AJAM	 1/4 TYP

Form E-1 (Front)

PRECISION WELDING AND FABRICATION, INC.

Structural Steel - Miscellaneous Metals

(207) 854-9330 Telephone
(207) 854-9694 Fax
(207) 854-1167 Accounting

MEMBER SSFNE STRUCTURAL STEEL FABRICATORS OF NEW ENGLAND

690A Stroudwater
P.O. Box 8
Westbrook, Maine 04098-08

COPY

TO: QUALITY ASSURANCE LABS INC.
ATTN: ART GALLANT
FROM: SOL GAY
1-29-08

REF: CONTINUITY LOG FOR AWS CARDS ATTACHED SMAW

- EDWARD NADEAU
- RONALD MOODY
- GARY ZARATE
- SOLOMON GAY

ALL WELDERS LISTED ABOVE HAVE CURRENT CERTIFICATIONS & HAVE HELD THESE CONTINUOUS SINCE DATE OF TESTING DATED ON CARDS. (AWS REQUIRES THESE CARD CERTIFICATIONS TO BE MAINTAINED WITH THEM EVERY SIX MONTHS.)

THANK YOU,

SOLOMON R. GAY

SAMPLE FORM FOR WELDING PROCEDURE SPECIFICATION (WPS)

Company Name PRECISION WELDING By B WELLS
Welding Procedure Specification No. D1.3 007 Rev 007 Date 9-16-08
Supporting Procedure Qualification Test Record(s) No. PREQUALIFIED
Welding Process(es) GMAW Type MANUAL
(Automatic, manual, etc.)
Mode of Transfer for GMAW SHORT-CIRCUITING
(Short circuiting, spray, etc.)

JOINTS (Table 4.1)
Type of Welded Joint(s) 4.1, 4.2A
Backing Yes ☐ No ☒
Backing Material Type
Groove Welded From:
one side ✓ both sides

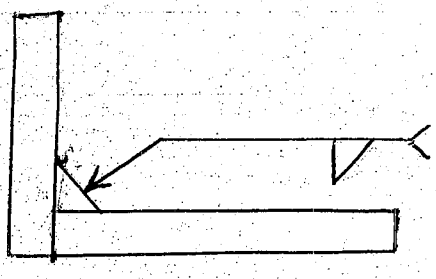
COATING(S)
Type N/A
Thickness

Sketch of Joint Details

BASE METAL (1.2)
Material specification type and grade:
Sheet steel A36 to A36
Support steel
Thickness Range:
Sheet Steel 16G - 3/16
Support Steel 16G - 3/16
Thickness 10 guage
Base Metal Preparation

4.1

4.2A



FILLER METAL (Table 1.1)
Specification A 5.18
Classification E705-6

POSITIONS (Table 1.2)
Position of Groove 3G
Position of Fillet 2F
Progression

PREHEAT (1.1.1 AND 5.1)
Preheat Temperature Min
Preheat Temperature Max

GAS (1.4.6.2)
Shielding Gas ARGON / CO_2
Percent Mixture 75% - 25%
Flow Rate 25 cfh

FLUX (1.4.5.2)

TECHNIQUE

Pass No.	Electrode Size	Welding Current		Travel Speed (or Weld Time for Arc Spot Welds)	Melting Rate	Wire Feed Speed
		Amperes	Volts			
1	.035	150 A	17.5 V.	8 IPM	4.25	250 (approx)

This procedure may vary due to fabrication sequence, fit-up, pass size, etc. within the limitation of variables given in ANSI/AWS D1.3 (98 (year)), *Structural Welding Code—Sheet Steel*.

Authorized by Date

Form A-2

SAMPLE FORM FOR WELDER AND WELDING OPERATOR QUALIFICATION TEST RECORD

Welder or Welding Operator's Name CHAD ANDERSON
Identification No. 0267 Qualification Date 4-15-08
Welder's Social Security No.

In Accordance with WPS No. D1.3-001 Revision 001
Welding Process(es) GMAW Type MANUAL
(Automatic, manual, etc.)
Mode of Transfer for GMAW SHORT CIRCUITING
(Short circuiting, spray, globular)

VARIABLE	ACTUAL VARIABLE USED IN QUAL.	QUALIFICATION RANGE
JOINT		
Joint Type	4.2A	4.2 A
Backing Material Type		
Groove Welded From: one side or both sides		
BASE METAL (4.7.1.1)		
Material Specification		
Sheet Steel	A36 to A36	SEE AWS to 1.2.1
Supporting Steel		
Sheet Thickness (4.7.2.1)		
Groove		
Fillet	10 GUAGE	16 guage → 3/16"
Arc Plug		
Arc Spot		
Arc Seam		
COATING(S)	NONE	NONE
Type		
Thickness		
POSITION (4.7.1.5 and 4.7.1.6)		
Groove		
Fillet	2F	1F, 2F
Arc Plug		
Arc Spot		
Arc Seam		
Progression		
GAS (4.7.1.4)	75%AR - 25%CO_2	75%AR - 25%CO_2
ELECTRODE (4.7.1.3 and 4.7.1.4)		
Size	.035	.035
Group Designation	E70S-6	E70S-X

VISUAL EXAMINATION RESULTS (4.6)
Specimen 1 ACCEPTABLE Specimen 2 ACCEPTABLE
Appearance ACCEPTABLE Cracks NONE Undercut NONE
Reinforcement ACCEPTABLE Diam of Arc Spot Nugget

Test Conducted By BRAD WELLS Per AWS D1.3-98
Laboratory Test No. Date of Test 4-15-08

The undersigned certify that the statements in this record are correct and that the test welds were prepared and tested in accordance with the requirements of 4.6 of ANSI/AWS D1.3 (98 (year)), *Structural Welding Code—Sheet Steel.*

Company Authorized By

Form A-3

J

SAMPLE FORM FOR WELDER AND WELDING OPERATOR QUALIFICATION TEST RECORD

Welder or Welding Operator's Name SOLOMON GAY
Identification No. 9877 Qualification Date 9-15-08
Welder's Social Security No.

In Accordance with WPS No. D1.3-001 Revision 001
Welding Process(es) GMAW Type MANUAL (Automatic, manual, etc.)
Mode of Transfer for GMAW SHORT CIRCUITING (Short circuiting, spray, globular)

VARIABLE	ACTUAL VARIABLE USED IN QUAL.	QUALIFICATION RANGE
JOINT		
Joint Type	4.2A	4.2 A
Backing Material Type		
Groove Welded From: one side or both sides		
BASE METAL (4.7.1.1)		
Material Specification		
Sheet Steel	A36 to A36	SEE AWS to 1.2.1
Supporting Steel		
Sheet Thickness (4.7.2.1)		
Groove		
Fillet	10 GUAGE	16 guage → 3/16"
Arc Plug		
Arc Spot		
Arc Seam		
COATING(S)	NONE	NONE
Type		
Thickness		
POSITION (4.7.1.5 and 4.7.1.6)		
Groove		
Fillet	2F	1F, 2F
Arc Plug		
Arc Spot		
Arc Seam		
Progression		
GAS (4.7.1.4)	75%AR - 25%CO_2	75%AR - 25%CO_2
ELECTRODE (4.7.1.3 and 4.7.1.4)		
Size	.035	.035
Group Designation	E705-6	E705-X

VISUAL EXAMINATION RESULTS (4.6)
Specimen 1 ACCEPTABLE Specimen 2 ACCEPTABLE
Appearance ACCEPTABLE Cracks NONE Undercut NONE
Reinforcement ACCEPTABLE Diam of Arc Spot Nugget

Test Conducted By BRAD WELLS Per AWS D1.3-98
Laboratory Test No. Date of Test 4-15-08

The undersigned certify that the statements in this record are correct and that the test welds were prepared and tested in accordance with the requirements of 4.6 of ANSI/AWS D1.3 (98 (year)), *Structural Welding Code—Sheet Steel.*

Company Authorized By

Form A-3

J

SAMPLE FORM FOR WELDER AND WELDING OPERATOR QUALIFICATION TEST RECORD

Welder or Welding Operator's Name Ben Berry
Identification No. B281 Qualification Date 4-15-08
Welder's Social Security No.

In Accordance with WPS No. D1.3-001 Revision 001
Welding Process(es) GMAW Type MANUAL (Automatic, manual, etc.)
Mode of Transfer for GMAW SHORT CIRCUITING (Short circuiting, spray, globular)

VARIABLE	ACTUAL VARIABLE USED IN QUAL.	QUALIFICATION RANGE
JOINT		
Joint Type	4.2A	4.2 A
Backing Material Type		
Groove Welded From: one side or both sides		
BASE METAL (4.7.1.1)		
Material Specification		
Sheet Steel	A36 to A36	See AWS to 1.2.1
Supporting Steel		
Sheet Thickness (4.7.2.1)		
Groove		
Fillet	10 GUAGE	16 guage → 3/16"
Arc Plug		
Arc Spot		
Arc Seam		
COATING(S)	NONE	NONE
Type		
Thickness		
POSITION (4.7.1.5 and 4.7.1.6)		
Groove		
Fillet	2F	1F, 2F
Arc Plug		
Arc Spot		
Arc Seam		
Progression		
GAS (4.7.1.4)	75%AR - 25% CO_2	75%AR - 25% CO_2
ELECTRODE (4.7.1.3 and 4.7.1.4)		
Size	.035	.035
Group Designation	E705-6	E705-X

VISUAL EXAMINATION RESULTS (4.6)
Specimen 1 ACCEPTABLE Specimen 2 ACCEPTABLE
Appearance ACCEPTABLE Cracks NONE Undercut NONE
Reinforcement ACCEPTABLE Diam of Arc Spot Nugget

Test Conducted By BRAD WELLS Per AWS D1.3-98
Laboratory Test No. Date of Test 4-15-08

The undersigned certify that the statements in this record are correct and that the test welds were prepared and tested in accordance with the requirements of 4.6 of ANSI/AWS D1.3 (98 (year)), *Structural Welding Code—Sheet Steel.*

Company Authorized By

Form A-3

J

SAMPLE FORM FOR WELDER AND WELDING OPERATOR QUALIFICATION TEST RECORD

Welder or Welding Operator's Name JEFF LUNT
Identification No. 2258 Qualification Date 4-15-08
Welder's Social Security No.

In Accordance with WPS No. D1.3-001 Revision 001
Welding Process(es) GMAW Type MANUAL (Automatic, manual, etc.)
Mode of Transfer for GMAW SHORT CIRCUITING (Short circuiting, spray, globular)

VARIABLE	ACTUAL VARIABLE USED IN QUAL.	QUALIFICATION RANGE
JOINT		
Joint Type	4.2A	4.2 A
Backing Material Type		
Groove Welded From: one side or both sides		
BASE METAL (4.7.1.1)		
Material Specification		
Sheet Steel	A36 to A36	SEE AWS to 1.2.1
Supporting Steel		
Sheet Thickness (4.7.2.1)		
Groove		
Fillet	10 GUAGE	16 guage → 3/16"
Arc Plug		
Arc Spot		
Arc Seam		
COATING(S)	NONE	NONE
Type		
Thickness		
POSITION (4.7.1.5 and 4.7.1.6)		
Groove		
Fillet	2F	1F, 2F
Arc Plug		
Arc Spot		
Arc Seam		
Progression		
GAS (4.7.1.4)	75%AR - 25% CO_2	75%AR - 25% CO_2
ELECTRODE (4.7.1.3 and 4.7.1.4)		
Size	.035	.035
Group Designation	E70S-6	E70S-X

VISUAL EXAMINATION RESULTS (4.6)
Specimen 1 ACCEPTABLE Specimen 2 ACCEPTABLE
Appearance ACCEPTABLE Cracks NONE Undercut NONE
Reinforcement ACCEPTABLE Diam of Arc Spot Nugget

Test Conducted By BRAD WELLS Per AWS D1.3-98
Laboratory Test No. Date of Test 4-15-08

The undersigned certify that the statements in this record are correct and that the test welds were prepared and tested in accordance with the requirements of 4.6 of ANSI/AWS D1.3 (98 (year)), *Structural Welding Code—Sheet Steel.*

Company Authorized By

Form A-3

SAMPLE FORM FOR WELDER AND WELDING OPERATOR QUALIFICATION TEST RECORD

Welder or Welding Operator's Name SOLOMON GAY
Identification No. 9877 Qualification Date 4-15-08
Welder's Social Security No.

In Accordance with WPS No. D13-001 Revision 1
Welding Process(es) GMAW-S Type MANUAL (Automatic, manual, etc.)
Mode of Transfer for GMAW SHORT CIRCUITING (Short circuiting, spray, globular)

VARIABLE	ACTUAL VARIABLE USED IN QUAL.	QUALIFICATION RANGE
JOINT		
Joint Type	3G - 4.1	4.1
Backing Material Type		
Groove Welded From: one side or both sides	ONE SIDE	EITHER
BASE METAL (4.7.1.1)		
Material Specification		
Sheet Steel	A36 to A36	SEE AWS TO 1.2.1
Supporting Steel		
Sheet Thickness (4.7.2.1)		
Groove	10 GUAGE	16 G → 3/16"
Fillet		
Arc Plug		
Arc Spot		
Arc Seam		
COATING(S)	NONE	NONE
Type		
Thickness		
POSITION (4.7.1.5 and 4.7.1.6)		
Groove	3G	1G, 2G, 3G
Fillet		
Arc Plug		
Arc Spot		
Arc Seam		
Progression		
GAS (4.7.1.4)	75% ARGON - 25% CO_2	75% AR - 25% CO_2
ELECTRODE (4.7.1.3 and 4.7.1.4)		
Size	.035	.035
Group Designation	E70S-6	E70S-X

VISUAL EXAMINATION RESULTS (4.6)
Specimen 1 ACCEPTABLE Specimen 2 ACCEPTABLE
Appearance ACCEPTABLE Cracks NONE Undercut NONE
Reinforcement ACCEPTABLE Diam of Arc Spot Nugget

Test Conducted By BRAD WELLS Per AWS D1.3-98
Laboratory Test No. Date of Test 4-15-08

The undersigned certify that the statements in this record are correct and that the test welds were prepared and tested in accordance with the requirements of 4.6 of ANSI/AWS D1.3 (98 (year)), *Structural Welding Code—Sheet Steel.*

Company Authorized By

Form A-3

SAMPLE FORM FOR WELDER AND WELDING OPERATOR QUALIFICATION TEST RECORD

Welder or Welding Operator's Name JEFF LUNT
Identification No. 2658 Qualification Date 4-15-08
Welder's Social Security No.

In Accordance with WPS No. D1.3-001 Revision 1
Welding Process(es) GMAW Type MANUAL (Automatic, manual, etc.)
Mode of Transfer for GMAW SHORT CIRCUITING (Short circuiting, spray, globular)

VARIABLE	ACTUAL VARIABLE USED IN QUAL.	QUALIFICATION RANGE
JOINT		
Joint Type	3G - 4.1	4.1
Backing Material Type		
Groove Welded From: one side or both sides	ONE SIDE	EITHER
BASE METAL (4.7.1.1)		
Material Specification		
Sheet Steel	A36 to A36	SEE AWS to 1.2.1
Supporting Steel		
Sheet Thickness (4.7.2.1)		
Groove	10 GUAGE	16guage → 3/16"
Fillet		
Arc Plug		
Arc Spot		
Arc Seam		
COATING(S)	NONE	NONE
Type		
Thickness		
POSITION (4.7.1.5 and 4.7.1.6)		
Groove	3G	1G, 2G, 3G
Fillet		
Arc Plug		
Arc Spot		
Arc Seam		
Progression		
GAS (4.7.1.4)	75% ARGON / 25% CO_2	75% Ar - 25% CO_2
ELECTRODE (4.7.1.3 and 4.7.1.4)		
Size	.035	.035
Group Designation	E705-6	E705-X

VISUAL EXAMINATION RESULTS (4.6)
Specimen 1 ACCEPTABLE Specimen 2 ACCEPTABLE
Appearance ACCEPTABLE Cracks NONE Undercut NONE
Reinforcement ACCEPTABLE Diam of Arc Spot Nugget

Test Conducted By BRAD WELLS Per AWS D1.3-98
Laboratory Test No. Date of Test 4-15-08

The undersigned certify that the statements in this record are correct and that the test welds were prepared and tested in accordance with the requirements of 4.6 of ANSI/AWS D1.3 (98 (year)), *Structural Welding Code—Sheet Steel.*

Company Authorized By

Form A-3

SAMPLE FORM FOR WELDER AND WELDING OPERATOR QUALIFICATION TEST RECORD

Welder or Welding Operator's Name: Ben Berry
Identification No.: 8281 Qualification Date: 4-15-08
Welder's Social Security No.:

In Accordance with WPS No.: D1.3-001 Revision: 1
Welding Process(es): GMAW Type: MANUAL (Automatic, manual, etc.)
Mode of Transfer for GMAW: SHORT CIRCUITING (Short circuiting, spray, globular)

VARIABLE	ACTUAL VARIABLE USED IN QUAL.	QUALIFICATION RANGE
JOINT		
Joint Type	3G - 4.1	4.1
Backing Material Type		
Groove Welded From: one side or both sides	ONE SIDE	EITHER
BASE METAL (4.7.1.1)		
Material Specification		
Sheet Steel	A36 to A36	SEE AWS to 1.2.1
Supporting Steel		
Sheet Thickness (4.7.2.1)		
Groove	10 GUAGE	16guage → 3/16"
Fillet		
Arc Plug		
Arc Spot		
Arc Seam		
COATING(S)	NONE	NONE
Type		
Thickness		
POSITION (4.7.1.5 and 4.7.1.6)		
Groove	3G	1G, 2G, 3G
Fillet		
Arc Plug		
Arc Spot		
Arc Seam		
Progression		
GAS (4.7.1.4)	75% ARGON / 25% CO_2	75% AR - 25% CO_2
ELECTRODE (4.7.1.3 and 4.7.1.4)		
Size	.035	.035
Group Designation	E705-6	E705-X

VISUAL EXAMINATION RESULTS (4.6)
Specimen 1: ACCEPTABLE Specimen 2: ACCEPTABLE
Appearance: ACCEPTABLE Cracks: NONE Undercut: NONE
Reinforcement: ACCEPTABLE Diam of Arc Spot Nugget:

Test Conducted By: Brad Wells Per: AWS D1.3-98
Laboratory Test No.: Date of Test: 4-15-08

The undersigned certify that the statements in this record are correct and that the test welds were prepared and tested in accordance with the requirements of 4.6 of ANSI/AWS D1.3 (98 (year)), *Structural Welding Code—Sheet Steel.*

Company: Authorized By:

Form A-3

BECKER
structural engineers, inc.

05120

OBSERVATION REPORT
Structural Steel

Date:	July 6, 2009
Time:	7:30am – 9:00 am
Temp:	60's
Weather:	Sunny

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI 2058

Observation Location:
Holdown anchor supports and deck edge configuration at the second floor slab. This review was completed in preparation of the slab placement later this week.

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Bolt Condition	☐	☐	☐	☒	
Weld Condition	☐	☐	☐	☒	
Anchor Bolts, Nuts, & Washers	☐	☒	☐	☐	See notes below
Grout/Leveling Plates	☐	☐	☐	☒	
Fit Up/Plumbness	☐	☐	☐	☒	
Metal Deck Welds	☒	☐	☐	☐	
Pour Stops	☐	☒	☐	☐	See notes below
Bracing	☐	☐	☐	☒	
Additional Items	☐	☐	☐	☒	
Additional Items	☐	☐	☐	☒	

Notes:
This visit was conducted to review holdown supports and deck edge details prior to the second floor placement. Holdown support locations were not measured, but were found in the general area expected. It was noted that Section 1/S3.2 was not completed at all necessary areas. At all perimeter locations where steel joists bear on steel beams , Section 1/S3.2 shall be followed. Specifically, a continuous steel angle shall be placed along with HSS 2 1/2 sections between joists. Anchor rods shall be welded to this

angle as specified. This item was discussed with the General Contractor, and areas requiring attention were identified.

Also, at locations where the steel joists span parallel with the steel perimeter beams, a continuous pour stop can be used in lieu of the steel angle, and the anchor rods shall be welded to the top flange of the beam as specified in Section 2/S3.2.

Please contact me with any questions.

Thank you -

Signed: James Fortin, P.E.

75 York Street, Portland, ME 04101-4701 ■ Tel 207-879-1838 ■ Fax 207-879-1822

BECKER
structural engineers, inc.

05120

OBSERVATION REPORT
Structural Steel

Date:	July 27, 2009
Time:	8:30am - 11:00 am
Temp:	70's - 80's
Weather:	Clouds, Sun, Humid

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI2058

Observation Location:
Structural Steel Framing at First Floor and Second Floor

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Bolt Condition	☒	☐	☐	☐	
Weld Condition	☐	☐	☐	☒	See Notes Below
Anchor Bolts, Nuts, & Washers	☒	☒	☐	☐	See Notes Below for areas requiring attention
Grout/Leveling Plates	☒	☐	☐	☐	
Fit Up/Plumbness	☒	☐	☐	☐	
Metal Deck Welds	☐	☐	☐	☒	
Pour Stops	☐	☐	☐	☒	
Bracing	☒	☒	☐	☐	See Notes Below for areas requiring attention
Additional Items	☐	☐	☐	☐	
Additional Items	☐	☐	☐	☐	

Notes:
A final review of the structural steel framing was completed to ensure that framing configurations, connection, and details have been incorpoarated as shown on the contract documents. Framing appears to be in general conformance with the contract drawings except as noted below. These items will require additional attention by the General Contractor to meet the drawing requirements. Contractor shall contact engineer to schedule follow-up visit once items have been addressed.

1. Diagonal struts at brace frame diagonal connections are missing or incorrecly installed. The angle shall extend from the base of the brace gusset plate to the underside of the deck, as shown on Section D/S1.7
2. At locations where joist bridging teminates at the concrete wall, the clip angle currently in place shall be bolted to the wall.
3. CMU walls at the stairs and elevators shall be attached to all steel beams located directly adjacent to the walls as shown on Section 5/S3.2. (all locations)
4. Column/brace anchor bolt attachments have been reviewed at all visible loactions. At Grids 9/N & 11/N, nuts were found missing.
5. All perimeter floor beams shall have bottom flange angle braces at 5'-0" on center maximum as identified in Sections 1 & 2 on S3.2.
6. GC is requested to submit final reports for field weld inspections.
7. At vertical HSS hangers that will support canopy framing, a diagonal angle brace shall be placed at each hanger per SSK-2 and Section 2/S3.1. *Note: Canopy framing is not in place and will be reviewed at a future visit.*
8. Anchor bolts not in place at first floor slab along Line Line A & Line 7 per Sections 1 & 2 on Dwg. S3.2. GC shall forward proposed repair to address this item to Engineer for review,
9. At second floor framing, W14x22 between Lines 1 & 3 east of Line E, double angle connection to W16 support beam has 3 rows of bolt holes. Bottom row of holes in angles do not have corresponding holes in W16 web. Holes shall be drilled in web for bolt installation.
10. At second floor, W16x31 along Line 3 from Line D.2 to E.8, the web stiffener at the gusset plate centerline is not in place. The connection of the W12/W10 beams interfere with installation of a full depth stiffener. Instead, a stiffener shall be placed at each side of the beam web from the beam bottom flange to the underside of the angle connection.

Please contact me with any questions reagrding the information provided above.

Thank you -

Signed: James Fortin, P.E.

75 York Street, Portland, ME 04101-4701 ■ Tel 207-879-1838 ■ Fax 207-879-1822

BECKER
structural engineers, inc.

05120

OBSERVATION REPORT
Structural Steel

Date:	August 13, 2009
Time:	8:00am - 11:00 am
Temp:	80's
Weather:	Sunny, Humid

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI2058

Observation Location:
Structural Steel Framing at First Floor and Second Floor (follow-up visit to review completion of previously documented items)

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Bolt Condition	☒	☐	☐	☐	
Weld Condition	☐	☐	☐	☒	
Anchor Bolts, Nuts, & Washers	☒	☐	☐	☐	
Grout/Leveling Plates	☐	☐	☐	☒	
Fit Up/Plumbness	☐	☐	☐	☒	
Metal Deck Welds	☐	☐	☐	☒	
Pour Stops	☐	☐	☐	☒	
Bracing	☒	☐	☐	☐	
Additional Items	☐	☒	☒	☐	See Notes Below
Additional Items	☐	☐	☐	☐	

Notes:
A follow-up review of the structural steel framing was completed to review completion of previously documented items. The following list was submitted following the initial steel inspection on July 27, 2009. Updates to the items are provided in bold.

1. Diagonal struts at brace frame diagonal connections are missing or incorrecly installed. The angle shall extend from the base of the brace gusset plate to the underside of the deck, as shown on Section D/S1.7
08/13/09: Areas have been reviewed and new angle braced have been installed that extend to the base of the brace frame gusset plate. <u>This item is CLOSED.</u>

75 York Street, Portland, ME 04101-4701 ■ Tel 207-879-1838 ■ Fax 207-879-1822

2. At locations where joist bridging teminates at the concrete wall, the clip angle currently in place shall be bolted to the wall.
08/13/09: Clip angles have been bolted to the wall. This item is CLOSED.
3. CMU walls at the stairs and elevators shall be attached to all steel beams located directly adjacent to the walls as shown on Section 5/S3.2. (all locations)
08/13/09: Some areas were found to be complete. Item is on-going.
4. Column/brace anchor bolt attachments have been reviewed at all visible loactions. At Grids 9/N & 11/N, nuts were found missing.
08/13/09: Placement of nuts was reviewed prior to concrete slab placement. Nuts were installed. This item is CLOSED.
5. All perimeter floor beams shall have bottom flange angle braces at 5'-0" on center maximum as identified in Sections 1 & 2 on S3.2.
08/13/09: Beam bottom flange braces appear to have been installed. This item is CLOSED.
6. GC is requested to submit final reports for field weld inspections.
08/13/09: Reports have not been submitted.
7. At vertical HSS hangers that will support canopy framing, a diagonal angle brace shall be placed at each hanger per SSK-2 and Section 2/S3.1. *Note: Canopy framing is not in place and will be reviewed at a future visit.*
08/13/09: Canopy framing has not been installed.
8. Anchor bolts not in place at first floor slab along Line Line A & Line 7 per Sections 1 & 2 on Dwg. S3.2. GC shall forward proposed repair to address this item to Engineer for review.
08/13/09: Simpson Titen HD anchors have been installed. This item is CLOSED.
9. At second floor framing, W14x22 between Lines 1 & 3 east of Line E, double angle connection to W16 support beam has 3 rows of bolt holes. Bottom row of holes in angles do not have corresponding holes in W16 web. Holes shall be drilled in web for bolt installation.
08/13/09: Bolts have been added. This item is CLOSED.
10. At the first and second floor, W16 beam along Line 3 from Line D.2 to E.8, the web stiffener at the gusset plate centerline is not in place. The connection of the W12/W10 beams interfere with installation of a full depth stiffener. Instead, a stiffener shall be placed at each side of the beam web from the beam bottom flange to the underside of the angle connection.
08/13/09: This item is not complete. Note that this occurs at both the first and second floor level.

Please contact me with any questions regarding the information provided above.

Thank you -

Signed: James Fortin, P.E.

BECKER
structural engineers, inc.

05120

OBSERVATION REPORT
Structural Steel

Date:	December 11, 2009
Time:	1:00pm – 2:30 pm
Temp:	20's
Weather:	Sunny

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI 2058

Observation Location: Exterior Canopy Framing (4 locations)

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Bolt Condition	☐	☒	☐	☐	
Weld Condition	☐	☒	☐	☐	
Anchor Bolts, Nuts, & Washers	☐	☐	☐	☒	
Grout/Leveling Plates	☐	☐	☐	☒	
Fit Up/Plumbness	☐	☐	☐	☒	
Metal Deck Welds	☐	☐	☐	☐	
Pour Stops	☐	☒	☐	☒	
Bracing	☐	☐	☐	☒	
Additional Items	☐	☒	☐	☐	Only as Noted Below
Additional Items	☐	☐	☐	☐	

Notes:

This visit was limited to review of the four exterior steel framed canopies. Overall, the framing appears to be installed in general conformance with drawing requirements. The following items were noted:

<u>All Canopies:</u>

1. Perimeter channel flanges do not align at the mitered corners
2. Welds at the mitered corners are not complete. Welds should also be ground smooth for uniform appearance.
3. All field weld shall be inspected by an AWS Certified Weld Inspector with a report submitted to Becker Structural Engineers for record.

Canopy along Line N - North:

4. The revised connection at Grid 13/N does not appear to have been completed as directed (instructions provided in November 19 report). The fillet weld at the south side of the plate is not complete, and the full penetration bevel weld at the top of the plate does not appear to have been placed. This welding is necessary for adequate attachment.

   From the November 19 report: At the Valley Street canopy, the MC 18 connection to the Second Floor Beam connection plate at Line 13 is not fitting up correctly. The connection plate appears rotated slightly such that the bolt holes do not line up. The item was discussed with the steel erector, and in lieu of fully bolting the connection, three bolt holes will be elongated slightly such that a minimum of (1) bolt in each row will be placed. Then the south side and the top of the MC18 connection plate will be welded to the W14 connection plate. Use a ¼" fillet weld along the south side, and a full penetration beveled weld along the top. These field welds require visual inspection from an AWS Certified Weld Inspector, with a report submitted to Becker Structural Engineers for Record.

Canopy along Line N & Line 1:

5. Bolts are missing at five locations connecting the C15 channels located against the exterior brick veneer. In lieu of bolting, the channel has been welded to the tab connection plate. These field welds shall be inspected by an AWS Certified Weld Inspector to ensure adequacy.

Canopy along Line 1 - West:

6. The bolts through the CMU wall are not adequately installed for connection of the tension rod to the CMU. Two bolts have missing nuts, and these bolts were spot welded to the steel plate at the inside face of the wall. This was discussed with Everett Stewart of Ganneston Construction, and will be repaired as follows: Shore the canopy, grind out out plug welds, and remove the nuts and plate at the interior side of the CMU wall. Cut away a minimal section of the wall as necessary to expose the bolts, and install coupler nuts to extend a threaded rod for proper attachment of the plate. Grout around the coupler nuts flush with the inside face of the wall, and replace the plate.

Please contact me with any questions.

Thank you -

Signed: James Fortin, P.E.

BECKER
structural engineers, inc.

05120

OBSERVATION REPORT
Structural Steel

Date:	January 13, 2010
Time:	N/A
Temp:	N/A
Weather:	N/A

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI 2058

Observation Location: Exterior Canopy Framing – THIS REPORT ISSUED TO PROVIDE UPDATED INFORMATION ON THE PREVIOUSLY LISTED ISSUES.

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Bolt Condition	☐	☒	☐	☐	
Weld Condition	☐	☒	☐	☐	
Anchor Bolts, Nuts, & Washers	☐	☐	☐	☒	
Grout/Leveling Plates	☐	☐	☐	☒	
Fit Up/Plumbness	☐	☐	☐	☒	
Metal Deck Welds	☐	☐	☐	☐	
Pour Stops	☐	☐	☐	☒	
Bracing	☐	☐	☐	☒	
Additional Items	☐	☒	☐	☐	Only as Noted Below
Additional Items	☐	☐	☐	☐	

Notes:

This report is issued to provide updated information. A follow-up visit was not completed. Updated information is provided in **bold.**

All Canopies:

1. Perimeter channel flanges do not align at the mitered corners.
2. Welds at the mitered corners are not complete. Welds should also be ground smooth for uniform appearance. **01/13/10: It is necessary that these welds are repaired or replaced to provide a continuous weld between the two channel sections. Following placement, the welds shall be ground for a smooth uniform appearance.**

75 York Street, Portland, ME 04101-4701 ▪ Tel 207-879-1838 ▪ Fax 207-879-1822

3. All field weld shall be inspected by an AWS Certified Weld Inspector with a report submitted to Becker Structural Engineers for record.

Canopy along Line N - North:

4. The revised connection at Grid 13/N does not appear to have been completed as directed (instructions provided in November 19 report). The fillet weld at the south side of the plate is not complete, and the full penetration bevel weld at the top of the plate does not appear to have been placed. This welding is necessary for adequate attachment. **01/13/10: 1) Improved access shall be provided to allow for inspection of the full penetration bevel weld along the top of the plate. This weld is required as specified below. In accordance with project specifications (Section 05120-3.02C.4.b), this weld shall by inspected by non-destructive testing (RT or UT inspection). 2) The skew between the two connection plates does not allow for completion of the 1/4" vertical fillet weld. GC shall document the length of the fillet weld placed, and forward to BSE. 3) Upon further review of the photos taken during the December 11 visit, it was discovered that the metal roof deck is not properly supported adjacent to the MC18 at Line 13. An L2x2x3/16 x 8" long shall be welded to the MC18 at Line 13 to support the deck . Attach per Section 4/S3.1.**

   From the November 19 report: At the Valley Street canopy, the MC 18 connection to the Second Floor Beam connection plate at Line 13 is not fitting up correctly. The connection plate appears rotated slightly such that the bolt holes do not line up. The item was discussed with the steel erector, and in lieu of fully bolting the connection, three bolt holes will be elongated slightly such that a minimum of (1) bolt in each row will be placed. Then the south side and the top of the MC18 connection plate will be welded to the W14 connection plate. Use a ¼" fillet weld along the south side, and a full penetration beveled weld along the top. These field welds require visual inspection from an AWS Certified Weld Inspector, with a report submitted to Becker Structural Engineers for Record.

Canopy along Line N & Line 1:

5. Bolts are missing at five locations connecting the C15 channels located against the exterior brick veneer. In lieu of bolting, the channel has been welded to the tab connection plate. These field welds shall be inspected by an AWS Certified Weld Inspector to ensure adequacy. **01/13/10: At all locations, (1) bolt has been installed. A minimum of a 1/4" fillet weld x 3" long is required between the channel leg and the connection tab plate.**

Canopy along Line 1 - West:

6. The bolts through the CMU wall are not adequately installed for connection of the tension rod to the CMU. Two bolts have missing nuts, and these bolts were spot welded to the steel plate at the inside face of the wall. This was discussed with Everett Stewart of Ganneston Construction, and will be repaired as follows: Shore the canopy, grind out out plug welds, and remove the nuts and plate at the interior side of the CMU wall. Cut away a minimal section of the wall as necessary to expose the bolts, and install coupler nuts to extend a threaded rod for proper attachment of the plate. Grout around the coupler nuts flush with the inside face of the wall, and replace the plate. **01/13/10: Per subsequent conversation for Everett Stewart, an alternate repair was completed. A written explanation of the repairs completed shall be submitted to BSE for review and record. A follow-up inspection can be conducted following receipt of the repair documentation.**

Please contact me with any questions.

Thank you -

Signed: James Fortin, P.E.

BECKER
structural engineers, inc.

OBSERVATION REPORT
Structural Steel

Date:	February 3, 2010
Time:	9:30 AM – 10:00 AM
Temp:	20's
Weather:	Snow Showers

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI 2058

Observation Location: Exterior Canopy Framing – FOLLOW-UP VISIT

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Bolt Condition	☒	☐	☐	☐	
Weld Condition	☐	☒	☐	☐	
Anchor Bolts, Nuts, & Washers	☐	☐	☐	☒	
Grout/Leveling Plates	☐	☐	☐	☒	
Fit Up/Plumbness	☐	☐	☐	☒	
Metal Deck Welds	☐	☐	☐	☐	
Pour Stops	☐	☐	☐	☒	
Bracing	☐	☐	☐	☒	
Additional Items	☐	☐	☐	☐	
Additional Items	☐	☐	☐	☐	

Notes:

This report is issued to provide updated information following a return visit to review the exterior canopy construction. Updated information is provided in **bold.**

All Canopies:

1. Perimeter channel flanges do not align at the mitered corners.
2. Welds at the mitered corners are not complete. Welds should also be ground smooth for uniform appearance. 01/13/10: It is necessary that these welds are repaired or replaced to provide a continuous weld between the two channel sections. Following placement, the welds shall be ground for a smooth uniform appearance. **02/04/10: The latest weld inspection report from QAL, dated 1/19/10, stated that "material other than weld material" was used to fill mitered corners. The corners of the canopy need to be fully welded with**

weld material. An inspection report is necessary certifying that foreign materials have been removed and the welds are satisfactory.

3. All field weld shall be inspected by an AWS Certified Weld Inspector with a report submitted to Becker Structural Engineers for record.

Canopy along Line N - North:

4. **The revised connection at Grid 13/N has been bolted with (5) or the (6) detailed bolts. At the missing bolt, the two plates have been fillet welded with a minimum 6" of 1/4" fillet weld. This condition is adequate as constructed.**

   It is necessary that the metal roof deck is supported adjacent to the MC18 at Line 13 using an L2x2x3/16 x 8" long welded to the MC18 at Line 13 to support the deck . Attach per Section 4/S3.1.

   The welded connection at the mitered joint between the cantilevered channel at Line 13 & the main MC18 canopy channel spanning parallel with the building shall be re-inspected to ensure the weld is complete and acceptable (see Item 2 above). GC shall ensure that weld has been repaired prior to re-inpsection. This connection supports the main channel. In lieu of repairing and re-inspecting this weld, an L2x2x1/4" x 8" long can be placed at the inside of the corner and welded to each channel with 3/16" fillet welds. This welding would require inspection by QAL.

Canopy along Line N & Line 1:

5. Bolts are missing at five locations connecting the C15 channels located against the exterior brick veneer. In lieu of bolting, the channel has been welded to the tab connection plate. These field welds shall be inspected by an AWS Certified Weld Inspector to ensure adequacy. 01/13/10: At all locations, (1) bolt has been installed. A minimum of a 1/4" fillet weld x 3" long is required between the channel leg and the connection tab plate. **02/04/10: QAL inspection report has been provided accepting the weld as identified above. This item is CLOSED.**

Canopy along Line 1 - West:

6. The bolts through the CMU wall are not adequately installed for connection of the tension rod to the CMU. Two bolts have missing nuts, and these bolts were spot welded to the steel plate at the inside face of the wall. This was discussed with Everett Stewart of Ganneston Construction, and will be repaired as follows: Shore the canopy, grind out out plug welds, and remove the nuts and plate at the interior side of the CMU wall. Cut away a minimal section of the wall as

necessary to expose the bolts, and install coupler nuts to extend a threaded rod for proper attachment of the plate. Grout around the coupler nuts flush with the inside face of the wall, and replace the plate. 01/13/10: Per subsequent conversation for Everett Stewart, an alternate repair was completed. A written explanation of the repairs completed shall be submitted to BSE for review and record. A follow-up inspection can be conducted following receipt of the repair documentation. **02/04/10: This item has not been reviewed after completion of above instructions.**

Please contact me with any questions.

Thank you -

Signed: James Fortin, P.E.

BECKER
structural engineers, inc.

OBSERVATION REPORT
Structural Steel

Date:	March 4, 2010
Time:	NA
Temp:	NA
Weather:	NA

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI 2058

Observation Location: Exterior Canopy Framing – Final Report

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Bolt Condition	☒	☐	☐	☐	
Weld Condition	☒	☐	☐	☐	
Anchor Bolts, Nuts, & Washers	☒	☐	☐	☒	
Grout/Leveling Plates	☐	☐	☐	☒	
Fit Up/Plumbness	☒	☐	☐	☐	
Metal Deck Welds	☐	☐	☐	☐	
Pour Stops	☐	☐	☐	☒	
Bracing	☐	☐	☐	☒	
Additional Items	☐	☐	☐	☐	
Additional Items	☐	☐	☐	☐	

Notes:

This report is issued to provide updated information following a return visit to review the exterior canopy construction. Updated information is provided in **bold.**

All Canopies:

1. Perimeter channel flanges do not align at the mitered corners.
2. Welds at the mitered corners are not complete. Welds should also be ground smooth for uniform appearance. 01/13/10: It is necessary that these welds are repaired or replaced to provide a continuous weld between the two channel sections. Following placement, the welds shall be ground for a smooth uniform appearance. 02/04/10: The latest weld inspection report from QAL, dated 1/19/10, stated that "material other than weld material" was used to fill mitered corners. The corners of the canopy need to be fully welded with weld material.

An inspection report is necessary certifying that foreign materials have been removed and the welds are satisfactory. **03/04/10: Repair work has been completed, and inspection report submitted. Item is CLOSED.**

3. All field weld shall be inspected by an AWS Certified Weld Inspector with a report submitted to Becker Structural Engineers for record.

Canopy along Line N - North:

4. The revised connection at Grid 13/N has been bolted with (5) or the (6) detailed bolts. At the missing bolt, the two plates have been fillet welded with a minimum 6" of 1/4" fillet weld. This condition is adequate as constructed.

   It is necessary that the metal roof deck is supported adjacent to the MC18 at Line 13 using an L2x2x3/16 x 8" long welded to the MC18 at Line 13 to support the deck . Attach per Section 4/S3.1.

   The welded connection at the mitered joint between the cantilevered channel at Line 13 & the main MC18 canopy channel spanning parallel with the building shall be re-inspected to ensure the weld is complete and acceptable (see Item 2 above). GC shall ensure that weld has been repaired prior to re-inpsection. This connection supports the main channel. In lieu of repairing and re-inspecting this weld, an L2x2x1/4" x 8" long can be placed at the inside of the corner and welded to each channel with 3/16" fillet welds. This welding would require inspection by QAL. **03/04/10: Inspection report has been submitted. Item is CLOSED.**

Canopy along Line N & Line 1:

5. Bolts are missing at five locations connecting the C15 channels located against the exterior brick veneer. In lieu of bolting, the channel has been welded to the tab connection plate. These field welds shall be inspected by an AWS Certified Weld Inspector to ensure adequacy. 01/13/10: At all locations, (1) bolt has been installed. A minimum of a 1/4" fillet weld x 3" long is required between the channel leg and the connection tab plate. 02/04/10: QAL inspection report has been provided accepting the weld as identified above. This item is CLOSED.

Canopy along Line 1 - West:

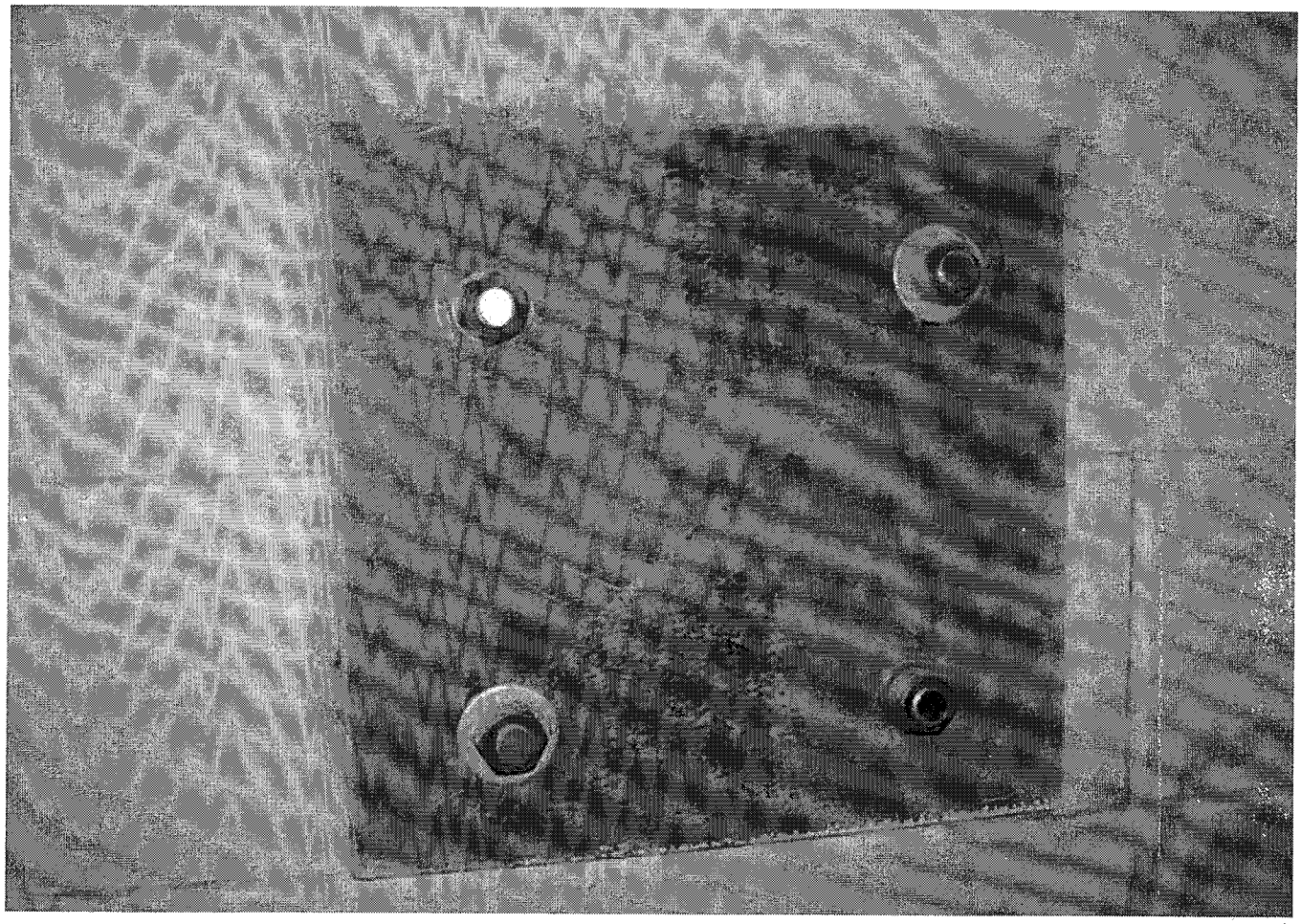
6. The bolts through the CMU wall are not adequately installed for connection of the tension rod to the CMU. Two bolts have missing nuts, and these bolts were spot welded to the steel plate at the inside face of the wall. This was discussed with Everett Stewart of Ganneston Construction, and will be repaired as follows: Shore the canopy, grind out out plug welds, and remove the nuts and plate at the interior side of the CMU wall. Cut away a minimal section of the wall as

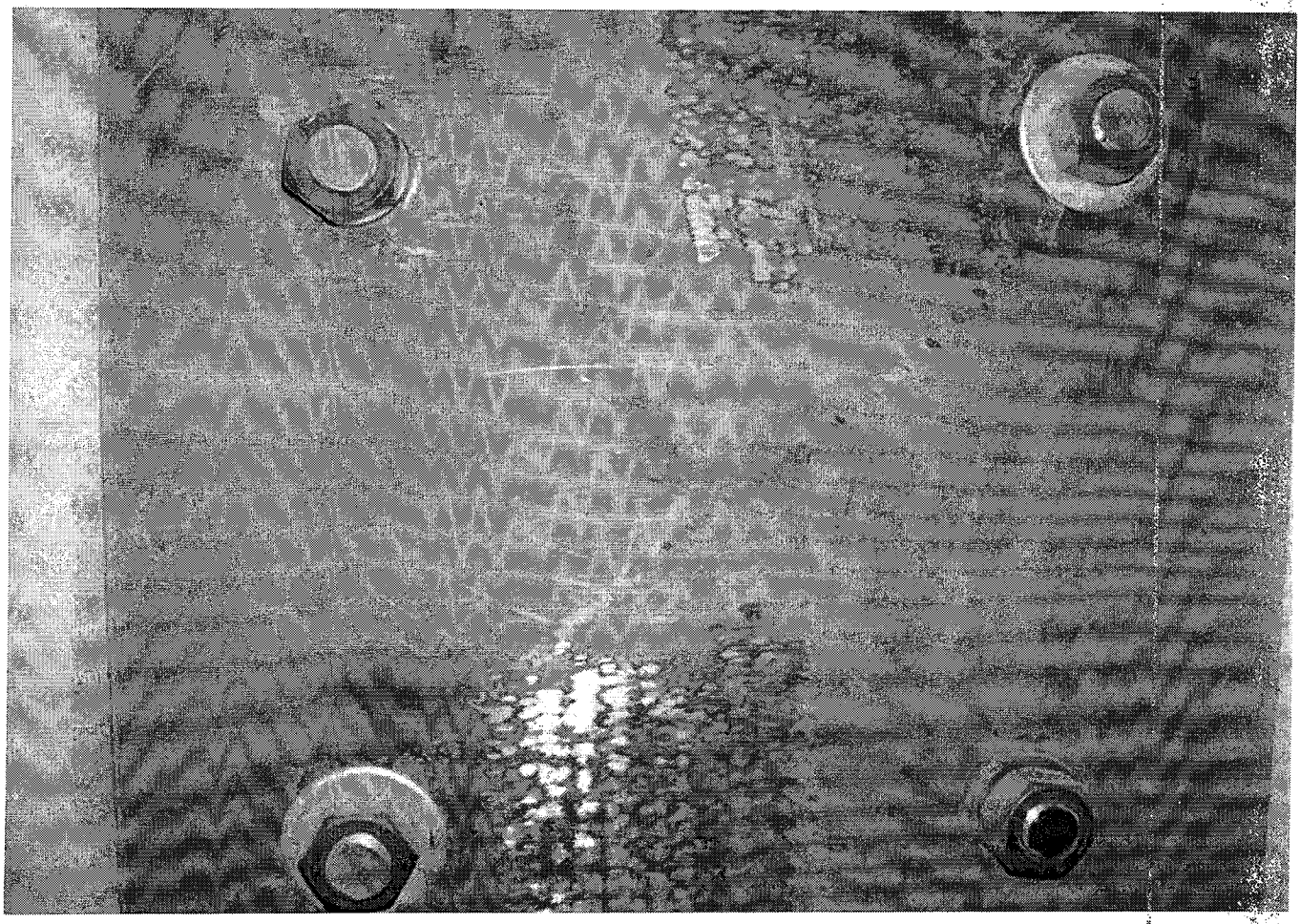
necessary to expose the bolts, and install coupler nuts to extend a threaded rod for proper attachment of the plate. Grout around the coupler nuts flush with the inside face of the wall, and replace the plate. 01/13/10: Per subsequent conversation for Everett Stewart, an alternate repair was completed. A written explanation of the repairs completed shall be submitted to BSE for review and record. A follow-up inspection can be conducted following receipt of the repair documentation. 02/04/10: This item has not been reviewed after completion of above instructions. **03/04/10: Documentation provided by General Contractor along with supporting photographs. Item is CLOSED.**

Please contact me with any questions.

Thank you -

Signed: James Fortin, P.E.





Ganneston Construction Corp
PO Box 27, North Belfast Avenue
Augusta, ME 04332-0027

Tel: (207) 621-8505
Fax: (207) 621-8508

LETTER OF TRANSMITTAL

GCC PROJECT NUMBER
GCC-80677

To:
GAWRON TURGEON ARCHITECTS
29 BLACK POINT ROAD
SCARBOROUGH, MAINE 04074

MONDAY 8:05 AM
Date: February 22, 2010
Attention: WENDI
RE: FLORENCE HOUSE PROJECT

WE ARE SENDING YOU X ATTACHED _____ UNDER SEPARATE COVER

COPIES	NO.	DESCRIPTION
3		COPY OF ENGINEERS REPORT - FLAT PLATE TRUSS REPAIR

REMARKS

Signed: *Michael L. Adams*
Sr. Project Manager

Date: 22-Feb-10

RECEIVED
FEB 2 2 2010

CARPENTER ASSOCIATES

CONSULTING ENGINEERS

February 8, 2010

Mr. Mike Adams
Ganneston Construction Corp.
3025 N. Belfast Avenue
P.O .Box 27
Augusta, ME 04332

Re: Bar Joist Damage

Dear Mike:

Based on the information reviewed, the bar joist repair in question appears to be as designed. The flat plate welded in place should replace the lost strength due to the damage.

Sincerely,

Randy Bragg, P.E

209142

687 STILLWATER AVENUE • OLD TOWN, MAINE 04468• 207-827-8001 • FAX 207-827-8234

Jenner J Helstrom

#	Test Date	Sup	Code	Process	Gas	Filler	Metal	Base Metal	Position	Thickness	Expires
1	02/05/02	G	D1.1	SMAW	N/A	F4		P1	A	U	06/15/09
2	11/29/06	G	B2.1	SMAW	N/A	E316-16		M8	ALL	LIMITED	06/15/09

AWS Certified Welder
Welders, Brazers and Operators

Jenner J Helstrom

Cert # 0203015W

SSN # XXX-XX-3667

1-800-443-9353

Information relating to identification and certification of the bearer of this card may be verified by calling or writing:
CERTIFICATION DEPARTMENT OF THE AMERICAN WELDING SOCIETY
550 N.W. LeJeune Road, Miami, FL 33126

AMERICAN WELDING SOCIETY

VALID ONLY IF ACCOMPANIED BY PHOTO ID

This Card is the property of AWS and shall be returned on demand.

IRONWORKER WELDER CERTIFICATION PROGRAM OF NORTH AMERICA

CERTIFIED WELDER

Jenner Helstrom

Membership #: 1368868

EFFECTIVE DATE **3/22/2009**

EXPIRATION DATE **3/22/2010**

Process: FCAW-S - B1a

GANNESTON CONSTRUCTION CORP.
P.O. BOX 27 AUGUSTA, ME 04332

X	REVIEWED		REVIEWED WITH NOTES RESUBMISSION REQUESTED
	REVIEWED WITH NOTES NO RESUBMISSION		NOT ACCEPTABLE

CHECK MARK INDICATES ACTION TAKEN

CHECKED BY _____ DATE 6/10/09

IRONWORKER WELDER CERTIFICATION PROGRAM OF NORTH AMERICA

CERTIFIED WELDER

Nobert D Page

Membership #: 1256614

EFFECTIVE DATE	EXPIRATION DATE
2/1/2009	2/1/2010

Process: SMAW - A7r

IRONWORKER WELDER CERTIFICATION PROGRAM OF NORTH AMERICA

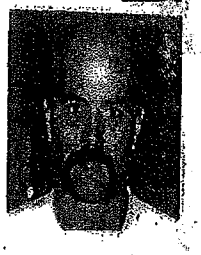
CERTIFIED WELDER

Nobert D Page

Membership #: 1256614

EFFECTIVE DATE	EXPIRATION DATE
6/23/2008	6/23/2009

Process: SMAW - A1



IRONWORKER WELDER CERTIFICATION PROGRAM OF NORTH AMERICA

CERTIFIED WELDER

Nobert D Page

Membership #: 1256614

EFFECTIVE DATE	EXPIRATION DATE
4/5/2009	4/5/2010

Process: FCAW-S - B1a

IRONWORKER WELDER CERTIFICATION PROGRAM OF NORTH AMERICA

CERTIFIED WELDER

Nobert D Page

Membership #: 1256614

EFFECTIVE DATE	EXPIRATION DATE
2/1/2009	2/1/2010

Process: SMAW - A7

IRONWORKER WELDER CERTIFICATION PROGRAM OF NORTH AMERICA

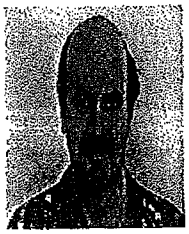
CERTIFIED WELDER

Nobert D Page

Membership #: 1256614

EFFECTIVE DATE	EXPIRATION DATE
2/1/2009	2/1/2010

Process: SMAW - A7a



James C Fortin

From: Paul Becker
Sent: Thursday, June 18, 2009 12:25 PM
To: James C Fortin
Subject: Fwd: Florence House 6-16-09

Sent from my iPhone

Begin forwarded message:

From: Darrell Gilman <dgilman@summitenv.com>
Date: June 18, 2009 7:55:57 AM EDT
To: Paul Becker <paul@beckerstructural.com>
Subject: Florence House 6-16-09

June 17, 2009

Summit Geo. Tech.

640 Main Street

Lewiston, Maine 04240

Ref: The Florence House - Project 14194

QAL-09-1011

Attn: Darrel Gilman

Dear Sir,

On 6/17/09, a site visit was made to the Florence House project in Portland. The following items were inspected:

1st Floor Framing – Lines 1-16, A-N

Bolts – In progress

Bracing connections – In progress

Joist connections – In progress

Bridging connections – In progress

Decking welds and fasteners – In progress

Embed connections – In progress

Note: Beams to embed connections on 1st floor framing at line N, 1.5-5 are at wrong elevation. Need RFI for relocation.

All welding which was in progress was IAW AWS D1.1 and applicable site DWGS.

Best Regards,

Arthur Gallant

CWI# 90100091

Quality Assurance Labs Inc.
NON-DESTRUCTIVE TESTING AND INSPECTION SERVICES
80 PLEASANT AVENUE • SOUTH PORTLAND, MAINE 04106 • TEL: (207) 799-8911 • FAX: (207) 799-7251

INSPECTION REPORT

CUSTOMER: SUMMIT GEO TECH ENG. PAGE 1 OF 1

ADDRESS: AUGUSTA , ME.

ATTENTION: DARREL GILMAN

COPIES:

PROJECT: FLORENCE HOUSE - PORTLAND , ME.

OWNER:

CONTRACTOR: GANNESTON CONSTRUCTION CORP.

JOB No.: 14194 | REPORT No.: QAL-09-1166 | P. O. NUMBER: | DATES INSPECTED: 07 - 06 - 09

REMARKS

>>>>> SITE VISIT TO PERFORM VISUAL INSPECTION OF STRUCTURAL STEEL CONNECTIONS : GRID LINES 1 - 7 , A - N ALL ELEVATIONS .

> REF. PREVIOUS FIELD REPORT DATED 06 - 29 - 09 .
A) ALL DECKING ATTACHMENTS NOW COMPLETE .
B) SECOND LEVEL JOIST AND BRIDGING CONNECTIONS NOW COMPLETE .
C) FIRST LEVEL PLAN SHOWS (1) LOWER HSS WIND BRACE WELD MISSING AT LOCATION 1 - G AS MARKED WITH SOAP STONE . ALL REMAINING WIND BRACE CONNECTIONS COMPLETED FOR ALL LEVELS .

> LOCATION 13 - N (2ND) LEVEL FRAMING PLAN SHOWS COLUMN TO W16X26 SHEAR TAB TO BEAM BOTTOM FLANGE WITH (2) MISSING BOLTS . ALL BEAM WEB BOLTS COMPLETE .

> SEVERAL COLUMN BASE ANCHOR BOLTS REQUIRE FINAL TORQUE .

> CANOPYS IN PROGRESS .

COMPLETED ITEMS COMPLY WITH SITE DOCUMENTS AND AWS D1.1 , D1.3 REQUIREMENTS FOR VISUAL ACCEPTANCE .

END ITEMS /////

MICHAEL W. DREW
CWI # 99050211
QC1 EXP: 06/01/11

FAA REPAIR STATION NUMBER RX5R187N
METHOD(S),PROCESS(ES),PROCEDURE(S) MERCURY FREE

ADDITIONAL INFORMATION - SEE ATTACHED: ☐ SKETCH(ES) ☐ SUPPLEMENTARY SHEET(S) ☐ NDT REPORTS ☐ VIDEO

SIGNATURES	CERTIFICATION	LEVEL	DATE M D Y
INSPECTOR M. Drew CWI # 99050211	ASNT	II	07 07 09
SUPERVISOR			

(FORM Q703 REV 0)

Quality Assurance Labs Inc.
NON-DESTRUCTIVE TESTING AND INSPECTION SERVICES
80 PLEASANT AVENUE • SOUTH PORTLAND, MAINE 04106 • TEL: (207) 799-8911 • FAX: (207) 799-7251

INSPECTION REPORT

CUSTOMER:	Summit Environmental Eng.	PAGE 1	OF 1
ADDRESS:	Augusta, ME		
ATTENTION:	Darrel Gilman		
COPIES:	file		
PROJECT:	Florence House - Portland, ME.		
OWNER:			
CONTRACTOR:	Ganneston Construction Corp.		

JOB No.:	REPORT No.:	P. O. NUMBER:	DATES INSPECTED:
14194	QAL-09-2174		12/16/2009

REMARKS

Performed Visual Inspection at two (2) locations.

At Canopy along Line N:
The MC18 connection plate had 12" of fillet weld varying in size from 1/4" to 0" along vertical side. The weld at top of plate could not be verified as to effective throat nor quality.

At Canopy along Line N & Line 1:
C15 connections were missing bolts. In lieu of bolts, the shear tab was welded to channel.

Acceptability can not be determined without the corrective actions being approved by EOR.

FAA REPAIR STATION NUMBER RX5R187N
METHOD(S),PROCESS(ES),PROCEDURE(S) MERCURY FREE

ADDITIONAL INFORMATION - SEE ATTACHED: ☐ SKETCH(ES) ☐ SUPPLEMENTARY SHEET(S) ☐ NDT REPORTS ☐ VIDEO

SIGNATURES		CERTIFICATION	LEVEL	DATE M	D	Y
INSPECTOR	R. H. Parechanian, Jr. CWI# 90100111	ASNT	II	01	12	10
SUPERVISOR						

(FORM Q703 REV 0)

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INSPECTION REPORT

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ADDRESS:	Augusta, ME		
ATTENTION:	Darrel Gilman		
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PROJECT:	Florence House - Portland, ME		
OWNER:			
CONTRACTOR:	Ganneston Construction Corp.		
JOB No.: 14194	REPORT No.: QAL-10-0100	P. O. NUMBER:	DATES INSPECTED: 01/19/2010

REMARKS

Performed Visual Inspection in accordance with AWS D1.1 2008 and AISC 360-05.

Canopy along Line N & Line 1: Welds were found to be acceptable.

Canopy along Line N - North: The required full penetration bevel weld at the top were the two plates come together is not possible to perform because of configuration.
As reported on QAL-09-2174, the fillet weld along skewed connection plate is 12" long and varies in size from 1/4" to 0".
These conditions do not meet the requirements of Becker Structural Engineers, Inc. letter dated January 13, 2010.

Welds at mitered corners: Mitered corners were not ground to a smooth uniform appearence. Material other than weld metal was used to fill mitered corners and ground. Unable to determine how much weld is at each corner.

Damaged Joist: Damaged joist were inspected and found to be acceptable per sketch provided by Ganneston.

FAA REPAIR STATION NUMBER RX5R187N
METHOD(S),PROCESS(ES),PROCEDURE(S) MERCURY FREE

ADDITIONAL INFORMATION : SHS ATTACHED: ☐ SKETCH(ES) ☐ SUPPLEMENTARY SHEET(S) ☐ NDT REPORTS ☐ VIDEO

SIGNATURES		CERTIFICATION	LEVEL	DATE M D Y
INSPECTOR	R. H. Parechanian, Jr. CWI# 90100111	ASNT	II	01 | 26 | 10
SUPERVISOR				

(FORM 0703 REV 0)

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INSPECTION REPORT

CUSTOMER:	Summit Environmental Eng.	PAGE 1	OF 1
ADDRESS:	Augusta, ME		
ATTENTION:	Darrell Gilman		
COPIES:	file		
PROJECT:	Florence House - Portland, ME		
OWNER:			
CONTRACTOR:	Ganneston Construction Corp.		

JOB No.:	REPORT No.:	P. O. NUMBER:	DATES INSPECTED:
14194	QAL-10-0264		02/17/2010

REMARKS

Performed Visual Inspection in accordance with AWS D1.1 2008 and AISC 360-05.
Spoke with James Fortin, Becker Eng. before performing inspection to determine scope of inspection.

Canopy along Valley Street:
At this canopy there are three (3) issues.
First issue is the installation of an angle to support the roof decking at the inside corner. The angle was installed with a fillet weld along the toe of the angle and a fillet weld along the vertical leg.
The welds on the vertical are rejected for poor quality.
The weld along the toe has 4" of 3/16" fillet weld which is acceptable. The balance of weld, approximately 4", is rejectable for poor quality.

Second issue is the installation of an angle at the outside corner. The welding and installation are acceptable.

Third issue was to determine the amount of weld at the mitered corners of channels. The only method to determine this was to remove some of the "liquid weld" until weld metal was found. The first flange inspected revealed there was no weld metal at this joint for at least 3". The other flanges revealed similar conditions.
The "liquid metal" was removed at the mitered outside corner of the channel web. It appears a sealing weld was installed and the balance was filled with the "liquid metal".

Canopy along the main entrance:
The one issue on this canopy was to determine the amount of weld at the mitered corners of channels. The inspection revealed that there was little to no weld metal for most of the joint and was similar to the Valley Street canopy.

Both canopies exhibited cracks forming in the "liquid metal" which could be seen from ground level.
A verbal report was given to James Fortin by phone. It was determined at this time that the 4" of weld on the deck angle support was adequate structurally and would be acceptable.
It was also determined that the mitered flanges would require repair.

FAA REPAIR STATION NUMBER RX5R187N
METHOD(S),PROCESS(ES),PROCEDURE(S) MERCURY FREE

ADDITIONAL INFORMATION - SEE ATTACHED: ☐ SKETCH(ES) ☐ SUPPLEMENTARY SHEET(S) ☐ NDT REPORTS ☐ VIDEO

SIGNATURES		CERTIFICATION	LEVEL	DATE M D Y
INSPECTOR	R. H. Parechanian, Jr. CWI# 90100111	ASNT	II	02 19 10
SUPERVISOR				

(FORM Q703 REV 0)

Quality Assurance Labs Inc.
NON-DESTRUCTIVE TESTING AND INSPECTION SERVICES
80 PLEASANT AVENUE • SOUTH PORTLAND, MAINE 04106 • TEL: (207) 799-8911 • FAX: (207) 799-7251

INSPECTION REPORT

CUSTOMER:	Summit Environmental Eng.	PAGE 1 OF 1	
ADDRESS:	Augusta, ME		
ATTENTION:	Darrel Gilman		
COPIES:	file		
PROJECT:	Florence House - Portland, ME		
OWNER:			
CONTRACTOR:	Ganneston Construction Corp.		
JOB No.: 14194	REPORT No.: QAL-10-0345	P. O. NUMBER:	DATES INSPECTED: 03/03/2010

REMARKS

Performed Visual Inspection in accordance with AWS D1.1 2008 and AISC 360-05.

Inspection was performed on a total of four (4) canopies.
All four canopies now have angles welded to channel webs at the inside corners. Angles are welded at toes of angles for a length of approximately 6". Welds are a minimum 3/16" fillet weld with acceptable quality.

All mitered flanges have been cleaned and welded. Welds have been ground reasonably smooth. Welds are of acceptable quality.

All mitered webs have been cleaned and additional weld metal deposited on the outside. Welds have been ground reasonably smooth. Welds are of acceptable quality.

At both flange and web miters, filler has been used to fill minor irregularities. Filler was ground to a smooth uniform appearance.

FAA REPAIR STATION NUMBER RX5R187N
METHOD(S),PROCESS(ES),PROCEDURE(S) MERCURY FREE

ADDITIONAL INFORMATION - SEE ATTACHED: ☐ SKETCH(ES) ☑ SUPPLEMENTARY SHEET(S) ☐ NDT REPORTS ☐ VIDEO

SIGNATURES	CERTIFICATION	LEVEL	DATE M D Y
INSPECTOR R. H. Parechanian, Jr. CWI# 90100111	ASNT	II	03 | 03 | 10
SUPERVISOR			

(FORM Q703 REV 0)

BECKER
structural engineers, inc.

MILL CERTIFICATIONS

PROJECT FLORENCE HOUSE

STRUCTURAL STEEL	☒RECEIVED	DATE: 7-13-09 ☐NOT RECEIVED
BOLTS	☒RECEIVED	DATE: 7-13-09 ☐NOT RECEIVED
WELD FILLER	☒RECEIVED	DATE: ☒NOT RECEIVED

ITEMS ABOVE MARKED "RECEIVED" HAVE NOT BEEN INCLUDED IN THIS REPORT DUE TO THE LARGE VOLUME. HARD COPIES ARE AVAILABLE UPON REQUEST.

SPECIAL INSPECTOR: James Fortin, P.E. DATE: 3-04-10

75 York Street, Portland, ME 04101-4701 ■ Tel 207-879-1838 ■ Fax 207-879-1822

EXHIBIT B

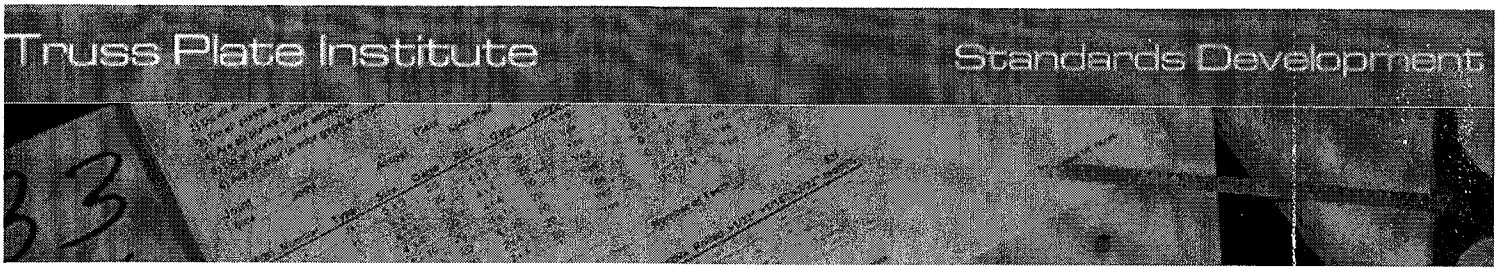
06100 Wood Construction

Project: Florence House
Date Prepared: September 2, 2008

Structural Schedule of Special Inspection Services

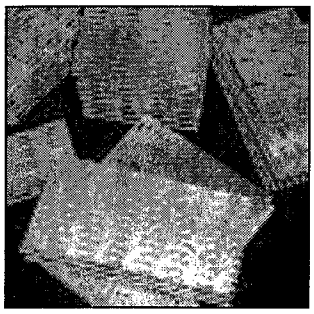
FABRICATION AND IMPLEMENTATION PROCEDURES – WOOD TRUSSES

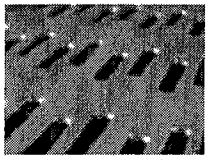
VERIFICATION AND INSPECTION IBC Section 1704.2	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS	AGENT	AGENT QUALIFICATION	TASK COMPLETED
1. Fabrications Procedures: Review of fabricator's written procedural and quality control manuals and periodic auditing of fabrication practices by an approved special inspection agency. At the completion of fabrication, the approved fabricator shall submit a certificate of compliance to the building code official stating that the work was performed in accordance with the approved construction documents. -OR- 2. TPI Inspection Program: Fabricator shall participate in the TPI Quality Assurance Inspection Program, and maintain a copy of the Quality Assurance Procedures Manual, QAP-90. Submit copy of certificate. All trusses shall bear the TPI Registered Mark.	Y	S	Fabricator shall submit one of the two qualifications	SI1	PE/SE or EIT	SEE CERT. INCLUDED
3. At completion of fabrication, the approved fabricator shall submit a certificate of compliance to the building code official stating that the work was performed in accordance with the approved construction documents	Y	S	IBC 1704.2.2	SI1	PE/SE or EIT	SEE LETTER INCLUDED



218 N. Lee Street, Suite 312
Alexandria, VA 22314
703/683-1010

TPI's Mission:
To maintain the truss industry on a sound engineering basis.





St. Louis, MO
(314) 434-1200
www.mii.com

Quality Assurance Inspection Program: Wood Truss Licensees

Select a state

CANADA

Adam Lumber, Inc.
TPI Plant No. 793
Waterloo, QC
(450) 539-1858

Clyvanor LTEE
TPI Plant No. 410
St.Goeges, Beauce, QC
(418) 228-7690

Maple Ridge Truss
TPI Plant No. 653
Pitt Meadows, BC
(604) 465-9176

Pyramid Truss
TPI Plant No. 475
Abbotsford, BC
(604) 854-6626

Solutions Prefab American Structures, Inc.
TPI Plant No. 774
Thetford Mines, QC
(418) 423-3377

Structure DLD, LTEE
TPI Plant No. 633
LaGuadeloupe Beauce, QC
(418) 459-6001

Structures RBRs
TPI Plant No. 930
Saints-Anges, QC
(418) 253-5454

Structures St. Joseph LTD
TPI Plant No. 590
St-Joseph De Beauce, QC
(418) 397-5712
WWW.STRUCTURESST-JOSEPH.COM

Viceroy Homes Limited
TPI Plant No. 616
Richmond, BC
(604) 274-8700

Timberfield Roof tryss
TPI Plant No. 903
London, ON
(519) 659-2711

MiTek Canada, Inc.
100 Industrial Road
Bradford, ON, Canada L3Z 3G7
Phone 905/952-2900
Toll Free 800/268-3434
Fax 905/952-2901

January 13, 2010

James Fortin, P.E.

Ref #: **B36792**

75 York Street
Portland, ME 04101

Dear James,

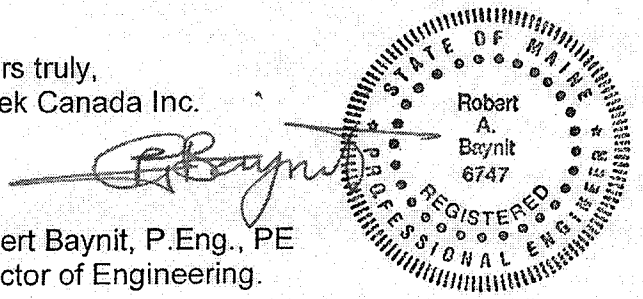
RE: Florence House

I am writing in regard to the above building to comment on the capacities of the trusses as per your email of Jan 4, 2010:

1. The roof trusses on the entire roof truss system were inspected by Stéphane Blanchette, P.Eng, and reported to MiTek Canada Inc for analysis and repair.
2. All trusses found to be damaged; including the 22 trusses marked with orange spray paint by Ganneston's subcontractor, were reviewed and evaluated.
3. The evaluation resulted in the need to reinforce 14 trusses with a scab truss nailed to the existing truss. The repair instructions were prepared and sealed by Ross Lee, PE on behalf of MiTek Canada Inc.
4. The level of damage of the remaining trusses was acceptable because the remaining bearing length with flush plate alone has sufficient bearing capacity. Therefore, these trusses have sufficient capacity to support the design loads.
5. Assuming all other roof parameters including and not limited to walls, foundation and roof bracing system are properly designed and installed, the entire as-built roof truss system is adequate to support the design loads.

I trust the above is as per your request but if you should have any questions please do not hesitate to contact me.

Yours truly,
MiTek Canada Inc.



Robert Baynit, P.Eng., PE
Director of Engineering.

Project: Florence House
Date Prepared: September 2, 2008

Structural Schedule of Special Inspections
WOOD CONSTRUCTION

VERIFICATION AND INSPECTION IBC Section 1704.6	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS	AGENT	AGENT QUALIFICATION	TASK COMPLETED
1. Fabrication of high-load diaphragms						
a. Verify wood structural panel sheathing for grade and thickness	Y	P	IBC 1704.6	SI1	PE/SE or EIT	JLF
b. Verify the nominal size of framing members at adjoining panel edges	Y	P	IBC 1704.6	SI1	PE/SE or EIT	JLF
b. Verify the nail or staple diameter and length	Y	P	IBC 1704.6	SI1	PE/SE or EIT	JLF
b. Verify the number of fastener lines	Y	P	IBC 1704.6	SI1	PE/SE or EIT	JLF
b. Verify the spacing between fasteners in each line and at edge margins	Y	P	IBC 1704.6	SI1	PE/SE or EIT	JLF
2. Load Tests for Joist Hangers: Provide evidence of manufacturer's load test in accordance with ASTM D1761 including the vertical load bearing capacity, torsional moment capacity, and deflection characteristics when there is no calculated procedure recognized by the code.	Y	S	IBC 1715 [submit ICBO reports]	SI1	PE/SE or EIT	N/A

BECKER
structural engineers, inc.

FLORENCE HOUSE FINAL REPORT – March 24, 2010				
1	1st floor wall framing	07/27/09	For the in-fill wall panels at the first floor level along Line 1, from Grid C to the site retaining wall, it is acceptable to replace the LVL wall studs with (2) 2x6 at 24" o.c. since these walls do not serve as a backing for brick veneer at these locations. 08/13/09: Wall studs have been constructed.	CLOSED
2	2nd floor wall framing	07/27/09	Due to inconsistencies in the levelness of the 2nd floor concrete slab on deck, gaps exist between the bottom plate of the wall panel and the PT sill plate bolted to the slab. At all locations where gaps exist, place full width by full depth hardwood shims below all vertical studs. Contractor has stated that shims will be cut to fit the various gap widths. Apply construction adhesive to each side of each shim prior to placing to provide positive connection between the plates. 08/24/09: It appears that shims have been installed as necessary, and joints have been sealed.	CLOSED
3	2nd & 3rd floor wall framing	07/27/09	At the 5 ¼" x 7" LVL supporting the 3H1 headers (see S1.4, 2 locations), it is acceptable to bear the LVL on (2) PT SYP #2 sill plates fastened to the concrete slab with (2) 5/8" diameter x 2 ¾" concrete embed Simpson Titen HD screws (countersink screws into top-most plate). 08/13/09: Plate and bolts have been placed, but LVL is not in place. 08/24/09: LVL & wood stud posts have been installed.	CLOSED
4	1st floor wall framing	07/31/09	At the first floor window openings along Line A,N,7 & 16, the top of the R.O. is at 11'-11" above the first floor slab. With a floor-to-floor height of 14'-1", a clear distance of between ½" to 8 ½" remains between the bottom of the steel beam and the top of the window header. At these locations, it is not necessary to install cripple studs above the header, but the header shall be attached to the jambs using the Simpson clips at the top and bottom of the header (as specified). If the header is at least 1" into the light-gauge track, the top Simpson angles can be omitted. If top angles cannot be installed due to interference with the metal track, a 4" long max section of the inside flange of the top track can be cut to fit a palm nailer for fastening the hanger to the jamb. At cut locations, fasten a 16" long Simpson CMST-12 ga strap (or equivalent) centered across the cut and screwed to the track flange. Note: When installing plywood, ensure that joints do not occur at the metal track. Sheathing fastener spacing shall follow drawing requirements. 08/13/09: Headers and clips have been installed without needing to cut the metal track.	CLOSED
5	1st floor wall framing	07/31/09	Along Line F, the jamb at Door 126 interferes with the HSS hangers for the canopy. At this location, increase the jamb at adjacent Window F to (6) 1 ¾" x 5 ½" LVL's, and extend the door header to this jamb. Install (2) LVL's for the door jamb extending from the slab to the underside of the header. Attach with Simpson LS50 angles, each side of jamb. 08/13/09: Item has been completed as described above.	CLOSED
6	2nd & 3rd floor wall framing	07/31/09	At all shear wall locations, attach the sheathing to the PT bottom plates with galvanized fasteners. 08/24/09: Fastening not complete at all areas. Confirmation is necessary that fasteners used are galvanized. Review of exterior wall sheathing attachments not yet completed. 09/25/09: Item was reviewed with LocBid during inspection of exterior shear wall plywood fastening.	CLOSED

7	2nd & 3rd floor framing	07/31/09	For floor truss strong-back attachment at wood stud walls, it is acceptable to replace the Simpson A34 angles specified on Section 2/S1.4 with a vertical 2x6 stud placed against the strong-back. Fasten all members together. 8/24/09: Strong-backs are being installed as specified above.	CLOSED
8	2nd floor wall framing	07/31/09	At SW-5 locations, the Titen HD anchors are not installed to attach the wall panel to the concrete slab. 08/24/09: Titen HD anchors have been installed.	CLOSED
9	2nd floor wall framing	07/31/09	At SW-4 locations, the 5/8" through bolts are not installed to attach the wall panel to the steel framing. 08/24/09: Correct fastening was not completed as these shear walls. Titen HD anchors were placed in lieu of through bolts. Areas shall be corrected to meet drawing requirements. 09/25/09: Work is not complete. One area could not be reviewed from below due to sheetrock installation. 10/19/09: It appears that through bolts have been placed as specified. Bolts were noted in all areas reviewed.	CLOSED
10	3rd floor framing	07/31/09	At LVL attachment between third floor trusses (along Line 10), the Simpson hanger flange nailing is incomplete. 08/24/09: Top flange hanger used, and fastening appears complete.	CLOSED
11	2nd & 3rd floor wall framing	07/31/09	At various locations, floor trusses do not line up with wall studs. Additional studs will be added as necessary. 09/02/09: Item occurs at areas of the third floor also. 09/25/09: Studs were added where necessary at all areas observed.	CLOSED
12	2nd & 3rd floor wall framing	07/31/09	At joints between panels, vertical studs shall be in contact and shall be attached together with (2) 10d nails at 6" o.c. 09/25/09: Fasteners were found at areas observed.	CLOSED
13	3rd floor framing	07/31/09	Between Lines G & H, drawing S1.4 specifies floor trusses to be aligned at edges of floor opening. GC shall confirm truss layout is adequate to suit floor opening location. 09/25/09: GC shall coordinate as necessary.	CLOSED
14	2nd floor wall framing	07/31/09	At numerous locations, the shear wall holdown bolt (per Detail B/S4.3) does not extend above the wall bottom plates. GC shall confirm locations, and submit recommendation for repair. 09/25/09: Threaded rods have been extended using coupler nuts, and holdowns are installed.	CLOSED
15	3rd floor	07/31/09	The CMU wall construction at the third floor cantilever bond beam (Section 9/S3.2) has un-mortared joints. 09/25/09: Work is on-going but not yet completed. 10/19/09: Joints have been filled in.	CLOSED
16	1st floor wall framing	08/13/09	At the first floor in-fill wall framing along Line F, the jamb attachment at Door 126 is missing a Simpson angle at the base of the jamb.	CLOSED

17	3rd floor & Roof Framing	08/24/09	PT ledgers against CMU walls that support floor and roof framing are not bolted to CMU wall as specified (see section 9/S4.2 & 4/S4& 4/S4.1). 09/25/09: Ledgers were found not bolted to the CMU walls as specified. ALL areas shall be reviewed when completed and prior to sheetrock installation. 10/19/09: PT ledgers were not visible for review during this visit due to installation of sheetrock. GC has stated that ledgers were bolted as specified. 3/24/10: Sheetrock was removed at one location at each level to confirm installation of anchors. Simpson Titen HD anchors were visible and located at spacing specified.	CLOSED
18	Roof Truss System	08/24/09	Configuration of Roof Trusses at Exterior Wall bearing end do not accommodate the 2x ribbon per Section 5/S4.1. An alternate detail to provide adequate stability of the truss shall be provided by the truss manufacturer. Also, Section 5/S4.1 will be revised to show alternate sheathing framing configuration and sheathing fastening. 09/25/09: Detail was observed from the third floor level (looking up). ** All fastening identified in CSKS-18 could not be reviewed due to roofing installation.	CLOSED
19	3rd floor framing	08/24/09	The 3B2 & 3H1 headers are pocketed into the CMU stair walls. At 3B2, the CMU wall does not line up with the header, and (1) LVL is not properly bearing on the wall. Contractor shall review area to ensure proper layout, and header bearing will need to be improved. Additional instruction shall be provided by Engineer to address this issue. 09/25/09: Fasten a 2x8 x 8" long PT ledger into solid grouted CMU located directly below the unsupported section of the header (ensure contact between ledger and header). Fasten the ledge to the CMU wall with (2) 5/8" x 6 5/8" embed adhesive anchors, centrally located on the plate. 10/19/09: Work was completed as instructed.	CLOSED
20	3rd floor framing	08/24/09	At North side of Stair 1, the PT ledger against the CMU wall is not in place for floor sheathing support. 10/19/09: Work is not yet completed. Same condition exists at south side of Stair 2. 03/24/10: Areas were not specifically reviewed, but adjacent areas were reviewed and noted to be constructed per drawing requirements.	CLOSED
21	3rd floor framing	08/24/09	Truss strong-backs are not attached at ends against CMU walls. 09/25/09: Areas were observed where strong-backs need to extend to the CMU wall for attachment. Strong-backs shall be positively attached to structure (ie. walls) at all ends. 10/19/09: Strong-backs have been attached as specified.	CLOSED
22	3rd floor framing	08/24/09	Truss strong-back missing at north side of stair 1 09/25/09: Strong-backs are in place as specified	CLOSED
23	2nd floor wall framing	08/24/09	Titen HD anchors are not installed at "recessed" wall locations per Section 7/S3.2. 09/25/09: Titen anchors were observed where specified.	CLOSED
24	2nd floor wall framing	08/24/09	At exterior wall locations, sill anchors are required at 4'-0" on center. Contractor shall review areas, and add anchors as necessary. At missing locations, install Simpson Titen HD anchors. 09/25/09: Anchors were found as specified at all areas observed.	CLOSED

25	2nd & 3rd floor wall framing	08/24/09	At the exterior face of the CMU walls, it has been determined by the General Contractor that sufficient space exists to replace the wood strapping with 2x4 studs. The framing shall be located against the CMU wall and shall create a flush sheathing condition with the wood stud walls on either side. Attach 2x4 studs to the 2x6 bearing wall studs on either side of the CMU wall using 10d nails at 6" on center. Additional instruction on constructing the 2x4 walls and attaching to the CMU wall will be provided by the Engineer shortly. 09/25/09: Work not completed along the Valley St. side of Stair 2 per CSKS-17 10/19/09: 2x4 wall has been constructed in front of CMU wall. Most of the wall is not accessible, but at one side, the attachment of this wall to the 2x6 bearing wall was observed, and follows CSKS-17.	CLOSED
26	Shear wall holdowns	09/02/09	The shear wall holdowns between the 2nd and 3rd floor wall panels are not in place per Det. C/S4.3. 09/25/09: Holdowns were found installed at all accessible locations. Photos were provided by the GC to confirm areas not accessible during the visit, as the top of the second floor wall panels was sheetrocked at all holdown locations, and installation of the holdown could not be verified	CLOSED
27	3rd floor header post	09/02/09	The (5) 2x8 wood post that supports the 3H1 corridor header at the south side of the building is not in place. 09/25/09: Post is in place as specified	CLOSED
28	2nd & 3rd floor wall framing	09/02/09	Built-up wood posts shall be fastened with (2) 10d nails at 6" o.c. as specified on Dwg. S1.0 09/25/09: Fastening of field-constructed built-up posts was found adequate at all locations reviewed.	CLOSED
29	Roof Framing	09/02/09	The RB1 header is pocketed into the CMU at Stair 1. The header does not line up adequately with the CMU wall to provide proper bearing. Contractor shall review area to ensure proper layout, and header bearing will need to be improved. Additional instruction shall be provided by Engineer to address this issue. 09/25/09: Fasten a 2x8 x 8" long PT ledger into solid grouted CMU located directly below the unsupported section of the header (ensure contact between ledger and header). Fasten the ledge to the CMU wall with (2) 5/8" x 6 5/8" embed adhesive anchors, centrally located on the plate. 10/19/09: Work was completed as instructed.	CLOSED
30	3rd floor - Shear Wall Plate Attachment	09/02/09	Additional fastening is necessary for the shear wall bottom plate attachments as specified in the Shear Wall Schedule on Dwg. S4.3 09/15/09: Fastening appears to be in general conformance with drawing requirements.	CLOSED
31	Roof Framing – Strong-backs	09/02/09	Truss bracing strong-backs have not yet been installed at 10'-0" on center as required. 09/25/09: Strong-backs appear to have been installed as necessary.	CLOSED

32	Stair 2 – CMU wall bracing	09/02/09	A wood stud wall is constructed above the top of the CMU wall at Stair 2 as shown in Section 10/S4.2. A roof joist has been located within 6" of the CMU wall, creating an interference with placement of the 2x4 brace diagonal. Instead, place 2x6 x 24" long blocking tight between the wall studs and the top & bottom of the truss, and install both the diagonal and horizontal brace in the next truss space away from the wall. Ensure roof sheathing is attached to the horizontal brace as specified. 09/25/09: Work has been completed as specified.	CLOSED
33	Roof Trusses	09/02/09	Numerous locations exist where the bearing seat at the corridor end of the truss is cracked and split. At some locations, the seat has completely failed, with a section of the bearing seat missing. GC has contacted the truss manufacturer's engineer to review the in-place trusses, and submit engineered repairs for all damaged trusses. All repairs shall be sealed by a Professional Engineer licensed in the State of Maine. Following completion of all repairs, truss manufacturer's engineer shall review all work, and submit a letter to the Owner and Engineer of Record certifying that the roof truss system is acceptable and in accordance with all project requirements. 09/25/09: Repair documentation is required for review. 10/19/09: PE stamped certification has not yet been received. 01/13/10: Certification letter provided.	CLOSED
34	Roof Framing	09/08/09	At locations where roof joists span parallel with exterior walls, and the roof joist slopes while the wall is kept level, a 2x6 ledger has been used to support the edge of the roof plywood. This ledger has not been detailed on the structural drawings (Section 6/S4.1 was specified in these areas, which considers a sloping wall top plate). Ledger shall be attached to each vertical wall stud with (4) 10d nails. Also, full-depth 2x6 blocking shall be placed between studs behind the ledger. Fasten blocking to the vertical studs, and fasten the ledger and the wall sheathing to the blocking with 10d nails at 4" o.c. 09/25/09: Work has not been completed. 10/19/09: GC has stated that the blocking and attachment details specified above have been completed, but the blocking cannot be reviewed due to sheetrock installation. GC stated that photos are available to certify the work was completed. It is requested that these photos are submitted for review. 3/24/10: Sheetrock and insulation was removed to expose two vertical studs. Ledger attachment to vertical studs was not found to be as specified. The as-built configuration was reviewed, taking into account the proximity of additional shear walls, and it was determined that the as-built capacity of the connection is acceptable.	CLOSED
35	Roof Sheathing Attachment	09/08/09	At all interior shear wall locations, roof plywood sheathing shall be fastened to wall top plates with 8d nails at 4" o.c. per Section 9/S4.1 09/25/09: GC has stated that this item was completed. ** Item cannot be reviewed due to roofing installation.	CLOSED
36	Corridor Bearing Walls	09/08/09	At corridor bearing walls, area between truss seats shall be blocked per Section 3/S4.1. At roof trusses, it is acceptable to install (1) 2x10. At floor level, add (2) 2x10. Attach blocking to LVL rim, and attach wall plates/sheathing to blocking. 09/25/09: Blocking was observed at all areas reviewed.	CLOSED

37	Roof Screen Post Attachment	09/08/09	At roof screen post attachments along the north and south sides, the roof trusses are to be restrained at the posts per Section 13/S4.2. The framing specified in Section 13 is installed, but when the post locations were determined, it was necessary to move the posts slightly to either align with the east & west walls, or to have adequate room to install the bolts. It appears that the maximum clear distance between the post and the truss bracing wall is under 2" (GC shall confirm, and notify engineer is distance is greater than this). To provide adequate restraint at the post, it is necessary to add (4) 2x6 blocking between roof trusses at the top and bottom of the truss. Ensure a tight fit between the blocking and the truss chords, and attach the blocking to the 2x4's in the bracing wall with 3/8" x 8" long RSS Structural Screws (pre-drill holes in 2x4 members). Place (3) RSS Screws at each vertical 2x4 located against the roof trusses, and at 8" o.c. +/- along the top and bottom horizontal 2x4 members. 09/25/09: Work is not complete. 10/19/09: Work has been completed.	CLOSED
38	Wood Wall Studs Near Holdowns	09/25/09	Wall studs have been cut for installation and fastening of shear wall holdowns. At non-load bearing walls, replace the cut section and scab a new 4'-0" long 2x6 section to the side of the full height stud, fastening with (2) 10d nails at 6" o.c. At load-bearing walls, add a new full height stud. Fasten all new sections to the plywood sheathing per drawing requirements. 10/19/09: Wall studs were found to be repaired as specified at locations observed.	CLOSED
39	Post Cap Attachment at Roof Beam	09/25/09	At the RH1 header adjacent to Stair 2, the Simpson post cap is not attached to the jamb post. 10/19/09: Post cap has been attached to jamb post as necessary.	CLOSED
40	Valley Street Canopy – North End	10/19/09	At the Valley Street canopy, the MC 18 connection to the Second Floor Beam connection plate at Line 13 is not fitting up correctly. The connection plate appears rotated slightly such that the bolt holes do not line up. The item was discussed with the steel erector, and in lieu of fully bolting the connection, three bolt holes will be elongated slightly such that a minimum of (1) bolt in each row will be placed. Then the south side and the top of the MC18 connection plate will be welded to the W14 connection plate. Use a ¼" fillet weld along the south side, and a full penetration beveled weld along the top. These field welds require visual inspection from an AWS Certified Weld Inspector, with a report submitted to Becker Structural Engineers for Record. 03/04/10: Item was tracked under separate report specific to the canopy. Final inspection report from AWS Certified Weld Inspector was submitted.	CLOSED

PHOTOS FOR SITEM 2L

LETTER OF TRANSMITTAL

Ganneston Construction Corp
PO Box 27, North Belfast Avenue
Augusta, ME 04332-0027

Tel: (207) 621-8505
Fax: (207) 621-8508

GCC PROJECT NUMBER
GCC-80677

To:

GAWRON TURGEON ARCHITECTS
29 BLACK POINT ROAD
SCARBOROUGH, MAINE 04074

TUESDAY 3:10pm

Date: February 16, 2010
Attention: WENDI
RE: FLORENCE HOUSE PROJECT

WE ARE SENDING YOU ___X___ ATTACHED _____ UNDER SEPARATE COVER

COPIES	NO.	DESCRIPTION
2		CLIPS PICTURES AS REQUESTED BY JIM FORTIN

REMARKS

HIS E MAIL KICKED IT BACK THUS WE ARE SENDING THESE FOR RECORD

Signed: *Michael L. Adams* Date: 16-Feb-10

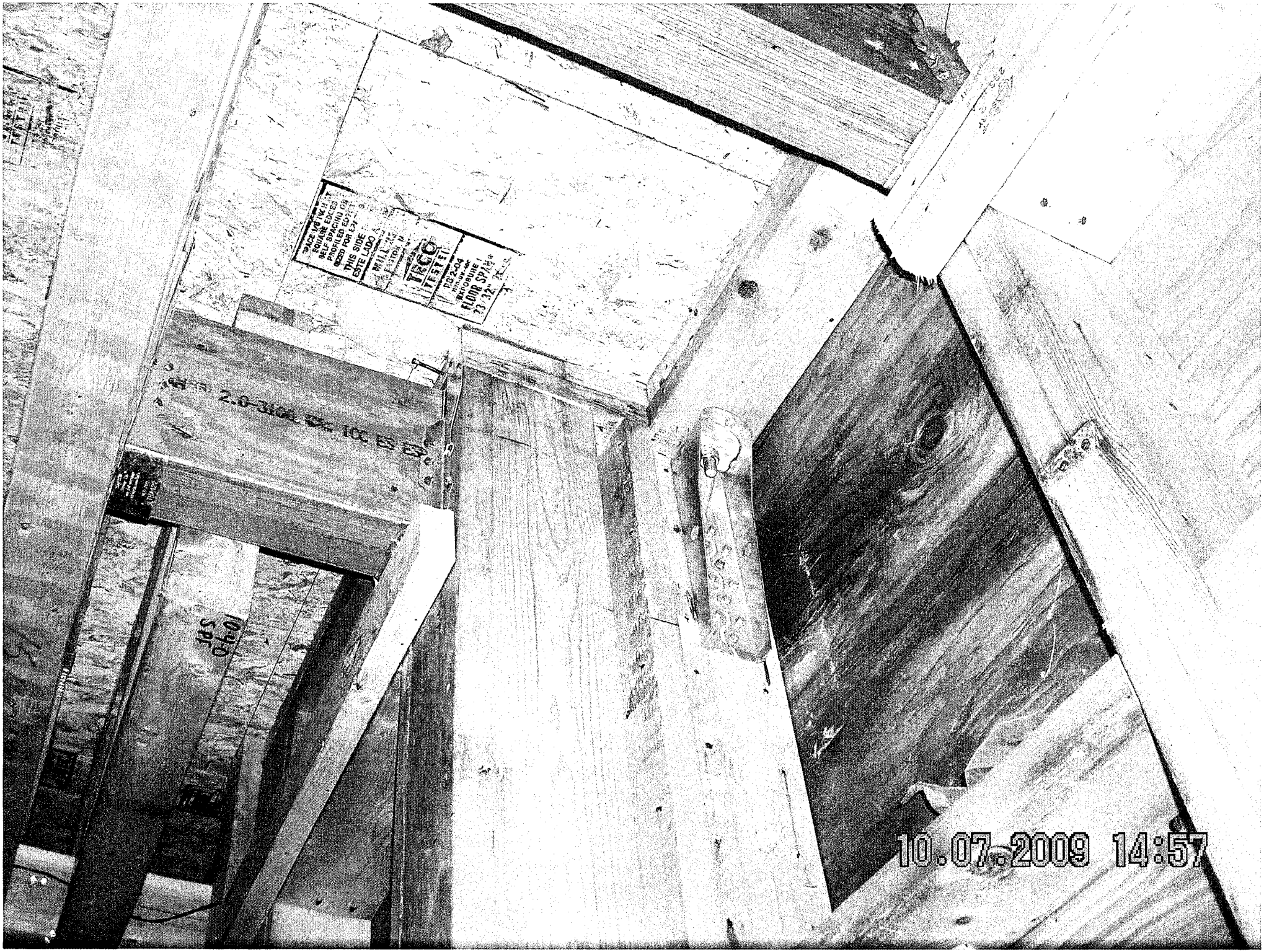
Sr. Project Manager

RECEIVED FEB 18 '10

BECKER STRUCTURAL ENGINEERS, INC.

FEB 22 2010

RECEIVED


10.07.2009 14:57

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26
10.07.2009 15:02

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10.07.2009 14:57

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06100

OBSERVATION REPORT
Rough Carpentry

Date:	June 18, 2009
Time:	1:30 pm-2:30 pm
Temp:	60's
Weather:	Cloudy / Rain Showers

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI2058

Observation Location: Review of panelized wood wall sections at the Hancock Lumber fabrication facility in Windham, Maine.

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Member Sizes	☒	☐	☐	☐	
Material Quality	☒	☐	☐	☐	
Bearing Condition	☐	☐	☐	☒	
Connections	☒	☐	☐	☐	
Nailing Pattern	☒	☐	☐	☐	
Bridging/Bracing	☒	☐	☐	☐	
Other:	☐	☐	☐	☐	
Other:	☐	☐	☐	☐	

Notes:

This report shall document review of pre-fabricated wood wall panels for the Florence House project. A review the panels was completed at the Hancock Lumber fabrication facility in Windham, Maine prior to shipment to the site. The sections of wall panels accessible for review were found to be in general conformance with the contract documensts.

Signed: James Fortin, P.E.

75 York Street, Portland, ME 04101-4701 ■ Tel 207-879-1838 ■ Fax 207-879-1822

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OBSERVATION REPORT
Rough Carpentry

Date:	July 27, 2009
Time:	8:30 am – 11:00 am
Temp:	70 – 80
Weather:	Clouds, Sun, Humid

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI2058

Observation Location: Review of wood construction completed to date.

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Member Sizes	☒	☐	☐	☐	
Material Quality	☒	☐	☐	☐	
Bearing Condition	☒	☐	☐	☐	
Connections	☒	☒	☒	☐	
Nailing Pattern	☒	☐	☒	☐	
Bridging/Bracing	☐	☐	☒	☐	
Other:	☐	☐	☐	☐	
Other:	☐	☐	☐	☐	

A preliminary review of the 2nd floor wood wall panels and third floor trusses was completed. The intent was to observe the framing and identify areas that were not in general conformance with the contract documents. At this time, all areas were in progress but appeared to be following the intent of the drawings. A few items were discussed with the General Contractor and framing sub-contractor. These items are provided below.

1. For the in-fill wall panels at the first floor level along Line 1, from Grid C to the site retaining wall, it is acceptable to replace the LVL wall studs with (2) 2x6 at 24" o.c. since these walls do not serve as a backing for brick veneer at these locations.

2. Due to inconsistencies in the levelness of the 2nd floor concrete slab on deck, gaps exist between the bottom plate of the wall panel and the PT sill plate bolted to the slab. At all locations where gaps exist, place full width by full depth hardwood shims below all vertical studs. Contractor has stated that shims will be cut to fit the various gap widths. Apply construction adhesive to each side of each shim prior to placing to provide positive connection between the plates.
3. At the 5 ¼" x 7" LVL supporting the 3H1 headers (see S1.4, 2 locations), it is acceptable to bear the LVL on (2) PT SYP #2 sill plates fastened to the concrete slab with (2) 5/8" diameter x 2 ¾" concrete embed Simpson Titen HD screws (countersink screws into top-most plate).

Please call with any questions.

Signed: James Fortin, P.E.

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OBSERVATION REPORT
Rough Carpentry

Date:	July 31, 2009
Time:	8:30 am-10:30 am
Temp:	70's
Weather:	Sun and Clouds

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI2058

Observation Location: Review of first and second floor wood construction completed to date.

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Member Sizes	☒	☐	☐	☐	
Material Quality	☒	☐	☐	☐	
Bearing Condition	☒	☐	☐	☐	
Connections	☒	☐	☒	☐	
Nailing Pattern	☒	☐	☐	☐	
Bridging/Bracing	☒	☐	☒	☐	
Other:	☐	☐	☐	☐	
Other:	☐	☐	☐	☐	

This site visit was completed to review wood construction completed to date. Review included the 1st & 2nd floor exterior wall framing, and the third floor trusses. A few items were discussed with the General Contractor and framing sub-contractor. These items are provided below.

A. At the first floor window openings along Line A,N,7 & 16, the top of the R.O. is at 11'-11" above the first floor slab. With a floor-to-floor height of 14'-1", a clear distance of between ½" to 8 ½" remains between the bottom of the steel beam and the top of the window header. At these locations, it is not necessary to install cripple studs above the header, but the header shall be attached to the jambs using the Simpson clips at the top and bottom of the header (as specified). If the header is at least 1" into the light-gauge

track, the top Simpson angles can be omitted. If top angles cannot be installed due to interference with the metal track, a 4" long max section of the inside flange of the top track can be cut to fit a palm nailer for fastening the hanger to the jamb. At cut locations, fasten a 16" long Simpson CMST-12 ga strap (or equivalent) centered across the cut and screwed to the track flange. Note: When installing plywood, ensure that joints do not occur at the metal track. Sheathing fastener spacing shall follow drawing requirements.

B. Along Line F, the jamb Door 126 interferes with the HSS hangers for the canopy. At this location, increase the jamb at adjacent Window F to (6) 1 ¾" x 5 ½" LVL's, and extend the door header to this jamb. Install (2) LVL's for the door jamb extending from the slab to the underside of the header. Attach with Simpson LS50 angles, each side of jamb.

C. For floor truss strong-back attachment at wood stud walls, it is acceptable to replace the Simpson A34 angles specified on Section 2/S1.4 with a vertical 2x6 stud placed against the strong-back. Fasten all members together.

The following items were noted during a walk-through of the second floor. These items were not discussed with the framing sub-contractor or general contractor prior to my departure.

1. At interior shear wall locations, attach the sheathing to the PT bottom plates with galvanized fasteners.
2. At SW-5 locations, the Titen HD anchors are not installed to attach the wall panel to the concrete slab.
3. At SW-4 locations, the 5/8" through bolts are not installed to attach the wall panel to the steel framing.
4. At LVL attachment between third floor trusses (along Line 10), the Simpson hanger flange nailing is incomplete.
5. At various locations, floor trusses do not line up with wall studs. Additional studs will be added as necessary.
6. At joints between panels, vertical studs shall be in contact and shall be attached together with (2) 10d nails at 6" o.c.
7. At third floor, between Lines G & H, drawing S1.4 specifies floor trusses to be aligned at edges of floor opening. GC shall confirm truss layout is adequate to suit floor opening location.
8. At numerous locations, the shear wall holdown bolt (per Detail B/S4.3) does not extend above the wall bottom plates. GC shall confirm locations, and submit recommendation for repair.
9. The CMU wall construction at the third floor cantilever bond beam (Section 9/S3.2) has un-mortared joints.

10. At locations where the radon pipe runs through the concrete floor haunches shown in CSKS-10, remove the crushed stone, wrap, and pour the concrete haunch around the pipe.

Please call with any questions.

Signed: James Fortin, P.E.

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OBSERVATION REPORT
Rough Carpentry

Date:	August 13, 2009
Time:	8:00 am-10:30 am
Temp:	80's
Weather:	Sunny & Humid

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI2058

Observation Location: Review of complete wall and floor wood construction completed to date.

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Member Sizes	☒	☐	☐	☐	
Material Quality	☒	☐	☐	☐	
Bearing Condition	☒	☐	☐	☐	
Connections	☒	☒	☒	☐	
Nailing Pattern	☒	☐	☐	☐	
Bridging/Bracing	☒	☐	☒	☐	
Other:	☐	☒	☒	☐	See notes (attached)
Other:	☐	☐	☐	☐	

This site visit was completed to review wood construction completed to date. Review included the exterior wall and interior bearing wall framing and floor trusses. A meeting was also conducted to review the roof trusses that were exhibiting cracks and excessive curling at the bearing ends. This issue will be tracked independent of this report.

Items noted during this visit are provided below, along with previously documented items. New information, as well as updates to previously documented items, are provided in bold.

If there are any questions with the information provided, please contact our office.

Thank you -

Signed: James Fortin, P.E.

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OBSERVATION REPORT
Rough Carpentry

Date:	August 24, 2009
Time:	10:00 am-12:00 pm
Temp:	80's
Weather:	Sunny & Humid

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI2058

Observation Location: Review of second floor wall framing and third floor wood construction. (including preliminary review of third floor wall framing and roof truss system)

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Member Sizes	☒	☐	☐	☐	
Material Quality	☒	☐	☐	☐	
Bearing Condition	☐	☒	☐	☐	As noted
Connections	☐	☒	☒	☐	As noted
Nailing Pattern	☒	☐	☐	☐	
Bridging/Bracing	☒	☐	☒	☐	As noted
Other:	☐	☒	☒	☐	See notes (attached)
Other:	☐	☐	☐	☐	

This site visit was completed to review wood construction completed to date. Review included the exterior wall and interior bearing wall framing at the second floor as well as the third floor trusses. A preliminary review of the third floor wall framing and roof trusses was completed, but additional review is necessary.

Items noted during this visit are provided below, along with previously documented items. New information, as well as updates to previously documented items, is provided in bold.

If there are any questions with the information provided, please contact our office.

Thank you -

Signed: James Fortin, P.E.

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OBSERVATION REPORT
Rough Carpentry

Date:	September 2, 2009
Time:	1:00 pm-3:00 pm
Temp:	80's
Weather:	Sunny

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI2058

Observation Location: Review of third floor wall framing and roof truss installation.

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Member Sizes	☒	☐	☐	☐	
Material Quality	☒	☐	☐	☐	
Bearing Condition	☐	☒	☐	☐	See notes (attached)
Connections	☐	☐	☒	☐	See notes (attached)
Nailing Pattern	☐	☒	☒	☐	See notes (attached)
Bridging/Bracing	☐	☐	☒	☐	See notes (attached)
Other:	☐	☒	☒	☐	See notes (attached)
Other:	☐	☐	☐	☐	

This site visit was completed to review third floor wall construction and roof truss installation completed to date. Construction appears to be progressing well. Items noted have been included in the following table. New information, as well as updates to previously documented items, is provided in bold.

The following items have not yet been reviewed. These items will be reviewed once the General Contractor states that all work is completed and ready for review. These items include:

1. Roof Plywood Attachment
2. Exterior Wall Sheathing Attachment and Plywood Fastening between wall panels
3. Shear wall nailing at bottom plates
4. Specific fastening details specified in the construction documents (Section 1/S4.1, for example)
5. Canopies (not yet installed)
6. Simpson H-clips at ends of all roof trusses.
7. Roof Screen Attachments to roof trusses, and associated details

If there are any questions with the information provided, please contact our office.

Signed: James Fortin, P.E.

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OBSERVATION REPORT
Rough Carpentry

Date:	September 8, 2009
Time:	1:30 pm-3:30 pm
Temp:	80's
Weather:	Sunny

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI2058

Observation Location: Review of roof sheathing attachments, and roof truss system.

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Member Sizes	☐	☐	☐	☒	
Material Quality	☒	☐	☐	☐	
Bearing Condition	☐	☐	☐	☒	
Connections	☐	☐	☒	☐	See notes (attached)
Nailing Pattern	☐	☒	☒	☐	Only as noted below
Bridging/Bracing	☐	☐	☒	☐	See notes (attached)
Other:	☐	☒	☒	☐	See notes (attached)
Other:	☐	☐	☐	☐	

This site visit was completed to review the attachments of the plywood roof sheathing to the roof joists, as well as review roof framing details with Dick Beaulieu and Derik Ouellette. All plywood roof sheathing was in place during this visit, and attachments appeared satisfactory where reviewed, except for a few minor areas that were spray-painted for identification. As a result of this visit, a few additional items were noted that have been included in the attached table. These items require additional attention to meet the drawing requirements.

The following list of items is provided to identify major structural areas that have not yet been reviewed. These items will be reviewed once the General Contractor states that all work is completed and ready for review. These items include:

1. Exterior Wall Sheathing Attachment and Plywood Fastening between wall panels
   Note: This inspection needs to occur prior to placement of any building wrap (ie. tyvek)
2. Shear wall nailing at bottom plates
3. Specific fastening details specified in the construction documents (Section 1/S4.1, for example)
4. Canopies (not yet installed)
5. Simpson H-clips at ends of all roof trusses.
6. Roof Screen Attachments to roof trusses, and associated details

Also, CSKS-18 has been completed in a majority of areas, but it is not possible to review all fastening as the area has been covered with additional blocking and plywood to construct the eave.

If there are any questions with the information provided, please contact our office.

Signed: James Fortin, P.E.

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OBSERVATION REPORT
Rough Carpentry

Date:	September 25, 2009
Time:	1:30 pm-3:30 pm
Temp:	80's
Weather:	Sunny

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI2058

Observation Location: Review of entire structure. Review completion of previously documented items.

At the request of the General Contractor, a site visit was completed to review construction and completion of previously documented items. There was concern that areas requiring re-inspection were being covered up due to sheetrock installation. The attached table has been updated based on today's review.

At this point, the following items remain that need to be reviewed when completed, and prior to installation of finishes. These items will be reviewed once the General Contractor states that all work is completed and ready for review. These items include:

1. Roof Screen Post Attachment to the Roof Trusses, and completion of associated details
2. Roof Truss Repairs at Damaged Areas
    a. In order to review this item, it is necessary that documentation is submitted from the truss manufacturer (Clyvanor). Documentation shall identify the trusses that require repair, the engineered design of the repair, and the criteria used to accept damaged trusses that were identified by the design team.
3. Repair of Damaged Steel Joists (2 locations)
4. Canopies (not yet installed)

If there are any questions with the information provided, please contact our office.

Signed: James Fortin, P.E.

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OBSERVATION REPORT
Rough Carpentry

Date:	October 19, 2009
Time:	10:30 am-12:30 pm
Temp:	60's
Weather:	Sunny

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI2058

Observation Location: Review completion of previously documented items.

Following notification by the General Contractor, a site visit was completed to review previously documented items that had been addressed. A majority of open items were addressed by the General Contractor, and these items have been updated in the attached table. A few items remain that require attention by the General Contractor. Once these items are completed, a return visit can be scheduled. These items need to be reviewed prior to installation of finishes that will block access.

Also, the following items need to be reviewed when the General Contractor states that all work is complete, and prior to installation of finishes.
These items include:

1. Repair of Damaged Steel Joists (2 locations) (includes PE stamped design submittal)
2. Canopies (installation not complete)

Please note that I will be on vacation from October 23rd to November 3rd, and will not have access to e-mail. A return site visit can be scheduled once I return.

Thank you –

Signed: James Fortin, P.E.

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EXHIBIT C

01000 Quality Assurance

Project: Florence House
Date Prepared: September 2, 2008

Quality Assurance Plan – Seismic and Wind

QUALITY ASSURANCE FOR SEISMIC RESISTANCE CHECK LIST [IBC 1705]

Seismic Design Category	C

☒ **FOR SEISMIC DESIGN CATEGORY C OR HIGHER:**

Structural:

☒ The seismic-force-resisting systems
- ☒ Steel Braced Frames and associated connections/anchorage
- ☐ Steel Moment Frames and associated connections
- ☒ Shear walls: ☐ CMU ☒ Wood ☐ Concrete ☒ Diaphragms: ☒ Floor ☒ Roof
- ☐ Other:

QUALITY ASSURANCE FOR WIND RESISTANCE CHECK LIST [IBC 1706]

Wind Exposure Category	C
Basic Wind Speed	100 mph

REQUIRED	NOT REQUIRED	NOT APPLICABLE	QUALITY ASSURANCE PLAN REQUIREMENTS (A Quality Assurance Plan is required where indicated below)
☐	☐	☒	In wind exposure Categories A and B, where the 3-second-gust basic wind speed is 120 miles per hour (mph) (52.8 *m/sec)* or greater.
☐	☐	☒	In wind exposure Categories C and D, where the 3-second-gust basic wind speed is 110 mph (49 *m/sec)* or greater.

Prepared by:

Signature 3/4/10 Date

Building Code Official's Acceptance:

Signature Date

Project: Florence House
Date Prepared: September 2, 2008

Structural Schedule of Special Inspections

SEISMIC RESISTANCE - STRUCTURAL

VERIFICATION AND INSPECTION IBC Section 1707	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS	AGENT	AGENT QUALIFICATION	TASK COMPLETED
1. Special inspections for seismic resistance. Special inspection as specified in this section is required for the following:			Seismic Design Category: C			
a. The seismic-force-resisting systems in structures assigned to Seismic Design Category C, D, E or F	Y	P	IBC 1707.1	SI1	PE/SE or EIT	JCF
2. Structural steel: Continuous special inspection for structural welding in accordance with AISC 341.	Y	P	IBC 1707.2	TA2	AWS-CWI	N/A
3. Structural wood:						
a. Continuous special inspection during field gluing operations of elements of the seismic-force-resisting system.	Y	C	IBC 1707.3	SI1	PE/SE or EIT	N/A
b. Periodic special inspections for nailing, bolting, anchoring and other fastening of components within the seismic-force-resisting system, including drag struts, braces and hold-downs	Y	P	IBC 1707.3	SI1	PE/SE or EIT	JCF
4. Cold-formed steel framing: Periodic special inspections during welding operations of elements of the seismic-force-resisting system. Periodic special inspections for screw attachment, bolting, anchoring and other fastening of components within the seismic-force-resisting system, including struts, braces, and hold-downs	N/A	N				
4. Seismic isolation system. Provide periodic special inspection during the fabrication and installation of isolator units and energy dissipation devices if used as part of the seismic isolation system	N/A	N	IBC 1707.8			

EXHIBIT D

Statements of Responsibility

Project: Florence House
Date Prepared: September 2, 2008

Contractor's Statement of Responsibility

Each contractor responsible for the construction or fabrication of a system or component designated in the Quality Assurance Plan must submit a Statement of Responsibility. The Statement of Responsibility is required for Seismic Design Category C or higher. Make additional copies of this form as required.

Project: Florence House

Contractor's Name: Gannesian Construction Corp

Address: PO Box 22, Augusta, Maine 04332

License No.:

Description of designated building systems and components included in the Statement of Responsibility:

Building Structural Systems

Contractor's Acknowledgment of Special Requirements

I hereby acknowledge that I have received, read, and understand the Quality Assurance Plan and Special Inspection program.

I hereby acknowledge that control will be exercised to obtain conformance with the construction documents approved by the Building Official.

Signature

8/15/10
Date

Contractor's Provisions for Quality Control

Procedures for exercising control within the contractor's organization, the method and frequency of reporting and the distribution of reports is attached to this Statement.

Identification and qualifications of the person(s) exercising such control and their position(s) in the organization are attached to this Statement.

Project: Florence House
Date Prepared: September 2, 2008

Contractor's Statement of Responsibility

Each contractor responsible for the construction or fabrication of a system or component designated in the Quality Assurance Plan must submit a Statement of Responsibility. The Statement of Responsibility is required for Seismic Design Category C or higher. Make additional copies of this form as required.

Project: Florence House

Contractor's Name: LOCBID Construction, Inc.

Address: P.O. Box 607 Sabattus, ME

License No.:

Description of designated building systems and components included in the Statement of Responsibility:

wood shear walls & floor/roof plywood diaphragms

Contractor's Acknowledgment of Special Requirements

I hereby acknowledge that I have received, read, and understand the Quality Assurance Plan and Special Inspection program.

I hereby acknowledge that control will be exercised to obtain conformance with the construction documents approved by the Building Official.

Derek L Ouellette 3/1/2010

Signature Date

Contractor's Provisions for Quality Control

Procedures for exercising control within the contractor's organization, the method and frequency of reporting and the distribution of reports is attached to this Statement.

Identification and qualifications of the person(s) exercising such control and their position(s) in the organization are attached to this Statement.

Project: Florence House
Date Prepared: September 2, 2008

Contractor's Statement of Responsibility

Each contractor responsible for the construction or fabrication of a system or component designated in the Quality Assurance Plan must submit a Statement of Responsibility. The Statement of Responsibility is required for Seismic Design Category C or higher. Make additional copies of this form as required.

Project: Florence House

Contractor's Name: Precision Welding & Fabrication

Address: 690 Stevens Ave. Westbrook, ME 04092

License No.:

Description of designated building systems and components included in the Statement of Responsibility:

Structural & misc steel

Contractor's Acknowledgment of Special Requirements

I hereby acknowledge that I have received, read, and understand the Quality Assurance Plan and Special Inspection program.

I hereby acknowledge that control will be exercised to obtain conformance with the construction documents approved by the Building Official.

Signature

3-3-10
Date

Contractor's Provisions for Quality Control

Procedures for exercising control within the contractor's organization, the method and frequency of reporting and the distribution of reports is attached to this Statement.

Identification and qualifications of the person(s) exercising such control and their position(s) in the organization are attached to this Statement.

Project: Florence House
Date Prepared: September 2, 2008

Fabricator's Certificate of Compliance

Each approved fabricator that is exempt from Special Inspection of shop fabrication and implementation procedures per section 1704.2 of the International Building Code must submit a *Fabricator's Certificate of Compliance* at the completion of fabrication.

Project: Florence House - Portland, ME.

Fabricator's Name: Precision Welding & Fabrication Inc.

Address: 690A Stadwater St. Westbrook, ME. 04092

Certification or Approval Agency: AISC - Standard for Steel Building Structures

Certification Number:

Date of Last Audit or Approval: October 2009

Description of structural members and assemblies that have been fabricated:

Div. # 05120 "Structural Steel"
Div. # 05500 "Metal Fabrications"

I hereby certify that items described above were fabricated in strict accordance with the approved construction documents.

Signature 3-8-10
Date

V.P.
Title

Attach copies of fabricator's certification or building code evaluation service report and fabricator's quality control manual

American Institute of Steel Construction

is proud to recognize

Precision Welding & Fabrication, Inc.

Westbrook, ME

for successfully meeting the quality certification requirements for

Standard for Steel Building Structures

Roger E. Ferch

Certification valid through October 2010

End of Special Inspections Report