Structural Special Inspections Report

Florence House

190 Valley Street Portland, Maine March 8, 2010

Report prepared by:

Structural Engineer of Record Becker Structural Engineers, Inc. 75 York Street Portland, Maine 04101

Florence House

Portland, Maine March 8, 2010

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EXHIBIT A

01000 Special Inspections - General



Statement of Special Inspections

Florence House 190 Valley Street Portland, Maine

Statement Prepared by
Structural Engineer of Record:
Becker Structural Engineers, Inc.
75 York Street
Portland, ME 04101
207. 879. 1838

Owner:

Avesta Florence House, LP 307 Cumberland Ave Portland, ME 04101

Architect of Record: Gawron Turgeon Architects 29 Black Point Road Scarborough, ME 04074 207.863.6307

Contractor:
Ganneston Construction
3025 North Belfast Avenue
Augusta, Maine 04332
207.621.8505

Project:

Project: Florence House Date Prepared: September 2, 2008

Florence House

Structural Statement of Special Inspections

Location:	190 Valley Street, Portland, Maine		
Owner:	Avesta Florence House, LP		
This Stateme	nt of Special Inspections encompass the follo	wing discipline: Structural	
Inspection an services appli	nt of Special Inspections is submitted as a condition of Structural Testing requirements of the Buicable to this project as well as the name of the approved agencies to be retained for conditions.	ilding Code. It includes a e Structural Special Inspect	schedule of Special Inspection tion Coordinator (SSIC) and the
inspection re Responsible (for correction Building Offic	al Special Inspection Coordinator shall kee sports to the Building Code Official (BCO) Charge (SRDP). Discovered discrepancies sl. If such discrepancies are not corrected, to tall and the Structural Registered Design Prosport relieve the Contractor of his or her respondence.	and the Structural Regis nall be brought to the immed he discrepancies shall be fessional in Responsible C	stered Design Professional in diate attention of the Contractor brought to the attention of the
	ts shall be submitted to the Building Offici Charge at an interval determined by the SSIC		istered Design Professional in
correction of	ort of Special Inspections documenting con any discrepancies noted in the inspections Use and Occupancy.		
Job site safety	y and means and methods of construction are	solely the responsibility of	the Contractor.
Interim Repor	t Frequency: \(\sum \begin{aligned} \sum Upon request of Building \end{aligned}\)	Official	or per attached schedule.
Prepared by:			ATE OF MANAGEMENT
James Fortin, I	P.E.		
	name of the Structural Registered Design	_	AMESC.
Professional	n Responsible Charge)	09/02/2008	JAMES C. FORTIN No. 9559
Signature		Date	DAME STORY
			Design Professional Seal
Owner's Author	orization:	Building Code Official's	Acceptance:
Signature	Date	Signature	Date

Project: Florence House

Date Prepared: September 2, 2008

Structural Statement of Special Inspections (Continued)

List of Ag	ents						
Project:	Florence House						
Location:	190 Valley Street, Portland,	Maine					
Owner:	Avesta Florence House, LP						
This Statement	of Special Inspections encom	pass the following discipline: Structural					
`	(Note: Statement of Special Inspections for other disciplines may be included under a separate cover) This Statement of Special Inspections / Quality Assurance Plan includes the following building systems:						
	Soils and Foundations Cast-in-Place Concrete Precast Concrete Syster Masonry Systems Structural Steel Wood Construction	n ☐ Special Cas	es				
Special Inche	otion Agonolos	Cimo	Address Telephone a mail				

Special Inspection Agencies	Firm	Address, Telephone, e-mail
STRUCTURAL Special Inspections Coordinator (SSIC)	James C. Fortin Becker Structural Engineers	75 York Street Portland, Maine 04101 (207) 879-1838 jim@beckerstructural.com
2. Special Inspector (SI 1)	James C. Fortin Becker Structural Engineers	75 York Street Portland, Maine 04101 (207) 879-1838 jim@beckerstructural.com
3. Special Inspector (SI 2)	Bill Peterlein, P.E. Summit Geoengineering Services	640 Main Street Lewiston, Maine 04240 (207) 795-6009 bpeterlein@summitenv.com
4. Testing Agency (TA 1)	Darrell Gilman Summit Environmental Consultants	640 Main Street Lewiston, Maine 04240 (207) 795-6009 dgilman@summitenv.com
5. Testing Agency (TA 2)		
6. Other (O1)		

Note: The inspectors and testing agencies shall be engaged by the Owner or the Owner's Agent, and <u>not</u> by the Contractor or Subcontractor whose work is to be inspected or tested. Any conflict of interest must be disclosed to the Building Official, prior to commencing work.

Project: Florence House

Date Prepared: September 2, 2008

Structural Statement of Special Inspections (Continued)

Final Report of Special Inspections (SSIC/SI 1)

[To be completed by the Structural Special Inspections Coordinator (SSIC/SI 1). Note that all Agent's Final Reports must be received prior to issuance.]

Project:

Florence House

Location:

190 Valley Street, Portland, ME

Owner:

Avesta Florence House, LP

Owner's Address:

307 Cumberland Avenue

Portland, ME 04101

Architect of Record:

Rebecca Dillon

Gawron Turgeon Architects

(firm)

(name)

Structural Registered Design

Professional in Responsible Charge:

Paul B Becker, P.E.

(name)

Becker Structural Engineers, Inc

(firm)

To the best of my information, knowledge and belief, the Special Inspections required for this project, and itemized in the *Statement of Special Inspections* submitted for permit, have been performed and all discovered discrepancies have been reported and resolved.

Interim reports submitted prior to this final report form a basis for and are to be considered an integral part of this final report.

Respectfully submitted,

Structural Special Inspection Coordinator

HAMES FORTH

(Type or print name)

BELLER STRUCTURAL ENGINERS, INC.

(Firm Name)

Signature

3/5/10

JAMES C. FORTIN No. 9559

CENSED SONAL EMPLOYMENT OF MALE SONAL EMPLOYM



ENVIRONMENTAL CONSULTING . GEOTECHNICAL ENGINEERING . CONSTRUCTION MATERIALS TESTING

March 10, 2010 Summit #14194

Becker Structural Engineers, Inc. 75 York Street Portland, ME 04101 Attn: Jim Fortin

Reference:

Testing and Inspection Activity Certification

Florence House Project

Dear Jim:

Summit Geoengineering Services (Summit) has provided materials testing and special inspection services to Avesta Florence House, LP and Florence House Housing Corporation during construction activities at the Florence House project located on Valley Street in Portland, Maine. Summit's responsibilities for these services were identified in the Statement of Statement of Special Inspections dated September 2, 2008 and prepared by Becker Structural Engineers, Inc.

Summit was present to observe construction activities assigned within the earthwork, cast-in-place concrete and masonry sections of the Statement of Structural Special Inspections during construction of the Florence House. Summit subcontracted Quality Assurance Laboratories to provide AWS certified inspection of structural steel welding activities, as required. Summit certifies that, based on on-site observations and related test results, inspection activities were completed in accordance with the supplied project design plans and specifications.

Attached please find a signed copy of the Special Inspector's/Agent's Final Report form for the project. In addition, Darrell Gilman of Summit will provide you with a summary of laboratory testing results and onsite inspection observations for your records.

Thank you for the opportunity to work with Becker Structural Engineers on this project. Please contact us if there are any questions regarding the information provided.

Sincerely yours.

Summit Geoengineering Services

muchal Walk

Michael J. Walsh, P.E.

Senior Engineer

Project: Florence House

Date Prepared: September 2, 2008

Structural Statement of Special Inspections (Continued)

Special Inspector's/Agent's Final Report Project: Florence House Special Inspector or Agent: Designation: To the best of my information, knowledge and belief, the Special Inspections or testing required for this project, and designated for this Inspector/Agent in the Statement of Special Inspections submitted for permit, have been performed and all discovered discrepancies have been reported and resolved. Interim reports submitted prior to this final report form a basis for and are to be considered an integral part of this final report. Respectfully submitted, Special Inspector or Agent: Licensed Professional Seal or **Certification Number**

01000.5 Disclaimers and Qualifications

The program of Structural/Special Tests and Inspections does not relieve the Contractor or its Subcontractors of their responsibilities and obligations for quality control of the work, for any design work which is included in the scope of services, and for full compliance with the requirements of the Construction Documents. Furthermore, the detection of, or the failure to detect, deficiencies or defects in work during testing and inspection conducted pursuant to the Program does not relieve the Contractor or its subcontractors of their responsibility to correct all deficiencies or defects, whether detected or undetected, in all parts of work, and to otherwise comply with all requirements of the Construction Documents. No warrantee is expressed or implied by the issuance of this document. Additional disclaimers and/or qualifications may be included in the Owner-Special Inspection agreement.

EXHIBIT B

01000 Qualifications of Inspectors and Testing Technicians Schedule of Structural Inspections **Project: Florence House**

Date Prepared: September 2, 2008

Structural Schedule of Special Inspections

Qualifications of Inspectors and Testing Technicians

The qualifications of all personnel performing Special Inspection and testing activities are subject to the approval of the Building Official. The credentials of all Inspectors and testing technicians shall be provided to the Special NOTE VERIFICATION THAT QUALIFIED INDIVIDUALS ARE AVAILABLE TO Inspector for their records. PERFORM STIPULATED TESTING AND/OR INSPECTION SHOULD BE PROVIDED PRIOR TO SUBMITTING STATEMENT. AGENT QUALIFICATIONS IN SCHEDULE ARE SUGGESTIONS ONLY; FINAL QUALIFICATIONS ARE SUBJECT TO THE DISCRETION OF THE REGISTERED DESIGN PROFESSIONAL PREPARING THE SCHEDULE.

Key for Minimum Qualifications of Inspection Agents:

When the Registered Design Professional in Responsible Charge or Special Inspector of Record deems it appropriate that the individual performing a stipulated test or inspection have a specific certification, license or experience as indicated below, such requirement shall be listed below and shall be clearly identified within the schedule under the Agent Qualification Designation.

PE/SE Structural Engineer - a licensed SE or PE specializing in the design of building structures PE/GE Geotechnical Engineer – a licensed PE specializing in soil mechanics and foundations

EIT Engineer-In-Training - a graduate engineer who has passed the Fundamentals of Engineering

examination

Experienced Testing Technician

ETT Experienced Testing Technician – An Experienced Testing Technician with a minimum 5 years

experience with the stipulated test or inspection

American Concrete Institute (ACI) Certification

ACI-CFTT Concrete Field Testing Technician - Grade 1

ACI-CCI Concrete Construction Inspector

ACI-LTT Laboratory Testing Technician - Grade 1&2

ACI-STT Strength Testing Technician

American Welding Society (AWS) Certification

AWS-CWI Certified Welding Inspector AWS/AISC-SSI Certified Structural Steel Inspector

American Society of Non-Destructive Testing (ASNT) Certification

ASNT Non-Destructive Testing Technician - Level II or III.

International Code Council (ICC) Certification

ICC-SMSI	Structural Masonry Special Inspector
ICC-SWSI	
	Structural Steel and Welding Special Inspector
ICC-SFSI	Spray-Applied Fireproofing Special Inspector
ICC-PCSI	Prestressed Concrete Special Inspector
ICC-RCSI	Reinforced Concrete Special Inspector

National Institute for Certification in Engineering Technologies (NICET)

NICET-CT	Concrete Technician – Levels I, II, III & IV
NICET-ST	Soils Technician - Levels I, II, III & IV

Geotechnical Engineering Technician - Levels I, II, III & IV **NICET-GET**

Other

EXHIBIT B

02300 Soil and Foundation Construction

Project: Florence House
Date Prepared: September 2, 2008
Structural Schedule of Special Inspections
SOILS & FOUNDATION CONSTRUCTION

VERIFICATION AND INSPECTION IBC Section 1704.7, 1704.8, 1704.9	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS	AGENT	AGENT QUALIFICATION	TASK COMPLETED
Verify existing soil conditions, fill placement and load bearing requirements						
a. Prior to placement of prepared fill, determine that the site has been prepared in accordance with the approved soils report.	Y	P	IBC 1704.7.1	SI2	PE/GE, EIT or ETT	
b. During placement and compaction of fill material, verify material being used and maximum lift thickness comply with the approved soils report.	Y	Р	IBC 1704.7.2	SI2	PE/GE, EIT or ETT	* '
c. Test in-place dry density of compacted fill complies with the approved soils report.	Y	р	IBC 1704.7.2	TAI	PE/GE, EIT or ETT	: :
2. Pile foundations:						
 a. Observe and record procedures for static load testing of piles. 	N/A	C.	IBC 1704.8°		PE/GE, EIT or ETT	
 Observe and record procedures for dynamic load testing of piles. 	N/A	C			PE/GE, EIT or ETT	
 c. Record installation of each pile and results of load test. Include cutoff and tip elevations of each pile relative to permanent reference. 	N/A	C			PE/GE, ETT or ETT	
d. Test welded splices of steel piles	N/A	Ċ	AWS D1.1		AWS-CWI	
 Pier foundations: Verify installation of pier foundations for buildings assigned to Seismic Design Category C, D, E or F. 	N/A	c	IBC 1704.9		PE/GE, EIT or BIT	
a. Verify pier diameter and length	N/A	C			PE/GE, EIT or ETT	
b. Verify pier embedment (socket) into bedrock	N/A	P			PE/GE, EIT or ETT	
c. Verify suitability of end bearing strata	N/A	P			PE/GE, EIT or ETT	

Project: Florence House Date Prepared: September 2, 2008

Structural Schedule of Special Inspections SOILS & FOUNDATION CONSTRUCTION

VERIFICATION AND INSPECTION IBC Section 1704.7, 1704.8, 1704.9	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS	AGENT	AGENT QUALIFICATION	TASK COMPLETED
Verify existing soil conditions, fill placement and load bearing requirements						
 a. Prior to placement of prepared fill, determine that the site has been prepared in accordance with the approved soils report. 	Y	P	IBC 1704.7.1	SI2	PE/GE, EIT or ETT	
 b. During placement and compaction of fill material, verify material being used and maximum lift thickness comply with the approved soils report. 	Y	P	IBC 1704.7.2	SI2	PE/GE, EIT or ETT	
 c. Test in-place dry density of compacted fill complies with the approved soils report. 	Y	р	IBC 1704.7.2	TA1	PE/GE, EIT or ETT	
2. Pile foundations:	á					
a. Observe and record procedures for static load testing of piles.	N/A	C	IBC 1704.8		PE/GE, EIT or ETT	
b. Observe and record procedures for dynamic load testing of piles.	N/A	С			PE/GE, EIT or ETT	
c. Record installation of each pile and results of load test. Include cutoff and tip elevations of each pile relative to permanent reference.	N/A	Ċ			PE/GE, EIT or ETT	
d. Test welded splices of steel piles	N/A	C	AWS D1.1		AWS-CWI	
3. Pier foundations: Verify installation of pier foundations for buildings assigned to Seismic Design Category C, D, E or F.	N/A	C	IBC 1704.9		PE/GE, EIT or ETT	
a. Verify pier diameter and length	N/A	C			PE/GE, EIT or ETT	
b. Verify pier embedment (socket) into bedrock	N/A	P	100		PE/GE, EIT or ETT	16.
c. Verify suitability of end bearing strata	N/A	P	71		PE/GE, EIT or ETT	

SUMMIT GEOENGINEERING SERVICES 434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: 626-9094



DAILY FIELD REPORT

Date:	2/3/09
Project:	Avesta Florence House
Project #:	14194
Site Contacts:	Mike Adams, Ganneston Construction
Purpose of Visit:	Preconstruction earthwork meeting.
Work Activities:	Met onsite with Alan Nichols, Ganneston Construction, and RE Coleman Excavation personnel to discuss earthwork requirements related to the project Getechnical Report prepared by Summit. The major points to the discussion were as follows: 1. Ganneston proposes to stage the preload due to site constraints. Summit has no issue with this approach provided the required materials, placement techniques, and preload timelines are adhered to for both locations. 2. Underlying frost within the preload areas is to be removed and subgrade proof rolled prior to placement of the Structural Fill. 3. Proposed backfill materials should be submitted for laboratory analysis to confirm compliance with design requirements. 4. Summit requests notification to coordinate compaction testing of preload fill lifts to confirm required densities are achieved. 5. Summit recommends that settlement monitoring plates be installed in each of the preload areas, surveyed and elevations recorded daily through the preload periods to confirm the fill has adequately compressed underlying soils prior to building foundation construction.
Test Results:	None.
Remarks:	See above.
Portal to Portal	Expenses Signed: Michael J. Walsh, P.E.
Leave:	8:30am Mileage: 11 cc:
Return:	10:00am Density Gauge:
TOTAL:	1.5 hrs Other:
	Sent: 2/3/09

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DAILY FIELD REPORT

Date:	3/11/09									
Project:	Avesta Florence House									
Project #:	14194	r .								
Site Contacts:	Mike Adams, Ganneston Construction									
Purpose of Visit:	Soil sampling.	÷								
Work Activities:		Obtained a sample of the existing onsite granular borrow for laboratory testing to the meet the project specification for structural fill and/or surcharge fill.								
		1								
		,								
Test Results:										
Remarks:										
Portal to Portal Leave: Return: TOTAL:	Expenses Signed: 1:30 Mileage: 11 cc: 2:30 Density Gauge: 1 Other:	Darrell A. Gilman								
		rell A. Gilman, CMT Manager /11/09								

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DAILY FIELD REPORT

Date:	3/30/09								
Project:	Avesta Florence House								
Project #:	14194								
Site Contacts:	Mike Adams, Ganneston Construction								
Purpose of Visit:	Compaction report from onsite test performed on 3-27-09.								
Work Activities:	A crew from Coleman Excavation placed and compacted 4" minus material along N-line from line 8.2 to 16. The material was compacted with a vibratory plate compactor. Sample # 14194.1-S4 maximum density determined to be 136.5pcf at 6.0% moisture.								
Test Results:	Compaction range: 96.6% to 98.8% All tests performed on line "N" from 8.2 to 16 exceed the 95% compaction requirement.								
					$C^{2} \rightarrow \chi$				
Remarks:									
Portal to Portal Leave: Return: TOTAL:	12:30 12:46 0.25	Expenses Mileage: Density Gauge: Other:		Signed: cc: Reviewed: Sent:	Darrell Gilman Darrell A. Gilman, CMT Manager 3/31/09				

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Return: TOTAL:

DAILY FIELD REPORT

		,			:
Date:	4/10/09				1
Project:	Avesta Florence	e House			
Project #:	14194				;
Site Contacts:	Mike Adams, (Ganneston Construc	etion		
Purpose of Visit:	Special Inspect	tions - Soils.			
Work Activities:	had caved in or section 1704.1, was visible, the	n the area of footing	g subgrade i - 1 - A. A make a det	that was to be i small area that ermination on t	had been excavated
Test Results:	N/A				1
Remarks:					
Portal to Portal		Expenses		Signed:	Frank Clark
Leave:	11:00am	Mileage:	11	cc:	
Return:	12:45pm	Density Gauge:			

1.75

Other:

Reviewed: Sent:

1.75 tolls

Darrell A. Gilman, CMT Manager 4/14/09

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TOTAL:

DAILY FIELD REPORT

Date:	4/13/09					
Project:	Avesta Florence	e House				•
Project #:	14194					
Site Contacts:	Mike Adams, (Ganneston Construct	tion			
Purpose of Visit:	Soil Compacti	on Testing.				
Work Activities:	compacted the inspection, sec vibratory roller 1704.7.2, 1-C.	oleman Excavation 4" minus material a tion 1704.7.2, 1-B. 7 The area was teste All tests taken excee area was inspected 1, 1-A.	long F-line The materia d for densi ed the requ	from line 7 al was compa ty per special ired 95 perce	to 16 per so acted with I inspection ont compa	special a 5-ton on, section ction. Prior to
Test Results: Remarks:	Dry density: 13	ection: 95.5 to 98 30.4 to 133.8 pcf 36.9 to 137.8 pcf re: 3.0 to 3.6%				
Portal to Portal						
Leave:		Expenses		Signed:	Fı	rank Clark
Return:	11:45am	Mileage:	11	cc:		*

Density Gauge:

Other:

2:30pm

2.75

Reviewed: Darrell A. Gilman, CMT Manager Date: 4/17/09

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DAILY FIELD REPORT

Date:

4/20/09

Project:

Avesta Florence House

Project #:

14194

Site Contacts:

Mike Adams, Ganneston Construction

Purpose of Visit:

Soil Compaction Testing.

Work Activities:

A crew from Coleman Excavation placed approximately a 12" lift and then compacted the 4" minus material along A-line to line 7 per special inspection, section 1704.7.2, 1-B. The material was compacted with a 5-ton vibratory roller. The area was tested for density per special inspection, section 1704.7.2, 1-C. All tests taken exceed the required 95 percent compaction. Prior to compaction the area was inspected and found to be suitable per special inspection, section

1704.7.1, 1-A.

Picked up two sets of four test cylinders for lab testing and controlled storage.

Test Results:

Percent compaction: 97.6 to 97.7 Dry density: 126.4 to 126.5 pcf Wet density: 130.1 to 130.5 pcf Percent moisture: 3.7 to 4.0%

Remarks:

Portal to Portal

Leave:		Expenses		Signed:	Mike Sullivan
Return:	9:00	Mileage:	11	cc:	
TOTAL:	10:00	Density Gauge:	1		•
	1	Other:		<u> </u>	

Reviewed:

Darrell A. Gilman, CMT Manager 4/21/09

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: 626-9094



DAILY FIELD REPORT

Date:

4/28/09

Project:

Avesta Florence House

Project #:

14194

Site Contacts:

Mike Adams, Ganneston Construction

Purpose of Visit:

Soil Compaction Testing.

Work Activities:

A crew from Coleman Excavation placed approximately a 12" lift and then compacted the 4" minus material along G-1,L.8-1,N-1&N-5.2 footing per special inspection, section 1704.7.2, 1-B. The material was compacted with a 5-ton vibratory roller. The area was tested for density per special inspection, section 1704.7.2, 1-C. All tests taken exceed the required 95 percent compaction. Prior to compaction the area was inspected and found to be suitable per special inspection, section 1704.7.1, 1-A.

Test Results:

Percent compaction: 95.0 to 98.7

Dry density: 123.0 to 127.9 pcf Wet density: 128.4 to 131.7 pcf Percent moisture: 3.0 to 4.6%

Remarks:

Portal to Portal

Leave:		Expenses		Signed:	Mike Sullivan
Return:	7:00	Mileage:	11	cc:	
TOTAL:	9:00	Density Gauge:	1		
	2	Other:		_	

Reviewed: Date: Darrell A. Gilman, CMT Manager 4/30/09

SUMMIT GEOENGINEERING SERVICES 434 Cony Road, Augusta, Maine 04330

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DAILY FIELD REPORT

Date:	5/4/09
Project:	Avesta Florence House - Portland, Maine
Project #:	14194
Site Contacts:	Mike Adams, Ganneston Construction
Purpose of Visit:	Subgrade soil inspection.
Work Activities:	A crew from Coleman Excavation has excavated for the interior column footings at L-9, L-11, L-12, L-13, L-14, J-14, J-13, and H-10. The subgrade soil was inspected per special inspection section 1704.7.1ab and was found acceptable with the following observations: -total material removed to bottom of footing ~6" to 8". -soil was consistent with the geotechnical report dated 8-9-2007. -a smooth edged bucket was used to excavate to final grade with no over excavation of subgrade soil. -the surface of the subgrade was compacted with a 500 lb. plate compactor with no
Remarks:	In conversation with Everett Stewart of Ganneston, a RFI had been issued for interior column footings to bear on native subgrade soil unless the material had been disturbed.
Portal to Portal Leave: Return: TOTAL:	Expenses Signed: Darrell Gilman 10:00 Mileage: 13 cc: 11:00 Density Gauge: 1 Other:

Reviewed: Darrell A. Gilman, CMT Manager Sent: 5/6/09

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: 626-9094



DAILY FIELD REPORT

Date:

7/28/09

Project:

Avesta Florence House

Project #:

14194

Site Contacts:

Mike Adams, Ganneston Construction

Purpose of Visit:

Soil Compaction Testing.

Work Activities:

A crew from Ganneston Construction placed approximately a 12" lift and then compacted the onsite structural fill material along line 15 from L-line to M-line. The material was compacted with a walk behind compactor. The crew then compacted a 1" to 2" lift of the onsite barrow material with 1 1/2" stone to make it easier to shim grade. All tests taken exceed the required 95 percent compaction.

Test Results:

Percent compaction: 96.0 to 98.7

Dry density: 125.8 to 127.8 pcf Wet density: 132.1 to 133.9 pcf Percent moisture: 4.7 to 5.0%

Remarks:

Portal to Portal

Leave:		Expenses		Signed:	Justin Rouillard
Return:	2:00pm	Mileage:	11	_cc:	
TOTAL:	3:30pm	Density Gauge:	X	_	
	1.5	Other:			
				_	

Reviewed: Date: Darrell A. Gilman, CMT Manager 7/30/09

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DAILY FIELD REPORT

Date:

8/3/09

Project:

Avesta Florence House

Project #:

14194

Site Contacts:

Mike Adams, Ganneston Construction

Purpose of Visit:

Soil Compaction Testing.

Work Activities:

A crew from Ganneston Construction placed approximately a 12" lift and then compacted the onsite structural fill material for the first floor. The material was compacted with a walk behind compactor. The crew then compacted a 1" to 2" lift of the onsite barrow material with 1 1/2" stone to make it easier to shim grade. All tests taken exceed the required 95 percent compaction.

Test Results:

Percent compaction: 96.9 to 101.1

Dry density: 125.4 to 130.9 pcf Wet density: 128.7 to 134.2 pcf Percent moisture: 2.5 to 3.8%

Remarks:

Portal to Portal

Leave:		Expenses		Signed:	Justin Rouillard
Return:	8:00	Mileage:	11	cc:	
TOTAL:	11:00	Density Gauge:	X		•
	3	Other:			:

Reviewed:

Darrell A. Gilman, CMT Manager 8/10/09

Date:

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: 626-9094



DAILY FIELD REPORT

Date:

8/27/09

Project:

Avesta Florence House - Portland, Maine

Project #:

14194

Site Contacts:

Mike Adams, Ganneston Construction

Purpose of Visit:

Density Testing

Work Activities:

Three compaction tests were taken on the outside of the foundation wall between line F and line J and between line 10 and line 16. A sample was retreived and brought to the lab for

testing. All three tests exceeded the 95% compaction requirement.

Test Results:

Percent compaction: 95.8% to 99.5%

Dry density: 122.6 to 127.4 pcf Wet density: 130.2 to 134.1 pcf Percent moisture: 5.3 to 6.6%

Remarks:

Grout was being placed at the 3rd level of the stairwell.

Portal to Portal		Expenses		Signed:	Justin Rouillard
Leave:	8:45am	Mileage:	11	cc:	
Return:	10:15am	Density Gauge:	X		
TOTAL:	1.5	Other:			•
				Reviewed:	Darrell A. Gilman, CMT Manager

Sent: 9/8/09

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: 626-9094



DAILY FIELD REPORT

Date:

9/1/09

Project:

Avesta Florence House - Portland, Maine

Project #:

14194

Site Contacts:

Mike Adams, Ganneston Construction

Purpose of Visit:

Density Testing

Work Activities:

Eight compaction tests were taken on the outside of the foundation wall along N line from 5 line to 14 line. Material was being placed via a conveyor truck and was being compacted

with a walk behind wacker in 12" lifts. All tests exceeded 95% compaction.

Test Results:

Percent compaction: 95.3% to 98.0%

Dry density: 122.6 to 125.4 pcf Wet density: 129.4 to 134.0 pcf Percent moisture: 5.7 to 7.3%

Remarks:

Grout was being placed at the 3rd level of the stairwell.

Portal to Portal		Expenses		Signed:	Justin Rouillard
Leave:	10:45am	Mileage:	22	cc:	
Return:	5:30am	Density Gauge:	X		
TOTAL:	6.75	Other:			
				Reviewed	Darrell A. Gilman, CMT Manager

Sent: Darrell A. Gilman, CMT Mana Sent: 9/8/09

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: 626-9094



DAILY FIELD REPORT

Date:

9/2/09

Project:

Avesta Florence House - Portland, Maine

Project #:

14194

Site Contacts:

Mike Adams, Ganneston Construction

Purpose of Visit:

Density Testing

Work Activities:

Fifteen compaction tests were taken on the outside of the foundation wall along N line from

5 line to 14 line. Material was being placed via a conveyor truck and was being compacted

with a walk behind wacker in 12" lifts. All tests exceeded 95% compaction.

Test Results:

Percent compaction: 95.2% to 98.3%

Dry density: 121.9 to 125.8 pcf Wet density: 128.3 to 132.7 pcf Percent moisture: 4.2 to 6.5%

Remarks:

Portal to Portal		Expenses		Signed:	Justin Rouillard
Leave:	8:30am	Mileage:	22	cc:	
Return:	6:30am	Density Gauge:	X		
TOTAL:	.10	Other:			
				Daviowed:	Domall A Gilman CMT Managar

Reviewed Sent: Darrell A. Gilman, CMT Manager 9/8/09

1

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: 626-9094



DAILY FIELD REPORT

Date:

9/4/09

Project:

Avesta Florence House - Portland, Maine

Project #:

14194

Site Contacts:

Mike Adams, Ganneston Construction

Purpose of Visit:

Density Testing

Work Activities:

Six compaction tests were taken on the outside of the foundation wall along N line from 5 line to 12 line. Material was being placed via a conveyor truck and was being compacted

with a walk behind wacker in 12" lifts. All tests exceeded 95% compaction.

Test Results:

Percent compaction: 95.3% to 98.2%

Dry density: 122.0 to 125.8 pcf Wet density: 133.3 to 128.2 pcf Percent moisture: 4.6 to 6.1%

Remarks:

Portal to Portal		Expenses		Signed:	Justin Rouillard
Leave:	7:00am	Mileage:	22	cc:	
Return:	6:45pm	Density Gauge:	X		
TOTAL:	11.75	Other:			
				Reviewed:	Darrell A. Gilman, CMT Manager

Sent: 9/10/09

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: 626-9094



DAILY FIELD REPORT

Date:

9/9/09

Project:

Avesta Florence House - Portland, Maine

Project #:

14194

Site Contacts:

Mike Adams, Ganneston Construction

Purpose of Visit:

Density Testing

Work Activities:

Eight compaction tests were taken on the outside of the foundation wall along N line from 4 line to 7 line. Material was being placed via frontend loader and was being compacted with

a walk behind wacker in 12" lifts. All tests exceeded 95% compaction.

Test Results:

Percent compaction: 99.0% to 105.3%

Dry density: 113.4 to 120.6 pcf Wet density: 116.8 to 123.7 pcf Percent moisture: 3.0 to 4.2%

Remarks:

Portal to Portal		Expenses		Signed:	Justin Rouillard
Leave:	8:00am	Mileage:	22	cc:	
Return:	4:30pm	Density Gauge:	X		
TOTAL:	8.5	Other:			
				Davison de	Damell A. Cilman, CMT Manager

Reviewed: Sent: Darrell A. Gilman, CMT Manager 9/10/09

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: 626-9094



DAILY FIELD REPORT

Date:

9/10/09

Project:

Avesta Florence House - Portland, Maine

Project #:

14194

Site Contacts:

Mike Adams, Ganneston Construction

Purpose of Visit:

Density Testing

Work Activities:

One compaction test was taken on the outside of the foundation wall at the intersection of the M.5 line and the 4.5 line. Material was being placed via front end loader and was being compacted with a walk behind wacker in 12" lifts. All tests exceeded 95% compaction.

Test Results:

Percent compaction: 103.5%

Dry density: 118.6 pcf Wet density: 122.3 pcf Percent moisture: 3.1%

Remarks:

Portal to Portal		Expenses		Signed:	Justin Rouillard
Leave:	10:00am	Mileage:	22	cc:	
Return:	2:30pm	Density Gauge:	X		
TOTAL:	4.5	Other:		_	
		_		Reviewed:	Darrell A. Gilman, CMT Manager

Sent:

9/14/09

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: 626-9094



DAILY FIELD REPORT

Date:

10/20/09

Project:

Avesta Florence House - Portland, Maine

Project #:

14194

Site Contacts:

Mike Adams, Ganneston Construction

Purpose of Visit:

Density Testing

Work Activities:

Michael Walsh of Summit arrived onsite to provide compaction testing support during backfilling of the segmental block retaining wall. A crew from R.E. Coleman was placing the first course of block from approximately Station 0+92 to Station 1+12. Met with Everett Stewart of Ganneston and reviewed backfill requirements in project specifications and segmental wall plans by SRG Engineering, Inc. Justin Rouillard of Summit was onsite for testing at 11:15 am. However, lifts were not completed prior to leaving site at 2:00pm.

Testing rescheduled for 10:00am tomorrow.

Ί	'est	Res	ults:
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Remarks:

Portal to Portal		Expenses		Signed:	M. Walsh/Justin Rouillard
Leave:	8:45am	Mileage:	11	cc:	•
Return:	2:00pm	Density Gauge:	X		
TOTAL:	3	Other:			
				Reviewed:	Darrell A. Gilman, CMT Manager

Sent:

10/27/09

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: 626-9094



DAILY FIELD REPORT

Date:

10/21/09

Project:

Avesta Florence House - Portland, Maine

Project #:

14194

Site Contacts:

Mike Adams, Ganneston Construction

Purpose of Visit:

Soil Compaction Testing Services.

Work Activities:

Performed soil compaction testing inside the retaining wall south side. A crew from R.E. Coleman placed and compacted retaining wall backfill material in approximately 8" lifts. The maximum dry density of the material being placed had changed since laboratory testing was conducted, therefore a new sample was obtained and compaction percentages had to be back figured using the new dry density value. The back figured values revealed that two of the four compaction tests taken today did not meet the 95% compaction requirement. All other compaction tests exceeded the 95% compaction requirement. Everett Stewart was made aware of the situation via telephone message.

Test Results:

Percent compaction: 92.2% to 97.1 Dry density: 104.2 to 109.7 pc Moisture %: 3.4 to 9.0%

Remarks:

Portal to Portal		Expenses		Signed:	Neil Davis
Leave:	9:45am	Mileage:	11	cc:	
Return:	1:15pm	Density Gauge:	X		
TOTAL:	3.5	Other:			
				Periowed:	Darrell A Gilman CMT Manager

Reviewed: Sent: Darrell A. Gilman, CMT Manager 10/26/09

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: 626-9094



DAILY FIELD REPORT

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10/29/09

Project:

Avesta Florence House

Project #:

14194

Site Contacts:

Mike Adams, Ganneston Construction

Purpose of Visit:

Soil Compaction Testing.

Work Activities:

Prior to my arrival, a crew from Coleman Excavation had placed and compacted the subbase fill for the upper and lower parking lots. We obtained a sample of the material for a grain size analysis and proctor. Compaction tests were taken and results will be back figured upon the completion of the proctor.

Test Results:

Percent compaction:

Dry density: 135.8 to 136.7 pcf Wet density: 139.1 to 141.7 pcf Percent moisture: 3.4 to 3.8 %

Remarks:

Portal to Portal

Leave:		Expenses		Signed:	Frank Clark
Return:	3:15pm	Mileage:	11	cc:	
TOTAL:	4:45pm	Density Gauge:	1		
	1.5	Other:			

Reviewed: Date: Darrell A. Gilman, CMT Manager

10/30/09



434 Cony Road, Augusta, Maine 04330 Tel: (207) 621.8334 Fax: (207) 626.9094

SUMMIT GEOENGINEERING FIELD DENSITY RESULTS SUMMARY

Proctor Information:

Avesta Florence House PROJECT NAME: CLIENT:

14194

PROJECT NO:

Avesta Florence House. LP

					In-place	Maximum			
Test				Moisture	Density DD	Density	Percent	Required	
Number	Date	Location:	Lift Elevation	Content %	PCF	PCF	Compaction	Compaction	Remarks
	_	3/27/2009 N-line at 8.2-line	Bottom of Footing	4.1	131.8	136.5	9.96	95	Pass
2	_	3/27/2009 N-line at 12-line	Bottom of Footing	3.4	132.1	136.5	96.8	95	Pass
3		3/27/2009 N-line at 16-line	Bottom of Footing	2.9	134.8	136.5	98.8	95	Pass
-		E-7	Bottom of Footing	4	126.5	129.5	97.7	1	95 Pass
2		A-1	Bottom of Footing	3.7	126.4	129.5	97.6	95	95 Pass
_		28-Apr N-5.2 footing	Bottom of Footing	3.4	123.5	129.5	95.4	95	95 Pass
2		N-1	Bottom of Footing	3	127.9	129.5	7.86	95	95 Pass
3		L.8-1	Bottom of Footing	3.6	126.6	129.5	97.8	95	95 Pass
4		G-1	Bottom of Footing	4.6	123	129.5	95	95	95 Pass
1		H-5	finished grade	3.4	127.1	129.5	98.1	95	95 Pass
2			finished grade	3.6	127.5	129.5	98.4	95	95 Pass
3			finished grade	3.8	127.6	129.5	98.5	95	95 Pass
4	_		finished grade	2.5	130.9	129.5	101.1	95	95 Pass
5	\Box		finished grade	2.6	125.4	129.5	6.96	95	Pass
_	_		3' BTW	9.9	123.6	128	9.96	95	Pass
2	_		3' BTW	5.3	127.4	128	99.5	95	Pass
3	<u>~ </u>		3' BTW	6.2	122.6	128	95.8	95	Pass
Ψ-	_	9/1/2009 Outside N line at 5.8	18' BTW	2.3	122.5	128	95.7	95	Pass
2	_	9/1/2009 Outside N line at 7.1	16.5' BTW	6.9	125.4	128	97.9	95	Pass
3	\perp	9/1/2009 Outside N line at 11	16' BTW	6.2	123.3	128	96.3	95	Pass
4		9/1/2009 Outside N line at 13	15'BTW	6.4	122.4	128	92.6	95	Pass
5		9/1/2009 Outside N line at 12	14'BTW	2.3	122.8	128	95.9	95	Pass
9		9/1/2009 Outside N line at 5.8	15'BTW	6.2	123.8	128	2.96	95	Pass
		9/1/2009 Outside N line at 11.5	13' BTW	5.9	125.4	128	86	95	Pass
∞ •	╛	9/1/2009 Outside N line at 6.5	14' BTW	6.2	122.6	128	95.3	36	Pass
- (9/2/2009	9/2/2009 Outside N line at 10	12' BTW	6.5	122.5	128	95.7	95	Pass
	9/2/2009	9/2/2009 Outside N line at 6.5	13' BTW	5.3	122.4	128	92.6		Pass
, C	9/2/2009	9/2/2009 Outside N line at 11	11' BTW	6.1	123	128	96.1	95	Pass
4 r	9/2/2009	9/2/2009 Outside N line at 7	11' BTW	6.2	122.5	128]2.36	95	Pass
o (9/2/2009	9/2/2009 Outside N line at 14		5.6	121.9	128	95.2	95	95 Pass
1 0	9/2/2009	Outside N line at 8	10' BTW	4.2	123.3	123	96.3	95	95 Pass
, c	9/2/2009	Outside N line at 9		4.8	122.4	128	92.6	95	95 Pass
0 0	9/2/2009	Outside N line at f	10' BTW 2500	5.5	123.6	128	9.96	95	95 Pass
ກ	9/2/2009	Outside N line at 5.5	9' BTW	6.1	123.1	128	96.2	95	95 Pass

35	88	550	880	iss	lss	Pass	ISS	ISS	1SS	ISS	SS	SS	55	SS			SS	SSI
95 Dass	95 Lass	95 Pass	95 Pass	95 Pass	95 Pass	95 Pe	95 Pass	95 Pass	95 Pass	95 Pass	95 Pass	95 Pass	95 Pass	95 Pass	95 Fail	95 Fail	95 Pass	95 Pass
95.4	96.1	95.3	98.4	96.4	98.3	66	105.3		102.2	101.6	101.4	103.3	100.5	103.5	93.6	92.2	96.1	97.1
128					128	114.5	114.5	114.5	114.5	114.5	114.5		114.5			•	113	113
6.2 122.1				1,	4.6 125.8	3 113.4	2.6 120.6	2.4 114.1	.1 117	4 116.3	5 16.1	4.1 118.3	`	·	•	•		9 109.7
9) (0	9	5	9	4		2	2	4	ĸ	4	4	4	3.1			45.4 8.4	5.4
9' BTW	8' BTW	13' BTW	12' BTW	11' BTW	10' BTW	12' BTW	11' BTW	10' BTW	9' BTW	8' BTW	7' BTW	6' BTW	5' BTW	7' BTW	7	7	7	4
9/2/2009 Outside N line at 16	9/2/2009 Outside N line at 16	9/2/2009 Outside N line at 5	9/2/2009 Outside N line at 6		1			9/9/2009 Outside N line at 4.8				9/9/2009 Outside N line at 6	9/9/2009 Outside N line at 6.3	6	10/21/09 Inside Retaining Wall - South Quarter	10/21/09 Inside Retaining Wall - 2nd to S Quarter	3 10/21/09 Inside Retaining Wall - South Quarter	10/21/09 Inside Retaining Wall - 2nd to S Quarter
12	11	12	13	14	15	· ·	7	(י)	4	5	9	~	∞	6	_	W	ന	4

EXHIBIT B

03300 Concrete Construction

Project: Florence House Date Prepared: September 2, 2008

Structural Schedule of Special Inspections CONCRETE CONSTRUCTION

VERIFICATION AND INSPECTION IBC Section 1704.4	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS	AGENT	AGENT QUALIFICATION	TASK COMPLETED
1. Inspection of reinforcing steel, including prestressing tendons, and placement	Y	P	ACI 318: 3.5, 7.1-7.7	SII	PE/SE or EIT	JUF
2. Inspection of reinforcing steel welding in accordance with Table 1704.3, Item 5B	N		Welding of Reinf Not Allowed	TA2	AWS-CWI	NIA
 Inspect bolts to be installed in concrete prior to and during placement of concrete where allowable loads have been increased 	Y	С	IBC 1912.5	SI1	PE/SE or EIT	TH
4. Verifying use of required design mix	Y	P	ACI 318: Ch 4, 5.2-5.4	SII	PE/SE or EIT	RF
5. At time fresh concrete is sampled to fabricate specimens for strength test, perform slump and air content test and temperature	Y	С	ASTM C 172 ASTM C 31 ACI 318: 5.6, 5.8	TA1	ACI-CFTT or ACI-STT	Serveludal
6. Inspection of concrete and shotcrete placement for proper application techniques	N/A	С	ACI 318: 5.9, 5.10		PE/SE or EIT	
7. Inspection for maintenance of specified curing temperature and techniques	Y	P	ACI 318: 5.11- 5.13	SII	PE/SE or EIT	SIF
8. Inspection of Prestressed Concrete		: #\!	/			
a. Application of prestressing force.	N/A	C	ACI 318: 18.20		PE/SE or EIT	
b. Grouting of bonded prestressing tendons in seismic force resisting system	N/A	C	ACI 318: 18.18.4		PE/SE or EIT	
9. Erection of precast concrete members	N/A	P	ACI 318; Ch 16		PE/SE or EIT	
10. Verification of in-situ concrete strength, prior to stressing of tendons in post-tensioned concrete and prior to removal of shores and forms beans and structural slabs	N/A	P	ACI 318: 6.2		ACI-STI	

$\underbrace{B\ E\ C\ K\ E\ R}_{\text{structural engineers, inc.}}$

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	1858

OBSERVATION REPORT Cast in Place Concrete

Date:	04/01/09
Time:	10:00 to 10:30 AM
Temp:	Mid 40's
Weather:	Sunny

Observation Location: Footings Line N from 8.2 to 16

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Reinforcement Size					
Quantity					
Condition					
Placement					
Embed/Anchors					
Lap Splices					
Hot Weather				\boxtimes	
Cold Weather				\boxtimes	
Bond Beams				\boxtimes	•
Additional Items					
Additional Items					

Notes:

Signed: Matthew J. Miller, P.E.

$\underbrace{B\ E\ C\ K\ E\ R}_{\text{structural engineers, inc.}}$

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	1858

OBSERVATION REPORT Cast in Place Concrete

Date:	Friday, 04/10/09
Time:	12:45PM
Temp:	Low 50's
Weather:	Sunny

Observation Location: Wall reinforcing and formwork (Valley Street side) along N line, 16 line to 8.2 line and 16 line, from the N corner to approximately 12 feet toward St. John's Street

	· · · · · · · ·		T		}
	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Reinforcement Size	M	П	П	П	
Quantity	Ø		П	П	
Condition	\boxtimes				3
Placement	\boxtimes				
Embed/Anchors			\boxtimes		
Lap Splices	\boxtimes				
Hot Weather				\boxtimes	
Cold Weather				\boxtimes	
Bond Beams				\boxtimes	4
Additional Items					
Additional Items					

Notes:

I met with Dan Beaulieu and Everett Stewart of Ganneston, and Mike Tindle of Newman concrete. Newman Concrete was seeking clarification of the pier and brick shelf conditions of the wall that was in place. Newman's crews were packing up for the weekend and it was agreed that we would meet again on Monday morning. The reinforcing that was in place at the time appeared to conform with the project documents.

Signed: Ethan A. Rhile, P.E.

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	1858

OBSERVATION REPORT Cast in Place Concrete

Date:	Monday, 04/13/09
Time:	7:55AM
Temp:	Low 30's
Weather:	Windy with sun, Chilly

Observation Location: Wall reinforcing and formwork (Valley Street side) along N line, 16 line to 8.2 line and 16 line, from the N corner to approximately 12 feet toward St. John's Street

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Reinforcement Size	M	П	П	П	Odminone
Quantity	Ø		П		
Condition	$\overline{\boxtimes}$				
Placement	\boxtimes				, and the same of
Embed/Anchors			$\overline{\boxtimes}$		
Lap Splices	\boxtimes				
Hot Weather				\boxtimes	
Cold Weather				\boxtimes	
Bond Beams				\boxtimes	
Additional Items					
Additional Items					

Notes:

At the time of my visit the project site was taking delivery of reinforcment. I met with Dan Beaulieu and Everett Stewart of Ganneston, and Mike Tindle of Newman concrete and presented draft sketches CSKS-01/02/03 to clarify the pier and brick shelf conditions. Field direction was given to: Raise 6 feet of brick shelf along N line from the corner up to 55'-3" to match the elevation along the 16 line, revise the location of the (5) number 8 reinforcing bars between 14 & 13 to be below the pier tops in these locations and to add (2)#5x6'-0" below the brick shelfs where the pier bond outs extended above the shelf.

Signed: Ethan A. Rhile, P.E.

$\underbrace{B\ E\ C\ K\ E\ R}_{\text{structural engineers, inc.}}$

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	1858

OBSERVATION REPORT Cast in Place Concrete

Date:	Monday, 04/13/09
Time:	2:30PM
Temp:	40's
Weather:	Windy with sun, Chilly

Observation Location: Wall reinforcing and formwork (Valley Street side) along N line, 16 line to 8.2 line and 16 line, from the N corner to approximately 12 feet toward St. John's Street

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Reinforcement Size	\square			П	Commone
Quantity	\boxtimes				
Condition	\boxtimes				
Placement	\boxtimes				
Embed/Anchors			\boxtimes		
Lap Splices	\boxtimes				
Hot Weather				\boxtimes	
Cold Weather				\boxtimes	
Bond Beams				\boxtimes	
Additional Items					
Additional Items				П	

Notes:

I returned to review the work discussed in the morning. A majority of the work discussed in the morning was completed. It was requested that Newman install (2) #6 in place of one of the #8 reinforcing bars between 13 & 14 that had been accidentially been cut to create the column bond outs.

Signed: Ethan A. Rhile, P.E.

$\frac{B E C K E R}{\text{structural engineers, inc.}}$

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI 2058

OBSERVATION REPORT Cast in Place Concrete

Date: Wednesday, 04/22/09
Time: 3:20DM 4:20 DM

Time: 3:30PM - 4:30 PM

Temp: Low 40's

Weather: Cloudy with Rain Showers

Observation Location: Strip footing along Line A and along Line 7 from Line A to west of Line F where the footing was already in place.

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Reinforcement Size	\boxtimes				
Quantity	\boxtimes				
Condition	X				
Placement	\boxtimes				
Embed/Anchors	X				
Lap Splices	$\overline{\boxtimes}$ l			П	
Hot Weather			П	$\overline{\square}$	
Cold Weather			П	Ø	
Bond Beams		ī	П	X	
Additional Items			П	Ø	
Additional Items			П	Ħ	
Notes:	i				

Work was found to be in general conformance with the construction documents. Reinforcement was located adequately and footing forms appear to be located to properly size the footing. The concrete sub-contractor was still working along Line 7 during this visit, and not all reinforcement had been placed. The reinforcement needed in this area was discussed with the concrete superintendent, and this work will be completed tomorrow prior to the concrete placement.

Please contact me with any questions or comments. Thank you -

Project:		Florence House				
Location:		Portland, Maine				
Becker Jo	b No:	SI 2058				
Date:	Friday, 05/01/09					
Time:	7:30AM - 9:00 AM					
Temp:	Low 40's					
Weather:	Cloudy with Rain Showers					

OBSERVATION REPORT

Cast in Place Concrete

Observation Location: Foundation Wall along F-Line and remainder of 16-Line, elevator pit mat with adjacent F4 footings at Grid M/5 & M/5.8.

Note: A preliminary review of the major foundation wall reinforcement was completed on Tuesday, April 28th, prior to standing the interior wall forms.

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Reinforcement Size	\boxtimes				
Quantity	\boxtimes				•
Condition	\boxtimes				
Placement	\boxtimes		\boxtimes		
Embed/Anchors	\square				
Lap Splices	\boxtimes				
Hot Weather				X	
Cold Weather				$\overline{\boxtimes}$	
Bond Beams				$\overline{\boxtimes}$	
Additional Items				$\bar{\boxtimes}$	
Additional Items				$\overline{\boxtimes}$	
Notes:					

Review of the areas referenced above found that rebar and embedments layout appeared in general conformance with the construction documents and supplemental sketches. Upon arrival, it was noted that the U-bars and slab dowels identified on sketch CSKS-5 were not in place. The concrete sub-contractor was aware of this, and had begun placing the u-bars prior to my departure. I discussed this with Everett, and he stated that he would confirm that the missing bars are in place prior to pouring the wall.

Please contact me with any questions or comments. Thank you -

Project:		Florence House			
Location:		Portland, Maine			
Becker Jo	b No:	SI 2058			
Date:	Wednesday, 05/06/09				
Time:	11:30AM - 1:00 PM				
Temp:	Low 50's				
Weather:	Cloudy				

OBSERVATION REPORT

Cast in Place Concrete

Observation Location: Foundation Walls along the remainder of 7-Line & A-Line, elevator pit walls, strip footing along Line 1 from Line L.8 to H, and site retaining wall footing off Line 1. Also spread footings at Grid J/14, L/14, J/13, L/13, L/12, L/11, J/10 & L/9 and CMU footings along Lines 13, 14, & L.

Note: A preliminary review of the major foundation wall reinforcement was completed on Monday, May 4^{th} , prior to standing the interior wall forms.

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Reinforcement Size	\boxtimes				
Quantity	×		X		
Condition	\boxtimes				
Placement	\boxtimes		\boxtimes		
Embed/Anchors	\boxtimes		\boxtimes		
Lap Splices					
Hot Weather				$\overline{\square}$	
Cold Weather				$\overline{\boxtimes}$	
Bond Beams				$\overline{\boxtimes}$	
Additional Items				$\overline{\boxtimes}$	
Additional Items				$\overline{\square}$	
Notes:					

Review of the areas referenced above found that rebar and embedments layout appeared in general conformance with the construction documents and supplemental sketches. A majority of the wall rebar was in placed and appeared adequate, but work was ongoing and the following items were noted that will need to be addressed prior to pouring. We understand that a concrete sub-contractor is likely aware of a majority of these items, but provide them for reference to ensure they are not missed:

1. Per CSKS10, it is specified to pour the (2) interior footings at Line 13 & 14 integrally. These footings are currently formed separately. Although preferable to place as one integral footing, it is adequate to fill space between footings with concrete as a separate placement.

- 2. Reinforcement in many of the interior footings needs to be straightened out and lifted off the ground to maintain rebar cover requirements.
- 3. Reinforcement in the CMU wall strip footings was in contact with the earth and needs to be placed on chairs to obtain proper rebar cover.
- 4. Z-bars specified on CSKS-10 are not yet in place.
- 5. Dowels for CMU walls shall be placed and secured prior to the concrete placement.
- 6. At footing F6A at Grid L/9, rebar grid is required at both top and bottom of footing (see footing schedule)
- 7. At high wall along Line 7, joist seats are not yet in place, as well as U-bars, and dowels per CSKS-5.
- 8. The brick shelf transition along Line 7 does not appear to be in the proper location. This was confirmed with the GC, and the area will be corrected. The step in the stem wall is not at the same location as the step in the brick shelf.
- 9. Anchor bolts are not yet in place at column locations.
- 10. Along Line 1, between L.8 & H, a few vertical wall dowels are not yet in place per Section 1/S2.2.

The General Contractor was informed of these items prior to my departure. The General Contractor is requested to review these items with the concrete sub-contractor, and review completion of these items prior to the concrete placement.

Please contact me with any questions or comments. Thank you -

Project:		Florence House				
Location:		Portland, Maine				
Becker Jo	b No:	SI 2058				
Date:	Wednesday, 05/13/09					
Time:	2:00PM - 3:00 PM					
Temp:	Low 60's					
Weather:	Sunny					

OBSERVATION REPORT
Cast in Place Concrete

Observation Location: Foundation footings along Line 1 from Line A to between Line C & E, including the footings for the entrance slab.

Note: A pre-steel and pre-wood framing meeting was also conducted during this visit.

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments	
Reinforcement Size	\boxtimes					
Quantity	\square		\boxtimes			:
Condition	\boxtimes					•
Placement						
Embed/Anchors	\boxtimes					•••••••••••••••••••••••••••••••
Lap Splices	\boxtimes					
Hot Weather				$\overline{\boxtimes}$		
Cold Weather						***************************************
Bond Beams				X		
Additional Items				$\overline{\boxtimes}$		manus and specific panels of
Additional Items		Π	П	Ā		

Review of the areas referenced above found that rebar layout appeared in general conformance with the construction documents

Please contact me with any questions or comments. Thank you -

Project:		Florence House				
Location:		Portland, Maine				
Becker Jo		SI 2058				
Date:		y, 05/15/09				
Time:	11:00 AM - 12:00 PM					
Temp:	Low 70's					
Weather:	Sunnv					

OBSERVATION REPORT

Cast in Place Concrete

Observation Location: Foundation Walls along Line 1 from Line A to between Lines C & E, including the entrance slabs walls.

Note: A preliminary review of the major foundation wall reinforcement along Line N and 1 was also completed during this visit.

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	
Doinforment Cine	57				Comments
Reinforcement Size	\boxtimes	Щ	Ш	Ш	
Quantity	\boxtimes	Ш			
Condition	\square				\$
Placement	X		П		
Embed/Anchors	$\overline{\boxtimes}$		П	Ħ	
Lap Splices	$\overline{\boxtimes}$		П	П	
Hot Weather		П		Ā	
Cold Weather				$\overline{\square}$	
Bond Beams				$\overline{\boxtimes}$	
Additional Items				\boxtimes	
Additional Items				\boxtimes	
NI - 4				·····	

Notes:

Review of the areas referenced above found that rebar layout appeared in general conformance with the construction documents. There were no comments provided, and the area was determined ready for concrete placement.

Please contact me with any questions or comments. Thank you -

Project:		Florence House			
Location:		Portland, Maine			
Becker Jo	b No:	SI 2058			
Date:	Tuesday, 05/19/09				
Time:	3:00 PM - 4:00 PM				
Temp:	60's				
Weather:	Sunny				

OBSERVATION REPORT

Cast in Place Concrete

Observation Location: Foundation Walls along the remainder of N-Line & 1-Line to the intersection of the site retaining wall, and site retaining wall off Line 1.

Note: A preliminary review of the major foundation wall reinforcement was completed on Friday, May 15th, prior to standing the interior wall forms.

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Reinforcement Size					
Quantity	\boxtimes				-
Condition	\boxtimes				
Placement			\boxtimes		
Embed/Anchors					
Lap Splices	\boxtimes				
Hot Weather				Ā	
Cold Weather				$\overline{\boxtimes}$	
Bond Beams				$\overline{\boxtimes}$	
Additional Items				Ā	
Additional Items				$\overline{\boxtimes}$	
Notes:					

Review of the areas referenced above found that rebar layout appeared in general conformance with the construction documents and supplemental sketches. A majority of the wall rebar was in placed and appeared adequate, but work was ongoing and the following items were noted that will need to be completed prior to pouring. We understand that a concrete sub-contractor is likely aware of a majority of these items, but provide them for reference to ensure they are not missed. The General Contractor has agreed to take pictures as necessary to confirm.

- 1. Per CSKS-5, U-bars are necessary between each joist pocket. This reinforcement was on site but had not yet been placed.
- 2. The bond-outs and joist angle seat embedments had not been placed within the forms.

3. Per CSKS-5, the #5 dowels extending from the top of the concrete wall into the elevated slab were not in place.

During the preliminary review on May 15th, it was noted that the additional #5 bars specified on Section 8/S2.3 were not in place. These bars were found to be in place during this visit.

The General Contractor was informed of these items prior to my departure. The General Contractor is requested to review these items with the concrete sub-contractor, and review completion of these items prior to the concrete placement.

Please contact me with any questions or comments. Thank you -

Project:		Florence House					
Location:		Portland, Maine					
Becker Jo	b No:	SI 2058					
Date:	Tuesday	y, 05/27/09					
Time:	9:00 AM - 10:00 AM						
Temp:	50's						
Weather:	Rain Sh	owers					

OBSERVATION REPORT
Cast in Place Concrete

Observation Location: Remainder of interior spread footings between Line 1 and Line 7 as well as foundation wall along Line 1, from approx. Line D to Line H. Note: The spread footing at Grid 1/N_t was also placed, but the rebar at this area was not in place during this visit.

Note: A preliminary review of this reinforcement was completed on Friday, May 22nd.

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Reinforcement Size	X		П	П	
Quantity	X		П	П	
Condition	$\overline{\boxtimes}$		П	Ħ	
Placement	Ø	П	П	Ħ	}
Embed/Anchors	$\overline{\square}$		П	П	
Lap Splices	$\overline{\nabla}$	П	Π		
Hot Weather	П	П	Π	Ħ	
Cold Weather	П		П	Ā	
Bond Beams	Π	TT.	П	Ħ	
Additional Items		П	П	ă	
Additional Items	Ħ	Ħ	Ħ	Й	
Notes:		. 		<u> </u>	

Review of the areas referenced above found that rebar layout appeared in general conformance with the construction documents.

Please contact me with any questions or comments. Thank you -

$\underbrace{B\ E\ C\ K\ E\ R}_{\text{structural engineers, inc.}}$

Project:	Florence House							
Location:	Portland, Maine							
Becker Job No:	SI2058							
27	**************************************							

OBSERVATION REPORT Cast in Place Concrete

Date:	07-06-09
Time:	7:30 am - 9:00 am
Temp:	60's
Weather:	Sunny

Observation Location: Slab Reinforcement at the First Floor.

Note: An initial visit to review first floor slab reinforcement was completed on July 2^{nd} . This is a follow-up visit to confirm that incomplete areas found on July 2^{nd} had been completed and the area was ready for slab palcement.

	Satisfactory	Jn-Satisfactory	Not Completed	Not Applicable		·
	S	\supset	Z	Z	Comments	4
Reinforcement Size	\square					*
Quantity	\boxtimes					
Condition	\boxtimes	П	П	П		
Placement	Ø	П	П	П		
Embed/Anchors	$\overline{\boxtimes}$	Ī	Π	П		
Lap Splices	$\overline{\square}$	Ī		П		
Hot Weather		П	П	X		
Cold Weather	П		П	Ø		
Bond Beams	同	П	Ħ	Й		
Additional Items	H	Ħ.	П	Ħ		
Additional Items				X		

Notes:

This site observation was conducted to review reinforcement to be placed in the first floor concrete slab on metal deck. Reinforcement was in place and was in general conformance with the construction documents and supplemental sketches (CSKS's) provided.

Project:		Florence House					
Location:		Portland, Maine					
Becker Jo	b No:	SI 2058					
Date:	Wednes	day, 08/05/09					
Time:	1:30PM	- 2:30PM					
Temp:	80's						
Weather:	Sunny						

OBSERVATION REPORT
Cast in Place Concrete

Observation Location: Slab-on-grade reinforcement and sub-grade

	Satisfactory	Un-Satisfactory	Completed	Not Applicable	
	Satisf	Un-S	Not C	Not A	Comments
Reinforcement Size	\boxtimes				
Quantity		\boxtimes			
Condition					
Placement		\boxtimes			
Embed/Anchors					
Lap Splices			\square		
Hot Weather				$\overline{\boxtimes}$	
Cold Weather				$\overline{\boxtimes}$	
Bond Beams				$\overline{\boxtimes}$	
Additional Items				$\overline{\boxtimes}$	
Additional Items				$\overline{\boxtimes}$	

Notes:

This visit was completed to review the placement of reinforcement for the concrete slab-on-grade. Information provided on Drawing S1.1 and CSKS-10 was reviewed. The following items were noted that will require attention by the General Contractor prior to slab placement. The GC is requested to take photographs of these items when completed and forward to BSE for record.

- 1. The (3) #5 dowels shown in Section B to be drilled and grouted into the existing foundation strip footing have not yet been placed.
- 2. The #4 bars around the perimeter of the slab and along Line M shall be continuous. They are currently broken at each column slab bout-out. Pull the bars away from the wall to allow for continuous bar placement. Provide 2'-0" lap splices between bars.
- 3. The #4 slab bars shall be located at the center of the slab depth. Provide taller chairs for bar placement.

Please contact me with any questions or comments. Thank you -





82 Goldthwaite Road • P.O. Box 1747 • Auburn, Maine 04210 93 Scott Drive • Westbrook, Maine 04092 50 Arthur Reno Road • West Bath, Maine 04530 Phone: (207) 777-7100 • Fax: (207) 777-7171 Phone: (207) 780-0523 • Fax: (207) 780-1521 Phone: (207) 386-5100 • Fax: (207) 386-5151

GANNESTON CONSTRUCTION CORP.

ATTN: MIKE ADAMS
3025 NORTH BELFAST AVE.
P.O. BOX 27
AUGUSTA, ME 04332
PH: (207) 621-8505
FX: (207) 621-8508

GANNESTON CONSTRUCTION CORP.
P.O. BOX 27 AUGUSTA, ME 04332
REVIEWED WITH NOTES
REVIEWE

Mix Design Submittals for:

FLORENCE HOUSE

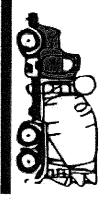
390 VALLEY ST. - PORTLAND, ME
GANNESTON CONSTRUCTION CORP - PROJECT # 677

As prepared by:
AUBURN CONCRETE

Remi Delcourt, Sales & Quality Control P.O. Box 1747 - 82 Goldthwaite Road Auburn, Maine 04210 Office: (207) 777-7100 Facsimile: (207) 777-7171

E-Mail: remi@auburnconcrete.com





93 Scott Drive • Westbrook, Maine 04092 82 Goldthwaite Road • P.O. Box 1747 • Auburn, Maine 04210

50 Arthur Reno Road • West Bath, Maine 04550

Submit Specific Item: Rejected Reviewed | Furnish as Corrected Revise and Resubmit

and for performing all work in a safe and of his or her Work with that of all other trades: and procedures of construction: coordination pertains solely to the fabrication processes or and correlated at the jobsite: information that responsible for: dimensions to be confirmed which the item is a component. Contractor is shall not include approval of an assembly of and specifications. Approval of a specific item compliance with the requirements of the plans this review do not relieve contractor from comments made on the shop drawings during in the Contract Documents. Corrections or general compliance with the information given with the design concept of the project and This review is only for general conformance satisfactory manner. to the means, methods, techniques, sequences

机工物

Becker Structural Engineers, Inc.

Date 2.18.09 By MJM

Phone: (207) 386-5100 • Fax: (207) 386-5151 Phone: (207) 780-0523 • Fax: (207) 780-1521 Phone: (207) 777-7100 • Fax: (207) 777-717





82 Goldthwaite Road • RO. Box 1747 • Auburn, Maine 04210 93 Scott Drive • Westbrook, Maine 04092 50 Arthur Reno Road • West Bath, Maine 04530 Phone: (207) 777-7100 • Fax: (207) 777-7171 Phone: (207) 780-0523 • Fax: (207) 780-1521 Phone: (207) 386-5100 • Fax: (207) 386-5151

Ganneston Construction Corp.

FLORENCE HOUSE 390 VALLEY ST. - PORTLAND, ME

3000PSI - Air Entrained, 3/4" Crushed Stone

Mix Design Submittal (Footings)

1/3	30/2009		3034S/		
			Weight-SSD (lbs)	Volume (Cu.Ft.)	Sources
CEMENT	, T I/II	ASTM C-150	517	2.63	DRAGON PRODUCTS COMPANY
COARSE	AGG	ASTM C-33: #57/#67	1700	10.28	PIKE INDUSTRIES
FINE AGO	GREGATE	ASTM C-33	1330	8.17	PORTLAND SAND & GRAVEL
WATER	U.S. GAL/CY:	33.0	275	4.41	CITY OF WESTBROOK
		AIR CONTENT (%):	6.0 +/- 1.5%	1.63	
 	WA	TER/CEMENT RATIO: SLUMP (Inches):	9.53 4.00	±1.00" 4"	MAR PER SPECS. PRIOR to WHTER REDUCER
		YIELD:	141.0 PCF	27.1 Cu.Ft.	WHTER REDUCER
GLENIUM	1 <i>7</i> 500	ASTM C494, TYPE A.F	3.00 oz/cwt	15.5 US 02/C)	BASF/MASTER BUILDERS
MICROAL OPTIONA		ASTM C-260	0.15 oz/cwt	0.8 US 0z/C	
POZZOLI	TH 100XR	ASTM C-494, Type B,D	2.00 oz/cwt	10.3 US oz/CY	BASF/MASTER BUILDERS
POZZUTE	C 20+	ASTM C-494, Type C,E	10.00 oz/cwt	51.7 US oz/CY	BASF/MASTER BUILDERS
*GLENIU	M 7500 dose is i	for MIDRANGE application	ins.		

GLENIUM 7500 meets the requirements of ASTM C494 for Type A (water-reducing) and Type F (high-range water-reducing). Plant of site addition of 2 - 3 fl.oz./cwt (10.3 - 15.5 fl.oz./cy) will be required to achieve end slump indicated.

www.aubumconcrete.com

MIX IDENTIFICATION #: DESIGN STRENGTH:

3034SA 3000 PSI @ 28 DAYS



PROJECT: CONTRACTOR: W/C: 0.53 MAX. C.A.: 3/4"

1/30/2009 3:11 PM

	manufacture promotestatic	TW- 15	ana a se aine		TEMPER		7	28	MOVING
DATE	PROJECT	ID#	SLUMP	% AIR	CONC	AIR	DAY	DAY	AVG. OF 3
1/10/2008	TURNER-LEEDS BRIDGES	14039-6			4.5		2050	3120	
1/12/2008 3/6/2008	TURNER-LEEDS BRIDGES TURNER-LEEDS BRIDGES	14039-12 14039-13	7.50 6.50	6,4 8,5	60 52		2785 2595	3980 3435	3512
3/8/2008	TURNER-LEEDS BRIDGES	14039-14	6.50	7.0	51		3010	3910	3775
4/5/2008	TURNER-LEEDS BRIDGES	14039-17		4.5	62		3410	4055	3800
4/5/2008	TURNER-LEEDS BRIDGES	14039-19		6.5	61		2620	3315	3760
4/5/2008 4/5/2008	TURNER-LEEDS BRIDGES TURNER-LEEDS BRIDGES	14039-20 14039-21		6.5	61 63		3390 3580	4420 4070	3930 3935
4/9/2008	RESIDENCE INN-AUBURN	14064-1	3.50	7.1 5.3	63		2830	4150	4213
4/10/2008	RESIDENCE INN-AUBURN	14064-2	3.75	5.0	63		3380	4470	4230
4/11/2008	RESIDENCE INN-AUBURN	14084-3	4.00	5.4	63		3010	4470	4383
4/15/2008 4/17/2008	RESIDENCE INN-AUBURN RESIDENCE INN-AUBURN	14064-4 14064-5	5.25 5.00	5.2 5.8	61 65		3140 3290	3885 4090	4275 4148
4/22/2008	RESIDENCE INN-AUBURN	14064-6	5.75	5.6	69		2710	3515	3830
4/25/2008	RESIDENCE INN-AUBURN	14064-7	5.00	6.5	65		2950	3770	3792
4/30/2008	RESIDENCE INN-AUBURN	14084-8	3.00	6.0	58		3350	4400	3895
5/5/2008 5/8/2008	RESIDENCE INN-AUBURN RESIDENCE INN-AUBURN	14084-9 14064-10	5.50 5.75	4.5 6.0	64 67		3030 2960	3540 3555	3903 3832
5/13/2008	RESIDENCE INN-AUBURN	14064-11	3.50	6.8	64		3310	3970	3688
5/19/2008	RESIDENCE INN-AUBURN	14064-12	5.25	6.2	85		3230	3470	3665
5/29/2008	RESIDENCE INN-AUBURN	14064-13	4.25	5.5	68		3900	4550	3997
7/2/2008	TURNER-LEEDS BRIDGES	14039-33	6.00	6.0	77		2850	3640	3887
7/2/2008	TURNER-LEEDS BRIDGES	14039-34	6.50	6.1	81		3100 2940	3850 3640	4012 3710
7/2/2008 7/2/2008	TURNER-LEEDS BRIDGES TURNER-LEEDS BRIDGES	14039-35 14039-36	6.50 6.00	8.0 8.2	81 84		2670	3355	3615
7/15/2008	GORHAM BY-PASS	889-10	6.00	6.6	81	83	2920	3200	3398
7/25/2008	St. MARYS ER ADDITION	14093-7	5.00	4.7	84		3060	3935	3497
7/29/2008	SL MARYS ER ADDITION	14093-8	4.00	5.4	87		3140	4095	3743
7/30/2008	St. MARYS ER ADDITION	14093-9	4.25	4.9	83		3390	4255 3735	4095 4028
8/1/2008 8/4/2008	St. Marys er addition St. Marys er addition	14093-10 14093-11	5.00 5.00	5.5 6.9	82 84		3030 2810	3200	3730
8/7/2008	St. MARYS ER ADDITION	14093-12	5.50	5.9	71		3040	4195	3710
8/9/2008	St. MARYS ER ADDITION	14093-13	5.00	6.8	79		3270	3795	3730
8/15/2008	SI. MARYS ER ADDITION	14093-14	4.00	8.9	86		2800	3190	3727
9/17/2008	ST. MARY'S MOB	14131-1	4.50	5.8	81		4550	4915	3967 407.2
9/18/2008 9/23/2008	ST. MARY'S MOB ST. MARY'S MOB	14131-2 14131-3	7.25 3.75	6.0 7.3	76 70		3290 3310	4110 4090	4372
9/23/2006	ST. MARYS MOB	14131-4	5.00	6.9	78		3800	4115	4105
9/30/2008	ST. MARY'S MOB	14131-5	5,50	6,6	74		2990	4055	4087
10/2/2008	ST. MARY'S MOB	14131-6	5,50	7.2	70		2990	3935	4035
10/2/2008	BOWDOIN COL. FITNESS CTR	14138-1	5.50	8,0	72		2840	3890	3960
10/6/2008 10/7/2008	ST, MARY'S MOB ST, MARY'S MOB	14131-7 14131-8	5,50 4,50	6.8 7.3	63 61		3320 3100	3825 3920	3883 3878
10/8/2008	ST. MARY'S MOB	14131-9	5.50	5.0	71		3680	4140	3962
10/13/2008	ST. MARY'S MOB	14131-10	5.25	5.9	73		3410	3910	3990
10/14/2008	BOWDOIN COL. FITNESS CTR	14138-2	5.50	5.6	70		3140	3905	3985
10/14/2008	BOWDOIN COL. FITNESS CTR	14138-3	5.50	6.5	68		3050	3775	3863
10/17/2008 10/21/2008	ST. MARY'S MOB BOWDOIN COL. FITNESS CTR	14131-11 14138-4	6.00 6.00	7.0 5.8	68 62		2740 3010	3555 4005	3745 3778
10/22/2008	SI, MARYS ER ADDITION	14093-18	5.00	5.8	60		3290	4100	3887
10/24/2008	SI. MARYS ER ADDITION	14093-19	6.25	6,0	57		1980	3270	3792
11/3/2008	BOWDOIN COL. FITNESS CTR	14138-5	8.00	7.8	51		2510	3155	3508
11/3/2008	BOWDOIN COL FITNESS CTR	14138-8	8.50	7.4	53		2830	3620	3348
11/14/2008	BOWDOIN COL. FITNESS CTR BOWDOIN COL. FITNESS CTR	14138-7 14138-8	8.00 8.00	7.2 5.8	67 67		3130 2860	3915 3485	3563 3673
11/14/2008 11/25/2008	BOWDOIN COL FITNESS CTR	14138-9	7.00	6.8	63		3150	3790	3730
Literation		343000	Esta						
COUNT:		56	51	55	55	2	56	56	50
matime.		a lerran	a nn	a F	E4	83	1980	3120	3398
RANGE:		LOW HIGH	3.00 8.50	4,5 8.5	51 87	83 83	4550	4915	4372
AVERAGE OF	ALL:		5.50	6.3	69	83	3061	3852	3886
STANDARD D			1,2	0.9	9.4	0.0	405	384	213
COEFFICIENT	OF VARIATION:		22.4	14.8	13,7	0.0	13.2	10.0	5.5

ACI 214 SUMMARY:

AVERAGE STRENGTH:
AVERAGE STRENGTH BASED ON:
STANDARD DEVIATION:
OVERALL COEFFICIENT OF VARIATION:
WITHIN-TEST STANDARD DEVIATION:
WITHIN-TEST COEFFICIENT OF VARIATION:
BATCH-TO-BATCH STANDARD DEVIATION:
RECOMMENDED STRENGTH:

3852 PSI 56 TESTS 384 PSI 10.0 % 114 PSI 3.0 % 366 PSI 3514 PSI

CONTROL IS EXCELLENT

CONTROL IS EXCELLENT





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Ganneston Construction Corp.

FLORENCE HOUSE 390 VALLEY ST. - PORTLAND, ME

3500PSI - Air Entrained, ¾" Crushed Stone

Mix Design Submittal (Foundation Walls)

1/30/2009

3534SA

		Weight-SSD (lbs)	Volume (Cu.Ft.)	Sources	
CEMENT, TI/II	ASTM C-150	564	2.87	DRAGON PRODUCTS COMPANY	
COARSE AGG FINE AGGREGATE	ASTM C-33: #57/#67 ASTM C-33	1700 1270	10.28 7.80	PIKE INDUSTRIES PORTLAND SAND & GRAVEL	
WATER U.S. GAL/CY	33.6	280	4.49	CITY OF WESTBROOK	
•	AIR CONTENT (%):	6.0 +/-1.5%	1.63		
WA	TER/CEMENT RATIO: SLUMP (Inches): YIELD:	0.50 4.00 140.9 PCF	±1.00"	MAX PROR TO LADMIXTURE	
GLENIUM 7500 MICROAIR	ASTM C494, TYPE A,F	3.00 oz/cwt 0.2 oz/cwt	16.9 US oz/CY 1.1 US oz/CY	BASF/MASTER BUILDERS BASF/MASTER BUILDERS	
OPTIONAL: POZZOLITH 100XR POZZUTEC 20+ '* GLENIUM 7500 dose is	ASTM C-494, Type B,D ASTM C-494, Type C.E for MIDRANGE applications	2.00 oz/cwt 10.00 oz/cwt	11.3 US oz/CY 56.4 US oz/CY	BASF/MASTER BUILDERS BASF/MASTER BUILDERS	

Flant of site addition of 2 - 3 fl.oz./cwt (11.3 - 16.9/16/22/24) will be required to achieve end slump indicated.

MIX IDENTIFICATION #: DESIGN STRENGTH:





PROJECT: CONTRACTOR:

					TEMPE	RATURE	7	28	MOVING
DATE	PROJECT	ID#	SLUMP	% AIR	CONC	AIR	DAY	DAY	AVG. OF 3
9/28/2006	GASWORKS	687-1	6.50	6.1	71	65	3240	4545	
3/13/2007	Davis Island Townhouses	4896-2	5.50	5.2	70		3230	4530	
3/15/2007	Davis Island Townhouses	4896-3	7.75	5.8	61		4160	5005	4693
3/19/2007	Davis Island Townhouses	4896-4	8.00	6.5	68		3990	4750	4762
3/23/2007	Davis Island Townhouses	4896-6	4.00	5.6	66		3120	3850	4535
4/3/2007	Davis Island Townhouses	4896-12	4.00	4.7	56		2990	3700	4100
4/12/2007	Davis Island Townhouses	4896-15	5.50	5.2	60		3080	3990	3847
4/24/2007	Davis Island Townhouses	4896-19	4.75	5.9	70		2930	3660	3783
4/26/2007	Davis Island Townhouses	4896-20	5.75	4.7	59		2430	3420	3690
6/12/2007	Davis Island Townhouses	4896-25	5.25	5.4	78		3200	3980	3687
12/7/2008	Wal-Mart Scarborough	1788-13	6.50	5.0	61	20	3550	4690	4030
12/7/2008	Wal-Mart Scarborough	1788-14	4.00	5	65	28	3730	4685	4452
12/27/2008	Wal-Mart Scarborough	1788-25	5.50	6.6	60	31	3830	4540	4638
COUNT:	•	13	13	13	13	4	13	13	11
RANGE:		LOW	4.00	4.7	56	20	2430	3420	3687
The state of the s		HIGH	8.00	6.6	78	65	4160	5005	4762
AVERAGE C	OF ALL:		5.62	5.5	65	36	3345	4257	4202
STANDARD	DEVIATION:		1.3	0.6	6.0	17.2	465	488	403
COEFFICIEN	NT OF VARIATION:		22.4	11.0	9.2	47.8	13.9	11.5	9.6

ACI 214 SUMMARY:

AVERAGE STRENGTH:
AVERAGE STRENGTH BASED ON:
STANDARD DEVIATION:
OVERALL COEFFICIENT OF VARIATION:
WITHIN-TEST STANDARD DEVIATION:
WITHIN-TEST COEFFICIENT OF VARIATION:
BATCH-TO-BATCH STANDARD DEVIATION:
RECOMMENDED STRENGTH:

4257 PSI

13 TESTS

488 PSI CONTROL IS VERY GOOD

11.5 %

51 PSI

1.2 % CONTROL IS EXCELLENT

485 PSI

Too few tests to determine recommended strength

BASED ON 13 TEST

 $\int_{cr}^{c} = \int_{c}^{c} + 1200 = 3500 + 1200 = 4700$ However Based on TEST of 3k Concrete $SS^{-}324$: $\int_{cr}^{c} = 3500 + 1.3 + (724) = 4014 \sqrt{}$





82 Goldthwaite Road • P.O. Box 1747 • Auburn, Maine 04210 93 Scott Drive • Westbrook, Maine 04092 50 Arthur Reno Road • West Bath, Maine 04530 Phone: (207) 777-7100 • Fax: (207) 777-7171 Phone: (207) 780-0523 • Fax: (207) 780-1521 Phone: (207) 386-5100 • Fax: (207) 386-5151

Ganneston Construction Corp.

FLORENCE HOUSE 390 VALLEY ST. - PORTLAND, ME

3000PSI - Non-Air Entrained, 3/4" Crushed Stone

Mix Design Submittal (Interior S-O-G, elevated slabs)

1/30/2009

3034SNA

			Weight-SS	D (lbs)	Volume	(Cu.Ft.)	Sources
CEMENT	, ті/п	ASTM C-150	517		2.	63	DRAGON PRODUCTS COMPANY
COARSE		ASTM C-33: #57/#67 ASTM C-33	1750 1440			.58 84	PIKE INDUSTRIES PORTLAND SAND & GRAVEL
WATER	U.S. GAL/CY:	33.6	280		4.	49	CITY OF WESTBROOK
		AIR CONTENT (%):	2.0 +	-/- 1.5%	0.	54	
	WA	TER/CEMENT RATIO: SLUMP (Inches):	0.54 4.00		± 1.00"	7.00" ± 1	1.00" (After Superplasticizer**)
		YIELD:	147.2	PCF	27.1	Cu.Ft.	
GLENIUM OPTIONA		ASTM C494, TYPE A,F	3.50 c	z/cwt	18.1	US oz/CY	BASF/MASTER BUILDERS
POLYME:	SH	ASTM C-1116 TYPE III ASTM C-494, Type C,E	1.5 ll 10.00 c	bs/cy z/cwt	51.7	US oz/CY	O'DEA CONCRETE PRODUCTS BASF/MASTER BUILDERS
4 COT TO KITT	S. F. PROOF TO SEE	C. ACTON ANTON					

* GLENIUM 7500 dose is for MIDRANGE applications.

^{1*}GLENIUM 7500 meets the requirements of ASTM C494 for Type A (water-reducing) and Type F (high-range water-reducing). Plant of site addition of 2 - 3 fl.oz./cwt (9.6 - 14.4 fl.oz./cy) will be required to achieve end slump indicated.

www.auburnconcrete.com

MIX IDENTIFICATION #: DESIGN STRENGTH:

30345NA 3000 PSI @ 28 DAYS



PROJECT: CONTRACTOR:

W/C: 0.54 MAX. C.A.: 3/4"

					TEMPE	RATURE	7	28	MOVING
DATE	PROJECT	ID#	SLUMP	% AIR	CONC	AIR	DAY	DAY	AVG. OF 3
1/24/08	84 MARGINAL WAY	765-65	5.50	2.6	68	25	3380	3855	
1/24/08	84 MARGINAL WAY	765-66	6.50	100 T 100	63	25	2740	3980	
	84 MARGINAL WAY	765-67	6.50	2.1	60	25	3400	4360	4065
1/24/08 1/24/08	84 MARGINAL WAY	765-68	5.00	2.5	60	28	3520	4105	4148
	84 MARGINAL WAY	765-69	6.75	3.3	50	29	3480	4105	4190
1/24/08	84 MARGINAL WAY	765-70	6.75	2.9	50	28	3450	4640	4283
1/24/08	84 MARGINAL WAY	765-71	6.25	2.0	77	20	3660	3810	4185
1/31/08	84 MARGINAL WAY	765-72	6.50	2.0	65	23	2830	3260	3903
1/31/08	84 MARGINAL WAY	765-72 765-73	6.75	2.6	61	25	3710	4000	3690
1/31/08	- 3 75 m - 3 m - 1	765-74	6.50	2.2	61	31	3290	3840	3700
1/31/08	84 MARGINAL WAY					33	3040	4000	3947
1/31/08	84 MARGINAL WAY	765-75	6.75	2.2	55 58	23	2920	3560	3800
2/8/08	84 MARGINAL WAY	765-76	7.00	2.7	, ·		2580	3495	3685
2/8/08	84 MARGINAL WAY	765-77	7.00	2.5	52	20		3 9 00	3652
2/8/08	84 MARGINAL WAY	765-78	6.50	2.5	48	26	3360	3505	3633
2/8/08	84 MARGINAL WAY	765-79	7.00	2.5	47	22	2600		
2/8/08	84 MARGINAL WAY	765-80	6.50	2.5	48	22	3470	4095	3833
2/29/08	84 MARGINAL WAY	765-84	6.00	2.5	72	4	3460	4095	3898
2/29/08	84 MARGINAL WAY	765-85	6.75	2.6	48	11	2900	3885	4025
2/29/08	84 MARGINAL WAY	765-86	5,00	2.4	52	25	3290	4165	4048
6/13/08	RESIDENCE INN-AUBURN	14064-17	6.50	3.8	72		2410	3395	3815
6/13/08	RESIDENCE INN-AUBURN	14064-18	6.00	3.9	71		2530	3035	3532
6/13/08	RESIDENCE INN-AUBURN	14064-19	5.75	4.1	73		2820	3385	3272
6/25/08	RESIDENCE INN-AUBURN	14064-23	5.00	4.5	78		3110	3980	3467
6/25/08	RESIDENCE INN-AUBURN	14064-24		4.8	78		2950	3425	3597
6/25/08	RESIDENCE INN-AUBURN	14064-25		4.5	75		2940	3830	3745
6/25/08	RESIDENCE INN-AUBURN	14064-26	-	4.9	75		3000	3635	3630
6/25/08	RESIDENCE INN-AUBURN	14064-27	5.75	3,9	77		2920	3700	3722
6/25/08	RESIDENCE INN-AUBURN	14064-28	pr - 14 m	4.0	75		2680	3495	3610
9/23/08	St MARYS ER ADDITION	14093-15	5.25	1.9	68		3690	4160	3785
9/23/08	St. MARYS ER ADDITION	14093-16	6.00	1.8	67		3540	4180	3945
9/23/08	St. MARYS ER ADDITION	14093-17	5.25	1.8	69		3770	4445	4262
10/30/08	ST. MARY'S MOB	14131-13	5.25	2.2	61		2980	4065	4230
10/30/08	ST, MARY'S MOB	14131-14	5.00	2.4	57		3290	3935	4148
10/30/08	ST. MARY'S MOB	14131-15	6.00	2.0	55		3190	4480	4160
12/3/08	BOWDOIN COLFITNESS CTR	14138-10	5.50	8.2	67		3940	4700	4372
12/30/08	ST. MARY'S MOB	14131-16	6.00	2.4	63		3320		
12/30/08	ST. MARY'S MOB	14131-17	5.50	2.0	59		3190		
(
COUNT:		39	39	38	39	19	39	35	33
RANGE:		LOW	5.00	1.8	47	4	2410	3035	3272
MARIOE.		HIGH	7.00	4.9	78	33	3940	4700	4372
AVERAGE OF	FALL:		6.04	2.9	63	23	3190	3900	3878
STANDARD D			0.6	0.9	9.7	6.5	377	382	266
	T OF VARIATION:		10.8	30.7	15.4	27.7	11.8	9.8	6.9
Teles of the 17	त भारत प्रतासन्त्रिकारणात्राम		- T- T- T-						

ACI 214 SUMMARY:

AVERAGE STRENGTH:
AVERAGE STRENGTH BASED ON:
STANDARD DEVIATION:
OVERALL COEFFICIENT OF VARIATION:
WITHIN-TEST STANDARD DEVIATION:
WITHIN-TEST COEFFICIENT OF VARIATION:
BATCH-TO-BATCH STANDARD DEVIATION:
RECOMMENDED STRENGTH:

3900 PSI		
35 TESTS	3	
382 PSI	CONTROL IS	EXCELLENT
9.8 %		
121 PSI		
3.1 %	CONTROLIS	VERY GOOD
362 PSI		
3512 PSI		





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Ganneston Construction Corp.

FLORENCE HOUSE 390 VALLEY ST. - PORTLAND, ME

4500PSI - Air Entrained, 3/4" Crushed Stone

Mix Design Submittal (Exterior Slabs, exposed site concrete)

1/30/2009

4534SA

ĺ		Weight-SSD (lbs)	Volume (Cı	<u>1.Ft.)</u>	<u>Sources</u>
CEMENT, TI/II	ASTM C-150	658	3.35	DRA	GON PRODUCTS COMPANY
COARSE AGG FINE AGGREGATE	ASTM C-33: #57/#67 ASTM C-33	1800 1100	10.89 6.75	PO	K & K EXCAVATION RTLAND SAND & GRAVEL
WATER US. GAL	/CY: 33.6	280	4.49		CITY OF WESTBROOK
	AIR CONTENT (%):	6.0 +/- 1.5%	1.63		
	WATER/CEMENT RATIO: SLUMP (Inches):	0.43 / 3.00	±1.00"	7.00" ± 1.	00" (After Superplasticizer**
	YIELD:	141.6 PCF	27.1	Cu.Ft.	
GLENIUM 7500*	ASTM C494, TYPE A,F	3.00 oz/cwt	19.7 U	JS oz/CY B	ASF/MASTER BUILDERS
MICROAIR OPTIONAL:	ASTM C-260	0.35 oz/cwt	2.3 t	JS oz/CY B	ASF/MASTER BUILDERS
POZZOLITH 100XR	ASTM C-494, Type B,D	2.00 oz/cwt	13.2 t	JS oz/CY B	ASF/MASTER BUILDERS
POZZUTEC 20+	ASTM C-494, Type C.E se is for MIDRANGE application	10.00 oz/cwt	65.8 U	JS oz/CY B	ASF/MASTER BUILDERS

GLENIUM 7500 meets the requirements of ASTM C494 for Type A (water-reducing) and Type F (high-range water-reducing). Plant of site addition of 2 - 3 fl.oz./cwt (13.2 - 197/fl.oz./cy) will be required to achieve end slump indicated.



P.O. Box 191, U.S. Route 1 • Thomaston, Maine 04861 • 207-594-5555

MILL TEST RESULTS
Laboratory at Thomaston, Maine

Date: January, 2008 Cement Type: I/II

CHEMICAL DATA	Percent	PHYSICAL DATA
Silicon Dioxide	20.0 4.2	Specific Surface
Ferric Oxide	3.0	Percent Passing 325 Mesh. 97.3 (Per ASTM C 430)
Calcium Oxide	61.5	Compressive Strength (psi) (Per ASTM C 109)
Magnesium Oxide	3.0	1 day 2300 3 day 4040
Sulphur Trioxide	3.6	7 day 5030 28 day
Loss on Ignition	1.4	•
Insoluble Residue	0.4	Vicat Setting Time (Per ASTM C 191)
Tricalcium Silicate	55	Initial (min.)
Dicalcium Silicate	15	Air Content (%) 6.8 (Per ASTM C 185)
Tricalcium Aluminate	6	Autoclave Expansion (%) 0.05 (Per ASTM C 151)
Sodium Oxide	0.5	Expansion in water (%) 0.012 (Per ASTM C 1038)
Potassium Oxide	1.1	Certified by:
Equivalent Alkalies(Chemical Analysis all per ASTM	1.2 C 114)	Jennifer K. Coltxon

We hereby certify that this cement complies with current ASTM C 150, AASHTO M-85 and CSA-A3000 - 03 Type GU specifications.



--- 2008 - 2009 GRADATION SUMMARY ---

3/4" Quarry Stone (# 67)

Source: K & K Excavation - Christian Hill Quarry, Auburn, Maine

								and a mi	
	Specific Gra	rvity: 2.67	í	Absorption:	0.40%		Tested by:	Summit La	bs 4/24/08
	2 ^{is}	11/2"	1"	%"	1/4"	*/***	#4	#8	#200
4/1/2008	K&K	100	100	99	69	49	10	3	0.6
4/4/2008	RD-CHQ	100	100	99	57	29	7	3	0.5
4/9/2008	KaK	100	100	99	58	30	7	2	0.5
4/11/2008	RD-A	100	100	98	62	31	8	4	0.5
4/16/2008	Kak	100	100	98	60	33	9	3	0.5
4/25/2008	K&K	100	100	98	53	29	9	3	0.5
5/5/2008	RID-CHQ	100	100	97	53	27	8	4	0.5
5/7/2008	Kak	100	100	97	51	27	7	3	0.5
5/9/2008	K&K	100	100	94	50	24	6	3	0.5
5/13/2008	KSK	100	100	98	50	24	8	3	0.4
5/16/2008	RD-CHQ	100	100	95	52	22	4	3	0.8
5/28/2008	Kak	100	100	97	51	25	6	2	0.5
5/30/2008	KSK	100	100	94	48	20	5	2	0.4
6/2/2008	Kak	100	100	95	55	27	7	3	0.5
6/5/2008	RD-W	100	100	96	48	21	6	3	0.5
6/16/2008	Kak	100	100	99	61	32	8	3	0.7
6/18/2008	Kek	100	100	96	45	23	5	2	0.6
6/19/2008	RD-A	100	100	97	55	27	6	3	0.5
		100	100	98	58 58	32	9	4	0.9
6/19/2008	RD-CHQ	100	100	99	53	27	9	3	0.5
6/30/2008	K&K	100	100	95	43	20	5	2	0.7
7/1/2008	RD-CHQ	100	100	99	43 54	20 24	7	3	0.4
7/1/2008	KAK				54	2 4 28	7	3	0.7
7/7/2008	K&K	100	100	97	60	32	8	3	0.5
7/10/2008	RD-W	100	100	93			7	3	0.6
7/16/2008	RD-CHQ	100	100	97	55 60	2 6 31	9	4	0.9
7/21/2008	RD-A	100	100	98	59 59	27	6	2	0.4
7/25/2008	K&K	100	100	98		21	5	3	0.5
7/28/2008	K&K	100	100	97	50				
8/1/2008	RD-CHQ	100	100	99	46	29	8	4	0.8
8/12/2008	K&K	100	100	97	46	22	6	3	0.6
8/13/2008	RD-A	100	100	94	42	21	8	3	0.7
8/19/2008	K&K	100	100	96	47	24	5	2	0.3
8/20/2008	RD-WB	100	100	97	55	32	8	3	0.4
8/22/2008	K&K	100	100	92	51	32	8	3	0.7
8/25/2008	K&K	100	100	94	53	27	6	3	0.7
9/3/2008	RD-W	100	100	94	54	31	9	4	0.7
9/25/2008	RD-A	100	100	96	53	32	9	4	0,3
9/26/2008	K&K	100	100	94	44	22	5	2	0.6
9/29/2008	RD-A	100	100	93	44	23	8	4	0.6
9/29/2008	K&K	100	100	98	48	27	6	2	0.4
10/1/2008	K&K	100	100	94	57	29	8	3	0.5
10/9/2008	RD-WB	100	100	91	42	22	6	3	0.6
10/15/2008	Kak	100	100	92	48	22	4	2	0.4
11/6/2008	KSK	100	100	91	52	31	8	3	0.6
11/7/2008	RD-WB	100	100	90	45	24	6	3	1.0
12/8/2008	RD-WB	100	100	90	49	30	10	5	0.4
1/6/2009	RD-A	100	100	91	46	22	4	2	0.3
AVERAGE		100.0	100.0	95.7	52.1	27.0	7.0	3.0	0.6
SPECIFICA	TION: #5	7 100	95-100	44-50-45-40-40.	25-60	, and the same of	0-10	0-5	<1.5
ASTM C	33 #6	7	100	90-100	****	20-55	0-10	0-5	<1.5



--- 2007 - 2008 - 2009 GRADATION SUMMARY ---

Concrete Sand

Source: Portland Sand & Gravel - Gray, Maine

	Spec	dific Gravity:	2.63		Absorption:	0.67%		Tested by:	Summit Lab	s 4/24/08	
		FM	Color	3/8**	#4	#8	#16	#30	#50	#100	#200
9/6/2007	RD-A	2.54	<1	100	97	91	77	53	22	5	1.2
9/7/2007	RD-W	2.53	•	100	97	91	76	53	24	6	1.4
9/19/2007	RD-PSG	2.43	<1	100	98	93	81	57	23	5	1.0
10/4/2007	RD-A	2,49		100	98	92	77	52	25	8	1.8
11/15/2007	RD-A	2,58	<1	100	98	91	75	51	22	6	1.2
11/21/2007	RD-A	2.59		100	97	90	75	51	22	6	1.4
12/12/2007	RD-W	2,60		100	98	90	75	50	22	6	1.3
12/17/2007	RD-A	2.53	<1	100	98	92	76	52	23	6	1.3
12/28/2007	RD-A	2.53		100	97	91	76	52	24	7	1.6
1/11/2008	RD-W	2.60		100	98	90	75	50	22	6	1.3
1/31/2008	RD-PSG	2.60	<1	100	98	91	74	49	22	6	1.3
2/12/2008	RD-PSG	2.59		100	98	90	75	50	23	6	1.4
2/15/2008	RD-PSG	2.63		100	98	90	73	48	22	6	1.2
2/25/2008	RD-PSG	2.68		100	98	89	72	47	21	5	1.2
2/28/2008	RD-W	2.65		100	98	89	73	48	22	6	1.2
3/10/2008	JC-W	2.63		100	98	89	73	48	22	6	1.3
3/13/2008	RD-A	2.70	<1	100	98	89	71	46	20	6	1.2
3/28/2008	RD-PSG	2.61		100	98	90	74	49	22	5	1.2
4/4/2008	RD-PSG	2.57		100	98	91	75	50	23	6	1.4
4/11/2008	RD-A	2.60		100	98	91	75	49	22	6	1.3
5/5/2008	RD-PSG	2.64		100	98	89	73	49	22	6	1.3
5/19/2008	RD-PSG	2.44		100	98	92	80	57	23	5	1,2
5/27/2008	RD-A	2.62		100	97	91	76	50	18	4	1.0
6/5/2008	RD-W	2.68		100	97	91	75	48	17	3	8.0
6/17/2008	RD-A	2.54	<1	100	97	91	77	54	23	5	1.2
6/20/2008	RD-PSG	2.52		100	97	91	77	53	23	6	1.3
7/1/2008	RD-PSG	2.49		100	98	92	79	56	22	5	1.1
7/10/2008	RD-W	2.71	<1	100	97	90	72	46	19	5	1.2
7/16/2008	RD-PSG	2.71		100	97	90	75	46	17	4	0.9
7/21/2008	RD-A	2.54	<1	100	98	92	77	53	21	5	1.1
7/30/2008	RD-A	2,45		100	96	93	80	56	23	5	1.1
7/31/2008	JC-W	2.70		100	97	91	76	48	15	4	0.8
8/13/2008	RD-A	2.57		100	97	89	74	53	24	6	1.3
8/21/2008	RD-WB	2.79		100	97	89	72	44	15	4	0.9
9/3/2008	RD-W	2.64	<1	100	96	90	75	50	20	5	1.2
9/25/2008	RD-A	2.65	<1	100	98	92	76	48	18	4	0.9
9/29/2008	RD-A	2.65		100	98	93	77	49	15	3	0.8
10/9/2008	RD-WB	2.75		100	97	90	74	46	15	3	0.7
11/7/2008	RD-WB	2.83	<1	100	98	93	75	40	10	1	0.3
11/13/2008	RD-WB	2.72		100	97	91	75	47	15	3	0.5
12/8/2008	RD-WB	2.74		100	98	93	77	45	13	2	0.4
1/6/2009	RD-PSG	2.63	< 1	100	99	90	73	49	21	5	0.8
AVERAGE		2.61		100.0	97.6	90.8	75.3	49.8	20.4	5.0	1.1
SPECIFICA ASTM C3		2.3 - 3.1		100	95-100	80-100	50-85	25-60	10-30	2-10	< 5

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: 626-9094



DAILY FIELD REPORT

Date:	4/1/09								
Project:	Avesta Florence	ee House							
Project #:	14194								
Site Contacts:	Mike Adams, (Mike Adams, Ganneston Construction							
Purpose of Visit:	Concrete Test	ing Services							
Work Activities:	3000 psi concresupplied by Au (Air entraining of four concret	ete for footings on N- ıburn Concrete, place	line from ed via truc nium 7500 cast and v	16-line to 7.1-link chute, and con (mid-range was will be picked up	ntained Micro Air ter reducer.) One set				
Test Results:	Slump: 4-3/4" Air%: 7.0% Concrete Tem Air Temp: 42	p: 61 to 62 deg.F							
Portal to Portal Leave:	11:15am	Expenses Mileage:	11	Signed:	Neil Davis				
Return:	1:45pm	Density Gauge:	1.1	_***					
TOTAL:	2.5	Other:							

Darrell A. Gilman, CMT Manager

4/1/09

Reviewed:

Sent:

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DAILY FIELD REPORT

Date:

4/16/09

Project:

Avesta Florence House

Project #:

14194

Site Contacts:

Mike Adams, Ganneston Construction

Purpose of Visit:

Concrete Testing Services

Work Activities:

A crew from Newman Concrete placed a total of 24 cubic yards for the footings on line 16 from F to M and line F from 7 to 16. All concrete was supplied by Auburn Concrete with a mix design of 3/4" aggregate and a design strength of 3000psi., with a mid-range water reducer (glenium 7500) added at the batch plant. One set (#2) of four concrete test cylinders was cast and will be retrieved at a later date for controlled storage and compressive strength testing.

Test Results:

Slump: 5-1/4" to 5-3/4"

Air%: 5.5 to 5.8%

Concrete Temp: 47 to 49 deg.F

Air Temp: 53 deg.F

Portal to Portal		Expenses		Signed:	Frank Clark
Leave:	9:30am	Mileage:	11	cc:	
Return:	12:15pm	Density Gauge:			
TOTAL:	2.75	Other:	2.75 tolls	_	

Reviewed:

Darrell A. Gilman, CMT Manager

Sent:

4/17/09

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DAILY FIELD REPORT

Date:

4/17/09

Project:

Avesta Florence House - Portland, Maine

Project #:

14194

Site Contacts:

Mike Adams, Ganneston Construction

Purpose of Visit:

Concrete testing services.

Work Activities:

A crew from Newman Concrete placed a total of 65cy of 3/4" aggregate, 3500 psi concrete for the foundation wall on line N from line 16 to 7.1, and line 16 from N to N-10'. A high range water reducer (HRWR) Glenium 7500 was added to the concrete at the batch plant (19.7oz/cy) for workability. The concrete was supplied by Auburn

Concrete and placed via pump truck.

Soil compaction tests were performed on the structural gravel below the foundation

footing along line 7 at D-15'.

Concrete:

Slump range: 5 1/2" to 7 1/2"

Air Content range: 5.4% to 5.7% Concrete Temp.: 56 to 58 deg. F

Air Temp.: 68 deg. F W/C Ratios: 0.49 to 0.50

Soil:

% Compaction: 98.1%

Remarks:

Test Results:

Two sets of four cylinders were cast for compressive strength testing.

Portal to Portal		Expenses		Signed:	Darrell Gilman
Leave:	9:30	Mileage:	13	cc:	
Return:	3:00	Density Gauge:	1	_	
TOTAL:	5.5	Other:		_	

Reviewed: Sent:

Darrell A. Gilman, CMT Manager 4/20/09

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DAILY FIELD REPORT

Date:

4/23/09

Project:

Avesta Florence House - Portland, Maine

Project #:

14194

Site Contacts:

Mike Adams, Ganneston Construction

Purpose of Visit:

Concrete Testing Services.

Work Activities:

A crew from Newman Concrete placed a total of 13.5cy of 3/4" aggregate, 3000 psi concrete for the footing on line 7 from line D-A and line A from 1-7. The concrete

was supplied by Auburn Concrete and placed via tailgate.

One set of four test cylinders was cast and will be picked up at a later date.

Concrete:

Slump range: 4 1/2" to 5"

Test Results:

Air Content range: 6.2% Concrete Temp.: 60 deg. F

Air Temp.: 45 deg. F W/C Ratios: 0.49

Remarks:

Portal to Portal		Expenses		Signed:	Mike Sullivan
Leave:	9:30	Mileage:	13	cc:	
Return:	11:30	Density Gauge:			
TOTAL:	2	Other:			

Reviewed: Sent: Darrell A. Gilman, CMT Manager

-501

4/23/09

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TOTAL:

DAILY FIELD REPORT

Date:	5/1/09				
Project:	Avesta Floren	ce House - Portland, l	Maine		
Project #:	14194				· •
Site Contacts:	Mike Adams,	Ganneston Constructi	on		:
Purpose of Visit:	Concrete Tes	ting Services.			
Work Activities:	concrete for the The concrete v	Newman Concrete pla ne walls on 16-line fro was supplied by Aubu ur test cylinders were	m A to F	, and F-line fron ete and placed v	n 16 to the south end. ia pump truck.
					÷
Test Results:		ange: 6.2% to 6.3% ap.: 64 to 67 deg. F		7 i.i.	
Remarks:					
Portal to Portal Leave: Return:	9:15am	Expenses Mileage: Density Gauge:	13	Signed:cc:	Neil Davis

4.5

Other:

Reviewed: Sent: Darrell A. Gilman, CMT Manager 5/4/09

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DAILY FIELD REPORT

Date:	5/4/09						
Project:	Avesta Florence House - Portland, Maine						
Project #:	14194						
Site Contacts:	Mike Adams, Ganneston Construction						
Purpose of Visit:	Concrete Testing Services and Cylinder Pick Up.						
Work Activities:	concrete for the Concrete work One set of four	lewman Concrete place footings on N-line was supplied by Aubutest cylinders were a two sets of four concrength testing.	from 8.2 to orn Concre cast and w	o the elevator ba te and placed vi- ill be picked up	se slab to L8 or a truck chute. at a later date.	1-line.	
Test Results:	Slump Range: 4 1/2" to 5" Air Content: 6.2% Concrete Temp.: 66 to 67 deg. F Air Temp.: 60 deg. F						
Remarks:							
Portal to Portal Leave: Return: TOTAL:	12:15pm 3:15pm	Expenses Mileage: Density Gauge:	13	Signed: _cc:	Neil Davis		
IUIAL:	3	Other:					

Reviewed: Darrell A. Gilman, CMT Manager Sent: 5/5/09

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TOTAL:

5

Other:

Reviewed:

Sent:

Darrell A. Gilman, CMT Manager

5/11/09

DAILY FIELD REPORT

Date:	5/8/09	
Project:	Avesta Florence House - Portland, Maine	
Project #:	14194	2 d
Site Contacts:	Mike Adams, Ganneston Construction	
Purpose of Visit:	Concrete Testing Services.	
Work Activities:	A crew from Newman Concrete placed a total of 38cy of 3/4" aggregate, 3000 p for the following: -footings on lines 13 and 14 from N to J -column piers J13 and L13 -four pads south of 13-line -retaining wall footing -footings on 1-line from A.5 to D and south of 1-line from A.5 to D. Newman C also placed a total of 33 cubic yards of 3/4" aggregate, 3500 psi concrete for elevants on 5 and 5.8-line from M to N, M-line from 5 to 5.8, A-line from 1 to 7, and from A to E. The concrete ways and below the A-lane Constant.	Concrete vator shaft and 7-line
	from A to F. The concrete was supplied by Auburn Concrete contained a mid-ra reducer (Glenium 7500), air-entraining admixture (Micro-Air), and was placed v truck.	
Test Results:	Slump Range: 5-1/2" to 6" Air Content range: 6.0% to 6.2% Concrete Temp.: 64 to 66 deg. F Air Temp.: 70 to 71 deg. F	
Remarks:	Two sets of four test specimen were cast for compressive strength testing.	* (
Portal to Portal Leave: Return:	Expenses Signed: Neil Davis 10:30am Mileage: 13 cc: 3:30pm Density Gauge:	

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DAILY FIELD REPORT

Date:

5/15/09

Project:

Avesta Florence House - Portland, Maine

Project #:

14194

Site Contacts:

Mike Adams, Ganneston Construction

Purpose of Visit:

Concrete Testing Services.

Work Activities:

A crew from Newman Concrete placed a total of 7cy of 3/4" aggregate, 3500 psi concrete for the foundation wall at 1line A to D. The concrete was supplied by Auburn Concrete and

contained a mid-range water reducer (Glenium 7500), and was placed via tail gate.

Test Results:

Slump Range: 6 1/4" to 6 3/4"

Air Content range: 7.0% to 7.2%

Concrete Temp.: No thermometer was available

Air Temp.: 70 deg. F

Remarks:

One set of four test specimen was cast for compressive strength testing.

Portal to Portal		Expenses		Signed:	Justin Rouillard
Leave:	8:30 AM	Mileage:	15	cc:	
Return:	10:30 PM	Density Gauge:			
TOTAL:	2	Other:			
				Reviewed:	Darrell A. Gilman, CMT Manager

Sent:

5/21/09

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Leave:

Return:

TOTAL:

11:00 AM

5:15 PM

6.25

Mileage:

Other:

Density Gauge:

DAILY FIELD REPORT

,					
Date:	5/20/09				
Project:	Avesta Florence House - Portland, Ma	ine			
Project #:	14194				
Site Contacts:	Mike Adams, Ganneston Construction			\$:	
Purpose of Visit:	Concrete Testing Services.				
Work Activities:	A crew from Newman Concrete places for the foundation wall at 1line H to M. The concrete was supplied by Auburn (Glenium 7500), and was placed via process.	I, along M to 1.75, 2 lin Concrete and contained	e M to N, ar	nd along	N to 7.1.
•					
					•
				÷ •	
•					. 972
				ŧ	
				; ·	1 21
Test Results:	Slump Range: 6 1/4" to 6 3/4"				
	Air Content range: 7.0% to 7.2%				
	Concrete Temp.: No thermometer was	available			
	Air Temp.: 70 deg. F				
Remarks:	Two sets of four test specimen were of	east for compressive stre	ength testing	ž.	
	1	F	S	j. i	
Portal to Portal	Expenses	Signed:	Adam L	yons	€

Reviewed: Darrell A. Gilman, CMT Manager Sent: 5/21/09

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DAILY FIELD REPORT

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5/22/09

Project:

Avesta Florence House - Portland, Maine

Project #:

14194

Site Contacts:

Mike Adams, Ganneston Construction

Purpose of Visit:

Concrete Testing Services.

Work Activities:

A crew from Newman Concrete placed a total of 5cy of 3/4" aggregate, 3500 psi concrete

for the foundation footing at 1line H to D.2. The concrete was supplied by Auburn

Concrete and contained a mid-range water reducer (Glenium 7500), and was placed via tail

gate.

Test Results:

Slump Range: 3 1/2" to 4"

Air Content range: 6.0% to 6.5%

Concrete Temp.: 73 deg. F

Air Temp.: 80 deg. F

Remarks:

One set of four test specimen was cast for compressive strength testing.

Portal to Portal		Expenses		Signed:	Justin Rouillard
Leave:	12:00 AM	Mileage:	25	cc:	
Return:	3:00 PM	Density Gauge:			
TOTAL:	3	Other:			
		_		Reviewed: Sent:	Darrell A. Gilman, CMT Manager 5/27/09

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DAILY FIELD REPORT

Date:

5/27/09

Project:

Avesta Florence House - Portland, Maine

Project #:

14194

Site Contacts:

Mike Adams, Ganneston Construction

Purpose of Visit:

Concrete Testing Services.

Work Activities:

A crew from Newman Concrete placed a total of 80cy of 3/4" aggregate, 3000 psi concrete for stairwell 1 and 3 footings, and all interior footings between line 1 and 5.8. The crew also placed 11cy of 3/4" aggregate, 3500psi concrete for the foundation wall along line 1 from "D" to "H". The concrete was supplied by Auburn Concrete and contained a midrange water reducer (Glenium 7500), Micro Air, and was placed via pump truck.

Test Results:

Slump Range: 5 1/2" to 6 3/4"

Air Content range: 7.0% to 7.6%

Concrete Temp.: No thermometer was available

Air Temp.: 53 deg. F

Remarks:

Three sets of four test specimen were cast for compressive strength testing.

(Sets 15, 16, and 17)

Portal to Portal		Expenses		Signed:	Adam Lyons
Leave:	11:30 AM	Mileage:	113	cc:	
Return:	5:45 PM	Density Gauge:			·
TOTAL:	6.25	Other:		····	
		_		Reviewed:	Darrell A. Gilman, CMT Manager

Sent:

5/29/09

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DAILY FIELD REPORT

Date:

6/18/09

Project:

Avesta Florence House - Portland, Maine

Project #:

14194

Site Contacts:

Mike Adams, Ganneston Construction

Purpose of Visit:

Concrete testing services.

Work Activities:

A crew from Newman Concrete placed a total of 18cy of 3/4" aggregate, 3,500psi

concrete for the retaining wall. Concrete was supplied by Auburn Concrete and

was placed via pump truck supplied bby Northeast Pumping Services.

Test Results:

Slump range: 4 1/2" to 4 3/4"

Air Content: 5.2%

Concrete Temp.: 68 deg. F Air Temp.: ~65 deg. F

One set of four cylinders was cast for compressive strength testing.

Remarks:

Portal to Portal		Expenses		Signed:	Darrell A. Gilman
Leave:	11:45	Mileage:	15	cc:	
Return:	2:15	Density Gauge:		_	
TOTAL:	2.5	Other:		_	

Reviewed: Sent: Darrell A. Gilman, CMT Manager 6/19/09

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DAILY FIELD REPORT

7/6/09	•
Avesta Florence House - Portland, Maine	* *
14194	1
Mike Adams, Ganneston Construction	
Concrete testing services.	
air concrete for the first floor slab. Concrete was supplied by Auburn	Concrete
Slump range: 5-1/2 to 7-1/2" Air Content: 2.0 to 2.5% Concrete Temp.: 67 to 75 deg. F Air Temp.: 62 to 75 deg. F Two sets (#19,20) of four cylinders was cast for compressive streng	th testing.
Mike Adams was notified of all testing results.	
6:45 Mileage: 11 cc: 11:15 Density Gauge:	ris Bennett
	Mike Adams, Ganneston Construction Concrete testing services. A crew from Pride Concrete placed a total of 80cy of 3/4" aggregate air concrete for the first floor slab. Concrete was supplied by Auburn and was placed via pump truck supplied by Northeast Pumping Serv Slump range: 5-1/2 to 7-1/2" Air Content: 2.0 to 2.5% Concrete Temp.: 67 to 75 deg. F Air Temp.: 62 to 75 deg. F Two sets (#19,20) of four cylinders was cast for compressive streng Mike Adams was notified of all testing results.

Reviewed: Darrell A. Gilman, CMT Manager Sent: 7/8/09

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DAILY FIELD REPORT

Date:

7/10/09

Project:

Avesta Florence House - Portland, Maine

Project #:

14194

Site Contacts:

Mike Adams, Ganneston Construction

Purpose of Visit:

Concrete Testing Services.

Work Activities:

A crew from Pride Concrete placed a total of 80cy of 3/4" aggregate, 3,000psi, no

air concrete for the second floor slab. Concrete was supplied by Auburn Concrete

and was placed via pump truck supplied by Northeast Pumping Services.

Test Results:

Slump range: 5-1/2 to 7-1/2"

Air Content: 2.0 to 2.4%

Concrete Temp.: 69 to 75 deg. F Air Temp.: 56 to 72 deg. F

Two sets (#21,22) of four cylinders was cast for compressive strength testing.

Remarks:

Mike Adams was notified of all testing results.

Portal to Portal		Expenses		Signed:	Kris Bennett
Leave:	6:30	Mileage:	11	cc:	1
Return:	11:45	Density Gauge:			:
TOTAL:	5.25	Other:		_	

Reviewed: Sent: Darrell A. Gilman, CMT Manager 7/8/09

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DAILY FIELD REPORT

Date:

8/6/09

Project:

Avesta Florence House - Portland, Maine

Project #:

14194

Site Contacts:

Mike Adams, Ganneston Construction

Purpose of Visit:

Concrete Testing

Work Activities:

A crew from pride floors place a total of 126cy of concrete for the first floor slab. The first 70cy of concrete was a 3000psi mix design with a mid range water reducer (glenium 7500) added during the batching process. The remaining 56cy of concrete was also a 3000psi mix design with a mid range water reducer but in addition reinforcing fiber mesh was added during the batching process as well. All was going well until the crew began to slow down because the slab on the south half of the building was only approximately 3" thick. This caused the 10th truck to exceed it 90 min time limit while it was unloading. I informed Everett and he said the he was going to keep the truck and let it finish unloading. However, he rejected the 11th truck because it had been batched for 120 mins. The wait for another truck to arrive caused a "cold joint" to occur.

Test Results:

Slump Range: 5.5 to 6.5

Air Content: 2.5 to 4.0

Concrete Temp: Air Temp: 80degrees

Remarks:

Portal to Portal		Expenses		Signed:	Justin Rouillard
Leave:	6:00	Mileage:	11	_cc:	
Return:	3:00	Density Gauge:			•
TOTAL:	9	Other:			*

Reviewed: Sent: Darrell A. Gilman, CMT Manager 8/10/09

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: (207) 626-9094



CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No:

14194

Project:

Avesta Florence House

Client:

Avesta Florence House, LP and Florence House

307 Cumberland Avenue Portland, ME 04101

Field Test Data

Set No.:

C1

Placement Date:

1-Apr-09

Lab Rec'd Date:

2-Apr-09

Location:

Footings on N-Line 16 to 12.5

Technician:

N. Davis

Supplier:

Auburn Concrete

Mix Designation: 3/4" Aggregate

Design Strength:

3000psi

Slump (initial)

in.

°F

Slump (placed)

in.

Air Content

%

Conc Temp.

62.0

Air Temp.

°F

Volume (yds)

42.0 8.0

5

7.0

of 16.0

Admixture:

Micro Air, Glenium 7500 (Mid-Range Water)

Laboratory Test Data

Sample No.	Test Date	Age	Туре	Unit Wt.	Area (in²)	Load (K)	Strength (psi)
C1a	8-Apr-09	7	2	144.3	28.27	102.7	3630
C1b	29-Apr-09	28	3	144.3	28.27	124.1	4390
C1c	29-Apr-09	28	2	144.9	28.27	123.8	4380
C1d					7		
					· · · · · · · · · · · · · · · · · · ·		

Average 28 Day (psi):

4385



1

Cone and Split

2

Cone and Shear

3

4

Columnar

5



6

Remarks:

Reviewed: Darrell Gilman, CMT Manager Sent:04-29-09

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CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No:

14194

Project:

Avesta Florence House

Client:

Avesta Florence House, LP and Florence House

307 Cumberland Avenue Portland, ME 04101

Field Test Data

Set No.:

C2

Placement Date:

16-Apr-09

Lab Rec'd Date:

17-Apr-09

Location:

Footing; Line F from 7 to 16, Line 16 from F to M

Technician:

F. Clark

Supplier:

Auburn Concrete

Mix Designation: 3/4" Aggregate Design Strength:

3000psi

Slump (initial)

in.

Slump (placed)

in. %

Air Content

5 1/4

Conc Temp.

Air Temp.

53.0

°F °F

Volume (yds)

16.0

5.5

49.0

of 24.5

Admixture:

Glenium 7500 (Mid-Range Water Reducer)

Laboratory Test Data

Sample No.	Test Date	Age	Туре	Unit Wt.	Area (in²)	Load (K)	Strength (psi)
C2a	23-Apr-09	7	6	147.5	28.27	103.7	3670
C2b	14-May-09	28	3	149.1	28.27	127.4	4510
C2c	14-May-09	28	3	148.2	28.27	123.7	4370
C2d							
,							

Average 28 Day (psi):

4440



Split **2** a Shear

3

Columnar



6

5

Remarks:

Sent: 5-14-09

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CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No:

14194

Project:

Avesta Florence House

Client:

Avesta Florence House, LP and Florence House

307 Cumberland Avenue Portland, ME 04101

Field Test Data

Set No.:

C3

Placement Date:

17-Apr-09

Lab Rec'd Date:

18-Apr-09

Location:

Foundation Wall Line N from 16 to 7.1

Wall Line 16 from N to N-10'

Technician:

D. Gilman

Supplier:

Auburn Concrete

Mix Designation: 3/4" Aggregate

Design Strength:

3500psi

Slump (initial)

in.

Slump (placed)

in.

Air Content

Conc Temp.

5.4

7 1/2

% °F

Air Temp.

57.0 68.0

٥F

Volume (yds)

20.0

of 65.0

Admixture:

Glenium 7500 (Mid-Range Water Reducer)

Laboratory Test Data

Sample No.	Test Date	Age	Туре	Unit Wt.	Area (in ²)	Load (K)	Strength (psi)
C3a	24-Apr-09	7	2	146.0	28.27	91.5	3240
C3b	15-May-09	28	3	146.8	28.27	109.4	3870
C3c	15-May-09	28	3	147.1	28.32	114.9	4060
C3d							

Average 28 Day (psi):

3965



Split 2

Shear 3

4

Columnar

5



6

Remarks:

Reviewed: Darrell Gilman, CMT Manager

Sent: 5-14-09

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CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No:

14194

Project:

Avesta Florence House

Client:

Avesta Florence House, LP and Florence House

307 Cumberland Avenue Portland, ME 04101

Field Test Data

Set No.:

C4

Placement Date:

17-Apr-09

Lab Rec'd Date:

18-Apr-09

Location:

Foundation Wall Line N from 16 to 7.1

Line 16 from N to N-10'

Technician:

D. Gilman

Supplier:

Auburn Concrete

Mix Designation: 3/4" Aggregate

Design Strength:

3500psi

Slump (initial)

in.

%

°F

Slump (placed)

in.

Air Content

Conc Temp.,

Air Temp.

°F

Volume (yds)

68.0 50.0

7 1/2

5.7

58.0

of 65.0

Admixture:

Glenium 7500 (Mid-Range Water Reducer)

Laboratory Test Data

Sample No.	Test Date	Age	Туре	Unit Wt.	Area (in ²)	Load (K)	Strength (psi)
C4a	24-Apr-09	7	2	144.5	28.27	92.9	3290
C4b	15-May-09	28	2	145.5	28.27	109.3	3870
C4c	15-May-09	28	2	145.9	28.27	107.1	3790
C4d							
				·			

Average 28 Day (psi):

3830



Split

Columnar



2

Shear 3

4

5

Other 6

Remarks:

Reviewed: Darrell Gilman, CMT Manager Sent: 5-14-09

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CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No:

14194

Project:

Avesta Florence House

Client:

Avesta Florence House, LP and Florence House

307 Cumberland Avenue Portland, ME 04101

Field Test Data

Set No.:

C5

Placement Date:

23-Apr-09

Lab Rec'd Date:

24-Apr-09

Location:

Line 7 From Line D to A, Line A From 1 to 7 Footing

Technician:

M. Sullivan

Supplier:

Auburn Concrete

Mix Designation: 3/4" Aggregate

Design Strength:

3000psi

5

6.2

60.0

45.0

13.5

Slump (initial)

in.

Slump (placed)

in.

Air Content

%

Conc Temp.

°F

Air Temp.

°F

Volume (yds)

of 13.5

Admixture:

Laboratory Test Data

Sample No.	Test Date	Age	Туре	Unit Wt.	Area (in²)	Load (K)	Strength (psi)
C5a	30-Apr-09	7	2	145.5	28.27	101.1	3570
C5b	21-May-09	28	6	147.5	27.91	126.0	4510
C5c	21-May-09	28	3	147.7	27.97	121.1	4330
C5d					***		

Average 28 Day (psi):

4420



Split

3

Columnar



4

5

6

Remarks:

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: (207) 626-9094



CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No:

14194

Project:

Avesta Florence House

Client:

Avesta Florence House, LP and Florence House

307 Cumberland Avenue Portland, ME 04101

Field Test Data

Set No.:

C6

Placement Date:

1-May-09

Lab Rec'd Date:

2-May-09

Location:

Walls on 16-Line from A to F (Lower Half)

Technician:

N. Davis

Supplier:

Auburn Concrete

Mix Designation: 3/4" Aggregate

Design Strength:

3500 psi

Slump (initial)

in.

Slump (placed)

in.

Air Content

%

٥F

Conc Temp.

Air Temp.

71.0

٥F

Volume (yds)

30.0

65.0

6

6.2

of 60.0

Admixture:

Micro Air, Glenium 7500 (Mid-Range Water Reducer)

Laboratory Test Data

Sample No.	Test Date	Age	Туре	Unit Wt.	Area (in ²)	Load (K)	Strength (psi)
C6a	8-May-09	7	2	143.9	28.27	97.2	3440
C6b	29-May-09	28	6	145.3	28.18	120.9	4290
C6c	29-May-09	28	3	143.3	28.46	117.7	4140
C6d							

Average 28 Day (psi):

4215



Split

Columnar



Shear 3

4

5

Other 6

Remarks:

Reviewed: Darrell Gilman, CMT Manager

Date: 5-29-09

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: (207) 626-9094



CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No:

14194

Project:

Avesta Florence House

Client:

Avesta Florence House, LP and Florence House

307 Cumberland Avenue Portland, ME 04101

Field Test Data

Set No.:

C7

Placement Date:

1-May-09

Lab Rec'd Date:

2-May-09

Location:

Walls on 16-Line from A to F (Upper Half)

Technician:

N. Davis

Supplier:

Auburn Concrete

Mix Designation: 3/4" Aggregate

Design Strength:

3500 psi

Slump (initial)

in.

Slump (placed)

in.

Air Content

%

Conc Temp.

64.0

°F

Air Temp. Volume (yds) 70.0 60.0

5 1/2

6.3

°F of 60.0

Admixture:

Micro Air, Glenium 7500 (Mid-Range Water Reducer)

Laboratory Test Data

Sample No.	Test Date	Age	Туре	Unit Wt.	Area (in ²)	Load (K)	Strength (psi)
C7a	8-May-09	7	2	143.8	28.27	91.1	3220
C7b	29-May-09	28	6	146.3	28.18	108.6	3850
C7c	29-May-09	28	1	144.5	28.46	111.2	3910
C7d					-··		
					<u>.</u>	7	

Average 28 Day (psi):

3880



Split









4

5

6

Remarks:

Reviewed: Darrell Gilman, CMT Manager

Date: 5-29-09

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CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No:

14194

Project:

Avesta Florence House

Client:

Avesta Florence House, LP and Florence House

307 Cumberland Avenue Portland, ME 04101

Field Test Data

Set No.:

C8

Placement Date:

4-May-09

Lab Rec'd Date:

5-May-09

Location:

Footings on N-Line from 8.2 to Elevator Base Slab

Technician:

N. Davis

Supplier:

Auburn Concrete

Mix Designation: 3/4" Aggregate

Design Strength:

3000psi

4 1/2

6.2

66.0

Slump (initial)

in.

 ${}^{\rm o}{
m F}$

Slump (placed)

in.

Air Content

%

Conc Temp.

Air Temp.

 $^{\circ}F$

Volume (yds)

60.0 8.5

of 17.0

Admixture:

Micro Air, Glenium 7500 (Mid-Range Water Reducer)

Laboratory Test Data

Sample No.	Test Date	Age	Туре	Unit Wt.	Area (in ²)	Load (K)	Strength (psi)
C8a	11-May-09	7	MS	147.1	28.27	103.0	3640
C8b	1-Jun-09	28	AL	147.6	28.27	130.6	4620
C8c	1-Jun-09	28	AL	146.9	28.27	122.2	4320
C8d							
			,				
N	1						

Average 28 Day (psi):

4470



Split

Shear

Columnar

5

Other 6

Remarks:

Reviewed: Darrell Gilman, CMT Manager

Date: 6-2-09

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: (207) 626-9094



CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No:

14194

Project:

Avesta Florence House

Client:

Avesta Florence House, LP and Florence House

307 Cumberland Avenue Portland, ME 04101

Field Test Data

Set No.:

C9

Placement Date:

8-May-09

Lab Rec'd Date:

9-May-09

Location:

Foundation Footings on 13-Line from N-Line to J-Line

Including Pads L13 and J13

Technician:

N. Davis

Supplier:

Auburn Concrete

Mix Designation: 3/4" Aggregate

Design Strength:

3000psi

Slump (initial)

in.

Slump (placed)

in.

Air Content

%

٥F

Conc Temp.

Air Temp.

70.0

Volume (yds)

19.0

64.0

5 1/4

6.0

٥F of 71.0

Admixture:

Micro Air, Glenium 7500 (Mid-Range Water Reducer)

Laboratory Test Data

Sample No.	Test Date	Age	Туре	Unit Wt.	Area (in ²)	Load (K)	Strength (psi)
C9a	15-May-09	7	2	146.2	28.27	101.1	3570
C9b	5-Jun-09	28	3	146.0	28.27	114.1	4040
C9c	5-Jun-09	28	3	146.9	28.27	114.5	4050
C9d							
	1						

Average 28 Day (psi):

4045



1

Split

Cone and Shear

3

Shear

4

Columnar

5

Other

6

Remarks:

Reviewed: Darrell Gilman, CMT Manager

Date: 6-5-09

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: (207) 626-9094



CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No:

14194

Project:

Avesta Florence House

Client:

Avesta Florence House, LP and Florence House

307 Cumberland Avenue Portland, ME 04101

Field Test Data

Set No.:

C10

Placement Date:

8-May-09

Lab Rec'd Date:

9-May-09

Location:

Foundation Walls; Elevator Shaft Walls on 5 and 5.8 Lines

from N to M and M-Line from 5 to 5.8

Technician:

N. Davis

Supplier:

Auburn Concrete

Mix Designation: 3/4" Aggregate

Design Strength:

3500psi

6.1

65.0

70.0

Slump (initial)

in.

Slump (placed)

5 3/4 in.

Air Content

%

Conc Temp.

٥F

Air Temp.

°F

Volume (yds)

of

Admixture:

Micro Air, Glenium 7500 (Mid-Range Water Reducer)

Laboratory Test Data

Sample No.	Test Date	Age	Type	Unit Wt.	Area (in ²)	Load (K)	Strength (psi)
C10a	15-May-09	7	3	143.1	28.27	93.2	3300
C10b	5-Jun-09	28	2	143.6	28.27	109.1	3860
C10c	5-Jun-09	28	2	143.7	28.27	110.0	3890
C10d							-

				. "			

Average 28 Day (psi):

3875



1

Split

Cone and Shear

Columnar

5



6

Remarks:

Reviewed: Darrell Gilman, CMT Manager Sent:6-5-09

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: (207) 626-9094



CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No:

14194

Project:

Avesta Florence House

Client:

Avesta Florence House, LP and Florence House

307 Cumberland Avenue Portland, ME 04101

Field Test Data

Set No.:

C11

Placement Date:

15-May-09

Lab Rec'd Date:

18-May-09

Location:

Foundation Wall A1 - D1

Technician:

J. Rouillard

Supplier:

Auburn Concrete

Mix Designation: 3/4" Aggregate

Design Strength:

3500psi

Slump (initial)

in.

Slump (placed)

in.

Air Content

Conc Temp.

Air Temp.

72.5

٥F ٥F

%

Volume (yds)

68.0

3 1/2

7.0

of 7.0

Admixture:

7.0 Glenium 7500 (Mid-Range Water Reducer), Micro Air

Laboratory Test Data

Sample No.	Test Date	Age	Туре	Unit Wt.	Area (in ²)	Load (K)	Strength (psi)
C11a	22-May-09	7	6	143.7	28.27	75.5	2670
C11b	12-Jun-09	28	6	144.2	28.27	127.8	4520
C11c	12-Jun-09	28	6	147.9	28.27	118.0	4170
Clld							
						~	
					···		

Average 28 Day (psi):

4345



Split

Columnar

Shear 3

4

5

Other

6

Remarks:

Reviewed: Darrell Gilman, CMT Manager Date: 6-15-09

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CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No:

14194

Project:

Avesta Florence House

Client:

Avesta Florence House, LP and Florence House

307 Cumberland Avenue Portland, ME 04101

Field Test Data

Set No.:

C12

Placement Date:

20-May-09

Lab Rec'd Date:

21-May-09

Location:

Along M to 1.75 Footing

Technician:

A. Lyons

Supplier:

Auburn Concrete

Mix Designation: 3/4" Aggregate

Design Strength:

3500psi

Slump (initial)

in.

Slump (placed)

in.

Air Content

%

Conc Temp.

٥F

Air Temp.

70.0

٥F

Volume (yds)

11.0

6 1/4

7.0

of 54.0

Admixture:

Mid-Range Water Reducer

Laboratory Test Data

Sample No.	Test Date	Age	Туре	Unit Wt.	Area (in ²)	Load (K)	Strength (psi)
C12a	27-May-09	7	2	145.5	28.27	111.6	3950
C12b	17-Jun-09	28	3	144.3	28.27	122.5	4330
C12c	17-Jun-09	28	3	144.2	28.27	120.5	4260
C12d							

Average 28 Day (psi):

4295



Split

2

Shear

4

Columnar

Other

1

5

6

Remarks:

Reviewed: Darrell Gilman, CMT Manager

Date: 5-28-09

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: (207) 626-9094



CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No:

14194

Project:

Avesta Florence House

Client:

Avesta Florence House, LP and Florence House

307 Cumberland Avenue Portland, ME 04101

Field Test Data

Set No.:

C13

Placement Date:

20-May-09

Lab Rec'd Date:

21-May-09

Location:

Along N Line Approximately at 4 Footing

Technician:

A. Lyons

Supplier:

Auburn Concrete

Mix Designation: 3/4" Aggregate

Design Strength:

3500psi

Slump (initial)

in.

Slump (placed)

in.

Air Content

%

Conc Temp. Air Temp.

70.0

٥F ٥F

Volume (yds)

33.0

6 3/4

7.2

of 54.0

Admixture:

Mid-Range Water Reducer

Laboratory Test Data

Sample No.	Test Date	Age	Туре	Unit Wt.	Area (in ²)	Load (K)	Strength (psi)
C13a	27-May-09	7	4	141.4	28.27	86.7	3070
C13b	17-Jun-09	28	6	141.8	28.27	108.1	3820
C13c	17-Jun-09	28	3	142.2	28.27	107.8	3810
C13d							

Average 28 Day (psi):

3815



Split

3

Columnar

2

4

5

Other 6

Remarks:

Reviewed: Darrell Gilman, CMT Manager

Date: 6-17-09

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CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No:

14194

Project:

Avesta Florence House

Client:

Avesta Florence House, LP and Florence House

307 Cumberland Avenue Portland, ME 04101

Field Test Data

Set No.:

C14

Placement Date:

22-May-09

Lab Rec'd Date:

23-May-09

Location:

Along Line 1 from H - D.2 Wall Footing

Technician:

J. Rouillard

Supplier:

Auburn Concrete

Mix Designation: 3/4" Aggregate

Design Strength:

3500psi .

3 1/2

6.0

73.0 80.0

Slump (initial)

in.

Slump (placed)

in.

Air Content

% °F

Conc Temp.

Air Temp.

Volume (yds)

°F 5.0

5.0 of

Admixture:

Glenium 7500 (Mid-Range Water Reducer)

Laboratory Test Data

Sample No.	Test Date	Age	Туре	Unit Wt.	Area (in ²)	Load (K)	Strength (psi)
C14a	29-May-09	7	4	147.7	28.27	91.2	3230
C14b	19-Jun-09	28	4	146.9	28.27	117.4	4150
C14c	19-Jun-09	28	4	146.6	28.27	119.4	4220
C14d							

Average 28 Day (psi):

4185



Cone and

Shear

Columnar

Other

1

Split 2

Shear 3

4

5

6

Remarks:

Reviewed: Darrell Gilman, CMT Manager

Date: 6-19-09

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CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No:

14194

Project:

Avesta Florence House

Client:

Avesta Florence House, LP and Florence House

307 Cumberland Avenue Portland, ME 04101

Field Test Data

Set No.:

C15

Placement Date:

27-May-09 28-Feb-09

Lab Rec'd Date: Location:

Stair 1 Footings

Technician:

A. Lyons

Supplier:

Auburn Concrete

Mix Designation:

3/4" Aggregate

Design Strength:

3000psi

Slump (initial)

in.

Slump (placed)

in.

Air Content

%

Conc Temp.

°F

Air Temp.

°F

Volume (yds)

53.0 10.0

6

7.6

of 91.0

Admixture:

Glenium 7500 (Mid-Range Water Reducer), Micro Air

Laboratory Test Data

Sample No.	Test Date	Age	Type	Unit Wt.	Area (in ²)	Load (K)	Strength (psi)
C15a	3-Jun-09	7	3	143.1	28.27	100.4	3550
C15b	24-Jun-09	28	3	138.3	28.27	126.7	4480
C15c	24-Jun-09	28	3	143.7	28.27	124.8	4420
C15d			,				

Average 28 Day (psi):

4450



1

Cone and Split

Cone and

Shear

Columnar

Other

6

Remarks:

3 4 5

Reviewed: Darrell Gilman, CMT Manager Date: 6-24-09

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: (207) 626-9094



CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No:

14194

Project:

Avesta Florence House

Client:

Avesta Florence House, LP and Florence House

307 Cumberland Avenue Portland, ME 04101

Field Test Data

Set No.:

C16

Placement Date:

27-May-09

Lab Rec'd Date:

28-May-09

Location:

Large Interior Footing

BJ5

Technician:

A. Lyons

Supplier:

Auburn Concrete

Mix Designation: 3/4" Aggregate

Design Strength:

3000psi

Slump (initial)

in.

Slump (placed)

in.

Air Content

Conc Temp.

% o_F

Air Temp.

53.0

°F

Volume (yds)

50.0

6

7.2

of 91.0

Admixture:

Glenium 7500 (Mid-Range Water Reducer), Micro Air

Laboratory Test Data

Sample No.	Test Date	Age	Туре	Unit Wt.	Area (in ²)	Load (K)	Strength (psi)
C16a	3-Jun-09	7	6	139.3	28.27	71.0	2510
C16b	24-Jun-09	28	4	139.2	28.27	96.6	3420
C16c	24-Jun-09	28	4	138.8	28.27	93.3	3300
C16d					<u> </u>		
		-		,			

Average 28 Day (psi):

3360



Cone and Split

2

Cone and Shear

3

4

Columnar

5

Other

1

6

Remarks:

Reviewed: Darrell Gilman, CMT Manager

Date: 6-24-09

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: (207) 626-9094



CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No:

14194

Project:

Avesta Florence House

Client:

Avesta Florence House, LP and Florence House

307 Cumberland Avenue Portland, ME 04101

Field Test Data

Set No.:

C17

Placement Date:

27-May-09

Lab Rec'd Date:

28-May-09

Location:

Foundation Walls Line 1 "D" to "H"

Technician:

A. Lyons

Supplier:

Auburn Concrete

Mix Designation: 3/4" Aggregate

Design Strength:

3500psi

Slump (initial)

in.

Slump (placed)

6 3/4 in.

Air Content

%

Conc Temp.

 $^{\rm o}{
m F}$

Air Temp.

53.0

 ${}^{o}F$

Volume (yds)

91.0

7.0

of 91.0

Admixture:

Glenium 7500 (Mid-Range Water Reducer), Micro Air

Laboratory Test Data

Sample No.	Test Date	Age	Туре	Unit Wt.	Area (in ²)	Load (K)	Strength (psi)
C17a	3-Jun-09	7	4	143.8	28.27	106.3	3760
C17b	24-Jun-09	28	4	144.4	28.27	142.5	5040
C17c	24-Jun-09	28	4	144.4	28.27	139.9	4950
C17d		1			P.,		
4							

Average 28 Day (psi):

4995



1

Split

2

Shear

Columnar

5



Other

6

Remarks:

Reviewed: Darrell Gilman, CMT Manager

Date: 6-24-09

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: (207) 626-9094



CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No:

14194

Project:

Avesta Florence House

Client:

Avesta Florence House, LP and Florence House

307 Cumberland Avenue Portland, ME 04101

Field Test Data

Set No.:

C18

Placement Date:

18-Jun-09

Lab Rec'd Date:

19-Jun-09

Location:

Retaining Wall

Technician:

D. Gilman

Supplier:

Auburn Concrete

Mix Designation: 3/4 Aggregate

Design Strength:

3500psi

5.2

68.0

85.0

8.5

Slump (initial)

in.

Slump (placed)

4 3/4

in.

٥F

Air Content

% ${}^{\circ}F$

Conc Temp.

Air Temp. Volume (yds)

of 18.0

Admixture:

Laboratory Test Data

Sample No.	Test Date	Age	Туре	Unit Wt.	Area (in ²)	Load (K)	Strength (psi)
C18a	25-Jun-09	7	4	147.4	28.27	101.7	3600
C18b	16-Jul-09	28	6	146.9	28.27	116.2	4110
C18c	16-Jul-09	28	6	146.9	28.27	111.4	3940
C18d							

Average 28 Day (psi):

4025



Cone and Split Shear

Shear

4

Columnar

5

Other 6

Remarks:

Reviewed: Darrell Gilman, CMT Manager

Date: 7-16-09

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: (207) 626-9094



CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No:

14194

Project:

Avesta Florence House

Client:

Avesta Florence House, LP and Florence House

307 Cumberland Avenue Portland, ME 04101

Field Test Data

Set No.:

C19

Placement Date:

6-Jul-09

Lab Rec'd Date:

7-Jul-09

Location:

First Floor Slab

Technician:

K. Bennett

Supplier:

Auburn Concrete

Mix Designation: 3/4" Aggregate

Design Strength:

3000psi

7 1/2

2.5

71.0

Slump (initial)

in.

Slump (placed)

in.

Air Content

%

Conc Temp.

°F

Air Temp.

°F

Volume (yds)

25.0

of 80.0

Admixture:

Glenium 7500 (Mid-Range Water Reducer)

Laboratory Test Data

Sample No.	Test Date	Age	Type	Unit Wt.	Area (in²)	Load (K)	Strength (psi)
C19a	13-Jul-09	7	6	147.7	28.27	68.7	2430
C19b	3-Aug-09	28	6	148.3	28.27	90.9	3210
C19c	3-Aug-09	28	6	148.0	28.27	89.2	3150
C19d							

Average 28 Day (psi):

3180



Cone and Split

2

Cone and Shear

3

Shear

4

Columnar

5



6

Remarks:

Reviewed: Darrell Gilman, CMT Manager

Date: 8-04-09

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: (207) 626-9094



CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No:

14194

Project:

Avesta Florence House

Client:

Avesta Florence House, LP and Florence House

307 Cumberland Avenue Portland, ME 04101

Field Test Data

Set No.:

C20

Placement Date:

6-Jul-09

Lab Rec'd Date:

7-Jul-09

Location:

Second Floor Slab

Technician:

K. Bennett

Supplier:

Auburn Concrete

Mix Designation: 3/4" Aggregate

Design Strength:

3000psi

7

Slump (initial)

in.

Slump (placed)

in. %

Air Content

Conc Temp.

2.4 73.0

Air Temp.

75.0

°F °F

Volume (yds)

of 80.0

65.0

Admixture:

Glenium 7500 (Mid-Range Water Reducer)

Laboratory Test Data

Sample No.	Test Date	Age	Type	Unit Wt.	Area (in ²)	Load (K)	Strength (psi)
C20a	13-Jul-09	7	6	147.5	28.27	70.8	2510
C20b	3-Aug-09	28	6	149.3	28.27	90.0	3180
C20c	3-Aug-09	28	6	150.0	28.27	88.0	3110
C20d							

Average 28 Day (psi):

3145



Split

Shear

4

Columnar

5

Other

6

Remarks:

Reviewed: Darrell Gilman, CMT Manager

Date: 8-04-09

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: (207) 626-9094



CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No:

14194

Project:

Avesta Florence House

Client:

Avesta Florence House, LP and Florence House

307 Cumberland Avenue Portland, ME 04101

Field Test Data

Set No.:

C23

Placement Date:

6-Aug-09

Lab Rec'd Date:

7-Aug-09

Location:

First Floor Slab

Technician:

J. Rouillard

Supplier:

Auburn Concrete

Mix Designation: 3/4" Aggregate

Design Strength:

3000psi

Slump (initial)

in.

Slump (placed)

6 1/2

Air Content

in.

Conc Temp.

% °F

Air Temp.

٥F

Volume (yds)

80.0

2.5

10.0 of 126.0

Admixture:

Glenium 7500 (Mid Range Water Reducer

Laboratory Test Data

Sample No.	Test Date	Age	Туре	Unit Wt.	Area (in ²)	Load (K)	Strength (psi)
C23a	13-Aug-09	7	6	148.3	28.27	93.3	3300
C23b	3-Sep-09	28	6	149.2	28.27	104.9	3710
C23c	3-Sep-09	28	6	149.0	28.27	109.9	3890
C23d				•			

Average 28 Day (psi):

3800



1

Cone and Split

2

Cone and Shear

3

Shear

4

Columnar

5

Other 6

Remarks:

Reviewed: Darrell Gilman, CMT Manager

Date: 9-7-09

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: (207) 626-9094



CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No:

14194

Project:

Avesta Florence House

Client:

Avesta Florence House, LP and Florence House

307 Cumberland Avenue Portland, ME 04101

Field Test Data

Set No.:

C24

Placement Date:

6-Aug-09

Lab Rec'd Date:

7-Aug-09

Location:

First Floor Slab

Technician:

J. Rouillard

Supplier:

Auburn Concrete

Mix Designation: 3/4" Aggregate Design Strength:

3000psi

6

3.0

Slump (initial)

in.

Slump (placed)

Air Content

in.

%

Conc Temp.

 $^{\rm o}{
m F}$

Air Temp.

80.0

°F

Volume (yds)

70.0 of 126.0

Admixture:

Glemium 7500 (Mid-Range Water Reducer)

Laboratory Test Data

Sample No.	Test Date	Age	Туре	Unit Wt.	Area (in²)	Load (K)	Strength (psi)
C24a	13-Aug-09	7	6	149.5	28.27	88.9	3150
C24b	3-Sep-09	28	1	149.2	28.27	108.6	3840
C24c	3-Sep-09	28	3	149.0	28.27	106.6	3770
C24d							

Average 28 Day (psi):

3805



Cone and

Shear

1

Split 2

Shear 3

4

5

Columnar

Other

6

Remarks:

Reviewed: Darrell Gilman, CMT Manager

Date: 9-8-09

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: (207) 626-9094



CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No:

14194

Project:

Avesta Florence House

Client:

Avesta Florence House, LP and Florence House

307 Cumberland Avenue Portland, ME 04101

Field Test Data

Set No.:

C25

Placement Date:

6-Aug-09

Lab Rec'd Date:

7-Aug-09

Location:

First Floor Slab

Technician:

J. Rouillard

Supplier:

Auburn Concrete

Mix Designation: 3/4 Aggregate

Design Strength:

3000psi

5 1/2

4.0

Slump (initial)

in.

Slump (placed)

in.

Air Content

%

Conc Temp.

°F

٥F

Air Temp.

Volume (yds)

80.0 120.0

of 126.0

Admixture:

Glenium 7500 (Mid-Range Water Reducer)

Laboratory Test Data

Sample No.	Test Date	Age	Туре	Unit Wt.	Area (in ²)	Load (K)	Strength (psi)
C25a	13-Aug-09	7	6	148.7	28.27	88.9	3140
C25b	3-Sep-09	28	6	146.4	28.27	101.5	3590
C25c	3-Sep-09	28	3	147.8	28.27	100.3	3550
C25d							

Average 28 Day (psi):

3570



1

Cone and Split

2

Cone and Shear

3

Shear

4

Columnar

5

Other 6

Remarks:

Reviewed: Darrell Gilman, CMT Manager

Date: 9-8-09

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: (207) 626-9094



CONCRETE COMPRESSIVE STRENGTH TEST RESULTS - ASTM C39

Project No:

14194

Project:

Avesta Florence House

Client:

Avesta Florence House, LP and Florence House

307 Cumberland Avenue Portland, ME 04101

Field Test Data

Set No.:

C26

Placement Date:

28-Aug-09

Lab Rec'd Date:

31-Aug-09

Location:

Stairwell Slab 1 to 3 Basement Level

Technician:

C. Young

Supplier:

Auburn Concrete

Mix Designation: 3/4" Aggregate

Design Strength:

3000psi

Slump (initial)

in.

Slump (placed)

in.

Air Content

% °F

Conc Temp.

Air Temp.

54.0

 ${}^{o}F$

Volume (yds)

7.0

5

3.0

72.0

of 15.0

Admixture:

Glenium 7500 (Mid-Range Water Reducer), 1%-Pozzutec 20%

Laboratory Test Data

Sample No.	Test Date	Age	Туре	Unit Wt.	Area (in²)	Load (K)	Strength (psi)
C26a	4-Sep-09	7	2	145.4	28.27	76.0	2690
C26b	25-Sep-09	28	4	146.5	28.27	95.5	3380
C26c	25-Sep-09	28	4	146.4	28.27 .	109.8	3880
C26d							
					_		

Average 28 Day (psi):

3630



Split

2

Shear

3

Columnar

Other

4

5

6

Remarks:

Reviewed: Darrell Gilman, CMT Manager

Date: 9-28-09

EXHIBIT B

04200 CMU Construction

Project: Florence House Date Prepared: September 2, 2008

Structural Schedule of Special Inspections MASONRY CONSTRUCTION – LEVEL 1 (NON-ESSENTIAL FACILITY)

VERIFICATION AND INSPECTION IBC Section 1704.5	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS	AGENT	AGENT QUALIFICATION	TASK COMPLETED
1. As masonry construction begins, the following shall be verified to ensure compliance:						
a. Proportions of site-prepared mortar.	Y	P	ACI530.1, 2.6A	SI1	PE/SE or EIT	TESTING MYCHULT
b. Construction of mortar joints.	Y	P	ACI530.1, 3.3B	SII	PE/SE or EIT	JÆ
c. Location of reinforcement and connectors.	Y	P	ACI530.1, 3.4, 3.6A	SII	PE/SE or EIT	F
d. Prestressing technique.	N/A	P	ACI530.1, 3.6B		PE/SE or EIT	
e. Grade and size of prestressing tendons and anchorages.	N/A	Р	ACI530.1, 2.4B, 2.4H		PE/SE or EIT	
2. The inspection program shall verify:						
a. Size and location of structural elements.	Y	P	ACI530.1, 3.3G	SI1	PE/SE or EIT	TH
b. Type, size and location of anchors, including other details of anchorage of masonry to structural members, frames or other construction.	Y	P	ACI530, 1.2.2(e), 2.1.4, 3.1.6	SII	PE/SE or EIT	T4
c. Specified size, grade and type of reinforcement.	Y	P	ACI530, 1.12, ACI530.1, 2.4, 3.4	SII	PE/SE or EIT	TUF
d. Welding of reinforcing bars.	Y	С	AC530, 2.1.10.6.2, 3.24 (b)	TA2	AWS-CWI	_
e. Protection of masonry during cold weather (temperature below 40°F) or hot weather (temperature above 90°F).	Y	P	IBC 2104.3, 2104.4; ACI530.1, 1.8C, 1.8D	SI1	PE/SE or EIT	
f. Application and measurement of prestressing force.	N/A	P	ACI530.1, 3.6B	No.	PE/SE or EIT	
3. Prior to grouting, the following shall be verified to ensure compliance:						
a. Grout space is clean.	Y	P	ACI530.1, 3.2D	SII	PE/SE or EIT	TV
b. Placement of reinforcement and connectors and prestressing tendons and anchorages.	Y	P	ACI530, 1.12, ACI530.1, 3.4	SII	PE/SE or EIT	TH
 Proportions of site-prepared grout and prestressing grout for bonded tendons. 	Y	Р	ACI530.1, 2.6B	SI1	PE/SE or EIT	TH
d. Construction of mortar joints.	Y	P	ACI530.1, 3.3B	SII	PE/SE or EIT	JUF
Grout placement shall be verified to ensure compliance with code and construction document provisions.	Y	С	ACI530.1, 3.5	SII	PE/SE or EIT	TH.
a. Grouting of prestressing bonded tendons.	N/A	C	ACI530.1, 3.6C		PE/SE or EIT	
5. Preparation of any required grout specimens, mortar specimens and/or prisms shall be observed.	Y	С	IBC 2105.2.2, 2105.3; ACI530.1, I.4	SII	PE/SE or EIT	TESTING MUNY
Compliance with required inspection provisions of the construction documents and the approved submittals shall be verified.	Y	P	ACI530.1, 1.5	SII	PE/SE or EIT	/

BECKE R structural engineers, inc.

OBSERVATION RE	EPORT				Date:	June 30, 2009
CMU					Time:	2:30pm - 3:30 pm
					Temp:	60's
	*				Weather:	Cloudy
Project: F	·					
	lorence	House	3	. 000000 e0000eeeeeeee		
Location:	ortland,	Maine)			
Becker Job No: S	31 2058			****		
Observation Location	ı: CMU v	valls a	t Elev	ator and	l Stairs adjacent to) Line N
9					· · · · · · · · · · · · · · · · · · ·	: :
	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable		Comments
Reinforcement Size						
Quantity					See note below	The state of the s
Condition						
Placement			Ш		Rebar positione bar location,	rs shall be used to ensure proper
Embed/Anchors				\boxtimes		en e
Lap Splices	xx - xxx]
Hot Weather		Щ		\boxtimes	· · · · · · · · · · · · · · · · · · ·	
Cold Weather		Ц		\boxtimes		
CMU Size		Ц	Щ	<u> </u>		
Layout/Fit-up/Plumbnes		Н	<u> </u>		1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	With the second control of the contr
Mortar/Grouting Proced	ure 🔼	L	L			ve grout, keep cavities free of mortar
Lift Height	П	П	П	\boxtimes	fins and droppin	ys.
Clean Outs		Н	H	X		MONOMINA MANAGEM MONOMINE COLLEGE SEE AMERICAN COLLEGE
Bond Beams		H	H	X	The second secon	AND THE RESIDENCE OF THE WASHINGTON OF THE WASHINGTON OF THE CONTROL OF THE CONTR
Additional Items			Ī		***************************************	AND THE RESERVE OF THE STREET

Notes:

This visit was conducted to review the start of CMU construction at the areas noted above. Work appears to be progressing satisfactorily, and was found to be in general conformance with the contract documents. One item was noted that will require attention by the General Contractor and masonry sub-contractor.

1. Vertical bars are required at the ends of CMU walls terminating against the concrete foundation wall, as shown in CSKS-11 and Dwg. S2.1.

Thank you -

Signed: James Fortin, P.E.



Transmittal

From: cc: Date: Re:	s C uts S	01	
Copies	Date	Description	
8		Masonry reinforcing certification of Compliance	
Appr Retur Subm	rned for correction oved & noted rn corrected hit copies for omit copies	Construction approval prints For review & comment For bids due For your use	
Remarks	:	BECKER STRUCTURAL ENGINEERS, INC. JUL 15 architectinterior de	

interior design master planning landscape architecture

Ganneston Construction Corp PO Box 27, North Belfast Avenue Augusta, ME 04332-0027

LETTER OF TRANSMITTAL

Tel: (207) 621-8505 Fax: (207) 621-8508 GCC PROJECT NUMBER
GCC-80677

rax. (201)	32 I-05UO	•	9	CC-00077	
29 BLACK	POINT R	ON ARCHITECTS COAD MAINE 04074	Attention: W	ily 13, 2009 ENDI .ORENCE HOUS	E PROJECT
WE ARE SE	ENDING '	YOU X ATTACHE	_	UNDER	SEPARATE COVER
COPIES 8	NO.	MASONRY REINVORCING CERT	DESCRIPT	a delication of the second	
		FOR APPROVAL			
		PLEASE RETURN 3 SETS TO GO	CC		
REMARKS		RECEIVED JUL	. 1 4 2009		
				-	
			.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		
Signed: J	Nichael S	l. Adams		Date:	13-Jul-09

Sr. Project Manager



Order 110000 Shipped From: Deerfield Shipped Date: 06/29/2009 Description CMU Shafts Ship Via: Barker Trucking BOL:176398-DEE Weight 22715 Lbs.

- Portland	sonry	Project: Florence House Masonry - Portland
	509	Barker Contract#: 10031509
COMPLIANCE	OH.	CERTIFICATE OF COMPLIANCE

		Sold To:
GRAY, ME 04039	216 LEWISTON RD	AUDETTE & SCE INC

Ship To:
190 Valley St Portland, ME 04102

E4183 67400	E4180 66800	E4179 63800	AU0910108501 68100	AU0910108401 69900	AU0910108301 66900	AU0910108001 66500	AU0910107901 73700	AU0910107801 68100
106800	104700	100900	107420	109600	104000	104700	114300	106400
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1.10	1.13	1.15	1.13	1.15	1.13	1.15	1.20	1.10
.030	.029	.026	.026	.022	.021	.031	.043	.018
.054	.054	.049	.055	.047	.049	.064	.064	.062
D9219	D9219	D9219	D9392	D9392	D9392	D9392	D9392	D9392

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Size

Weight

Grade

Supplier

Heat#

Yield

Tensile

Elng% Bend

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Coater

Batch

Cntrl#

GANNESTON CONSTRUCTION CORP. P.O. BOX 27 AUGUSTA, ME 04332

REVIEWED WITH NOTES NO RESUBMISSION REVIEWED REVIEWED WITH NOTES
RESUBMISSION REQUESTED NOT ACCEPTABLE

SHE CATUBLY

CHECK MARK IN

BARKER STEEL certifies that the above material conforms to all current job plans and specifications. We cortify that all manufacturing processes for this steel have occurred in the United to



Barker Contract#: 10031509

CERTIFICATE OF COMPLIANCE

Coater Batch	P S		Mn	C	Elng% Bend C	Elng%	Tensile	Yield	Heat#	Size Weight Grade Supplier	Grade	Weight	Size
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			T										
GRAY, ME 04039						bs.	22715 Lbs.	:		S	CMU Shafts		110000
216 LEWISTON RD		, ,				7	Weight			ر	Description	D	Order
ALIDETTE & SOE NO		Sold To:	'n					rucking	Ship Via: Barker Trucking	9009	06/29/2	Shipped Date: 06/29/2009	Ship
Project: Florence House Masonry - Portland	Flore	roject	т.					8-DEE	BOL:176398-DEE	ā	: Deerfie	Shipped From: Deerfield	Ship

NucorSteel	NucorSteel	NucorSteel	NucorSteel	NucorSteel	NucorSteel	NucorSteel	NucorSteel	NucorSteel	Supplier
E4183	E4180	E4179	AU0910108501	AU091010840	AU091010830	AU091010800	AU091010790	AU091010780	Heat#
67400	66800	63800	1 68100	1 69900	1 66900	1 66500	1 73700	1 68100	Yield
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13.2	13.7	16.2	15.0	14.3	16.6	13.2	14.5	13.7	Elng%
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.43	.43	.41	.44	.44	.41	.42	.41	.42	ဂ
1.10	1.13	1.15	1.13	1.15	1.13	1.15	1.20	1.10	Mn
.030	.029	.026	.026	.022	.021	.031	.043	.018	⊽
.054	.054	.049	.055	.047	.049	.064	.064	.062	ß
									Coater
									Batch
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1.10	1.13	1.15	1.13	1.15	1.13	1.15	1.20	1.10
.030	.029	.026	.026	.022	.021	.031	.043	.018
.054	.054	.049	.055	.047	.049	.064	.064	.062
D9	D9	D9	D9	D9	D9	D9	D9	D9
219	219	D9219	392	392	392	392	392	392

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GANNESTON CONSTRUCTION CORP. P.O. BOX 27 AUGUSTA, ME 04332 REVIEWED WITH NOTES NO RESUBMISSION REVIEWED TES ACTION TAKEN REVIEWED WITH NOTES
RESUBMISSION REQUESTED NOT ACCEPTABLE

CHECKED BY.

CHECK MARK

DATE 2

BARKER STEEL certifies that the above material conforms to all current job plans and specifications. We certify that all manufacturing processes for this steet have occurred in the United States

10/25/ 2 Ball



Order Shipped From: Deerfield Shipped Date: 06/29/2009 Size 110000 Weight Description CMU Shafts Grade **Supplier** Ship Via: Barker Trucking BOL: 176398-DEE Heat# Yield Tensile 22715 Lbs. Weight Elng%

	Ship To: 190 Valley St Portland, ME 04102	Sold To: AUDETTE & SCE INC 216 LEWISTON RD GRAY, ME 04039	CERTIFICATE OF COMPLIANCE Barker Contract#: 10031509 Project: Florence House Masonry - Portland
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E4183 674	E4180 66800	E4179 638	AU0910108501 681	AU0910108401 699	AU0910108301 669	AU0910108001 665	AU0910107901 737	AU0910107801 681
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1.10	1.13	1.15	1.13	1.15	1.13	1.15	1.20	1.10
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D9219	D9219	D9219	D9392	D9392	D9392	D9392	D9392	D9392

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CHECKED BY

HARKER STEEL certifies that the above material conforms to all current job plans and specifications. We certify that all manufacturing processes for this steet have occurred in the United States

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		Tanaila	Heat# Vield	Size Weight Grade Supplier	Grade	Weight	Size
Ship To: 190 V							
GKAY	Ubs.	22715 Lbs.		6	CMU Shafts		00001F
216 LI		Weight		,	Description		Order
Sold To: ALIDE:			Ship Via: Barker Trucking		06/29/2	Shipped Date: 06/29/2009	Shipp
Project: Florence Ho			BOL:176398-DEE	ā	: Deerfiel	Shipped From: Deerfield	Shipp

C			
Mn	Ship	Solo	Bar
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S Coater	190 Valley St Portland, ME 04102	AUDETTE & SCE INC 216 LEWISTON RD GRAY, ME 04039	CERTIFICATE OF COMPLIANCE Barker Contract#: 10031509 Project: Florence House Masonry - Portland
Batch	102	INC	Y COMPLIAN Orthory - Portland
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BARKER STEEL certifies that the above material conforms to all current job plans and specifications. We certify that all manufacturing processes for this steel have occurred in the United States.



S	P	Mn	C	Elng% Bend C Mn P S	Elng	Tensile	Yield	Heat#	Size Weight Grade Supplier	Grade	Weight	Size
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, <u>, ,</u>	Ship To:	<u>S</u>										
<u>و ک</u>		(ıt ∪bs.	Weight 22715 Lbs.			ន៍ក	Description CMU Shafts	7	Order 0000117
ΔΙ	Sold To:	ည					Trucking	Ship Via: Barker Trucking	9009	: 06/29/2	Shipped Date: 06/29/2009	Ship
-lorenc	Project: Florence	<u>"</u>					38-DEE	BOL:176398-DEE	eld.	ı: Deerfie	Shipped From: Deerfield	Ship
	4											

С			
Mn	Ship To:	Sold To:	Bark Proj
þ	То:	To:	CEI (er Cor ect: Flo
S Coater	190 Valley St Portland, ME 04102	AUDETTE & SCE INC 216 LEWISTON RD GRAY, ME 04039	CERTIFICATE OF COMPLIANCE Barker Contract#: 10031509 Project: Florence House Masonry - Portland
Batch)2 2	O NC	COMPLIAN y - Portland
Cntrl#			CE

				.43	.43	.41	.44	.44	.41	.42	.4	.42
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Batch	102	INC RD	COMPLIAN 9 Portland
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NucorSteel NucorSteel

NucorSteel NucorSteel

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NucorSteel

NucorSteel NucorSteel

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BARKER STEEL certifies that the above material conforms to all current job plans and specifications. We certify that all manufacturing processes for this steet have occurred in the United States.

Mit & Call



CERTIFICATE OF COMPLIANCE

Size Weight Grade Supplier			0000IF CMU Shafts	Order Description	Shipped Date: 06/29/2009	Shipped From: Deerfield	
lier Heat#					Ship Via: Barker Trucking	BOL:176398-DEE	
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CHECKED BY

Audette &



S.C.E., INC. Since 1973 216 Lewiston Road Gray, Maine 04039

LETTER OF TRANSMITTAL

Gray, Wallic 0-1033	DATE TULL SE 2000 JOB NO. SAL DO
(207) 657-3313 Fax (207) 657-4113	July 8-2009 JOB NO. 80627 ATTENTION MT Ke Adams
TO Ganneston Construction Corp.	Plorence House
POBOX 27 - North Belfast Avenue	
poles.	Masonny Submitted Spec
Augusta, ME 04332-0027 D) E G E I	ME Masonry Submitted Spec Section 04810
JUL 92	009
GANNESTO WE ARE SENDING YOU Attached ☐ Under Separate come	
	Plans □ Samples □ Specifications
	certificate of Compliance
COPIES DATE NO.	DESCRIPTION
8 7-8-09 1 Reinforcing Bars- 1	Certificate of Compliance Spec Section
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THESE ARE TRANSMITTED as checked below:	
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434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: 626-9094



DAILY FIELD REPORT

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7/2/09

Project:

Avesta Florence House - Portland, Maine

Project #:

14194

Site Contacts:

Mike Adams, Ganneston Construction

Purpose of Visit:

Grout Testing

Work Activities:

A crew from Audette & S C E Inc. placed grout for the first five feet of stairwell one and

two. One set (#1) of four grout cubes were cast and will be retrieved at a later date for

controlled storage and compressive strength testing.

Test Results:

Grout Test Results:

Grout Temperature: 65 deg. F Air Temperature: 60 deg F

Slump: 10"

Remarks:

Portal to Portal		Expenses		Signed:	Kris Bennett
Leave:	9:45 AM	Mileage:	15	_cc:	<i>i</i>
Return:	10:45 AM	Density Gauge:			
TOTAL:	1	Other:			
		_		Reviewed: Sent:	Darrell A. Gilman, CMT Manager 7/9/09

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: 626-9094



DAILY FIELD REPORT

Date:	7/20/09					
Project:	Avesta Florence	e House - Portland,	Maine			
Project #:	14194					
Site Contacts:	Mike Adams, G	anneston Construct	ion			
Purpose of Visit:	Mortar & Grou	it testing				
Work Activities:	with mortar and like me to do an	I asked the mason if were now mixing g ad he asked if I would set of 4 grout cube	rout. I talk ld put toget	ed with Mik her a grout l	te Adams about wl	nat he would uple of the
Test Results:						•
Remarks:						
	÷)
Portal to Portal	10.00 434	Expenses Miles as	11	Signed:	Justin Rouill	ard
Leave: Return:	10:00 AM 11:00 AM	Mileage: Density Gauge:	11	_ cc:		
TOTAL:	11:00 Alvi	Other:				
101111.				Reviewed: Sent:	Darrell A. Gilman, CM 7/21/09	IT Manager

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: 626-9094



DAILY FIELD REPORT

Project: Avesta Florence House - Portland, Maine Project #: 14194 Site Contacts: Mike Adams, Ganneston Construction Purpose of Visit: Mortar Testing Work Activities: Retrieved one grout box and brought it back for laboratory testing. Also cast one set of 3 mortar cubes for compressive strength testing. Sample was taken directly out of the mixe and at the time the sample was taken the mason's were working on the interior stairwell. Test Results: Remarks: Portal to Portal Leave: 10:00 AM Mileage: 11 cc: Return: 11:30 AM Density Gauge: TOTAL: 0 Darrell A. Gilmar, CMT Manager								
Project #: 14194 Site Contacts: Mike Adams, Ganneston Construction Purpose of Visit: Mortar Testing Work Activities: Retrieved one grout box and brought it back for laboratory testing. Also cast one set of 3 mortar cubes for compressive strength testing. Sample was taken directly out of the mixer and at the time the sample was taken the mason's were working on the interior stairwell. Test Results: Portal to Portal Leave: 10:00 AM Mileage: 11 ce: Return: 11:30 AM Density Gauge: TOTAL: 1.5 Other:	Date:	7/21/09						
Site Contacts: Mike Adams, Ganneston Construction Purpose of Visit: Mortar Testing Work Activities: Retrieved one grout box and brought it back for laboratory testing. Also cast one set of 3 mortar cubes for compressive strength testing. Sample was taken directly out of the mixer and at the time the sample was taken the mason's were working on the interior stairwell. Test Results: Remarks: Portal to Portal Leave: 10:00 AM Mileage: 11 ce: Return: 11:30 AM Density Gauge: TOTAL: 1.5 Other:	Project:	Avesta Florence E	Iouse - Portland, I	Maine				
Purpose of Visit: Mortar Testing Work Activities: Retrieved one grout box and brought it back for laboratory testing. Also cast one set of 3 mortar cubes for compressive strength testing. Sample was taken directly out of the mixer and at the time the sample was taken the mason's were working on the interior stairwell. Test Results: Remarks: Portal to Portal Leave: 10:00 AM Mileage: 11 ce: Return: 11:30 AM Density Gauge: TOTAL: 1.5 Other:	Project #:	14194					:	
Work Activities: Retrieved one grout box and brought it back for laboratory testing. Also cast one set of 3 mortar cubes for compressive strength testing. Sample was taken directly out of the mixer and at the time the sample was taken the mason's were working on the interior stairwell. Test Results: Remarks: Portal to Portal Leave: 10:00 AM Mileage: 11 cc: Return: 11:30 AM Density Gauge: TOTAL: 1.5 Other:	Site Contacts:	Mike Adams, Gan	neston Constructi	on				
mortar cubes for compressive strength testing. Sample was taken directly out of the mixed and at the time the sample was taken the mason's were working on the interior stairwell. Test Results: Remarks: Portal to Portal Leave: 10:00 AM Mileage: 11 cc: Return: 11:30 AM Density Gauge: TOTAL: 0.5 Other:	Purpose of Visit:	Mortar Testing						
Portal to Portal Expenses Signed: Justin Rouillard Leave: 10:00 AM Mileage: 11 cc: Return: 11:30 AM Density Gauge: TOTAL: 1.5 Other:	Work Activities:	mortar cubes for c	ompressive streng	gth testing.	Sample was	taken dire	ectly out of the	e mixer
Portal to Portal Expenses Signed: Justin Rouillard Leave: 10:00 AM Mileage: 11 cc: Return: 11:30 AM Density Gauge: TOTAL: 1.5 Other:								
Portal to Portal Expenses Signed: Justin Rouillard Leave: 10:00 AM Mileage: 11 cc: Return: 11:30 AM Density Gauge: TOTAL: 1.5 Other:								٠
Portal to Portal Expenses Signed: Justin Rouillard Leave: 10:00 AM Mileage: 11 cc: Return: 11:30 AM Density Gauge: TOTAL: 1.5 Other:								
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Leave: 10:00 AM Mileage: 11 cc: Return: 11:30 AM Density Gauge:	Portal to Portal		Expenses		Signed:	Just	in Rouillard	
TOTAL: 1.5 Other:				11	_			
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REVIEWEG: DAITER A. CHIMAII, CAVI MANAGER	TOTAL:	1.5	Other:		Reviewed:	Darrell A. G	ilman, CMT Mana	iger

7/24/09

Sent:

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621-8334 Fax: 626-9094



DAILY FIELD REPORT

Date:

8/20/09

Project:

Avesta Florence House - Portland, Maine

Project #:

14194

Site Contacts:

Mike Adams, Ganneston Construction

Purpose of Visit:

Grout testing

Work Activities:

When I arrived the contractor was mixing up a batch of grout, when the batch was

thoroughly mixed I cast one set of 4 grout cubes to be picked up later for compressive

strength testing.

Test Results:

Remarks:

Grout was being placed at the 3rd level of the stairwell.

Portal to Portal		Expenses		Signed:	Frank Clark
Leave:	9:00am	Mileage:	11	cc:	
Return:	10:15am	Density Gauge:		_	
TOTAL:	1.25	Other:	2.00 tolls	_	
		_		Reviewed:	Darrell A. Gilman, CMT Manager

Sent: 8/21/09

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GROUT COMPRESSIVE STRENGTH TEST RESULTS - ASTM C1019

Project No:

14194

Project:

Avesta Florence House

Client:

Avesta Florence House, LP and Florence House

307 Cumberland Avenue Portland, ME 04101

Field Test Data

Set No.:

G1

Placement Date:

2-Jul-09

Lab Rec'd Date:

3-Jul-09

Location:

Stair Well 1 & 2, 5'

Technician:

K. Bennett

Supplier:

Audette & SCE

Mix Designation: CMU Block Grout

Design Strength:

3000psi

Slump (inches)

Grout Temp. (°F) 65.0

Air Temp. (°F)

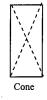
62.0

10

Laboratory Test Data

Sample No.	Test Date	Age	Type	Wt. (lb.)	Unit Wt.	Area (in ²)	Load (K)	Strength (psi)
Cla	9-Jul-09	7	3	4.78	120.6	11.17	31.3	2800
C1b	30-Jul-09	28	3	4.88	119.1	11.29	44.1	3910
Clc	30-Jul-09	28	3	4.82	125.4	11.22	46.5	4150
C1d								

Average 28 Day (psi):



Cone and Split

2

Cone and Shear

3

Shear

Columnar

1

4

5

6

Other

Remarks:

Reviewed: Darrell Gilman, CMT Manager

Date: 7-31-09

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621.8334 Fax: (207) 626.9094



GROUT COMPRESSIVE STRENGTH TEST RESULTS - ASTM C1019

Project No:

14194

Project:

Avesta Florence House

Client:

Avesta Florence House, LP and Florence House

307 Cumberland Avenue Portland, ME 04101

Field Test Data

Set No.:

G2

Placement Date:

20-Jul-09

Lab Rec'd Date:

22-Jul-09

Location:

Block Wall at the Intersection of Line M and Line 16

Technician:

J. Rouillard

Supplier:

Audette & SCE

Mix Designation: CMU Grout

Design Strength:

3000psi

Slump (inches)

11

Grout Temp. (°F)

62.0

Air Temp. (⁰F)

75.0

Laboratory Test Data

Sample No.	Test Date	Age	Type	Wt. (lb.)	Unit Wt.	Area (in²)	Load (K)	Strength (psi)
G2a	27-Jul-09	7	3	4.86	123.0	11.27	19.7	1740
G2b	17-Aug-09	28	6	4.62	124.6	10.83	25.5	2350
G2c	17-Aug-09	28	3	4.80	123.6	10.96	27.6	2510
G2d	14-Sep-09	56	6	4.70	124.5	11.01	28.2	2560

Average 28 Day (psi): 2430 Cone Cone and Cone and Columnar Other Split Shear 1 2 3 5 4 6

Remarks:

Reviewed: Darrell Gilman, CMT Manager

Date: 9-14-09

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621.8334 Fax: (207) 626.9094



GROUT COMPRESSIVE STRENGTH TEST RESULTS - ASTM C1019

Project No:

14194

Project:

Avesta Florence House

Client:

Avesta Florence House, LP and Florence House

307 Cumberland Avenue Portland, ME 04101

Field Test Data

Set No.:

G3

Placement Date:

20-Aug-09

Lab Rec'd Date:

21-Aug-09

Location:

Third Floor Level Stairwell

Technician:

F. Clark

Supplier:

Audette & SCE

Mix Designation: CMU Block Grout

Design Strength:

3000psi

Slump (inches)

Grout Temp. (°F) 64.0

Air Temp. (⁰F)

87.0

11

Laboratory Test Data

Sample No.	Test Date	Age	Type	Wt. (lb.)	Unit Wt.	Area (in²)	Load (K)	Strength (psi)
G3a	27-Aug-09	7	6	4.84	116.2	12.07	25.0	2070
G3b	17-Sep-09	28	6	4.58	115.7	11.59	24.5	2110
G3c	17-Sep-09	28	6	4.62	114.6	11.73	29.5	2510
G3d	15-Oct-09	56	3	4.73	113.9	12.04	35.5	2950

Average 28 Day (psi): 2310 Cone Cone and Cone and Shear Other Columnar Split Shear 1 2 4 5 6 3

Remarks:

Reviewed: Darrell Gilman, CMT Manager

Date: 10-15-09

S U M M I T ENVIRONMENTAL CONSULTANTS INC.

434 Cony Road, Augusta, Maine 04330 Phone: (207) 621.8334 Fax: (207) 626.9094

MORTAR COMPRESSIVE STRENGTH TEST RESULTS

Project No:

14194

Project:

Avesta Florence House

Client:

Avesta Florence House, LP 307 Cumberland Avenue

Portland, ME 04101

Field Test Data

Set No.:

M1

Placement Date:

21-Jul-09

Lab Rec'd Date:

23-Jul-09

Location:

Stairwell Wall

Techician:

T TO 111 1

Supplier:

J. Rouillard

Mix Designation: Type "S"

Audette SCE

Design Strength:

1800psi

Laboratory Test Data

Sample No.	Test Date	Age	Туре	Wt. (lb.)	Unit Wt.	Area (in²)	Load (K)	Strength (psi)
Mla	28-Jul-09	7	1	0.60	127.8	4.03	12.1	3000
M1b	18-Aug-09	28	1	0.60	125.4	4.03	14.4	3560
M1c	18-Aug-09	28	6	0.62	132.1	4.04	10.6	2620
					,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	-		

Average 28 Day (psi):

Cone Cone and Cone and Shear Columnar Other

Split Shear

1 2 3 4 5 6

Remarks:

Reviewed: Darrell Gilman, CMT Manager
Date:8-18-09

EXHIBIT B

05120 Steel Construction

Project: Florence House
Date Prepared: September 2, 2008
Structural Schedule of Special Inspections - STEEL CONSTRUCTION

3. Material verification of structural steel (IBC Sect 1708.4): a. Identification markings to conform to ASTM standards specified in the approved construction b. Manufacturers' certified mill test reports. 7. S. ASTM A 6 or ASTM A 5 or ASTM A 5 or ASTM A 6 or ASTM A 5 or ASTM A	VERIFICATION AND INSPECTION IBC Section 1704.3	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS		AGENT QUALIFICATION	TASK COMPLETED
specified in the approved construction documents. Y S SITM material specifications; AISC LRPD, Section A3.45,							
2. Inspection of high-strength bolting a. Bearing-type connections. b. Slip-critical connections. y P AISC LRPD Section M.2.5 b. Slip-critical connections. y C or P (method dependent) 3. Material verification markings to conform to ASTM standards specified in the approved construction documents. b. Manufacturer's certified mill test reports. y S ASTM A 6 or ASTM A 6 or ASTM AS 68 SII PE/SE or EIT Sulfurnous Manufacturer's certified mill test reports. y S ASTM A 6 or ASTM A 6 or ASTM AS 68 SII PE/SE or EIT Sulfurnous Manufacturer's certified mill test reports. y S ASTM A 6 or ASTM A 6 or ASTM AS 68 SII PE/SE or EIT Sulfurnous Manufacturer's certified mill test reports. 4. Material verification markings to conform to AWS specification in the approved construction documents. a. Identification markings to conform to AWS specification in the approved construction documents. b. Manufacturer's certificate of compliance required. y S AWS DI.1 PE/SE or EIT Sulfurnous Manufacturer's certificate of compliance required. y S AWS DI.1 SII PE/SE or EIT Sulfurnous Manufacturer's certificate of compliance required. y S AWS DI.1 SII PE/SE or EIT Sulfurnous Manufacturer's certificate of compliance required. y S AWS DI.1 SII PE/SE or EIT Sulfurnous Manufacturer's certificate for all field welders who will be welding (BC 1704.3.1): a. Structural steel: 1) Complete and partial penetration groove welds. y C AWS DI.1 SII PE/SE or EIT Sulfurnous Manufacturer's certificate for all field welders who will be welding (BC 1704.3.1): a. Structural steel: 1) Complete and partial penetration groove welds. y C AWS DI.1 TA2 AWS-CWI JA 4) Single-pass fillet welds-5716" y P AWS DI.3 TA2 AWS-CWI JA TA2 AWS-CWI JA TA2 AWS-CWI JA	specified in the approved construction documents.	Y	S	ASTM material specifications; AISC 335, Section A3.4; AISC LRFD,	SII	PE/SE or EIT	IV
a. Bearing-type connections. Y P AISC LRFD Section M.5. b. Slip-critical connections. Y C or P (method dependent) 174.2 AWS/AISC-SSI APPLIED 3. Material verification of structural steel (IBC Sect 1708.4): a. Identification markings to conform to ASTM standards specified in the approved construction documents. b. Manufacturers' certified mill test reports. 4. Material verification of weld filler materials: a. Identification markings to conform to ASTM Specification in the approved construction of weld filler materials: a. Identification markings to conform to ASTM A 568 BIG Sect 1708.4 5. Material verification of weld filler materials: a. Identification markings to conform to ASTM A 568 BIG Sect 1708.4 5. Material verification of weld filler materials: a. Identification markings to conform to AWS specification in the approved construction documents. b. Manufacturer's certificate of compliance required. c. Inspection of welding (IBC 1704.3.1): a. Structural steel: 1) Complete and partial penetration groove welds. Y C AWS D1.1 SII PE/SE or EIT TA2 AWS-CWI JAA TA2 AWS-CWI JAA TA3 AWS-CWI JAA TA4 AWS-CWI JAA TA5 AWS-CWI JAA TA6 AWS-CWI JAA TA7 AWS-CWI JAA TA7 AWS-CWI JAA AWS-CWI JAA AWS-CWI JAA TA7 AWS-CWI JAA TA7 AWS-CWI JAA TA7 AWS-CWI JAA TA8 AWS-CWI JAA AWS-CWI JAA TA8 AWS-CWI JAA TA9 A		Y	S		SI1	PE/SE or EIT	JY
b. Slip-critical connections. J. C. or P. (method dependent) J. Material verification of structural steel (IBC Sect 1708.4): a. Identification markings to conform to ASTM standards specified in the approved construction documents. b. Manufacturers' certified mill test reports. J. S. Saltim A 568 IBC Sect 1708.4 SII PE/SE or EIT Submitted of ASTM A 568 IBC Sect 1708.4 SII PE/SE or EIT Submitted of ASTM A 568 IBC Sect 1708.4 SII PE/SE or EIT Submitted of ASTM A 568 IBC Sect 1708.4 SII PE/SE or EIT Submitted of ASTM A 568 IBC Sect 1708.4 SII PE/SE or EIT Submitted of ASTM A 568 IBC Sect 1708.4 SII PE/SE or EIT Submitted of ASTM A 568 IBC Section A3.5 SII PE/SE or EIT Submitted of ASTM A 568 IBC Sectio							
3. Material verification of structural steel (IBC Sect 1708.4): a. Identification markings to conform to ASTM standards specified in the approved construction documents. b. Manufacturers' certified mill test reports. 4. Material verification of weld filler materials: a. Identification markings to conform to AWS specification in the approved construction documents. a. Identification markings to conform to AWS specification in the approved construction documents. b. Manufacturer's certificate of compliance required. 7. Submit current AWS D1.1 welder certificate for all field welders who will be welding on this project. 6. Inspection of welding (IBC 1704.3.1): a. Structural steel: 1) Complete and partial penetration groove welds. 4) Single-pass fillet welds> 5/16" 7 Y P 7 AWS D1.1 7 AWS-CWI		Y		Section M2.5	TA2	AWS/AISC-SSI	ATTACUTED
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a. Details such as bracing and stiffening. Y P SII PE/SE or EIT JU							25
		Y	P		SI1	PE/SE or EIT	-TUF
	b. Member locations.						
c. Application of joint details at each connection. Y P SII PE/SE or EIT	c. Application of joint details at each connection.						JK

Project: Florence House Date Prepared: September 2, 2008

Structural Schedule of Special Inspection Services FABRICATION AND IMPLEMENTATION PROCEDURES – STRUCTURAL STEEL

VERIFICATION AND INSPECTION IBC Section 1704.2	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS	AGENT	AGENT QUALIFICATION	TASK COMPLETED
Fabrications Procedures: Review of fabricator's written procedural and quality control manuals and periodic auditing of fabrication practices by an approved special inspection agency. At the completion of fabrication, the approved fabricator shall submit a certificate of compliance to the building code official stating that the work was performed in accordance with the approved construction documents. -OR- 2. AISC Certification	Y	S	Fabricator shall submit one of the two qualifications	SII	PE/SE or EIT	MSC CERTIFICATION
3. At completion of fabrication, the approved fabricator shall submit a certificate of compliance to the building code official stating that the work was performed in accordance with the approved construction documents.	Y	S	IBC 1704.2.2	SII	PE/SE or EIT	on

GANNESTON CONSTRUCTION CORE P.O. BOX 27 AUGUSTA, ME 04332 4 merican Institute of Iteel Constri

is proud to recognize

BERNEMIZZION BEGNERLED
BERNEMED MITH NOTES

Z.ML1-Precision Welding & Fabrication

Westbrook, ME

for successfully meeting the quality certification requirements for

CHECK WARK NO BERNEWIRSTON REVIEWED WITH NOTES

BEVIEWED

Standard for Steel Building Structures

DEC IV THE P ڪ டப Reviewed Furnish as Corrected Rejected Revise and Resubmit Submit Specific Item:

This review is only for general conformance with the design concept of the project and , general compliance with the information given in the Contract Documents. Corrections or comments made on the shop drawings during this review do not relieve contractor from compliance with the requirements of the plans and specifications. Approval of a specific item shall not include approval of an assembly of which the item is a component. Contractor is responsible for: dimensions to be confirmed and correlated at the jobsite: information that pertains solely to the fabrication processes or to the means, methods, techniques, sequences and procedures of construction: coordination of his or her Work with that of all other trades: and for performing all work in a safe and satisfactory manner.

Becker Structural Engineers, Inc.

Certification valid through (October 200

Roger E. Ferch

VULCRAFT SALEM NH

SECTION 05200 - PART 1, 1.04, H, 2.3.

Vulcraft Sales Corporation

PAGE 01/01 P.O.#16166 # 80677-5100

FLORENCE HOUSE

March 3, 2009

Larry Cunha Larene Steel **5 Mills Point** Middleton, MA 01949-2506 Fax: 978-646-0090

Vulcraft File #: 032-08-6527

Vulcraft of New York, Inc inspects joists according to the SJI specifications 5.12, 104.13 and 1004.10 as stated in the Code of Standard Practice, section 4. In order to maintain SJI plant certification, inspection must be performed at the plant. The SJI compliance letter should cover any inspection requirements , if there is something more specific they need addressed please contact me.

Sincerely,

Jason M. Thornton District Sales Manager **Vulcraft Sales Corporation** Salem NH Office

loo on Souto

GANNESTON CONSTRUCTION CORP.

P.O. BOX 27 AUGUSTA, ME 04332 REVIEWED WITH NOTES REVIEWED RESUBMISSION REQUESTED

REVIEWED WITH NOTES NOT ACCEPTABLE NO RESUBMISSION

CHECK MARK,

1

SECTION 05200 - PART 1 - 1.04, H, 3

FLORENCE HOUSE \$ 80677-5100

Vulcraft Sales Corporation

March 3, 2009

Larry Cunha Larene Steel 5 Mills Point Middleton, MA 01949-2506 Fax: 978-646-0090

Vulcraft File #: 032-08-6527

2	3		
			STRUCTION CORP. SUSTA, ME 04332
1	- REVIEWED		REVIEWED WITH NOTES RESUBMISSION REQUESTY:
	REVIEWED WITH NOTES NO PESUBWISSION		NUT ACCEPTABLE
	1279)	3/6/09

Vulcraft performs visual inspection of all joist and joist girder products during their production. We do not generate inspection reports for individual projects. All products are deemed acceptable for shipment unless they are set off by our inspectors. Any corrective action is also inspected prior to the product being released for shipment.

In lieu of inspection reports, you may choose either in-house inspection performed by an inspection agency of your choosing, or we can perform a full-scale load test. For in-house inspection by an outside agency, we will provide the day and time of production to you. The chosen agency should be at our facility at that time in order to witness the fabrication process.

For a full scale load test, the member to be tested can be chosen in-house by our QA Supervisor, or by the Engineer-of-Record. Full scale load test includes verifying the member load capacity and deflection using a series of concentrated loads that simulate the loading from the SII Load Table. The tested member must safely support 1.65 x theoretical design capacity of critical components. The member tested will not be utilized within the project. Cost for a full-scale test is \$750 per joist. If the member to be tested is chosen by the Engineer-of-Record, please indicate on your approved bill of materials the joist to be tested.

Sincerely,

Jason M. Thornton District Sales Manager Vulcraft Sales Corporation

Salem NH Office

Reviewed Furnish as Corrected Rejected Revise and Resubmit

This review is only for general conformance with the design concept of the project and general compliance with the information given in the Contract Documents. Corrections or comments made on the shop drawings during this review do not relieve contractor from compliance with the requirements of the plans and specifications. Approval of a specific item shall not include approval of an assembly of which the item is a component. Contractor is responsible for: dimensions to be confirmed and correlated at the jobsite: information that pertains solely to the fabrication processes or to the means, methods, techniques, sequences and procedures of construction: coordination of his or her Work with that of all other trades: and for performing all work in a safe and satisfactory manner.

Becker Structural Engineers, Inc.

Vulcraft Sales Corporation | 5 Manor Parkway, Suite B Salem NH 03079 P: 603 894 11.46 F: 603 894 1149

3-16-05

WTM

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SECTION 05200 - PART 1, 1.04, G



P.O.# 16166 株 8**06**77-51的

VULCRAFT SALES CORPORATION

February 26, 2009

Contact LARRY CUNHA

Company Name: LARENE STEEL PRODUCTS

Address 5 MILLS POINT

City: MIDDLETOWN

ST: MA

Zip Code 01949

Reference: FLORENCE HOUSE / PORTLAND, ME

Purchase Order Number: 810-25 Vulcraft Number: 032-08-6527

Reviewed Rejected	Furnish as Corrected Revise and Resubmit
Submit Sne	cific Item

This review is only for general conformance with the design concept of the project and general compliance with the information given in the Contract Documents. Corrections or comments made on the shop drawings during this review do not relieve contractor from compliance with the requirements of the plans and specifications. Approval of a specific item shall not include approval of an assembly of which the item is a component. Contractor is responsible for: dimensions to be confirmed and correlated at the jobsite: information that pertains solely to the fabrication processes or to the means, methods, techniques, sequences and procedures of construction: coordination of his or her Work with that of all other trades: and for performing all work in a safe and satisfactory manner.

Becker Structural Engineers, Inc.

Date 3-16-09 By MJM

Gentlemen:

Vulcraft Division, Nucor Corporation, hereby certifies that we are a member of the Steel Joist Institute. Vulcraft open web steel joists are designed and manufactured in accordance with the standard joist specifications of the Steel Joist Institute.

These joists will safely support a uniformly distributed load as designated in the applicable Steel Joist Institute load table, for the particular type and span, when field applications are in accordance with these specifications.

Cordially,

Sinda R Chappell

Linda R Chappell

LINDA R CHAPPEII
DISTRICT SALES CLERK

GANNESTON CONSTRUCTION CORP.
P.O. BOX 27 AUGUSTA, ME 04332

REVIEWED RESUBMISSION REQUESTED

REVIEWED WITH NOTES
NO RESUBMISSION

CHECK MARK INDICATES ACTION TAKEN

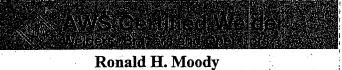
CHECKED BY

Solomon R. Gay
Cert # 0702070W SSN # XXX-XX-9877



1-800-443-9353	AMERICAN WELDING SOCIET
Information relating to Identification and Egytfication of the transfer of this Card may be wellfuct by Calling of writing a Gentification Department of the American Wellong Society 550 N.W. IL eleumolistical Milatin FF 3312p	VALID ONLY IF ACCOMPANIED BY PHOTO ID This Card is the property of AWS and shall be returned on demand.
Reviewed Revise and Resubmit Submit Specific Item: This review is only for general conformance with the design concept of the project and general compliance with the information given in the Contract Documents. Corrections or comments made on the shop drawings during this review do not relieve contractor from compliance with the requirements of the plans and specifications. Approval of an assembly of which the item is a component. Contractor is responsible for: dimensions to be confirmed and correlated at the jobsite: information that pertains solely to the fabrication processes or to the means, methods, techniques, sequences and procedures of construction: coordination of his or her Work with that of all other trades: and for performing all work in a safe and satisfactory manner. Becker Structural Engineers, Inc. Date By	GANNESTON CONSTRUCTION CORP. P.O. BOX 27 AUGUSTA, ME 04332 REVIEWED RESUBMISSION REQUESTED REVIEWED RESUBMISSION REQUESTED REVIEWER MITH NOTES NO RESURMISSION CHECK MARKING CATES ASTION TAKEN CHECKED BY DATE 2 23/05
omon R Gay	

Solomon R. Gay						
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Ronald H. Moody
Test Date Sup Code Process Gas Filler Metal Base Metal Position Thickness Expires
1 06/10/96 G D1.1 SMAW N/A F4 P A L 06/25/09

Edward P Nadeau Cert # 0009030W SSN #

SSN # XXX-XX-2162

AMERICAN WELDING SOCIETY

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-VERIFY GERTIFICATION
HIMS BEEN
LIDDAYED.

Scott J Flaherty
Cert # 0510038W SSN # XXX-XX-5584



VALID ONLY IF ACCOMPANIED BY PHOTO ID

This Card is the property of AWS and shall be returned on demand.

	ott J Flaherty		D======	Gas	Filler	Metal	Base Metal	Position	Thickness	Expires
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PERFORMANCE QUALIFICATION TEST RECORD

Pipe 16 個 26 個 56 個 66 図 6GR [(T)Min 1/8" Max 3/4" Dlarneter 24" Range 24" end greater Plate 16 図 26 図 36 図 46 図 (T)Min 1/8" Max 3/4" Consumable Insert	Eye correction required Yes 🗆 No 🗆		Type of Eye	Correc	1	Eye glasses Contect len: Magniflers		
Welder 第 Operator □ Outsified with AWS WPS No. 1.QC-W2 Supplement No. G Text No. 01-SM-F4-P-A-L Outsified with AWS WPS No. 1.QC-W2 Supplement No. G Automatic □ Automatic □ Machina □ Processies	MARY BONALO N.		Social Sac	urity #		7265		
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Backing Yes 體 No Consumble flash Yes No May Double Weldon or Single Weldon 器 Short Circuiting are (GMAW) Yes No 獨 No 獨 Yes No 孤 No 獨 Yes No 獨 Yes No 孤 No 獨 Yes No 孤 No 獨 Yes No 孤 No 孤 Yes No 孤 No 孤 Yes No 函 Yes No 孤 No 孤 No 孤 Yes No 孤 No	3/116/11/19		٠ -					
Pass 题 Fail C Radiographic test results Bend test results NA Pass 题 Fail C Pass 图 C PAss Pass Pass 图 C PAss Pass 图 C PAss Pass Pass Pass Pass Pass Pass Pass	Double Welder Cor Single Welded #		Short Circuit	uð stc	(GMAW)	Yas C	No 📆	
Brooks: Pips 16 個 26 圏 56 脚 66 圏 6G用口 (T)Min 1/8* Max 3/4* Dlameter 24* Range 24* end greater Plate 16 圏 26 圏 36 側 46 圏 (T)Min 1/8* Max 3/4* Consumable Insert 口 Backing type 圏 Fillet: Pipe 1F 題 2F 脚 3F 圏 4F 圏 5F 圏 (T)Min 1/8* Max Unlimited Plate 1F 圏 2F 脚 3F 圏 4F 圏 (T)Min 1/8* Max Unlimited	Test results Visual test results Bend test results NA Pass 图		Radiographic	大麻将【 / 8	sults 	NA WE F	ess 🗋 Feil [⊋
Pipe 1F 2F 3F 3F 4F 1F 1F 1F 1F 1F 1F 1F 1F 1	Pips 1G 图 2G 图 5G 閣 6G 图 6G Plate 1G 图 2G 图 3G 图 4G 图 Consumable Insert C Backing type 簡	(T)Min 1	/8" Max 3/	4.	arneter <u>· 2</u>	nangu	Se and Steam	
Single Side ® Double Side The above named person qualified for the welding process(es) used in this tast within the limits of essential variables shown above, including materials and filter metal variables of the AWS Standard for welder certification and ANSI/AWS D1.1 Code or Standard. I nereby certify that I was not involved in the training of the above name notificial as a welder: Date Tested 6/10/95 Signed by Thomas E. Gilles Fest Supervisor AWS CWI No. 88070281 Signed by Thomas E. Gilles Formand E. Gilles Formand E. Gilles Formand E. Gilles Title	P404 17 88 25 88 df 88				· ·			
Signed by Thomas E. Giles Signed by Thomas E. Giles ANSI/AWS D1:1 Code or Standard. I nereby cartify that I was not involved in the training of the above name ansividual as a welder: Date Tested Signed by Thomas E. Giles AWS CWI No. 88070281 Signed by Thomas E. Giles This	Single Side Oouble Side							
Date Tested 6/10/96 Signed by Thomas E. Gilles Fest Supervisor Aws CWI No. 88070281 Signed by Thomas E. Gilles Trice	shown above, including materials and little of ANSVAWS D1.1 Code of Standard.	ding processies eral veriables of I nareby certify) used in this the AWS Star that I was no	tast w ndard f at invoi	ithin the li for welder vad in the	mits of easi certification training of	ential variables i and the above has	mes
Signed by Thomas E. Gillus Condinator-WELDING CENTER, Eastern Maine Technical College Title	7 (N O (O C	Signi	ed by Thom	85 E. G		Lit	25/	4
	- He	// ```			B1 ER, Easter		chnical Colleg	6

Form QC-WF1-- Performance Qualification Test Record 5

AWS OC-3

PERFORMANCE QUALIFICATION TEST RECORD

Eye correction required Yes () No ()	Type of Eye Correction: Eye glasses Contact lenses Flagnifiers
Name GAY, SOLOMON R.	Social Sacurity # 6000-9877
Welder 題 Operator □	Test No. D1-SM-F4-P-A-L
Qualified with AWS WPS No. 1-QC-W2 Suppleme	SIR PUL
Processies) SMAW Manual B	Si Sewi-Watowstor CT Caretiment
Test base metal specification SA 35	To SA 38
Material number (M or P Number) P1	To P1
Shielding Gas NA Flow Rate	, NA
AWS Filler metal classification E7018	F no 4 Size 2/32° & 1/8°
Backing Yes 間 No □ Double Welded □ or Single Welded 圖 Current AC □ DC 第	Consumable insert Yes □ No 題 Short Circuiting ard (GMAW) Yes □ No 題 Back Purging Yes □ No 題
Tout results	: Since we carried process of prints
Visual test results Pass 職 Fail □ Bend test results NA □ Pass 融 Fail □ PAOCESS(es) QUALIFIED FOR SMAW	Radiographic test results NA 類 Pass 〇 Fail 〇
POSITION(s) QUALIFIED FOR: Groove: Fipe 1G 题 2G 题 5G 题 6G 题 6GR □ (T)M Make 1G 题 2G 题 3G 题 4G 题 (T)M Consumable insert □ Becking type 題	Ain 1/8" Max 3/4" Diameter 24" Range 24" and greeter Ain 1/8" Max 3/4"
F 牟 I 泰 C :	•
	1/8" Max Unlimited 1/8" Max Unlimited
Vertical Up 職 Down 日 Weld Deposit Min Single Side 鵬 Double Side 己	 :
thown above, including materials and ther metal variable ANSI/AWS D1.1 Code or Standard. I haveby o	ess(es) used in this test within the limits of essential-variables les of the AWS Standard for walder certification and certify that I was not involved in the training of the adoxe named
ndividual as a welder:	
Date Tested 8/2/96	Signed by Thomas E. Gfrest Test Supervisor
A. I	AWS CYN No. 88070281
signed by Thomas E. Giles 1 2 Co.	ordinator-WELDING CENTER, Eastern Mains Technical College
igreed by Thomas E. Giles	Title
Composate Herresentative	

Form GC WF1 -- Performance Qualification Test Record

۶A

WELDING TEST CENTER / Eastern Maine Community College 354 Hogan Road, Bangor, ME 04401 (207) 974-4662, FAX (207) 974-4608 tgiles@emcc.edu

FAX TRANSMITTAL

FOL III	imediai	ė mem s	ery ro.					
	Name:	Sol						
	Organi2	zation:	Precision	Welding	and Fabric	ation, Inc.		
	FAXN	umber:	(207) 854	-9694			angenggen i prob of allefa as assertes securios e	
,			f pages, inc	luding th	is page:	4		•
					: :			
From:								
	Name:	Thom	as Giles				····	
•	Office:	Weld	ing Test Ce	enter		***		
	Date:	01/29/0)8		. :			
	•							

Message:

Attached please find the supporting WPS for the welder certifications you mentioned. This procedure is from the AWS QC 3-89 "Standard for AWS Certified Welders" WPS# 1-QC-W2 and is pre-qualified per AWS D1.1 Structural Welding Code - Steel.

If problems in transmission are encountered, please contact the sender at: (207)974-4662 or FAX (207)974-4608.

WELDER, WELDING OPRATOR, OR TACK WELDER QUALIFICATION TEST RECORD

Name of Welder	Jason Joiner		144-00-0-1-1		
Name	Precision We		Identification N		
Velding Procedure Specification No.		<u>1</u> Rev.	N/A	_ Date	Aug. 20, 2008
Variables		ord Actual Value			
FF 11 440 Hz = (4)1		ed in Qualification	on Qu	alification F	Range
Process/Type [Table 4.10, Item (1)]	SMAW				
lectrode (single or multiple) [Table 4.10, Item			ALL		
urrent Polarity	115 A DC+				
ocition [Table 4.10, Itam /6]]	2F - Tack tes	+	1F, 2F		
Position [Table 4.10, Item (6)] Weld Progression [Table 4.10, Item (6)]	N/A	-	N/A		
Weld Frogression [Fable 4.10, hell (0)]	1071				
acking (YES or NO) [Table 4.10 Item (7)]	NO		NO		
/laterial/Spec.	Group 1	to Group 1			
Base Metal					
Thickness: (Plate)	NIZA		N1/0		
Groove	N/A		N/A UNLIMITED		
Fillet Thickness: (Pipe/Tube)	1/2 inch		ONTHALLED		
Groove	N/A		N/A		
Fillet	N/A		UNLIMITED		
Diameter :(Pipe)PJP					
Groove	N/A		N/A		
Fillet	N/A		N/A		
ller Metal [Table 4.10, Item (3)]					, <u> </u>
Spec. No.	A5.1				
Class	E7018		E4 E2 E2 E4		
F-No. [Table 4.10, Item (2)]	F 4		F4, F3, F2, F1		
as/Flux Type [Table 4.10item (3)]	N/A N/A		⊢ _{N/A}		
her	IWA				
	Acc		PECTION (4.8.1) S or NO	YES	
		Bend Test Resi	ults (4.30.5)		
Type	Result		Туре		Result
		Fillet Test Re	esults (4.30.2.3 and		
ppearance	Acceptable		Fillet Size	0.1875	
acture Test Root Penetration	Acceptable		Macroetch	N/A	
escribe the location, nature, and size	e of any crack o	r tearing of the s	specimen.)		
	oy_Brad Wells C	CWI # 0005022	1 Test Number		
Organizatio	on Maine Oxy		Dat	e Aug 20, :	2008
		RADIOGRAP	HIC TEST RESULT		1)
Film Identification	_		Film Identification		Demorts
Number	Results	Remarks	Number	Kesuits	Remarks
Interpreted b	by		Test Number		·
Organizatio	on		Date		
/e, the undersigned, certify that the stelled, and tested in accordance with ode Steel.	tatements in this the requiremen	record are cont ts of section 4 c	rect and that the tes of AWS D1.1, (t welds wer) Struc	re prepared, ctural Welding
Manufacturer or Contracto	or		Authorized E	y	
managada of of contract			Dat	e	

AUS QC*3 89 88 0784265 0006245 5

Keywords — AWS Certified Welders
Welders
Welder Certification
Welder Qualification

AWS QC 3-89

Standard for AWS Certified Welders

Prepared by AWS Qualification and Certification Committee

Under the Direction of AWS Education and Certification Council

Approved by AWS Board of Directors October 31, 1989

Abstract

This Standard describes a program administrated by the American Welding Society (AWS) for the certification of welders. The AWS Certified Welder Program allows for the transference of welder qualification from employer to employer without retesting. The standard establishes the minimum requirements for welder participation in the program and the methods to be used by employers for adopting the program.



American Welding Society

550 N.W. LeJeune Road, Miami, Florida 33126

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Information Handling Services, 199

QC 3 PERFORMANCE WELDING

PROCEDURE SPECIFICATION

Material specification ASTM A - 36	
Weiding process Shielded Metal Arc (SMAW)	The first of the second state of the second st
Manual or machine Manual	
Position of welding Vertical and Overhead	
Position of welding YOT TOBE CITE OF OUR AND	
Filler metal epecification AWS A5.1	And the second s
Filler metal classification <u>E7018</u>	
Flux N/A	
Shielding gas N/A	Flow rate N/A
Single or multiple pass <u>Multiple pase</u>	
Single or multiple arcSingle arc	
Welding current Direct current	
Polarity Electrode positive	
Welding progression Vert - up weave, OH - strip	nger beads
Root treatment None	
Preheat and Interpass temperature 70° F min 500) ^o max
Preheat and Interpass temperature 70 F MILLE	111/4-1
Postheat treatment None	

WELDING PROCEDURE

b	Floatrada	· Welding	current	Travel	
Pass no.	Electrode size	Amperes *	Volts	speed	Joint detail
All	8/32"	75-120	19-23	Manual	
	or 1/8″	110-780	19-23	Manuel	
		*Consult M			45°
		apeoific r			
				`	3/8"
					1/4"
				•	
· -	. [·		

This procedure may vary due to fabrication sequence, fit -up, pass size, etc., within the limitation of variables given in 4B, C, or D and 5.1.2 of AWS D1.1, (<u>1986</u>) Structural Welding Code.

Procedure no. 1 - QC - #2 Revision no. 0	Manufacturer or contractor American Welding 3001 ety Authorized by RE Bourbell
	DateMay 15, 1969

\$1-6

SUPPLEMENT 1

PERFORMANCE TEST DESCRIPTION Test Number: D1-SM-F4-P-A-L

Welding Process:

Shielded Metal Arc Welding (SMAVV)

Base Material:

ANSI/ASTM A-36, 3/8" thickness (rolling direction perpendicular to bevelled

leage)

Malerial Form:

Plate

Filler Metal:

ANSI/AWS A5.1, Class E-7018 (Group: F4)

Weld Joint Detail:

ANSI/AWS D1.1, Para, 5.19

Backing:

Prequalified steel plate from ANSI/AWS_D1.1

Welding Position:

3G & 4G (See ANSI/AWS A3.0-89 Figure 14)

Weld Progression (Vert):

uр

WPS No.:

1-QC-W2

Test Required:

Visual plus one root bend and one face bend. Radiography may be used in

lieu of bend tests.

LIMITS OF WELDER QUALIFICATION

Code:

ANSI/AWS D1.1

Weld Process:

SMAW

Base Material:

Prequalified steels from ANSI/AWS D1.1

Filler Melai:

Group F4 and lower

Thickness:

Groove: 1/8" thru 3/4"

Fillet Weld Size:

All

Positions:

All

Weld Progression:

Vertical, up

Backing:

Required for full penetration, single groove welds

Pipe/lubing Dia.:

Over 24"

Material Form:

Plate, pipe, shapes, strip, tubing as fisted in ANSI/AWS D1.1

S1-3

Name of Welder	Ben Berry	<u> </u>	·		
Name	Precision \	Nelding	Identification		N - 7 07
Welding Procedure Specification No.	2	Rev.	N/A	Date	Nov. 7, 07
		ecord Actual Valu	96		:
Variables	·Kı	ecoru Actuar valu Jsed in Qualificati	on . C	Qualification l	Range
(4)		in Qualificati		,	
Process/Type [Table 4.10, Item (1)]	SMAW		ALL	•	
Electrode (single or multiple) [Table 4.10, Iten	1/8 E/UIO		1765		
Current Polarity	110 A DC	<u>r </u>	· ·		
•• • • • • • • • • • • • • • • • • • • •					
	-20		1G, 2G, 3G		
Position [Table 4.10, Item (6)]	3G		UP UP		•
Weld Progression [Table 4.10, Item (6)]	UP			•	
			·		
(70)	YES		YES :		·
Backing (YES or NO) [Table 4.10 Item (7)]	Group 1	to Group 1			
Material/Spec.	Gloup i	- 10 O/OSF	•		
Base Metal	·				
Thickness: (Plate)	3/8 inch		1/8 " to 3/4 "		
Groove	N/A		UNLIMITED		
Fillet Thickness: (Pipe/Tube)					·
Groove	N/A		1/8 " to 3/4 "		
Fillet	N/A		UNLIMITED		
Diameter :(Pipe)PJP					
Groove	N/A		1/8" to 3/4 OVE		
Fillet	N/A		OVER 24" DIA	•	
Filler Metal [Table 4.10, Item (3)]		•			
Spec. No.	A5.1				•
Class	E7018		E4 52 52 51		
F-No. [Table 4.10, Item (2)]	F 4		F4, F3, F2, F1		
Gas/Flux Type [Table 4.10item (3)]	N/A			•	•
Other .	N/A		N/A		
	· · · · · ·	VOLUM INCO	PECTION (4.8.1)		
·		cceptable YES	or NO	YES	
	A A	d Bend Test Resu			-
		d Bend Test Ivest	Type		Result
Туре	Result	<u> </u>	1,750		
			- 		
		Fillet Test Re	sults (4.30.2.3 an	d 4.30.4.1)	
	N/A	•	Fillet Size	N/A	<u> </u>
Appearance	N/A		Macroetch	· N/A	
Fracture Test Root Penetration (Describe the location, nature, and size	of any crack	or tearing of the s	specimen.)		
(Describe the location, nature, and size	3 07 2119 212	:0\A/I # 000E022	Test Numb	per N/A	
Inspected b	y Brad Wells	CWI # 0005022	n restrains	ate Nov. 7, 2	2007
Organizatio	n Maine Oxy		-	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	
		RADIOGRAP	HIC TEST RESUL	TS (4.30.3.1) .
		IONDICOION	Film Identification	on .	
Film Identification	Deculto	Remarks	Number	Results	Remarks
Number	Results	Por Slee			•
0-	Accept	Joi, Jay.			
		1 11 /	T-ut Number		•
Interpreted b		Miller.	Test Number	1116	100
Organizatio	n · A 4	<u>. L</u>	Date	- 11/8	 U /
	atoments in th	is record are corr	ect and that the te	st welds were	e prepared.
We, the undersigned, certify that the st welded, and tested in accordance with	the requireme	nts of section 4 of	f AWS D1.1, () Struct	tural Welding
welded, and tested in accordance with			•	•	
Code Steel.	•		•		•
Contracto	r		Authorized l	Зу <u>·</u>	
Manufacturer or Contracto	"		- Da		

ANNEX E				Ard Olivia
),((((C)) =		AB TAC	K WELDER QUALIFICA	ATION TEST RECORD
		in a Tar	(. '	
Type of Weider The		70	identific	eation No. 6 10 -2162
Name <u>Follower</u> Welding Procedure Sp	acification No.	1	Rev Ala	Date
Welding Procedure op			Record Actual Values	Overlife tion Hanne
			Used in Qualification	Qualification Range
Variables			FCAID -	
Process/Type [Table 4.	10, (tem (1)]	Itam (8)1	04: 67.11	- ALL
Electrode (single or mu	Tubiel Lismania	1 (101) (4)]	TRET	
CurrentPolarity			. '7	160 260
Position (Table 4.10, Ite	em (4)}		02G	
Weld Progression [To	abla 4.10, Item (6))]	والمواجدة والمستور والمستور والمواجدة والمواجد	
ALEC NOVE	Tuble 4 to Item!	711	518	- CAT
Backing (YES or NO)	(ADIS 4.10) Notif	· · · · · · · · · · · · · · · · · · ·	to	
Materia/Spec. Base Metal				
Thickness: (Plate)			, 11	1 minted
Groove			1/4	(nim.B)
Fillet	ما	•		
Thickness: (Pipe/tube	θļ		ala	[24] warter
Groove Fillet			- dia	- Contract of the Contract of
Dlameter: (Pipe)			1/4	
Groove			7/4	
Fillet	15 - m 15117		way water prompt party of the first beauty of the same	
Filler Metal (Table 4.10,	, Item (3)]		B5.20	
Spec, No.			E71T-1	
Class F-No. [Table 4.10, Ite	m (2)]			7 - 10
Gas/Flux Type [Table 4	.10, Item (3)]		75% Anyon 0.76 CC	71/9
Other			- Lifeth	
			FOTON (4 8 1)	
•		VISUAL INSP	ECTION (4 8 1)	
		Acceptable 1	- 15 miles (4.30.5)	
			st Mosults (4.30.5)	Result
Туре		Result	Туре	
			11 00 0 0 and 4 3/1 4 1)	
			(4.30.2.3 and 4.30.4.1) Fillet Size	
Appearance			Macroetch	
Fracture Test Root Pe	Heitration	of mov crack or te	aring of the specimen.)	
			Test Number	
nspected by			Date	
Organization			Date	
	P.A	DIOGRAPHIC TE	ST RESULTS (4,30.3.1) .	·
and the second second			Film Identification	Results Remarks
Film Identification	Results	Remarks	Number	Results Hemarks
Number	Acceptable			
EDWARD				
D.	m PN			
nterpreted by	A 1		Date <u>5/554</u>	
Organization		and to the resource	and that the 1961	welds were prepared, welded, a tural Welding Code—Steel.
we, the undersigned, ce	auth that the states	nemas in unia rocort not section 4 of AW	S D1.1, (tural Welding Code—Steel.
ested in accordance with	n the requirements	I DI BOULDII T OFFITE	(year)	•
	etee	•	Authorized By	
vianufacturer or Contra	CIG(Date	

Form E-4

WELDING PROCEDURE SPECIFICATION (WPS) Yes D PREQUALIFIED QUALIFICATION RECORDS (PQR) Yes D OF PROCEDURE QUALIFICATION RECORDS (PQR)

OF PROCEDURE QUALIFICAT	Identification
Company Name Pricesian (a) library statements Welding Process(es) Train Supporting POR No.(4) 176 - Child Hourt	Revision MA Date Date 4-13-04
Echipped of MacIsaid	Authorized by Semi-Automatic
Name Burney Called	Type—Manuel [Automatic [
Company Tracess(e5)	. Machine
Welding Process(e5) TCAIN FOR Supporting POR No.(4) 1716 - Child Harl	
Supporting (a	POSITION Position of Groove:
11058	Position of Groove:
JOINT DESIGN USED	Position of Down Up Down
Type: Double Weld	Vertical Prograssion.
Single IX	ELECTRICAL CHARACTERISTICS
Backing: Yes I'm No Line 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	ELECTRICAL CARTAGO
Backing Market	Short-Circuiting
Back Gouging: Yes No Method	Transfer Mode (China)
Groove Angle: Hathod	Current: AC DCEP DCEN Pulsed
Back Gouding: Yes No No	Current: AC [] DCEP [2] DCCIVE
Dank Alive A. A.	Other
BASE METALS 0210	Tungsten Electrode (GTAW)
BASE METALS	Size:
the state of the s	Type:
Type or Grade	of f
Thickness: Gloop	TECHNIQUE 1 1 Coale Poot
Dismeter (Pipe)	
	Stringer or Weave Elead: The Stringer or Single Pace (per cide)
FILLER METALS	Number of Electrodes
AWS Classification AWS Classification AWS Classification	Number of Electrodes Longitudinal Longitudinal
AWS Classification	Electrode Spacing Lateral
	Angle
	Angle
200	3411
CUIFI DING	
SHIELDING GAS AREAN /CO2	Control Tibe to Work Distance
Flux Composition 7.00-2.00	Control Tibe to Work Distance
Composition 7.00-2.50 Flow Rate 35-40 CFn	Contact Tube to Work Distance
Composition 7.00-2.50 Flow Rate 35-40 CFn	Contact Tube to Work Distance
Composition 7.00-2.50	Contact Tube to Work Distance
Electrode-Flux (Class) Flow Rate 35-10 CFn Gas Cup Size 58:11	Contact Tube to Work Distance Peening Interpass Cleaning:
Electrode-Flux (Class) Flow Rate 35-10 CFn Gas Cup Size 58:11	Contact Tube to Work Distance Peening Interpass Cleaning:
Electrode-Flux (Class) Flow Rate 3.2 (O)	Contact Tube to Work Distance Peening Interpass Cleaning:
Electrode-Flux (Class) Flow Rate 3.2 (O)	Contact Tube to Work Distance Peening Interpass Cleaning:
Electrode-Flux (Class) Flow Rate 3.1 (OLER Electrode-Flux (Class) Electrod	Contact Tube to Work Distance Peening Interpess Cleaning:
Electrode-Flux (Class) Flow Rate 3.1 (OLER Electrode-Flux (Class) Electrod	Contact Tube to Work Distance Peening Interpess Cleaning:
Electrode-Flux (Class) Electrode-Flux (Class) PREHEAT Preheat Temp., Min Interpass Temp., Min WELDING	Contact Tube to Work Distance Peening Interpess Cleaning: Dock Contact POSTWELD HEAT TREATMENT Temp. Time PROCEDURE PROCEDURE
Electrode-Flux (Class) Electrode-Flux (Class) Electrode-Flux (Class) Electrode-Flux (Class) Flow Rate SCLIDEF Flow Rate Flow Rate SCLIDEF Max PREHEAT Preheat Temp., Min Interpass Temp., Min WELDING	Contact Tube to Work Distance Peening Interpess Cleaning: Dock Contact POSTWELD HEAT TREATMENT Temp. Time PROCEDURE JOHN CONTACT TO THE TIME TO THE
Electrode-Flux (Class) Electrode-Flux (Class) Electrode-Flux (Class) Electrode-Flux (Class) Flow Rate SCLIDCEN Gas Cup Size Flow Rate SCLIDCEN Gas Cup Size Flow Rate SCLIDCEN Max Flow Rate Flow R	Contact Tube to Work Distance Peening Interpess Cleaning: Dock Contact
Electrode-Flux (Class) Electrode-Flux (Class) PREHEAT Preheat Temp., Min Interpass Temp., Min Pass or Weld Class Composition 7.30-2.00 Flow Rate 3.2.00 Flow Rate 3.2.00 Max WELDING Pass or Weld Class Diem. Polarity	Contact Tube to Work Distance Peening Interpess Cleaning: Dock Contact POSTWELD HEAT TREATMENT Temp. Time PROCEDURE JOHN CONTACT TO THE TIME TO THE
Electrode-Flux (Class) Electrode-Flux (Class) Electrode-Flux (Class) Electrode-Flux (Class) Flow Rate Gas Gup Size Freheat Temp., Min Interpass Temp., Min WELDING Pass or Filler Metals Cu	Contact Tube to Work Distance Peening Interpess Cleaning: Dock Contact
Electrode-Flux (Class) Electrode-Flux (Class) Electrode-Flux (Class) Flow Rate SCLIDGE Flow Rate Fl	Contact Tube to Work Distance Peening Interpass Cleaning: Down C mounts CHURE HOME ON Sport O
Electrode-Flux (Class) Electrode-Flux (Class) Electrode-Flux (Class) Flow Rate SCLIDGE Flow Rate Flo	Contact Tube to Work Distance Peening Interpass Cleaning: Down C mounts CHURE HOME ON Sport O
Electrode-Flux (Class) Electrode-Flux (Class) Electrode-Flux (Class) Flow Rate SCLIDGE Flow Rate Flo	Contact Tube to Work Distance Peening Interpass Cleaning: Down C mounts CHURE HOME ON Sport O
Electrode-Flux (Class) Electrode-Flux (Class) Electrode-Flux (Class) Flow Rate SCLIDCEN Flow Rate Flow Rate SCLIDCEN Flow Rate Flo	Contact Tube to Work Distance Peening Interpass Cleaning: Down C mounts CHURE HOME ON Sport O
Electrode-Flux (Class) Electrode-Flux (Class) Electrode-Flux (Class) Flow Rate SCLIDCEN Flow Rate Flow Rate SCLIDCEN Flow Rate Flo	Contact Tube to Work Distance Peening Interpass Cleaning: Down C mounts CHURE HOME ON Sport O
Electrode-Flux (Class) Electrode-Flux (Class) Electrode-Flux (Class) Flow Rate SCLIDCEN Flow Rate Flow Rate SCLIDCEN Flow Rate Flo	Contact Tube to Work Distance Peening Interpass Cleaning: Down C mounts CHURE HOME ON Sport O
Electrode-Flux (Class) Electrode-Flux (Class) Electrode-Flux (Class) Flow Rate SCLIDGE Flow Rate Flo	Contact Tube to Work Distance Peening Interpass Cleaning: Down C mounts CHURE HOME ON Sport O
Electrode-Flux (Class) Electrode-Flux (Class) Electrode-Flux (Class) Flow Rate SCLIDGE Flow Rate Flo	Contact Tube to Work Distance Peening Interpass Cleaning: Down C mounts CHURE HOME ON Sport O
Electrode-Flux (Class) Electrode-Flux (Class) PREHEAT Preheat Temp., Min Interpass Temp., Min Pass or Weld Layer(s) Process Class Diam. Cut Verticent	Contact Tube to Work Distance Peening Interpass Cleaning: Down C mounts CHURE HOME ON Sport O
Electrode-Flux (Class) Electrode-Flux (Class) Electrode-Flux (Class) Flow Rate SCLIDCEN Flow Rate Flow Rate SCLIDCEN Flow Rate Flo	Contact Tube to Work Distance Peening Interpass Cleaning: Down C mounts CHURE HOME ON Sport O
Electrode-Flux (Class) Electrode-Flux (Class) Electrode-Flux (Class) Flow Rate SCLIDCEN Flow Rate Flow Rate SCLIDCEN Flow Rate Flo	Contact Tube to Work Distance Peening Interpass Cleaning: Down C mounts CHURE HOME ON Sport O
Electrode-Flux (Class) Electrode-Flux (Class) Electrode-Flux (Class) Flow Rate SCLIDCEN Flow Rate Flow Rate SCLIDCEN Flow Rate Flo	Contact Tube to Work Distance Peening Interpass Cleaning: Down C mounts CHURE HOME ON Sport O
Electrode-Flux (Class) Electrode-Flux (Class) Flow Rate Gas Cup Size Flow Rate Gas Cup Size Freheat Temp., Min Interpass Temp., Min Pass or Weld Layer(s) Frocess Class Diam. Polarity	Contact Tube to Work Distance Peening Interpass Cleaning: Down C mounts CHURE HOME ON Sport O

300

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A CONTRACTOR OF THE PROPERTY O

		INT AVENUE	N-DESTRUCTIVE TEST SOUTH PORTLAND, MAIL	CO D I COM PART	ADT.			
	 		INSPECT					or I
	MAY PR	ECISION WEI	DING & FABRICATIO	N P.O. NO.			PACE I	
CUSTOMER'S NA	EPORT NO	. QAL-04-28	30	PROCEDURE	NO: 0913		QUANTILL	<u> </u>
CAPROCICETIES	WELDEF	TEST PLAT	2	JOB NO.:		<u>. </u>	y to \	18"
105(1) 02	Iridium	1:192 SIZK	.118 X .132	CURICS: 24	.9 1011	MA:		YRONT LY
OURCE: TYPE				SINCLE Y	SIZE;	4½ x 10 HORE	ENS: 0,010*	
FILM: TVPE	<u>11 · </u>	SPKV	× 100	ролиги	, ME-101		พน	M ZIDK
iųi: seck AS		WTRE GROU	or: I	SENSITIVITY:	.025		, N/A so	NHCK STOR (A
MATERIAL TY			eniess: 1"	ACCEPTANCE	STANDARD:	AWS D1.1	OVIHER	
RADIOGRAPHIC	SET-CO.	Company		ELLIPTICAL		H		
AMERICA	SVIPA-	CONTENAN OF PART	ACCUMANTAL MARKET STATE OF THE	DUMBLE	NEW TOTAL	1000	400	18-4 0# 01
TUNE	NUMBER				a Cettainteer on a			
EDWARD	0-1	9	 					
					 			
					 			
	: 				1			
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		· · · · · · · · · · · · · · · · · · ·				-		
	<u></u>							· ·
			RI	MARKS				
···								
			•					
				,	METHOO;5	i), PROCE68(E5), PRO	XCEDURE(S) MERC	URY FREE
						0	118	·
DEFINITUATE 1. CTICL 1. Porestry 3. Incomplete Punion	7	. Includers Gas Births Shrink	11. JäVLo 12. Jugillood 13. Vaddaroot 3a. Vald	DIGNATURE:	D, Pratt #2: 05/2004	368772 Jac	11	
1. International houses		No represent Deliver	15. Intertital management	DATE: USA			(FORM	XXXX REV O

1937881703

Name of Welder	Precision Welding	Identification No. 4000-2257	
Name	1 Rev.	N/A Date Nov. 0	7, 200
Welding Procedure Specification No.			
Variables	Record Actual Value	S.	
variables	Used in Qualification	n Qualification Range	•
Process/Type [Table 4.10, Item (1)]	FCAW		
Electrode (single or multiple) (Table 4.10, Item	(£ .045 E71T1-M-H8	ALL	
Electrode (single of multiple) [Table 4.15, test	215 A DC positive		. :
Current Polarity	21071 20 -	7	: '
	·		
		1G	
Position [Table 4.10, Item (6)]	1G	F .	
Weld Progression [Table 4.10, Item (6)]	•		
		•	
•		YES	
Backing (YES or NO) [Table 4.10 Item (7)]	YES	. 123	
Material/Spec.	Group 1 to Group 1		
Base Metal	•		
Thickness: (Plate)		UNLIMITED	
Groove	1"	UNLIMITED	
Fillet	N/A	OHERWITED .	
Thickness: (Pipe/Tube)	N/A	UNLIMITED	
Groove		UNLIMITED	
Fillet	N/A		
Diameter :(Pipe)	N/A	OVER 24" DIA.	
Groove	N/A .	OVER 24" DIA.	
Fillet	N/A		
Filler Metal [Table 4.10, Item (3)]	A5.20		
Spec. No.	E71T1-1M-H8		
Class	F6	F6	
F-No. [Table 4:10, Item (2)]	N/A · ·		
Gas/Flux Type [Table 4.10item (3)]	N/A	N/A	
Other			
	VISUAL INSPE	CTION (4.8.1)	
	Acceptable YES of	rNO. <u>YES</u>	}
· · · · · · · · · · · · · · · · · · ·	Guided Bend Test Result	s (4.30.5)	
Tuno	Result	Type Resul	
Туре			
	N/A	N/A	
	Fillet Test Resi	ults (4.30.2.3 and 4.30.4.1) Fillet Size N/A	
Appearance	N/A	Fillet Size N/A Macroetch N/A	
Llion	N/A		
racture Test Root Penetration Describe the location, nature, and size	of any crack or tearing of the spi	edifferi.)	٠ لــــــا
Inspected by	Brad Wells CWI # 00050221	Lest Multiper IVA	<u>.</u>
Organization	Maine Oxy	Date November 7, 2007	<u>('</u>
·			
	RADIOGRAPHI	C TEST RESULTS (4.30.3.1)	
Film Identification		Film Identification	_
Number	Results Remarks	Number Results Remarks	<u> </u>
0-1	Accept Slag		
	farm Miller	Test Number	
Interpreted by		Date 11/8/07	
Organization	CXITI		.d.
Ve, the undersigned, certify that the sta	tements in this record are correc	it and that the test welds were prepare JWS D.1.1 () Structural Weld	dina
Ve, the undersigned, certily that the sta velded, and tested in accordance with the	ne requirements of section 4 of A	VVS D1.1. () Structurar vven	a 19
Code Steel.			
,040		Authorized Dy	
Manufacturer or Contractor		Authorized By	
		Date	

Otality Assurance Lads Inc. NON-DESTRUCTIVE TESTING AND INSPECTION SERVICES SOUTH PORTLAND, MAINE 04106 TEL: (207) 799-8911 FAX: (207) 799-7251

80 PLEASANT AVENUE .

INSPECTION REPORT

. L			1	VOI LU	HONTALI	<u> </u>		 	
CUSTOMER	'S NAME: PR	ECISION WI	ELDING		P.O. NO.:			PAGE	2 оғ
RADIOGRAP.	adiography report no.: QAL-07-2139			PROCEDUR	е хо.: 1		QUAN	QUANTITY: 3	
PART NO.:				n B 101 (1811 18 11 1811 1811 1811 1811 18 11 1811 1811 1811 1811 1811 1811 1811 1811	JOB NO.:		·		
SOURCE: T	YPE IR-192	SIZE	.118"		curies: l	4.1 KV:	МА	:	SFD: 16"
٠.					SINGLE V	- -			FRONT
FILM: TYP	e II	SPEE	D: 100		DOUBLE	SIZE:	41/2X10 scr	EENS: .010	" BACK FILM SIDE
ıqı: size	B WIRE	GRO	up: 1		SENSITIVITY	. ,010" .	SHD	·1:	SOURCE SIDE
MATERIAL:	TYPE CS/S	S THIC	KIVESS: 1.2	50"-3/8"	ACCEPTANCE	STANDARD:	AWSD1,1		
RADIOGRAP	HIC SET-UP	C D	E		ELLIPTICAL		H + + + + + + + + + + + + + + + + + + +	OTHER	
SERIAL NUMBER	VIEW NUMBER	CONDITION OF PART (See Definitions)	ACCEPT	REJECT	SERLAL NUMBER	VIEW NUMBER	CONDITION OF PART (See Definitions)	ACCEPT	REJECT
J.Lunt			<u> </u>						
1" 1G	0-1	5		/					
Bragdon	0-1	5	V						
1"			· • · · · · · · · · · · · · · · · · · ·						
3G			1.00.7.3						
Berry	0-1	2,5							
3/8"									
3G									
		:							:
				·			·		
		<u> </u>							
									-
				<u></u>					
				REM	ARKS			···	
TNITIONS: ack rosing complete Fusion complete penetrations	6. Inclus 7. Gas H 8. Shrink n 9. No Ap 10. Film .	oles : parent Defects	11. Hi/Lo 12. Surface 13. Undercut 14. Vold 15. Internal co	-	SIGNATURE: AS M/D/DATE: 11/08/2	aron Miller v 007	LEVEL: I	dik	7

WELDING PROCEDURE SPECIFICATION (WPS) Yes PREQUALIFIED V QUALIFIED BY TESTING

Ben. B or PROCEDURE QUALIFIC	CATION RECORDS (POR) Yes T
Company Name Precision Welding + Fabr Welding Process(es) FCAW Supporting PQR No.(s) PREQUALIFIED	Identification #
JOINT DESIGN USED Type: Single Double Weld Backing: Yes No	POSITION Position of Groove: 2 G Fillet: Vertical Progression: Up Down D
Backing Material: A STM A36 Root Opening	Transfer Mode (GMAW) Short-Circuiting Globular Spray
BASE METALS Material Spec. ASTM A36 Type or Grade Thickness: Groove / " Fillet Diameter (Pipe)	Current: AC DCEP DCEN Pulsed DCEN Tungsten Electrode (GTAW) Size: Type:
FILLER METALS AWS Specification AWS 5.25 AWS Classification E7/7-1	TECHNIQUE Stringer or Weave Bead: STRINGER / WEAVER Multi-pass or Single Pass (per side) MULTI PASS Number of Electrodes ONE Electrode Spacing Longitudinal
SHIELDING Flux Gas Argon / COn	Lateral Angle
Gas Cup Size 5/g 1	Interpass Cleaning: Down Mice horizing
PREHEAT Preheat Temp., Min Per AWS TABLE 3.2 (D1.1) Interpass Temp., Min Max	Chipping hammer, annder on Spath POSTWELD-HEAT TREATMENT

Pass or		Filler Metals		WELDIN	. Current		T	T
Weld Layer(s) Pro	Process	Class	Diam.	Type & Polarity	Anips or Wire Feed Speed	Volts	Travel Speed	Joint Details
		·		Verti Core			-	\K15° >/
1-13	FCAW	E717-1	, O45	DC+	ZOO AMPS.	26.5		
					V (14) (15)		- - -	707 7 1 E
m E-1 (Fron	.]_							> 11/4 16

UIAITY ASSITANCE LADS ITC.
NON-DESTRUCTIVE TESTING AND INSPECTION SERVICES 80 PLEASANT AVENUE • SOUTH PORTLAND, MAINE 04106 • TEL: (207) 799-8911 • FAX: (207) 799-7251 INSPECTION REPORT PAGE 1 OF P.O. NO.: N/A JUSTOMER'S NAME: PRECISION WELDING QUANTITY: 0913 PROCEDURE NO.: RADIOGRAPHY REPORT NO.: QAL-02-843 JOB NO.: N/A PART NO.: TEST COUPON 20" MA: N/A SFD: kv: N/A 30 CURIES: .107 X .118 SOURCE: TYPE Iridium 192 SIZE: V FRONT SINGLE SIZE: 41/2 x 10 SCREENS: 0.010" BACK DOUBLE **SPEED:** 100 FILM: TYPE II FILM SIDE SOURCE SIDE V SHIM: N/A SENSITIVITY: .020 iqi: size ASTM "B" WIRE GROUP: I ACCEPTANCE STANDARD: ASME SECTION IX THICKNESS: 1" MATERIAL: TYPE C/S OTHER RADIOGRAPHIC SET-UP ELLIPTICAL CONDITION COMPARI 2,5 BEN B. **FCAW**

DEFINITIONS:

1. Crack 2. Porosity

3. Incomplete Fusion

4. Incomplete penetration 5. Slag

6. Inclusions

7. Gas Holes

9. No Apparent Defects 10. Film Artifacts

11. HI/Lo

12. Surface 13. Undercut

14. Void 15. Internal concavity

SIGNATURE: R. Russell #2687716

M / D / Y 12/09/2002 DATE:

LEVEL:

(FORM XXXX REV 0)

Name of Welder	Precision Welding	Identification No. 2258
Name	1 Rev.	N/A Date Jan 15, 2008
Welding Procedure Specification No.	Titev.	TWA
Variables	Record Actual Value	S .
Variables	Used in Qualification	
Process/Type [Table 4.10; Item (1)]	FCAW	7
Electrode (single or multiple) [Table 4.10, Item		T ALL
	215 A DC Pos	
Current Polarity	21071 807 80	┤ :
Position [Table 4.10, Item (6)]	3G	1G, 2G, 3G
Weld Progression [Table 4.10, Item (6)]	V-up	V-up
Backing (YES or NO) [Table 4:10 Item (7)]	YES	YES
Material/Spec.	Group 1 to Group 1	<u>.</u>
Base Metal		
Thickness: (Plate)		
Groove	1 inch	UNLIMITED
Fillet	N/A	UNLIMITED
Thickness: (Pipe/Tube)		
Groove	N/A	UNLIMITED
Fillet	N/A	UNLIMITED
Diameter :(Pipe)PJP		
Groove	N/A	OVER 24" DIA.
Fillet	N/A	OVER 24" DIA.
Filler Metal [Table 4.10, Item (3)]	•	
	A5.18	
Spec. No.	E71T1-M-H8	
Class F-No. [Table 4.10, Item (2)]	F6	F6
Gas/Flux Type [Table 4.10 item (3)]	C-25	
	N/A	N/A
Other		
	VISUAL INSPE	CTION (4.8.1)
·	Acceptable YES	
	Guided Bend Test Result	
Tuna	Result	Type Result
Туре	result	
	Fillet Test Res	ults (4.30.2.3 and 4.30.4.1)
	N/A	Fillet Size N/A
Appearance Fracture Test Root Penetration	N/A	Macroetch N/A
(Describe the location, nature, and size	of any crack or tearing of the sp	ecimen.)
(Describe the location, hattire, and size	0, 0, 1, 1, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0,	Test Number N/A
	Brad Wells CWI # 00050221	Date Jan 15, 2008
Organization	Maine Oxy .	Date 3an 13, 2000
	RADIOGRAPH	IC TEST RESULTS (4.30.3.1)
Film Identification	_ :	Film Identification Number Results Remarks
Number	Results Remarks	Number Results Remarks
0-1	N. 19. 15	
	ACCEPT	
Interpreted by	John Weese	Test Number <u>08-008-2</u>
Organization		Date /-/6-08
Organization	9:01	
We, the undersigned, certify that the sta	tements in this record are correc	ct and that the test welds were prepared,
welded, and tested in accordance with t	he requirements of section 4 of A	AWS D1.1, () Structural Welding
Code Steel.		
0.000		
Manufacturer or Contractor		Authorized By
		Date

ame of Welder	Solomon Gay	<u> </u>	<u>. </u>
Name	Precision Welding /	. Identification	No. 9877 🗸 · · ·
Welding Procedure Specification No.	1 Rev.	N/A	Date Jan 15, 2008
Voluming 1 1000d and opposite and the			
Variables	Record Actual Value	es	
24,143,15	Used in Qualification	on o	ualification Range
Process/Type [Table 4.10, Item (1)]	FCAW		
Electrode (single or multiple) [Table 4.10, Item		ALL	•
electrode (single of multiple) (Table 4: 10; item	215 A DC Pos		
Current Polarity	21374 1307 130		
		`.\ .	
		10.00.00	
osition [Table 4.10, Item (6)]	3G	1G, 2G, 3G	
Weld Progression [Table 4.10, Item (6)]	V-up	V-up	
		•	
acking (YES or NO) [Table 4.10 Item (7)]	YES	YES	
Material/Spec.	Group 1 to Group 1		
ase Metal			
Thickness: (Plate)			•
Groove	1 inch	UNLIMITED	<u> </u>
Fillet	N/A	UNLIMITED	· · · · · · · · · · · · · · · · · · ·
Thickness: (Pipe/Tube)		· · · · · · · · · · · · · · · · · · ·	
Groove	N/A	. UNLIMITED	
Fillet	N/A	UNLIMITED.	
Diameter :(Pipe)PJP	•		
Groove	N/A	OVER 24" DIA	
Fillet	N/A	OVER 24" DIA	· · · · · · · · · · · · · · · · · · ·
iller Metal [Table 4.10, Item (3)]		· .	
Spec. No.	A5.18	·	
Class	E71T1-M-H8		
F-No. [Table 4.10, Item (2)]	F6	F6	
Gas/Flux Type [Table 4.10 item (3)]	C-25		
Sas/Flux Type [Table 4. Tolletti (3)] Other	N/A	│ _{N/A}	•
JU 161			
	VISUAL INSP	ECTION (4.8.1)	
		or NO	YES
	Guided Bend Test Resu		
Type	Result	Type	Result
1,750			
		1.	
		sults (4.30.2.3 an	d 4.30.4.1)
ppogrance	N/A	Fillet Size	N/A
ppearance racture Test Root Penetration	N/A	Macroetch	.N/A
racture Test Root Penetration Describe the location, nature, and size			
	•		or N/A
	Brad Wells CWI # 00050221	Test Numb	
Organization	Maine Oxy	_ Da	te <u>Jan 15, 2008</u>
•			TO (4.00.0.1)
	RADIOGRAPI	IIC TEST RESUL	
Film Identification		Film Identification	
Number	Results Remarks	Number	Results Remarks
0 = 1	N.A.D		
	ccept		· ·
		Test Number	08-0082
· · · · · · · · · · · · · · · · · · ·	John Weese	_	
Organization		_ Date	1-16-08
/e, the undersigned, certify that the sta	tements in this record are corre	ct and that the tes	t welds were prepared,
elded, and tested in accordance with ti	ne requirements of section 4 of	AWS D1.1, () Structural Welding
	io i again airiaina ar a a ann air i air		
ode Steel.		•	
		Authorized E	
Manufacturar or Contractor	· · · · · · · · · · · · · · · · · · ·	·/\UIII\UIIZCG L	
Manufacturer or Contractor		_ Dat	

Quality Assurance Labs Inc. NON-DESTRUCTIVE TESTING AND INSPECTION SERVICES

	80 PLEASAN	T AVENUE •	SOUTH PC	ODE OTL	CALDED	C. (207) 799-	8911 • FAX:	(201) 183-1231	
			IN	SPECTI	ON REP	JNI			
CUSTOMER'S NA		P.O. NO.:				_			
RADIOGRAPHY F	REPORT NO.:	QAL-08-00	82		PROCEDURE !			QUANTII	ry: 2
PART NO.: SE	E BELOW			<u> </u>	JOB NO.: 22	258/ 9877			
SOURCE: TYPE	Iridium 1	92 size:	.118		CURIES: 75	KV:	MA:	SF	rd: 12" FRONT
			100		SINGLE U	SIZE:	4½ x 10 scri	ens: 0,005"	
FILM: TYPE	<u> </u>	SPEED	: 100		DO ODDIE			F	ITW SIDE
iqi: size B		GROU	P: 1		SENSITIVITY:		SHIM	: S	OURCE SIDE V
MATERIAL: TY		THICK	NESS: 1"+	1/4" BB	ACCEPTANCE S	STANDARD:	AWS D1.1	OTHER	
RADIOGRAPHIC	SET-UP				**************************************				
A		D	E	F	LIPTICAL		H .	·	
SERIAL	VIEW	CONDITION	ACCEPT	REJECT	SERLAL NUMBER	VIEW NUMBER	CONDITION	ACCEPT	REJECT
NUMBER	NUMBER	OF PART (See Definitions)	ACCLI				(See Definitions)		
JEFF					soloman			· · ·	
LUNT	0-1	9			gay	0-1	9	J.	
2258		-4			9877				
		· ·				: 	 		
								·	
 		<u> </u>							
			•	·					
					1		,		
				RÉM	ARKS	· · ·		<u> </u>	<u> </u>
					,			. ,	
DEFINITIONS:							11/1	7/	
1. Crack 2. Porosity	6. Inclu 7. Gas :	Holes	11. HI/Lo 12. Surface 13. Undercu	, ,	و البوسية والمناسخ بيونية والمساسخ بيري	olm Weese	(f.br. (/)	leer	
3. Incomplete Fusion 4. Incomplete penetration 5. Sing		ik pparent Defects i Artifacis	13. Underco 14. Void 15. Internal		M / D DATE: 01/16/	/ Y /2008	LEVEL:	II :	

(FORM XXXX REV 0)

WELDING PROCEDURE SPECIFICATION (WPS) Yes PREQUALIFIED QUALIFIED BY TESTING OF PROCEDURE QUALIFICATION RECORDS (PQR) Yes

						on#		By 5. CAN
					Revision	1/A: De		Date
Company N	lame Inscis	ion la Jeldi	and tobacc	TION INC.	Authorized			Semi-Automatic
Company is	ocess(es)	FC4 is	ý					Automatic
weiding Fit	PQR No.(s)	00000	in Since		Mac	hine 🗌		
Supporting	P.O.H (40.(3)	Tack Character			·			
IOINT DES	IGN USED			-	POSITION	and to	ar Rea	Fillet*
-					Position of	Groove.10		Fillet:
iype. Single ⊠		Doub	le Weld 🗌		Vertical Pro	gression.	Up Do	WII []
Ducking. /	∕es⊠ No[OTEDICTIO	<u>c</u>
5	Backing Mat	erial:		•	ELECTRIC	AL CHARA	CTERISTIC	
	17.50	Doot Face	Dimension			. (0144)4	Λ Short	-Circuiting 🗌
A A A A	.io. 45°	, Hao	แนร์ เมบา		Transfer Mo	ode (GMAV	v) Short	ılar 🗍 Spray 🔀
Back Gourd	ng: Yes	No 🔀	Metho	d		- DCE		N Pulsed
Back Googi	ing.						P X DCL	N Pulsed
BASE MET	ALC.	·			Other		774140	
BASE ME	ec. <u>As</u>	$\tau m = 6$	136		Tungsten E	lectrode (G	i IAVV)	•
Material Sp	de							
Type or Gre	de Groove	1"	Fillet			Type:		
Discostor (5	Pipe)							
Diameter (r	ipe/				TECHNIQU	JE	~	سسرین در ا
					Stringer or	Weave Bea	ad: <u>Sang</u> a	1 / WEAVE 2001
FILLER ME	TALS	4 . 4 <	·		Multi-pass	or Single P	ass (per side	a) MULTI PASS
AWS Speci	fication	A003	<u> 3.00</u>		Number of	Electrodes	016	
AWS Class	ification	A FI	- 1		Electrode S	Spacing		tudinal
							Later	al
							Angle) <u> </u>
SHIELDING	3	_		1			•	
Flux		Gas	ARGON	1 002	Contact Tu	he to Work	Distance	3/-1:1
			•	<u>15% - 25%</u>	Deening	1-06		
Electrode-F	Flux (Class)_	Flo	w Rate <u>35</u>	-40 CFH	Internass (Cleaning:	hand wi	Re Brush
		Gas	s Cup Size	7/8 ··	microado (- Llance	700 00 0	volso or sparish
					DOSTME	D HEAT TI	REATMENT	,
PREHEAT				- 4.5				
Preheat Te	mp., Min <u>1</u> 2	er ALUS	TABLE 3.	2 (01.1)	Time			
Interpass T	mp., Min_P emp., Min_		Max_		Titise			
•								
		•		WITH DINK	PROCEDURE			
							[.	
		Filler	Metals	C	urrent			
Pass or	1			Type &	Amps or Wire		Travel	
Weld	_	Class	Diam.	Polarity	Feed Speed	Volts	Speed	Joint Details
Layer(s)	Process	Class	- Clain.	, 0.3,				•
			,					•
,	'					1	ļ	/9

Form E-1 (Front)

FCAW [8717-1 .045

WELDING PROCEDURE SPECIFICATION (WPS) PREQUALIFIED QUALIFIED BY TESTING		
or PROCEDURE QUALIFICATION RECORDS (PQR)	Yes 🗌	

or PROCEDURE GO.	Identification #2
	Revision A/A Date By 5. Colum
P: :	Date
Company Name Trecision Walding of TAM	Type—Manual Semi-Automat
Welding Process(es) 571AU	
Supporting PQR No.(s) Preaminfield	
JOINT DESIGN USED	POSITION
Type:	Position of Groove: 3G Fillet:
Single Double Weld Double	Vertical Progression: Up 🗍 Down 🗍
Backing: Yes No No	
Backing Material:	ELECTRICAL CHARACTERISTICS
Boot Face Dimension	
Groove Angle: 15° Radius (J-U)	- Italiste Mode (CIMAY) Short
Back Gouging: Yes No No Method	
	Current: AC DOEF DOEN THE
BASE METALS	Other
Material Spec. <u>ASTM A36</u>	Tungsten Electrode (GTAW)
Type or Grade	Size: Type:
Thickness: Groove 3/8" Fillet	
Diameter (Pipe)	
	TECHNIQUE
FILLER METALS	Stringer or Weave Bead: STRINGER
AWS Specification AUS A5.1	Multi-pass or Single Pass (per side) <u>multi-Pass</u>
AWS Classification <u>£7018</u>	Number of Electrodes
	Electrode Spacing Longitudinal Lateral
	Angle
SHIELDING	V
Flux Gas	On the A Tabe to Work Distance
Composition	
Electrode-Flux (Class) Flow Rate	Peening
Gas Cup Size	Interpass Cleaning:
PREHEAT 2 2 20 6	POSTWELD HEAT TREATMENT
Preheat Temp., Min Re AWS TABLE 3.2 (I	DI.1) Temp
Interpass Temp., Min Max	Time
intorpass rompi,	
W	FI DING PROCEDURE

				VACCOUNT	3 PROCEDONE		1	T
Deec or		Filler	Metals	<u> </u>	Current :			
Pass or Weld Layer(s)	Process	Class	 Diam.	Type & Polarity	Amps or Wire Feed Speed	Volts	Travel Speed	Joint Details
111		, .	3/32"		75-120		manual	, 45°
W.L.			or.		110-180		manusu	R
						-		783
								+ 1 14"

WELDING PROCEDURE SPECIFICATION (WPS) Yes PREQUALIFIED V QUALIFIED BY TESTING or PROCEDURE QUALIFICATION RECORDS (PQR) Yes

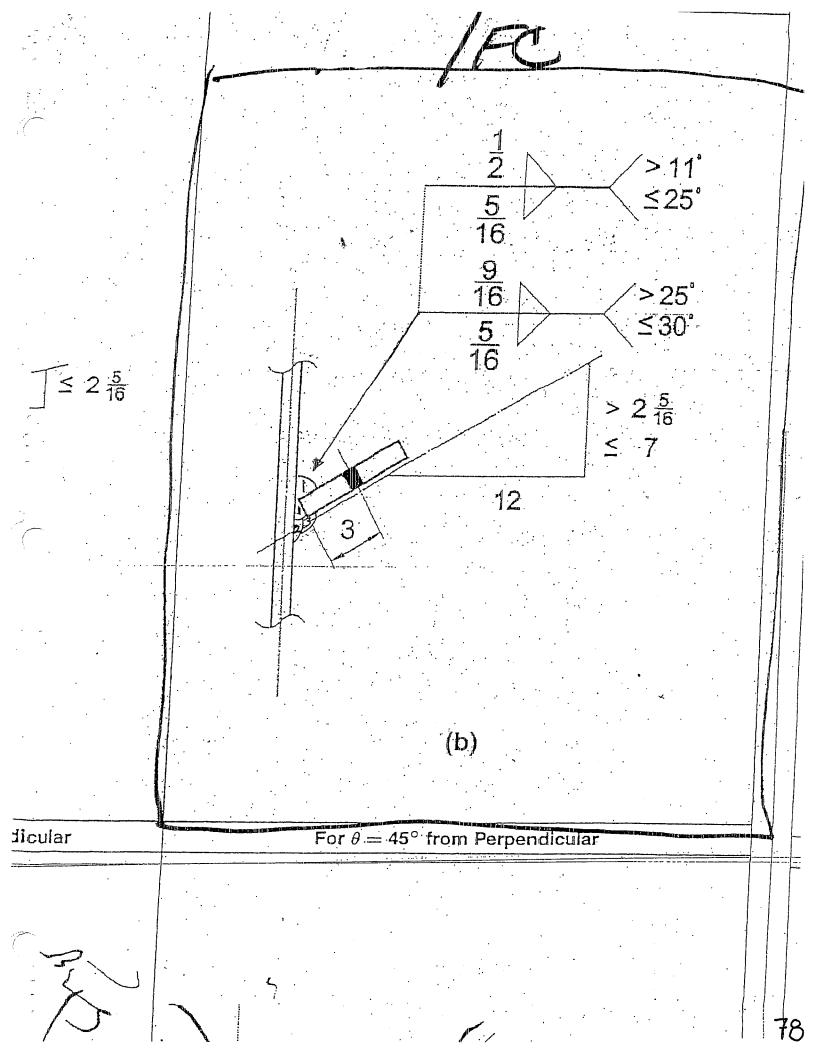
					Identific	cation #	PRECESI	0N 40 L
	٠,				Revisio	n	Date_12;	5107 By BYELLS
Company	v Name	PRECEST	cory M	VELD INC	Authori:	zed by		Date
Welding	Process(es	FCAL	ע		Туре—	Manual 🔲		Semi-Automatic 🔀
Supporting	a POR No)_FCAL (s)_BT	r - P4 -	GF	^	lachine 🗌	•	Automatic 🗌
Supportin	ng rought no.	· · · · · · · · · · · · · · · · · · ·	1					· ·
LOWET D	COLON LIGHT	on Free	2/1		POSITION)N		
JOINT DE	ESIGN USE	PARTER	<u>.</u> .				14	Fillet:
				- ··		Progression		
Single X			uble Weld [┛ .	. verucar	i Togression	. Op	20
Backing:	Yes 🔲 🗈	₁ο iXí			FLECTI	RICAL CHAP	ACTERIS	TICS
	Backing M			Va .		TICAL CHAI	TACTENIO	1100
Root Ope	ning <u>O</u>	Hoot Fa	ce Dimensi	on <u>"8 m</u>	<u>r</u> ^ Transfor	Mode (GMA	Sh	ort-Circuiting
Groove A	ngle:	(2) H	adius (J–U,)	_ mansier	MODE (GM)		obular Spray
Back Gou	iging: Yes	☐ No 🄀	Meti	100		40 D		CEN Pulsed.
BASE ME					Other			
Material S	3pec <i></i> 3	POUR I	to loga	严正	_ Tungster	Electrode		·
Type or G	rade	A 36	to A	992	.		· · · · · · · · · · · · · · · · · · ·	
Thickness	: Groove	3/16	Fillet		<u>.</u>	Type:		 .
Diameter	(Pipe)							
				•	TECHNI	QUE		
FILLER M	FTALS		······································		Stringer	or Weave Be	ead: <u>\$</u>	TRINGER
AWS Spec	cification.	A5.1	B		Multi-pas	s or Single	Pass (per s	ide) STOLCE
AWS Clas	sification	87171	æ im.	90,9m	Number (of Electrode	s ONE	
ATTO OIGS		<u> </u>			Electrode	Spacing	Lor	ngitudinal
-							Lat	eral
				· · · · · · · · · · · · · · · · · · ·	_			gle
SHIELDIN		<u></u>	s ARE	- 1 CO	• • •	•	•	•
Flux					.e/ Contact 7	Tube to Worl	k Distance	3/4"
:			mposition _	75%-25	Pogning	blobis	·	
Electrode-	Flux (Class			40 cfh	Interpose	Cleaning:	Clar ha	mmer furthe Gooth
		Ga	s Cup Size	5/8	. Interhass	Clearing.	3109 100	(Wille Co-11)
			·- 		- DOCTIVE	LD HEAT T	DEATMENT	
PREHEAT		· · · · · · · · · · · · · · · · · · ·	· ~ .	1 2- A	PUSTANE			
Preheat Te	mp., Min	LABLE :	7-17	I PER A	lemp			
Interpass 7	Temp., Min_	SAME	Max_		. Ime			
								
			• .					
			<u> </u>	WELDIN	G PROCEDURE	· · · · · · · · · · · · · · · · · · ·		
m		Filler	Metals	. (Current -			
Pass or			1	Type & .	Amps or Wire	1	Travel	
Weld		Class	Diam.	Polarity	Feed Speed	Volts	Speed	Joint Details
Layer(s)	Process	Class	Diam.	Polarity	1 ccd Opcca		Option	
1			ĺ				1	
			,	1				
	}			}				1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
, .	-02.	E7171C	0.45	DC+	550 IRM	29	101PM	
1	1-CHU	1M,90	,045		1	1 7 3	+25%	
		f I			550 Am ± 10% (195-605)	ITO	- 206	1 - 3
		9M	, .		1	16- 01	(7.5-)	
					(495-605)	164-211	(13.5)	不
	,					' •	(13.3)	
	ļ							
							·	
		1			•			

WELDING PROCEDURE SPECIFICATION (WPS) Yes PREQUALIFIED ____ QUALIFIED BY TESTING _____ or PROCEDURE QUALIFICATION RECORDS (PQR) Yes ____

·	Identification # 703
	Revision Date 12/5/07 By Buells
Company Name PRECESTION () PLOTING	Authorized by Date
Welding Process(es) FCALL	Type—Manual Semi-Automatic X
Supporting PQR No.(s) Progradified - Filet	Machine Automatic
Supporting PQH No.(s) Ptos Octivities 1 Titles	, mas mis
JOINT DESIGN USED	POSITION
Type: Filet - Skewed	
Single Double Weld	Position of Groove: Fillet: 1F Vertical Progression: Up Down
Backing: Yes No 🛛	73.4354.459.45
Backing Material:	ELECTRICAL CHARACTERISTICS
Root Opening Root Face Dimension	
Groove Angle: Radius (J–U)	Transfer Mode (GMAW) Short-Circuiting
Back Gouging: Yes No Method	. Globular 🗌 Spray 🗍
Back Goldging. 165	Current: AC DCEP DCEN Pulsed
BASE METALS	Other
Material Spec. 6000 T to 6000 II	Tungsten Electrode (GTAW)
Type or Grade A36 to A992	Size:
Thickness: Groove Fillet 5/16-4/16	Туре:
Diameter (Pipe)	
· · · · · · · · · · · · · · · · · · ·	TECHNIQUE
FILLER METALS	Stringer or Weave Bead: 8TOTNASE
AWS Specification A 5.18	Multi-pass or Single Pass (per side) Stroke / MULTI
AWS Classification 2717-1C, 1m, 9C 9M	Number of Electrodes ONS
	Electrode Spacing Longitudinal
	Lateral
SHIELDING	Angle
Flux Gas AR600 - CO2	21.
Composition 752-25%	Contact Tube to Work Distance 3/4-("
Electrode-Flux (Class) Flow Rate 40ch	Peening Nov2
Gas Cup Size 5/8	Interpass Cleaning: Chipping hammer-
PREHEAT	POSTWELD HEAT TREATMENT
Preheat Temp., Min Por Dal 1 Table 3,2 - AWS	Temp.
Interpass Temp., Min	Time

WELDING PROCEDURE

					·		T	T -	
Pas	s or		Filler I	Metals	(Current]		
We	eld	Process	Class	Diam.	Type & Polarity	Amps or Wire Feed Speed	Volts	Travel Speed	Joint Details
\	3	FCAW	671746 1m,9C 9M	.045	DC+	5 50IPM ± 10% (495-605)	2°(± 7% (27-31)	10.1Pm ± 25% (7.5-) (3.5)	SZE ATTACHED SHEET



WELDING PROCEDURE SPECIFICATION (WPS) Yes PREQUALIFIED _____ QUALIFIED BY TESTING or PROCEDURE QUALIFICATION RECORDS (PQR) Yes ____

	Identification # 704
	Revision Date 12/5/67-By Budell
Company Name PRECISION WELVING	Authorized by Date
Welding Process(es) FCA(L)	Type—Manual 🗌 Semi-Automatic 🖸
Supporting POR No.(s) PREQUALIFIED	Machine 🗌 Automatic 🗌
0 dpp 0 1 mg 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1	
JOINT DESIGN USED	POSITION
Type: FILLET	Position of Groove: Fillet: 2 F
Single Double Weld	Vertical Progression: Up Down
Backing: Yes No 🔀	
Backing Material:	ELECTRICAL CHARACTERISTICS
Root Opening Root Face Dimension	
Groove Angle: Radius (J–U)	Transfer Mode (GMAW) Short-Circuiting
Back Gouging: Yes No Method	Globular 🗍 Spray 🗍
Dack Godging. 100 E. 1,10 E.	Current: AC DCEP X DCEN Pulsed
BASE METALS	Other
Material Spec. ASTM A36	Tungsten Electrode (GTAW)
Type or Grade	Size:
Thickness: Groove Fillet _ 4 /8	Туре:
Diameter (Pipe)	
Diameter (1 1po)	TECHNIQUE
FILLER METALS	Stringer or Weave Bead: STRINGER
AWS Specification ASUS	Multi-pass or Single Pass (per side) うていして
AWS Classification <u>\$717-16,1m,90,9m</u>	Number of Electrodes ONE
AVV3 Classification Z CT 1-1 C, TT1; TC; UTT	Electrode Spacing Longitudinal
•	Lateral
0.115.0110	Angle
SHIELDING Flux Gas ARGON CO2	
Flux Gas HT(LOD) (CO ₂ Composition 756. 25%	Contact Tube to Work Distance 3/4-1
	Peening None
Electrode-Flux (Class) Flow Rate 40 c. Gas Cup Size 3/8	Interpass Cleaning: Chipping hammer-
Gas Oup Size	WERE BRUSH
	POSTWELD HEAT TREATMENT
PREHEAT Dog Tillo 30 IN I - ANS	Temp.
Preheat Temp., Min Pel Table 3.24 DI.I ANS	Time
Interpass Temp., Min SAME Max 1	Time
WELDING PR	OCEDURE

Г	· · · · · · · · · · · · · · · · · · ·		Filler	Metals	····	G PROCEDURE Current		ļ.	}]
	Pass or Weld _ayer(s)	Process	Class	Diam.	Type & Polarity	Amps or Wire Feed Speed	Volts	Travel Speed	Joint Details	
	1	FCAW	87179C 1m,9C	.cA5	DC+	300IPM ± 10% (270-330)	26 ± 79° (24.5 -) 28)	8IPM ±25% (6-10)		- 5/6+Y

WELDING PROCEDURE SPECIFICATION (WPS) Yes ☑ PREQUALIFIED ✓ QUALIFIED BY TESTING or PROCEDURE QUALIFICATION RECORDS (PQR) Yes ☐

	Identification # Promision 705
Posses Valence	Revision 1 Date 12/5/67 By Blads Authorized by Date
Company Name TRECISION WELDING	
Welding Process(es) FCA(x)	TypeManual Semi-Automatic A
Supporting PQR No.(s) (R)COUNTETED	Machine Automatic Automatic
JOINT DESIGN USED	POSITION
Type: \TLLET	Position of Groove: Fillet: 2F
Single Double Weld	Vertical Progression: Up Down
Backing: Yes No 🔀	
Backing Material:	ELECTRICAL CHARACTERISTICS
Root Opening Root Face Dimension	
Groove Angle: Radius (J–U)	Transfer Mode (GMAW) Short-Circuiting
Back Gouging: Yes No Method	Globular Spray
	Current: AC DCEP DCEN Pulsed
BASE METALS	Other
Material Spec. ASTM A 36	Tungsten Electrode (GTAW)
Type or Grade	Size:
Thickness: Groove Fillet = 1/8	Type:
Diameter (Pipe)	
Diamotor (i ipo)	TECHNIQUE
FILLER METALS	Stringer or Weave Bead: STRINGER
AWS Specification A 5.18	Multi-pass or Single Pass (per side) STICALE
AWS Classification ETITE IM 90,9M	Number of Electrodes ONE
AVVS Classification Z +11-1 e_ 111 , ac , 4111	Electrode Spacing Longitudinal
	Lateral
	Angle
SHIELDING	Aige
Flux Gas ARion (Co.	3/1-1"
Composition <u>75%-25%</u>	Contact Tube to Work Distance 3/4 -1"
Electrode-Flux (Class) Flow Rate 40 CFA	Peening NONE
Gas Cup Size 5/€	Interpass Cleaning: Chipping framme / wirebush
	DOCTIVELD HEAT TOGATMENT
PREHEAT O THE SOUND	POSTWELD HEAT TREATMENT
Preheat Temp., Min Per Table 3.2 of DI.1-AWS	
nterpass Temp., MinSAm_&Max	Time
WELDING P	ROCEDURE

Pass or		Filler	Metals	(Current				
Weld Layer(s)	Process	Class	Diam.	Type & . Polarity	Amps or Wire Feed Speed	Velts	Travel Speed	Joint Details	٠
1	FCAW	E71719 im,90 9M	.045	DC+	550 IRM ± 10% (495-605)		10 tom + 25% (7.5-)	1 k	

WELDING PROCEDURE SPECIFICATION (WPS) Yes PREQUALIFIED QUALIFIED BY TESTING OF PROCEDURE QUALIFICATION RECORDS (PQR) Yes

	,	•		•	Identi	fication#_	S是030	_ .		
Comp	any Name	Page	1:3			ionl	_ Date <u>」ス</u>	<u>-5-07</u>	By Burn	
Weldi	ng Process	- RECTY	TON W	POING		rized by	,		Date	
Suppo	orting PQR	No (e)	2000			-Manual			Semi-Automa	
Suppe	ing Fun	140.(S)P	KEROSHLE	(-I.s. ()		Machine _	J	: .	Automa	ıtic 🗌
TAIOL	DESIGN L	ISED			POSIT	TION!				
U.	FILLET			•						•
			Double Wel	4 [T]		n of Groove			Fillet: 2F	
Backin	g: Yes 🗌	No 🔽	Donnie Mei	4∟.	veruca	ıı Progressi	on: Up 🗌	Down _		
Dackin	Backin	n Material	•	·	<u> </u>	DIOA! OU	1010TED:	77100		
Boot O	pening	y waterial. Root	Eace Dimer	eión .	ELECT	HIGAL CH	ARACTERI	STICS		
Groove	Angle		Radius (.l	U)	Transfe	r Mode (Gi	4000	had Olive	.w. C1	
Back G	ouging: Y	es . No	Σ∛ M M	ethod		i Mode (Gi		hort-Circ	Jitung ∐] Spray [].:	
				<u></u>	Current	AC I	DCEP 🔯 . I	CEN	J Splay [].:	
BASE	//ETALS				Other		. پرکل دیادی		r dised [-]	•
	Spec.	ASTY	1 A36		- · · · · · · · · · · · · · · · · · · ·	n Flectrode	(GTAW)	· · · · · · · · · · · · · · · · · · ·		
	Grade									
Thickne	ss: Groov	, ,	Fille	t .4 1/8		Type:	<u> </u>			
	er (Pipe)								•	,
					TECHN	IQUE				
FILLER	METALS						3ead:	torave		
	ecification	A5.	18		Multi-pa:	ss or Single	Pass (per	side) Sz	313618	
AWS Cla	assification					of Electrod	es 610		10000	`` '
	٠.		•			e Spacing		ngitudina		
4	14 A A							teral		
SHIELDI	NG.							gle .		· 4
Flux	<u> </u>		ias	*		···	• •	-	• •	
	. :	C	Composition		Contact	Tube to Wo	rk Distance			
Electrode	-Flux (Clas		low Rate			NON		·		
		G	as Cup Size		Interpass	Cleaning:	WIRE R	RUSH (HIPPING	
		<u> </u>		:: .					HAMMER	
PREHEAT					POSTWE	LD HEAT	TREATMEN	Τ: .		-
Preheat T	emp., Min_	PER TABL	E 3.20	DI. 1-AL	V9 Temp			<u></u>		
Interpass	Temp., Min	SAMO.	Max		Time				<u> </u>	
	<u> </u>	<u></u>	* * * .			, e is				
•										
 			······································		IG PROCEDURE	<u> </u>	·			•
Pass or	1: :	Filler	Metals	1.00	Current ,			: <u> </u>		7
Weld					Amps or Wire	1	Travel			
Layer(s)	Process	Class	Diam.	Polarity	Feed Speed	Volts	Speed	loi	nt Details	
	-	· · · · · · · · · · · · · · · · · · ·	 	1				504	. Domino	-
• •		1.						·		
	1	1.	· .							
]. :	· .]				1	77	•	
•			1.7	No. 1	ا محند ا				•.	1, .
	SMAW	E-7018	8	DC+	115A		4 Ian			1 :
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· [.	1		1 .4.1.1
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rm F-1 /Fro				<u> </u>						J

PRECISION WELDING AND FABRICATION, INC.

Structural Steel - Miscellaneous Metals

(207) 854-9330 Telephone (207) 854-9694 Fax (207) 854-1167 Accounting



690A Stroudwater P.O. Box & Westbrook, Maine 04098-08



TO: QUALITY ASSURANCE LABS INC.

ATTN: ART GALLANT

FROM: SOL GAY

1-29-08

REF: CONTINUITY LOG FOR AWS CARDS ATTACHED SMAW

- EDWARD NADEAU
- RONALD MOODY
- GARY ZARATE
- SOLOMON GAY

ALL WELDERS LISTED ABOVE HAVE CURRENT CERTIFICATIONS & HAVE HELD THESE CONTINUOUS SINCE DATE OF TESTING DATED ON CARDS. (AWS REQUIRES THESE CARD CERTIFICATIONS TO BE MAINTAINED WITH THEM EVERY SIX MONTHS.)

THANK YOU,

SOLOMON R. GAY

Company Name PRECISION WELDE		_ By By	1845
Welding Procedure Specification No. DI.3 COT	Rev_001_	Date <u>4 - 1</u>	<u>6-68</u>
Supporting Procedure Qualification Test Record(s) No.	<u> </u>		
Nelding Process(es) <u>GMAW</u>	Type MAN		- V
		utomatic, manual, e	(C.)
Mode of Transfer for GMAW SHORT - CIRCUS	(Short circuiting, spray,	etc.)	
			
OINTS (Table 4.1) Type of Welded Joint(s) 4.1.4.2 A	COATING(S) TypeU	a	
Type of Welded Joint(s) 4.1, 4.2 A	rype ⊘ ↓r Thickness		
Backing Yes No X		sh of Joint Data!!-	
Backing Material Type	Sket	ch of Joint Details	
Groove Welded From:			
one side both sides			
ASE METAL (1.2)			
Material specification type and grade:	41 [17	
Sheet steel A36 to A36	J. **	٠	
Support steel			
Thickness Range: Sheet Steel 166 - 3/16	and the second s		
Sheet Steel 166 - 316 Support Steel 166 - 316			
Thickness 10 quage			
Base Metal Preparation			
	4.2A		
LLER METAL (Table 1.1)		\mathcal{L}	V
Specification A 5.18			
Classification £705-6			
OSITIONS (Table 1.2) Position of Groove36		=-1	
Position of Fillet 2F	PREHEAT (1.1.1 AND Preheat Temperature		
Progression	Preneat Temperature Preheat Temperature		
	r remeat remperature	J. ITIGA	
AS (1.4.6.2) Shielding Gas	Flow Rate 2	5 cfh	الراب و معطوم و الموسود الرابعي منه الرابع الرابعية الموسود الرابعية الرابعية الموسود الرابعية الموسود الرابعية الموسود الرابعية الموسود الرابعية
Percent Mixture 75% 25%	1 IOW I Idle		
.UX (1.4.5.2)			
TE	CHNIQUE		
Welding Current	t Travel Speed		Wire
Pass	(or-Weld-Time-for-		Feed
No. Electrode Size Amperes V	olts Arc Spot Welds)	Melting Rate	Speed
	1 CV 0		250
1 150 A 17	7.5 V. 8 IPm	4.25	250 (apport)
		4.2	
		The state of the s	
is procedure may vary due to fabrication sequence	fit-un nass size etc within t	the limitation of val	iables given in
is procedure may vary due to fabrication sequence, ISI/AWS D1.3 (fit-up, pass size, etc. within t -Sheet Steel	the limitation of val	iables given in
is procedure may vary due to fabrication sequence, ISI/AWS D1.3 (fit-up, pass size, etc. within t Sheet Steel.	the limitation of val	iables given in

Revision OO	
Type MANUAL	
(Aut	tomatic, manual, etc.)
of trave	
(Short circuiting, spray, glo	obular)
ACTUAL VARIABLE USED	
	QUALIFICATION RAN
선회 1000 등 집단하는 사람은 하는 사람	
4.2 A	4.2 A
	- 1.5 / 1
A36 to A36	SEE AwSto 12.1
10 GUAGE	16 quage -> 3/16
	-
	· · · · · · · · · · · · · · · · · · ·
A	
LONE	NONE
	1F, 2F
75% AR - 25 8.60	75% AR - 25% C
	10/0111-03/00
<u> </u>	
2 40> -6	<u> </u>
Λ	
Specimen 2/+cc	EDTABLE
Undercut_	NONE // AU
	1 315
	-Dev 10-11
Per ALDS NIT	3-98
	5-08
Duit VI IDSt 111	D 00
aro correct and that it	
are correct and that the test we	los were prepared and tested
//	ng Coae—Sneet Steel.
	Type MANOAL (Au STIRUS (Short circuiting, spray, glo ACTUAL VARIABLE USED IN QUAL. 4.2A A36 to A36 IO GOAGE TSTAR - 25702 Specimen 2 Acc Undercut Diam of Arc Spot Nugget Per AWS M.

Company Form A-3

Welder or Welding Operator's Name Soconon Identification No. 9877 Welder's Social Security No.	Qualification Date	1-15-08
In Accordance with WPS No	Revision — OOL Type — MAWAL (Auto	omatic, manual, etc.)
Mode of Transfer for GMAWSHORT CTRCC	(Short circuiting, spray, glo	bular)
VARIABLE	ACTUAL VARIABLE USED IN QUAL.	QUALIFICATION RANGE
JOINT Joint Type Backing Material Type Groove Welded From: one side or both sides	4.2A	4.2 A
BASE METAL (4.7.1.1) Material Specification Sheet Steel Supporting Steel	A36 to A36	SEE AWS to 1,2.1
Sheet Thickness (4.7.2.1) Groove Fillet Arc Plug Arc Spot Arc Seam	10 GUABE	16 quage -> 3/16"
COATING(S) Type Thickness	NONE	NONE
POSITION (4.7.1.5 and 4.7.1.6) Groove Fillet Arc Plug Arc Spot Arc Seam Progression	2 F	1F, 2F
GAS (4.7.1.4)	75% AR - 25 BCO2	75% AR - 25% CO2
ELECTRODE (4.7.1.3 and 4.7.1.4) Size Group Designation	035 E 705-6	
VISUAL EXAMINATION RESULTS (4.6) Specimen 1 A CCEPT ABLE Cracks NO Reinforcement A CCEPTABLE		20 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
Test Conducted By BRAO WELLS Laboratory Test No.	Per AWS SI Date of Test 4	.3-98 15-08

Authorized By

SAMPLE FORM FOR			

VARIABLE JOINT Joint Type Backing Material Type Groove Welded From: one side or both sides BASE METAL (4.7.1.1) Material Specification Sheet Steel Supporting Steel Sheet Thickness (4.7.2.1) Groove Fillet Arc Plug Arc Spot		natic, manual, etc.) Jlar) QUALIFICATION RANGE
VARIABLE JOINT Joint Type Backing Material Type Groove Welded From: one side or both sides BASE METAL (4.7.1.1) Material Specification Sheet Steel Supporting Steel Sheet Thickness (4.7.2.1) Groove Fillet Arc Plug Arc Spot	IN QUAL.	QUALIFICATION BANGE
JOINT Joint Type Backing Material Type Groove Welded From: one side or both sides BASE METAL (4.7.1.1) Material Specification Sheet Steel Supporting Steel Sheet Thickness (4.7.2.1) Groove Fillet Arc Plug Arc Spot	하는 그래면 회사 이 유민은 감독하는	QUALIFICATION BANGE
Joint Type Backing Material Type Groove Welded From: one side or both sides BASE METAL (4.7.1.1) Material Specification Sheet Steel Supporting Steel Sheet Thickness (4.7.2.1) Groove Fillet Arc Plug Arc Spot	4.2A	GOALII IOAI ION NANGE
Material Specification Sheet Steel Supporting Steel Sheet Thickness (4.7.2.1) Groove Fillet Arc Plug Arc Spot		4.2 A
Sheet Steel Supporting Steel Sheet Thickness (4.7.2.1) Groove Fillet Arc Plug Arc Spot		
Groove Fillet Arc Plug Arc Spot	A36 to A36	SEE AWS to 1,2.1
	ID GUAGE	16 quage -> 3/16"
Arc Seam		
COATING(S) Type Thickness	None	NONE
POSITION (4.7.1.5 and 4.7.1.6) Groove Fillet Arc Plug Arc Spot Arc Seam Progression	<u> </u>	1F, 2F
- 1788 - 1885 (1985) 18 20 18 18 18 18 18 18 18 18 18 18 18 18 18	5% AR - 25 2002	75% AR - 25% CO2
Size Group Designation	.035 E705-6	. 035 E705~ X
/ISUAL EXAMINATION RESULTS (4.6) Specimen 1 A CCCPT A BLE Appearance ACCEPTABLE Cracks Nove Reinforcement ACCEPTABLE	Specimen 2 Acce Undercut Diam of Arc Spot Nugget	COTABLE NOUE
est Conducted By Bako WEIS aboratory Test No.	Per AWS 51.7 Date of Test 4-15	3-98 5-08
he undersigned certify that the statements in this record are coordance with the requirements of 4.6 of ANSI/AWS D1.3 (
Company	correct and that the test welling (year).	ds were prepared and tested in g Code—Sheet Steel.

SAMPLE FORM FOR WELDER AND WELDING OPERATOR QUALIFICATION TEST RECORD

Identification No. 2758 Welder's Social Security No.	Qualification Date1	-15-08
In Accordance with WPS No		matic, manual, etc.)
	(Short circuiting, spray, glob	ular)
VARIABLE	ACTUAL VARIABLE USED IN QUAL:	QUALIFICATION RANGE
JOINT Joint Type Backing Material Type Groove Welded From: one side or both sides	4.2A	4.2 A
BASE METAL (4.7.1.1) Material Specification Sheet Steel Supporting Steel	A36 to A36	SER AWS to 1,2.1
Sheet Thickness (4.7.2.1) Groove Fillet Arc Plug Arc Spot Arc Seam	IO GUAGE	16 quage -> 3/16 "
COATING(S) Type Thickness	Buog	None
POSITION (4.7.1.5 and 4.7.1.6) Groove Fillet Arc Plug Arc Spot Arc Seam		1F, 2F
Progression GAS (4.7.1.4)	75%AR -252002	75% AR - 25% CO
Size Group Designation	. 035 . E 705-6	.035 .2705-X
Specimen 1. ACCEPTABLE Cracks NOA Reinforcement ACCEPTABLE	Y 1804 Y 14 Y 12 Y 14 Y 14 Y 14 Y 17 Y 1 Y 1 Y 1 Y 1 Y 1 Y 1 Y 1 Y 1 Y	OTABLE NOUE
est Conducted By BRAO WILLS aboratory Test No.	Per AWS 51.7 Date of Test 4-19	3-98 5-08
he undersigned certify that the statements in this record coordance with the requirements of 4.6 of ANSI/AWS D1.3	Lare correct and that the test well (1986), Structural Welding (1987)	ds were prepared and tested in g Code—Sheet Steel.
ompany	Authorized By	

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Velder or Welding Operator's Name <u>Sole imer</u> dentification No. <u>GS</u> 子子 Velder's Social Security No.	Qualification Date <u>4</u> -	-15-08
NACCORDANCE WITH WPS No		natic, manual, etc.)
Mode of Transfer for GMAW SHORT CTROUTTE	(Short circuiting, spray, globu	lar)
VARIABLE	ACTUAL VARIABLE USED IN QUAL.	QUALIFICATION RANGE
OINT Joint Type Backing Material Type	34-4.1	4,1
Groove Welded From: one side or both sides	ONZ SIDE	EITHER
MASE METAL (4.7.1.1) Material Specification Sheet Steel	A36 to A36	58 AWS 12.1
Supporting Steel Sheet Thickness (4.7.2.1) Groove Fillet Arc Plug Arc Spot	10 GUAGE	166-3/16"
Arc Seam OATING(S)	None	None
Type Thickness		
OSITION (4.7.1.5 and 4.7.1.6) Groove Fillet Arc Plug Arc Spot	36	16,26,36
Arc Seam Progression		
AS (4.7.1.4)	75% AREON - 25% CO2	45% AR-25% CO.
LECTRODE (4.7.1.3 and 4.7.1.4) Size Group Designation	. 63.5 £705-6	.035 £705 · X
Specimen 1 ACCEPTABLE.	Specimen 2 Acces	the second secon
Appearance ACCEPTABLE Cracks NONE Reinforcement ACCEPTABLE	Undercut_ Diam of Arc Spot Nugget	- NONC
est Conducted By TORAD WELLS aboratory Test No.	Per AWS DI.3 Date of Test 4-15-6	-9g o8
he undersigned certify that the statements in this record coordance with the requirements of 4.6 of ANSI/AWS D1.3	(<u>18</u>), Structurar Weldin (year)	ds.were prepared and tested g Code—Sheet Steel.
Company	Authorized By	

SAMPLE FORM FOR WELDER AND WELDING OPERATOR QUALIFICATION TEST RECORD

Welder or Welding Operator's Name リフレー Identification No. ススラ8 Welder's Social Security No.	Qualification Date	4-15-08
In Accordance with WPS No. D1.3-001 Welding Process(es) GMAW Mode of Transfer for GMAW SHORT CIRCL		tomatic, manual, etc.)
VARIABLE JOINT Joint Type	ACTUAL VARIABLE USED IN QUAL. 34—4,1	QUALIFICATION RANGE 격, I
Backing Material Type Groove Welded From: one side or both sides	ONE SIDE	EITHER
BASE METAL (4.7.1.1) Material Specification Sheet Steel	_A36 to A36	SEE AWS to 1.2.1
Supporting Steel Sheet Thickness (4.7.2.1) Groove Fillet Arc: Plug Arc Spot Arc: Seam	VO GUAGE	(tigivaje → 316
COATING(S) Type Thickness	None	SOOL
OSITION (4.7.1.5 and 4.7.1.6) Groove Fillet Arc Plug Arc Spot Arc Seam	34	16,26,36
Progression	75% AREON /25%	CO2 75% AR-25%
AS (4.7.1.4) LECTRODE (4.7.1.3 and 4.7.1.4) Size Group Designation	4 5 6 AKON 122 B 	.035 .255 .2765-X
Appearance 113000 Tolors	Specimen 2 Acc 2009 Undercu Diam of Arc Spot Nuggo	EPTABLE t_NONE
Reinforcement ACCEPTAGLE est Conducted By BOAO WELLS aboratory Test No.	Per Au S Di. Date of Test 4-1	3 = 18 5 - 08
ne undersigned certify that the statements in this reconcerdance with the requirements of 4.6 of ANSI/AWS D	ord are correct and that the test of 1.3 (welds were prepared and tested in ding Code—Sheet Steel.

Welder or Welding Opera	tor's Name Bio (Oualification Date	4-15-08
Identification No. 82 Welder's Social Security 1	10	Qualification Date	
In Accordance with WPS Welding Process(es)	No. <u>D1.3-601</u> SMAW		omatic, manual, etc.)
Mode of Transfer for GMA	W SHORT CIRCU	アナエル ケ (Short circuiting, spray, glo	obular)
		ACTUAL VARIABLE USED	The state of the s
VA	RIABLE	IN QUAL.	QUALIFICATION RANGE
JOINT Joint Type Backing Material Type		34-41	4.1
Groove Welded From: one side or both sides		ONE SIDE	EITHER
BASE METAL (4.7.1.1) Material Specification Sheet Steel		_A36to_A36	55E /WS to 1.2.1
Supporting Steel Sheet Thickness (4.7.2. Groove Fillet	\mathfrak{v}	D GUAGE	16quage → 3116"
Arc Plug Arc Spot Arc Seam			
COATING(S) Type Thickness		NONE	SUOU
POSITION (4.7.1.5 and 4. Groove Fillet Arc Plug	7.1.6)	34	16,24,36
Arc Spot Arc Seam Progression			
GAS (4.7.1.4)		75% AREON /25%	Co2 75% AR-25%
ELECTRODE (4.7.1.3 and Size Group Designation	4.7.1.4)	.035 2705-6	,035 E765-X
VISUAL EXAMINATION R	ESULTS (4.6)		
Specimen 1 ACCE Appearance ACCE Reinforcement ACCE	PTABLE	Specimen 2 <u>/ CC</u> O/US Undercu Diam of Arc Spot Nugge	
est Conducted By aboratory Test No	RAO WELLS	Per Alus D.K. Date of Test 4-1	5-18 5-68

(year) Authorized By

Company Form A-3

05120

BECKER structural engineers, inc.

OBSERVATION	ON REPORT
Structural Stee	

Date:	July 6, 2009
Time:	7:30am – 9:00 am
Temp:	60's
Weather:	Sunny

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI 2058

Observation Location:

Holdown anchor supports and deck edge configuration at the second floor slab. This review was completed in preparation of the slab placement later this week.

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Bolt Condition				\boxtimes	
Weld Condition				\square	
Anchor Bolts, Nuts, & Washers		\boxtimes			See notes below
Grout/Leveling Plates				\boxtimes	
Fit Up/Plumbness					
Metal Deck Welds	\boxtimes				
Pour Stops		\boxtimes			See notes below
Bracing				\boxtimes	
Additional Items				\boxtimes	
Additional Items				\boxtimes	

Notes:

This visit was conducted to review holdown supports and deck edge details prior to the second floor placement. Holdown support locations were not measured, but were found in the general area expected. It was noted that Section 1/S3.2 was not completed at all necessary areas. At all perimeter locations where steel joists bear on steel beams, Section 1/S3.2 shall be followed. Specifically, a continuous steel angle shall be placed along with HSS 2 1/2 sections between joists. Anchor rods shall be welded to this

05120

$\underbrace{B\ E\ C\ K\ E\ R}_{\text{structural engineers, inc.}}$

angle as specified. This item was discussed with the General Contractor, and areas requiring attention were identified.

Also, at locations where the steel joists span parallel with the steel perimeter beams, a continuous pour stop can be used in lieu of the steel angle, and the anchor rods shall be welded to the top flange of the beam as specified in Section 2/S3.2.

Please contact me with any questions.

Thank you -

Signed: James Fortin, P.E.

BECKER structural engineers, inc.

ORSE	RVATION	J REPO)RT
<u> </u>	IVALIOI	7 1 V L I I V	21 X I
Structu	ral Steel		

Date:	July 27, 2009
Time:	8:30am - 11:00 am
Temp:	70's - 80's
	Clouds, Sun, Humid

Project: Florence	House
Location: Portland,	Maine
Becker Job No: SI2058	
Observation Location:	
Structural Steel Framing at Firs	Floor and Second Floor

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Bolt Condition					
Weld Condition				\boxtimes	See Notes Below
Anchor Bolts, Nuts, & Washers					See Notes Below for areas requiring attention
Grout/Leveling Plates	\boxtimes				
Fit Up/Plumbness	\boxtimes				
Metal Deck Welds				\boxtimes	1
Pour Stops				\boxtimes	
Bracing		\boxtimes			See Notes Below for areas requiring attention
Additional Items					
Additional Items					

Notes:

A final review of the structural steel framing was completed to ensure that framing configurations, connection, and details have been incorporated as shown on the contract documents. Framing appears to be in general conformance with the contract drawings except as noted below. These items will require additional attention by the General Contractor to meet the drawing requirements. Contractor shall contact engineer to schedule follow-up visit once items have been addressed.

- 1. Diagonal struts at brace frame diagonal connections are missing or incorrectly installed. The angle shall extend from the base of the brace gusset plate to the underside of the deck, as shown on Section D/S1.7
- 2. At locations where joist bridging teminates at the concrete wall, the clip angle currently in place shall be bolted to the wall.
- 3. CMU walls at the stairs and elevators shall be attached to all steel beams located directly adjacent to the walls as shown on Section 5/S3.2. (all locations)
- 4. Column/brace anchor bolt attachments have been reviewed at all visible loactions. At Grids 9/N & 11/N, nuts were found missing.
- 5. All perimeter floor beams shall have bottom flange angle braces at 5'-0" on center maximum as identified in Sections 1 & 2 on S3.2.
- 6. GC is requested to submit final reports for field weld inspections.
- 7. At vertical HSS hangers that will support canopy framing, a diagonal angle brace shall be placed at each hanger per SSK-2 and Section 2/S3.1. *Note: Canopy framing is not in place and will be reviewed at a future visit.*
- 8. Anchor bolts not in place at first floor slab along Line Line A & Line 7 per Sections 1 & 2 on Dwg. S3.2. GC shall forward proposed repair to address this item to Engineer for review,
- 9. At second floor framing, W14x22 between Lines 1 & 3 east of Line E, double angle connection to W16 support beam has 3 rows of bolt holes. Bottom row of holes in angles do not have corresponding holes in W16 web. Holes shall be drilled in web for bolt installation.
- 10. At second floor, W16x31 along Line 3 from Line D.2 to E.8, the web stiffener at the gusset plate centerline is not in place. The connection of the W12/W10 beams interfere with installation of a full depth stiffener. Instead, a stiffener shall be placed at each side of the beam web from the beam bottom flange to the underside of the angle connection.

Please contact me with any questions reagrding the information provided above.

Thank you -

Signed: James Fortin, P.E.

BECKER structural engineers, inc.

OBS	ERVA]	TION	REPO)RT	

Date:	August 13, 2009
	8:00am - 11:00 am
Temp:	80's
Weather:	Sunny, Humid

Project:	Florence House	W 5 ***** WALLALI WAY
Location:	Portland, Maine	,),,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,
Becker Job No:	SI2058	

Observation Location	on:
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Structural Steel Framing at First Floor and Second Floor (follow-up visit to review completion of previously documented items)

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Bolt Condition	\boxtimes				THE PROPERTY OF THE PROPERTY OF THE TRANSPORT OF THE TRAN
Weld Condition				\boxtimes	S CONTROL CONT
Anchor Bolts, Nuts, & Washers					
Grout/Leveling Plates				\boxtimes	
Fit Up/Plumbness				\boxtimes	
Metal Deck Welds				\boxtimes	
Pour Stops				\boxtimes	
Bracing					
Additional Items		\boxtimes	\boxtimes		See Notes Below
Additional Items					

Notes:

A follow-up review of the structural steel framing was completed to review completion of previously documented items. The following list was submitted following the initial steel inspection on July 27, 2009. Updates to the items are provided in bold.

1. Diagonal struts at brace frame diagonal connections are missing or incorrectly installed. The angle shall extend from the base of the brace gusset plate to the underside of the deck, as shown on Section D/S1.7

08/13/09: Areas have been reviewed and new angle braced have been installed that extend to the base of the brace frame gusset plate. This item is CLOSED.

2. At locations where joist bridging teminates at the concrete wall, the clip angle currently in place shall be bolted to the wall.

08/13/09: Clip angles have been bolted to the wall. This item is CLOSED.

3. CMU walls at the stairs and elevators shall be attached to all steel beams located directly adjacent to the walls as shown on Section 5/S3.2. (all locations)

08/13/09: Some areas were found to be complete. Item is on-going.

4. Column/brace anchor bolt attachments have been reviewed at all visible loactions. At Grids 9/N & 11/N, nuts were found missing.

08/13/09: Placement of nuts was reviewed prior to concrete slab placement. Nuts were installed. <u>This item is CLOSED.</u>

5. All perimeter floor beams shall have bottom flange angle braces at 5'-0" on center maximum as identified in Sections 1 & 2 on S3.2.

08/13/09: Beam bottom flange braces appear to have been installed. <u>This item is CLOSED.</u>

6. GC is requested to submit final reports for field weld inspections.

08/13/09: Reports have not been submitted.

7. At vertical HSS hangers that will support canopy framing, a diagonal angle brace shall be placed at each hanger per SSK-2 and Section 2/S3.1. *Note: Canopy framing is not in place and will be reviewed at a future visit.*

08/13/09: Canopy framing has not been installed.

8. Anchor bolts not in place at first floor slab along Line Line A & Line 7 per Sections 1 & 2 on Dwg. S3.2. GC shall forward proposed repair to address this item to Engineer for review.

08/13/09: Simpson Titen HD anchors have been installed. This item is CLOSED.

9. At second floor framing, W14x22 between Lines 1 & 3 east of Line E, double angle connection to W16 support beam has 3 rows of bolt holes. Bottom row of holes in angles do not have corresponding holes in W16 web. Holes shall be drilled in web for bolt installation.

08/13/09: Bolts have been added. This item is CLOSED.

10. At the first and second floor, W16 beam along Line 3 from Line D.2 to E.8, the web stiffener at the gusset plate centerline is not in place. The connection of the W12/W10 beams interfere with installation of a full depth stiffener. Instead, a stiffener shall be placed at each side of the beam web from the beam bottom flange to the underside of the angle connection.

08/13/09: This item is not complete. Note that this occurs at both the first and second floor level.

Please contact me with any questions regarding the information provided above.

Thank you -

Signed: James Fortin, P.E.

BECKER structural engineers, inc.

OBSE	RVATIO	N REF	ORT
Structu	ral Steel		5

	the second contract to the second contract to
December 11, 2009	
1:00pm – 2:30 pm	,
20's	:
Sunny	
	December 11, 2009 1:00pm – 2:30 pm 20's Sunny

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI 2058

Observation Location:	Exteri	or Ca	nopy	Framing	g (4 locations)
	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Bolt Condition		\boxtimes			
Weld Condition		\square			
Anchor Bolts, Nuts, & Washers				\boxtimes	
Grout/Leveling Plates					
Fit Up/Plumbness				\boxtimes	
Metal Deck Welds					
Pour Stops		\boxtimes			
Bracing				\boxtimes	
Additional Items		\boxtimes			Only as Noted Below
Additional Items					

Notes:

This visit was limited to review of the four exterior steel framed canopies. Overall, the framing appears to be installed in general conformance with drawing requirements. The following items were noted:

All Canopies:

- 1. Perimeter channel flanges do not align at the mitered corners
- 2. Welds at the mitered corners are not complete. Welds should also be ground smooth for uniform appearance.
- 3. All field weld shall be inspected by an AWS Certified Weld Inspector with a report submitted to Becker Structural Engineers for record.

Canopy along Line N - North:

4. The revised connection at Grid 13/N does not appear to have been completed as directed (instructions provided in November 19 report). The fillet weld at the south side of the plate is not complete, and the full penetration bevel weld at the top of the plate does not appear to have been placed. This welding is necessary for adequate attachment.

From the November 19 report: At the Valley Street canopy, the MC 18 connection to the Second Floor Beam connection plate at Line 13 is not fitting up correctly. The connection plate appears rotated slightly such that the bolt holes do not line up. The item was discussed with the steel erector, and in lieu of fully bolting the connection, three bolt holes will be elongated slightly such that a minimum of (1) bolt in each row will be placed. Then the south side and the top of the MC18 connection plate will be welded to the W14 connection plate. Use a ½" fillet weld along the south side, and a full penetration beveled weld along the top. These field welds require visual inspection from an AWS Certified Weld Inspector, with a report submitted to Becker Structural Engineers for Record.

Canopy along Line N & Line 1:

5. Bolts are missing at five locations connecting the C15 channels located against the exterior brick veneer. In lieu of bolting, the channel has been welded to the tab connection plate. These field welds shall be inspected by an AWS Certified Weld Inspector to ensure adequacy.

Canopy along Line 1 - West:

6. The bolts through the CMU wall are not adequately installed for connection of the tension rod to the CMU. Two bolts have missing nuts, and these bolts were spot welded to the steel plate at the inside face of the wall. This was discussed with Everett Stewart of Ganneston Construction, and will be repaired as follows: Shore the canopy, grind out out plug welds, and remove the nuts and plate at the interior side of the CMU wall. Cut away a minimal section of the wall as necessary to expose the bolts, and install coupler nuts to extend a threaded rod for proper attachment of the plate. Grout around the coupler nuts flush with the inside face of the wall, and replace the plate.

Please contact me with any questions.

Thank you -

Signed: James Fortin, P.E.

BECKER structural engineers, inc

OBSERVATION	N REPORT				Date:	January 13, 2010	1
Structural Steel	7.				Time:	N/A	}
					Temp:	N/A	· :
					Weather:	N/A	
Project:	Florence	House)				•
Location:	Portland,	Maine)				
Becker Job No:	SI 2058				91-10-7-7-7-7-7-7-7-7-7-7-7-7-7-7-7-7-7-7		
Observation Loc	ation: Exter	ior Ca	nopy	Framing –	THIS REPOR	T ISSUED TO PROVIDE	UPDATE
INFORMATION O	N THE PREV	IOUSL	Y LIS	TED ISSU	IES.		÷
INFORMATION O	N THE PREV	iousi	_Y LIS	TED ISSU	JES.		; ;
INFORMATION O	N THE PREV	IOUSL	_Y LIS	STED ISSU	IES.		; ;
INFORMATION O	IN THE PREV				IES.		;
INFORMATION O	7				IES.		
INFORMATION O	7				JES.		:
INFORMATION O	7				IES.		
INFORMATION O	7				IES.		
INFORMATION O	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	JES.	Comments	
Bolt Condition	7				IES.	Comments	
Bolt Condition Weld Condition	□ Satisfactory	Un-Satisfactory		□□ Not Applicable	IES.	Comments	
Bolt Condition Weld Condition Anchor Bolts, Nuts	□ Satisfactory				JES.	Comments	
Bolt Condition Weld Condition Anchor Bolts, Nuts & Washers	Satisfactory				JES.	Comments	
Bolt Condition Weld Condition Anchor Bolts, Nuts & Washers Grout/Leveling Pla	Satisfactory				JES.	Comments	
Bolt Condition Weld Condition Anchor Bolts, Nuts & Washers Grout/Leveling Pla Fit Up/Plumbness	Satisfactory				JES.	Comments	
Bolt Condition Weld Condition Anchor Bolts, Nuts & Washers Grout/Leveling Pla Fit Up/Plumbness Metal Deck Welds Pour Stops	Satisfactory				JES.	Comments	

Notes:

This report is issued to provide updated information. A follow-up visit was not completed. <u>Updated information is provided in **bold**.</u>

All Canopies:

Additional Items

Additional Items

- 1. Perimeter channel flanges do not align at the mitered corners.
- 2. Welds at the mitered corners are not complete. Welds should also be ground smooth for uniform appearance. 01/13/10: It is necessary that these welds are repaired or replaced to provide a continuous weld between the two channel sections. Following placement, the welds shall be ground for a smooth uniform appearance.

Only as Noted Below

3. All field weld shall be inspected by an AWS Certified Weld Inspector with a report submitted to Becker Structural Engineers for record.

Canopy along Line N - North:

4. The revised connection at Grid 13/N does not appear to have been completed as directed (instructions provided in November 19 report). The fillet weld at the south side of the plate is not complete, and the full penetration bevel weld at the top of the plate does not appear to have been placed. This welding is necessary for adequate attachment. 01/13/10: 1) Improved access shall be provided to allow for inspection of the full penetration bevel weld along the top of the plate. This weld is required as specified below. In accordance with project specifications (Section 05120-3.02C.4.b), this weld shall by inspected by non-destructive testing (RT or UT inspection). 2) The skew between the two connection plates does not allow for completion of the 1/4" vertical fillet weld. GC shall document the length of the fillet weld placed, and forward to BSE. 3) Upon further review of the photos taken during the December 11 visit, it was discovered that the metal roof deck is not properly supported adjacent to the MC18 at Line 13. An L2x2x3/16 x 8" long shall be welded to the MC18 at Line 13 to support the deck. Attach per Section 4/S3.1.

From the November 19 report: At the Valley Street canopy, the MC 18 connection to the Second Floor Beam connection plate at Line 13 is not fitting up correctly. The connection plate appears rotated slightly such that the bolt holes do not line up. The item was discussed with the steel erector, and in lieu of fully bolting the connection, three bolt holes will be elongated slightly such that a minimum of (1) bolt in each row will be placed. Then the south side and the top of the MC18 connection plate will be welded to the W14 connection plate. Use a ½" fillet weld along the south side, and a full penetration beveled weld along the top. These field welds require visual inspection from an AWS Certified Weld Inspector, with a report submitted to Becker Structural Engineers for Record.

Canopy along Line N & Line 1:

5. Bolts are missing at five locations connecting the C15 channels located against the exterior brick veneer. In lieu of bolting, the channel has been welded to the tab connection plate. These field welds shall be inspected by an AWS Certified Weld Inspector to ensure adequacy. 01/13/10: At all locations, (1) bolt has been installed. A minimum of a 1/4" fillet weld x 3" long is required between the channel leg and the connection tab plate.

Canopy along Line 1 - West:

6. The bolts through the CMU wall are not adequately installed for connection of the tension rod to the CMU. Two bolts have missing nuts, and these bolts were spot welded to the steel plate at the inside face of the wall. This was discussed with Everett Stewart of Ganneston Construction, and will be repaired as follows: Shore the canopy, grind out out plug welds, and remove the nuts and plate at the interior side of the CMU wall. Cut away a minimal section of the wall as necessary to expose the bolts, and install coupler nuts to extend a threaded rod for proper attachment of the plate. Grout around the coupler nuts flush with the inside face of the wall, and replace the plate. 01/13/10: Per subsequent conversation for Everett Stewart, an alternate repair was completed. A written explanation of the repairs completed shall be submitted to BSE for review and record. A follow-up inspection can be conducted following receipt of the repair documentation.

Please contact me with any questions.

Thank you -

Signed: James Fortin, P.E.

BECKER structural engineers, inc.

OBSERVATION	REPO	ORT				Date:	February 3, 2010	······
Structural Steel	Structural Steel					Time:	9:30 AM – 10:00 AM	
						Temp:	20's	***************************************
						Weather:	Snow Showers	:
Project:	Flore	ence l	House					
Location:	· / · · · · · · · · · · · · · · · · · ·	***************************************	Maine	******	AMERICAN PROPERTY OF THE PROPE			
Becker Job No:			IVIAIIIG					
Decker Job No.	SI 20	บอช						
Observation Loca	ation:	Exteri	ior Ca	nopy	Framing -	- FOLLOW-UP	VISIT	
		T						
		Satisfactory	Un-Satisfactory	Not Completed	Not Applicable		Comments	
Bolt Condition		\boxtimes						
Weld Condition Anchor Bolts, Nuts & Washers	· · · · · · · · · · · · · · · · · · ·						ti de la compatica de prima de la colonia como como a colonia de la colonia de compatica de la colonia de la c	
Grout/Leveling Plat	tes				\boxtimes	there is a second to a second	AND	
Fit Up/Plumbness	Y				$\overline{\boxtimes}$		nomen and announcement of the control of the contro	
Metal Deck Welds Pour Stops		Н					P - 1900000000000000000000000000000000000	
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Notes:

This report is issued to provide updated information following a return visit to review the exterior canopy construction. Updated information is provided in **bold**.

All Canopies:

- 1. Perimeter channel flanges do not align at the mitered corners.
- Welds at the mitered corners are not complete. Welds should also be ground smooth for uniform appearance. 01/13/10: It is necessary that these welds are repaired or replaced to provide a continuous weld between the two channel sections. Following placement, the welds shall be ground for a smooth uniform appearance. 02/04/10: The latest weld inspection report from QAL, dated 1/19/10, stated that "material other than weld material" was used to fill mitered corners. The corners of the canopy need to be fully welded with

weld material. An inspection report is necessary certifying that foreign materials have been removed and the welds are satisfactory.

3. All field weld shall be inspected by an AWS Certified Weld Inspector with a report submitted to Becker Structural Engineers for record.

Canopy along Line N - North:

4. The revised connection at Grid 13/N has been bolted with (5) or the (6) detailed bolts. At the missing bolt, the two plates have been fillet welded with a minimum 6" of 1/4" fillet weld. This condition is adequate as constructed.

It is necessary that the metal roof deck is supported adjacent to the MC18 at Line 13 using an L2x2x3/16 x 8" long welded to the MC18 at Line 13 to support the deck. Attach per Section 4/S3.1.

The welded connection at the mitered joint between the cantilevered channel at Line 13 & the main MC18 canopy channel spanning parallel with the building shall be re-inspected to ensure the weld is complete and acceptable (see Item 2 above). GC shall ensure that weld has been repaired prior to re-inpsection. This connection supports the main channel. In lieu of repairing and re-inspecting this weld, an L2x2x1/4" x 8" long can be placed at the inside of the corner and welded to each channel with 3/16" fillet welds. This welding would require inspection by QAL.

Canopy along Line N & Line 1:

5. Bolts are missing at five locations connecting the C15 channels located against the exterior brick veneer. In lieu of bolting, the channel has been welded to the tab connection plate. These field welds shall be inspected by an AWS Certified Weld Inspector to ensure adequacy. 01/13/10: At all locations, (1) bolt has been installed. A minimum of a 1/4" fillet weld x 3" long is required between the channel leg and the connection tab plate. 02/04/10: QAL inspection report has been provided accepting the weld as identified above. This item is CLOSED.

Canopy along Line 1 - West:

6. The bolts through the CMU wall are not adequately installed for connection of the tension rod to the CMU. Two bolts have missing nuts, and these bolts were spot welded to the steel plate at the inside face of the wall. This was discussed with Everett Stewart of Ganneston Construction, and will be repaired as follows: Shore the canopy, grind out out plug welds, and remove the nuts and plate at the interior side of the CMU wall. Cut away a minimal section of the wall as

necessary to expose the bolts, and install coupler nuts to extend a threaded rod for proper attachment of the plate. Grout around the coupler nuts flush with the inside face of the wall, and replace the plate. 01/13/10: Per subsequent conversation for Everett Stewart, an alternate repair was completed. A written explanation of the repairs completed shall be submitted to BSE for review and record. A follow-up inspection can be conducted following receipt of the repair documentation. 02/04/10: This item has not been reviewed after completion of above instructions.

Please contact me with any questions.

Thank you -

Signed: James Fortin, P.E.

BECKER structural engineers, inc.

OBSERVATION	REPO	DRT				Date:	March 4, 2010	
Structural Steel						Time:	NA	
Structural Steel						Temp:	NA	
						Weather:	NA	
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Project:	Flore	ence l	House	Additional and a series of the				
Location:	Portl	and,	Maine					
Becker Job No:	SI 20			,				
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Additional Items		Ш						

Notes:

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All Canopies:

- 1. Perimeter channel flanges do not align at the mitered corners.
- 2. Welds at the mitered corners are not complete. Welds should also be ground smooth for uniform appearance. 01/13/10: It is necessary that these welds are repaired or replaced to provide a continuous weld between the two channel sections. Following placement, the welds shall be ground for a smooth uniform appearance. 02/04/10: The latest weld inspection report from QAL, dated 1/19/10, stated that "material other than weld material" was used to fill mitered corners. The corners of the canopy need to be fully welded with weld material.

An inspection report is necessary certifying that foreign materials have been removed and the welds are satisfactory. 03/04/10: Repair work has been completed, and inspection report submitted. Item is CLOSED.

3. All field weld shall be inspected by an AWS Certified Weld Inspector with a report submitted to Becker Structural Engineers for record.

Canopy along Line N - North:

4. The revised connection at Grid 13/N has been bolted with (5) or the (6) detailed bolts. At the missing bolt, the two plates have been fillet welded with a minimum 6" of 1/4" fillet weld. This condition is adequate as constructed.

It is necessary that the metal roof deck is supported adjacent to the MC18 at Line 13 using an L2x2x3/16 x 8" long welded to the MC18 at Line 13 to support the deck . Attach per Section 4/S3.1.

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Canopy along Line N & Line 1:

5. Bolts are missing at five locations connecting the C15 channels located against the exterior brick veneer. In lieu of bolting, the channel has been welded to the tab connection plate. These field welds shall be inspected by an AWS Certified Weld Inspector to ensure adequacy. 01/13/10: At all locations, (1) bolt has been installed. A minimum of a 1/4" fillet weld x 3" long is required between the channel leg and the connection tab plate. 02/04/10: QAL inspection report has been provided accepting the weld as identified above. This item is CLOSED.

Canopy along Line 1 - West:

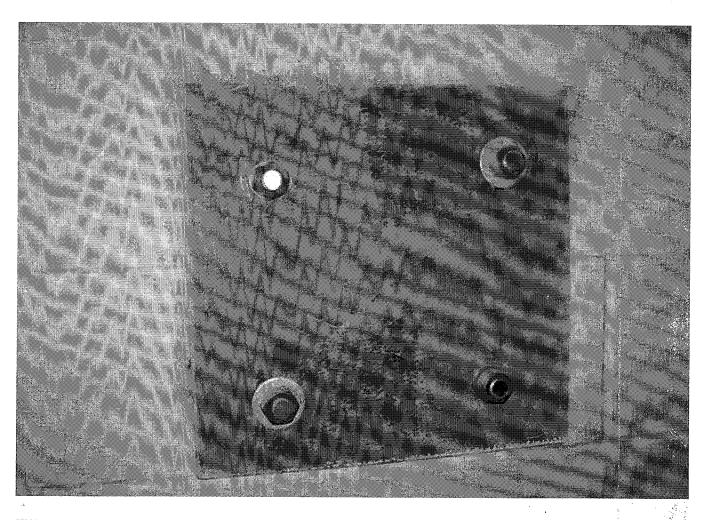
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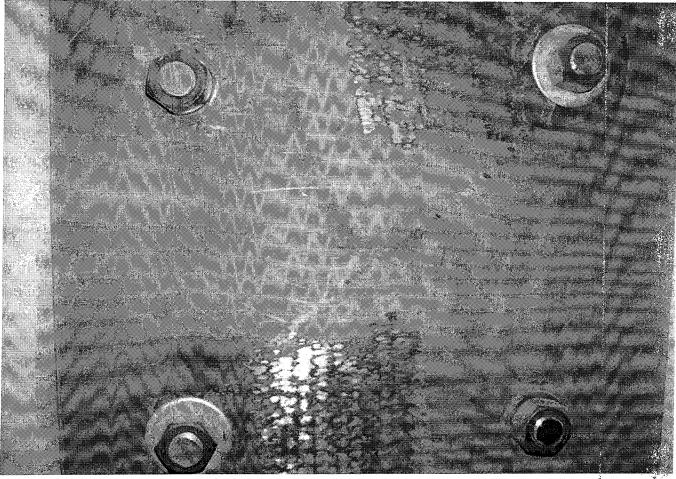
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Please contact me with any questions.

Thank you -

Signed: James Fortin, P.E.





Ganneston Construction Corp PO Box 27, North Belfast Avenue Augusta, ME 04332-0027

FEB 2 2 2010

LETTER OF TRANSMITTAL

Fax: (207) 6	21-8505 321-8508				GCC PROJECT NUM GCC-80677	BER
To:					MONDAY	8:05 AI
29 BLACK	POINT R	N ARCHITECTS COAD MAINE 04074		Attention:	February 22, 2010	PROJECT
WE ARE SE	ENDING	YOU <u>X</u>	ATTACHE	(D	UNDER SI	EPARATE COVER
COPIES	NO.			DESCR	IPTION	
3		COPY OF ENGIN	EERS REPOR	T - FLAT PL	ATE TRUSS REPAIR	
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REMARKS						
IVEID/II/IV						
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						3.



CARPENTER ASSOCIATES

CONSULTING ENGINEERS

February 8, 2010

Mr. Mike Adams
Ganneston Construction Corp.
3025 N. Belfast Avenue
P.O .Box 27
Augusta, ME 04332

Re: Bar Joist Damage

Dear Mike:

Based on the information reviewed, the bar joist repair in question appears to be as designed. The flat plate welded in place should replace the lost strength due to the damage.

Sincerely,

Randy Bragg, P.E

209142

Jenner J Helstrom			•		•• • • • • • • • • • • • • • • • • • • •			177.47
# Test Date Sup Code	Process	Gas	Filler	Metal	Base Metal	Position	Thickness	Expires ·
					73.1	7	13	06/15/09
1 02/05/02 G D1.1	SMAW	n/a	F4		21	A		
2 11/29/06 G B2.1	SMAW	N/A	E316'-16		м8	ALL	LIMITED	06/15/09
Z 11/23/00 0 DEI-								



Jenner J Helstrom Cert # 0203015W

SSN # XXX-XX-3667

1-800-443-9353
Information relating to identification and certification of the bearer of this card may be verified by calling or writing:
CERTIFICATION DEPARTMENT OF THE AMERICAN WELDING SOCIETY
550 N.W. LeJeune Road, Miami, FL 33126



AMERICAN WEL

VALID ONLY IF ACCOMPANIED BY PHOTO ID

This Card is the property of AWS and shall be returned on demand.



IRONWORKER WELDER CERTIFICATION PROGRAM OF NORTH AMERICA



CERTIFIED WELDER

Jenner Helstrom

Membership #: 1368868

EFFECTIVE DATE

EXPIRATION DATE

3/22/2009

3/22/2010

Process: FCAW-S - B1a



	GANNESTON CO	NS JG	STRUCTION CORP. USTA, ME 04332	
X	REVIEWED		REVIEWED WITH NOTES RESUBMISSION REQUESTED	
	REVIEWED WITH NOTES NO RESUBMISSION		NOT ACCEPTABLE	
	CHECK MARK INC	2	TES ACTION TAKEN DATE 6/10/0	1



IRONWORKER WELDER CERTIFICATION PROGRAM OF NORTH AMERICA

CERTIFIED WELDER

Nobert D Page

Membership #: 1256614

2/1/2009

2/1/2010

Process: SMAW - A7r





IRONWORKER WELDER CERTIFICATION PROGRAM OF NORTH AMERICA

CERTIFIED WELDER

Nobert D Page

Membership #: 1256614

EFFECTIVE DATE

EXPIRATION DATE

6/23/2008

6/23/2009

Process: SMAW - A1





IRONWORKER WELDER CERTIFICATION PROGRAM OF NORTH AMERICA



CERTIFIED WELDER

Nobert D Page

Membership #: 1256614

EFFECTIVE DATE

EXPIRATION DATE

4/5/2009 4/5/2010 Process: FCAW-S - B1a





TRONWORKER WELDER CERTIFICATION PROGRAM OF NORTH AMERICA



CERTIFIED WELDER

Nobert D Page

Membership #: 1256614

EFFECTIVE DATE

EXPIRATION DATE

2/1/2009 2/1/2

2/1/2010

Process: SMAW - A7





IRONWORKER WELDER CERTIFICATION PROGRAM OF NORTH AMERICA



CERTIFIED WELDER

Nobert D Page

Membership #: 1256614

EFFECTIVE DATE

EXPIRATION DATE

2/1/2009

2/1/2010

Process: SMAW - A7a



James C Fortin

From:

Paul Becker

Sent:

Thursday, June 18, 2009 12:25 PM

To:

James C Fortin

Subject: Fwd: Florence House 6-16-09

Sent from my iPhone

Begin forwarded message:

From: Darrell Gilman < dgilman@summitenv.com>

Date: June 18, 2009 7:55:57 AM EDT

To: Paul Becker < paul@beckerstructural.com>

Subject: Florence House 6-16-09

June 17, 2009

Summit Geo. Tech.

640 Main Street

Lewiston, Maine 04240

Ref: The Florence House - Project 14194

QAL-09-1011

Attn: Darrel Gilman

Dear Sir,

On 6/17/09, a site visit was made to the Florence House project in Portland. The following items were inspected:

1st Floor Framing – Lines 1-16, A-N

Bolts – In progress			
Bracing connections – In progress	•.		
Joist connections – In progress		÷	Ba 2
Bridging connections – In progress			
Decking welds and fasteners – In progress			
Embed connections – In progress		,	
Note: Beams to embed connections on 1 st floor framing at line N, for relocation.	1.5-5 are at wrong elevatio	n. Need	RFI
All welding which was in progress was IAW AWS D1.1 and appl DWGS.	icable site	f	111
			s f
	Best Regards,		
	Arthur Gallant	e.	

CWI# 90100091

Quality Assurance Labs Inc. NON-DESTRUCTIVE TESTING AND INSPECTION SERVICES NON-DESTRUCTIVE TESTING AND INSPECTION SERVICES SOUTH PORTLAND, MAINE 04106 • TEL: (207) 799-8911 • FAX: (207) 799-7251

AND DESCRIPTION OF THE PROPERTY OF THE PROPERT
ARTERIOR CONTRACTOR OF THE PROPERTY OF THE

INSPECTION REPORT			
CUSTOMER: SUMMIT GEO TECH ENG.	PAGE	l of	1
ADDRESS: AUGUSTA, ME.	.;		-
ATTENTION: DARREL GILMAN			
COPIES:			
PROJECT: FLORENCE HOUSE - PORTLAND, ME.			
OWNER:	· · · · · · · · · · · · · · · · · · ·		
CONTRACTOR: GANNESTON CONSTRUCTION CORP.			
JOB No.: 14194 REPORT No.: QAL-09-1166 P.O. NUMBER: DATES INSPECTED	D: 07 - 06	- 09	;
REMARKS	· . · .		÷ .
			ė
>>>> SITE VISIT TO PERFORM VISUAL INSPECTION OF STRUCTURAL STEEL CON	NECTIONS	: GRII	LINES
I-7, A-N ALL ELEVATIONS.			
> REF. PREVIOUS FIELD REPORT DATED 06-29-09.		•	
A) ALL DECKING ATTACHMENTS NOW COMPLETE.	. *		
B) SECOND LEVEL JOIST AND BRIDGING CONNECTIONS NOW COMPLETE. C) FIRST LEVEL PLAN SHOWS (1) LOWER HSS WIND BRACE WELD MISSING AT	LOCATIO	N 1-G	AS
MARKED WITH SOAP STONE. ALL REMAINING WIND BRACE CONNECTIONS CO	OMPLETE	O FOR	ALL
LEVELS.			
> LOCATION 13-N (2ND) LEVEL FRAMING PLAN SHOWS COLUMN TO W16X26 SH BOTTOM FLANGE WITH (2) MISSING BOLTS. ALL BEAM WEB BOLTS COMPLET	IEAR TAI	з то в	EAM
> SEVERAL COLUMN BASE ANCHOR BOLTS REQUIRE FINAL TORQUE.			
> CANOPYS IN PROGRESS.			
COMPLETED ITEMS COMPLY WITH SITE DOCUMENTS AND AWS D1.1, D1.3 REQUIR ACCEPTANCE.	EMENTS	FOR V	ISUAL
END ITEMS ////			1
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	MICHA	EL W. DRE	W
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ADDITIONAL INFORMATION - SEE ATTACHED: SKETCH(ES) SUPPLEMENTARY SHEET(S) NOT REF	CERTIFICAT	VIDE	DATE
SIGNATURES		TENET W	
INSPECTOR M. Drew CWI # 99050211 weeken how	ASNT	II 07	07 09
SUPERVISOR	<u> </u>	(FORM Q7	23 BEV (1)

Quality Assurance Labs Inc. NON-DESTRUCTIVE TESTING AND INSPECTION SERVICES 80 PLEASANT AVENUE • SOUTH PORTLAND, MAINE 04106 • TEL: (207) 799-8911 • FAX: (207) 799-7251

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(FORM Q703 REV 0)

	INSPECTION REPORT				
CUSTOMER:	Summit Environmental Eng.	PAGE	1	OF	1
ADDRESS:	Augusta, ME				
ATTENTION:	Darrel Gilman				
COPIES:	file			*******	
PROJECT:	Florence House - Portland, ME.				
OWNER:					
CONTRACTOR:	Ganneston Construction Corp.				
JOB No.: 1419	P4 REPORT No.: QAL-09-2174 P. O. NUMBER: DATES INSPECTE	D: 12/16/	2009		Was de la constant de
	REMARKS	enter:		Spiritely.	
Performed Vi	sual Inspection at two (2) locations.			E-SE-OCCIONAL	
At Canopy ald					
not be verified	nnection plate had 12" of fillet weld varying in size from 1/4" to 0" along vertical side. To along vertical side. To along vertical side. To along vertical side.	ne weld at	top qı	plate	could
	- ,				
	ong Line N & Line 1: ons were missing bolts. In lieu of bolts, the shear tab was welded to channel.				
	and was welded to chamier.		,		
Acceptability	can not be determined without the corrective actions being approved by EOR.				. W. A W. B
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भिकार दिस्	SIGNATURES	CERTIFICA	TION .	М	DATE D Y
INSPECTOR]	R. H. Parechanian, Jr. CWI# 90100111	ASNT	=	01	12 10
SUPERVISOR					

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Quality Assurance Labs Inc. NON-DESTRUCTIVE TESTING AND INSPECTION SERVICES SO PLEASANT AVENUE SOLITH ROOT! AND MAINE PARCE.

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ADDRESS: AUGUSTA, ME ATTENTION Darrel Gilman COPPLE: File PROCECT: Florence House - Portland, MIL DWNKE: CONTRACTON: Gameston Construction Corp. 108 No.: 14194 REPORT No. (OAL-10-010) REPORTES: Performed Visual Inspection in accordance with AWS D1.1 2008 and AISC 360-05. Canupy along Line N & Line I: Welds were found to be acceptable. Canopy along Line N - North: The required full penetration bevel weld at the top were the two plates course tagether is not possible to perform because of configuration. As reported on QAL-09-2174, the fillet weld along skewed connection plate is 12" long and varies in size from 1/4" to 0". These conditions do not meet the requirements of Becker Structural Engineers, Inc. letter dated January 13, 2010. Welds at mitered corners: Mitered corners were not ground to a smooth uniform appearence. Material other than weld metal was used to fill mitered corners and ground. Unable to determine how much weld is at each corner. Damaged Joist: Damaged joist were inspected and found to be acceptable per sketch provided by Ganneston.		INSPECTION REPORT			77
ADDRESS: Augusta, ME ATTENTION: Darrel Gilman COPIES: file PROJECT: Florence House - Portland, MLL CONTRACTOR: Gameston Construction Corp. JOB No.: 14194 REPORT No.: QAL-10-01(0) P. O. NUMBER: DATES INSPECTED: 01/19/2010 TEMARKS Performed Visual Inspection in accordance with AWS D1.1 2008 and AISC 360-05. Canopy along Line N & Line 1: Welds were found to be acceptable. Canopy along Line N - North: The required full penetration bevel weld at the top were the two plates come together is not possible to perform because of configuration. As reported on QAL-09-2174, the fillet weld along skewed connection plate is 12" long and varies in size from 1/4" to 0". These conditions do not meet the requirements of Becker Structural Engineers, Inc. letter dated January 13, 2010. Welds at mitered corners: Mitered corners were not ground to a smooth uniform appearence. Material other than weld metal was used to fill mitered corners and ground. Unable to determine how much weld is at each corner. Damaged Joist: Damaged joist were inspected and found to be acceptable per sketch provided by Ganneston.	CUSTOMER:	Summit Environmental Eng.	PAGE 1	OF	1
PROJECT: Florence House - Portland, M12 OWNER: CONTRACTOR: Garmeston Construction Corp. HOB No. 14194 REPORT No. QAL-10-0100 P. O. NUMBER: DATES INSPECTED: 01/19/2010 FLORENCE STREET STRE	ADDRESS;	Augusta, ME			•
PROJECT: Florence House - Portland, M13 OWNER: CONTRACTOR: Gameston Construction Corp. JOB No.: 14194 REPORT No.: QAL-10-0100 P. O. NUMBER: DATES INSPECTED: 01/19/2010 Performed Visual Inspection in accordance with AWS D1.1 2008 and AISC 360-05. Canopy along Line N & Line 1: Welds were found to be acceptable. Canopy along Line N - North: The required full penetration bevel weld at the top were the two plates come together is not possible to perform because of configuration. As reported on QAL-09-2174, the fillet weld along skewed connection plate is 12" long and varies in size from 1/4" to 0". These conditions do not meet the requirements of Becker Structural Engineers, Inc. letter dated January 13, 2010. Welds at mitered corners: Mitered corners were not ground to a smooth uniform appearence. Material other than weld metal was used to fill mitered corners and ground. Unable to determine how much weld is at each corner. Damaged Joist: Damaged joist were inspected and found to be acceptable per sketch provided by Ganneston.	ATTENTION:	Darrel Gilman			
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FAA REPAIR STATION NUMBER RX5R187N METHOD(S), PROCESS(ES), PROCEDURE(S) MERCURY FREE

ADDITIONAL INFORMATION - SKE ATTACHED:	SKETCH(RS) SUPPL	MANUNTARY SUPETION	NOT REPORTS	VIDE	3
WEST TOTAL T	SIGNATURES)		CERTIFICATIO	N M	DATE D Y
INSPECTOR R. H. Parechanian, Jr.	CWI# 9010011	and the	ASNT	11 01	28 10
SUPERVISOR		\sim			J 1

(FORM 0703 REV 0)

Quality Assurance Labs Inc. NON-DESTRUCTIVE TESTING AND INSPECTION SERVICES 80 PLEASANT AVENUE • SOUTH PORTLAND, MAINE 04106 • TEL: (207) 799-8911 • FAX: (207) 799-7251

(FORM Q703 REV 0)

	IN	SPECTION REPO	Ri		
CUSTOMER: Sur	nmit Environmental Eng.			PAGE 1	of 1
Address: Aug	gusta, ME				
ATTENTION: Dat	rell Gilman				
COPIES: file					3
PROJECT: Flo	rence House - Portland, ME				Å gr
OWNER:					
contractor: Gai	neston Construction Corp.				
JOB No.: 14194	REPORT No.: QAL-10-0264	P. O. NUMBER:		n: 02/17/2010	A .
JOB NO.	REPORT NO. OFME-TO-OZO-	REMARKS	DATES INSPECTE	D: UZ/17/2010	U
First issue is the insalong the toe of the The welds on the very The weld along the quality. Second issue is the insalong the properties of the properties of the point for at least 3". The "liquid metal" very along the properties of the point for at least 3".	are three (3) issues. stallation of an angle to suppor angle and a fillet weld along the artical are rejected for poor quatoe has 4" of 3/16" fillet weld installation of an angle at the of the amount of weld a "liquid weld" until weld metal. The other flanges revealed sing stallation of an angle at the of the amount of weld and the other flanges revealed sing stallation of the amount of weld and the other flanges revealed sing stallation of an angle at the other flanges revealed stallation of an angle at the other flanges revealed stallation of an angle at the other flanges revealed stall	ality. which is acceptable. The balance putside corner. The welding and in at the mitered corners of channels I was found. The first flange insper	e of weld, approximanstallation are acceptor. The only method to ected revealed there	itely 4", is reject table. to determine this was no weld m	s was to
Canopy along the m	an anemarica.				e e e e e e e e e e e e e e e e e e e
The one issue on thi	s canopy was to determine the	amount of weld at the mitered coint and was similar to the Valley S	orners of channels. T Street canopy.	he inspection r	evealed that
A verbal report was adequate structurally	ited cracks forming in the "liq given to James Fortin by phon and would be acceptable. ed that the mitered flanges wor	uid metal" which could be seen for ne. It was determined at this time uld require repair.	rom ground level. that the 4" of weld o	n the deck angl	le support was
		PAIR STATION NUMBER RX: CESS(ES),PROCEDURE(S) N			
ADDITIONAL INFORMATIO	N-SEE ATTACHED: SKETCH	(es) Supplementary sties:	T(\$) NDT REP	ORTS	VIDEO
	SIGNA	ATURES /) / /		CERTIFICATION LEVEL	
INSPECTOR R. H. F	Parechanian, Jr. CWI# 901001	it to buff foreban	104)	ASNT II	02 19 10

Quality Assurance Labs Inc. NON-DESTRUCTIVE TESTING AND INSPECTION SERVICES NON-DESTRUCTIVE TESTING AND INSPECTION SERVICES TEL: (207) 799-8911 • FAX: (207) 799-7251

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	INSPECTION REPORT				i	
CUSTOMER:	Summit Environmental Eng.		PAGE	1	OF	1
ADDRESS:	Augusta, ME		·		7	<u> </u>
ATTENTION:	Darrel Gilman				:	
COPIES:	file				:	
PROJECT:	Florence House - Portland, ME					
OWNER:						
CONTRACTOR:	Ganneston Construction Corp.					
JOB No.: 1419	4 REPORT No.: QAL-10-0345 P. O. NUMBER: DATES INS	SPECTED	. 03/03	3/201	; 0	*
	REMARKS				,	
Inspection was All four canop	sual Inspection in accordance with AWS D1.1 2008 and AISC 360-05. It is performed on a total of four (4) canopies. It is now have angles welded to channel webs at the inside corners. Angles are weld It is now have a minimum 3/16" fillet weld with acceptable quality.	led at to	es of ang	gles fo	or a le	ength of
All mitered fla	nges have been cleaned and welded. Welds have been ground reasonably smooth.	Welds a	re of acc	ceptal	ble au	ıalitv.
All mitered we	bs have been cleaned and additional weld metal deposited on the outside. Welds he are of acceptable quality.					
At both flange	and web miters, filler has been used to fill minor irregularities. Filler was ground t	o a smo	oth unife	Drm a	рреаг	ance.
	FAA REPAIR STATION NUMBER RX5R187N METHOD(S), PROCESS(ES), PROCEDURE(S) MERCURY FR MATION- SEE ATTACHED: SKETCH(ES) SUPPLEMENTARY SHEET(S) N SIGNATURES H. Parechanian, Jr. CWI# 90100111	DT REPOR	TS CERTIFICA		M 03	DATE P V 03 10
PERVISOR	0 1]		İ	1	1



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PROJECT FLORENCE HOUSE

STRUCTURAL STEEL

☐ RECEIVED DATE: 7-13-09 ☐ NOT RECEIVED

BOLTS

☑RECEIVED DATE: 7-13-09 ☐NOT RECEIVED

WELD FILLER

☑RECEIVED DATE: ☑NOT RECEIVED

ITEMS ABOVE MARKED "RECEIVED" HAVE NOT BEEN INCLUDED IN THIS REPORT DUE TO THE LARGE VOLUME. HARD COPIES ARE AVAILABLE UPON REQUEST.

SPECIAL INSPECTOR: James Fortin, P.E.

DATE: 3-04-10

EXHIBIT B

06100 Wood Construction

Project: Florence House Date Prepared: September 2, 2008

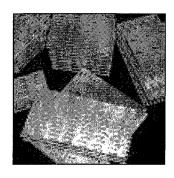
Structural Schedule of Special Inspection Services FABRICATION AND IMPLEMENTATION PROCEDURES – WOOD TRUSSES

VERIFICATION AND INSPECTION IBC Section 1704.2	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS	AGENT	AGENT QUALIFICATION	TASK COMPLETED
Fabrications Procedures: Review of fabricator's written procedural and quality control manuals and periodic auditing of fabrication practices by an approved special inspection agency. At the completion of fabrication, the approved fabricator shall submit a certificate of compliance to the building code official stating that the work was performed in accordance with the approved construction documents. -OR- 2. TPI Inspection Program: Fabricator shall participate in the TPI Quality Assurance Inspection Program, and maintain a copy of the Quality Assurance Procedures Manual, QAP-90. Submit copy of certificate. All trusses shall bear the TPI Registered Mark.	Y	S	Fabricator shall submit one of the two qualifications	SII	PE/SE or EIT	SEE COUT.
3. At completion of fabrication, the approved fabricator shall submit a certificate of compliance to the building code official stating that the work was performed in accordance with the approved construction documents	Y	S	IBC 1704.2.2	SII	PE/SE or EIT	SHE CETTER



TPI's Mission:

To maintain the truss industry on a sound engineering basis.





Quality Assurance Inspection Program: Wood Truss Licensees

Select a state



CANADA

Adam Lumber, Inc. TPI Plant No. 793 Waterloo, QC (450) 539-1858

Clyvanor LTEE TPI Plant No. 410 St.Goeges, Beauce, QC (418) 228-7690

Maple Ridge Truss TPI Plant No. 653 Pitt Meadows, BC (604) 465-9176 Pyramid Truss TPI Plant No. 475 Abbotsford, BC (604) 854-6626

Solutions Prefab American Structures, Inc. TPI Plant No. 774 Thetford Mines, QC (418) 423-3377

Structure DLD, LTEE TPI Plant No. 633 LaGuadeloupe Beauce, QC (418) 459-6001

Structures RBRs TPI Plant No. 930 Saints-Anges, QC (418) 253-5454 Structures St. Joseph LTD TPI Plant No. 590 St-Joseph De Beauce, QC (418) 397-5712 WWW.STRUCTURESST-JOSEPH.COM

Viceroy Homes Limited TPI Plant No. 616 Richmond, BC (604) 274-8700

Timberfield Roof tryss TPI Plant No. 903 London, ON (519) 659 2711



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January 13, 2010

MiTek Canada, Inc. 100 Industrial Road Bradford, ON, Canada L3Z 3G7 Phone 905/952-2900 Toll Free 800/268-3434 Fax 905/952-2901

James Fortin, P.E.

Ref #: **B36792**

75 York Street Portland, ME 04101

Dear James,

RE: Florence House

I am writing in regard to the above building to comment on the capacities of the trusses as per your email of Jan 4, 2010:

- 1. The roof trusses on the entire roof truss system were inspected by Stéphane Blanchette, P.Eng, and reported to MiTek Canada Inc for analysis and repair.
- 2. All trusses found to be damaged; including the 22 trusses marked with orange spray paint by Ganneston's subcontractor, were reviewed and evaluated.
- The evaluation resulted in the need to reinforce 14 trusses with a scab truss nailed to the existing truss. The repair instructions were prepared and sealed by Ross Lee, PE on behalf of MiTek Canada Inc.
- 4. The level of damage of the remaining trusses was acceptable because the remaining bearing length with flush plate alone has sufficient bearing capacity. Therefore, these trusses have sufficient capacity to support the design loads.
- 5. Assuming all other roof parameters including and not limited to walls, foundation and roof bracing system are properly designed and installed, the entire as-built roof truss system is adequate to support the design loads.

I trust the above is as per your request but if you should have any questions please do not hesitate to contact me.

Yours truly,

MiTek Canada Inc.

Robert Baynit, P.Eng., PE Director of Engineering.

Project: Florence House Date Prepared: September 2, 2008

Structural Schedule of Special Inspections WOOD CONSTRUCTION

VERIFICATION AND INSPECTION IBC Section 1704.6	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS	AGENT	AGENT QUALIFICATION	TASK COMPLETED
1. Fabrication of high-load diaphragms						
Verify wood structural panel sheathing for grade and thickness	Y	P	IBC 1704.6	SII	PE/SE or EIT	T.F
b. Verify the nominal size of framing members at adjoining panel edges	Y	Р	IBC 1704.6	SII	PE/SE or EIT	√1F
b. Verify the nail or staple diameter and length	Y	P	IBC 1704.6	SII	PE/SE or EIT	Ju
b. Verify the number of fastener lines	Y	P	IBC 1704.6	SII	PE/SE or EIT	JIF
b. Verify the spacing between fasteners in each line and at edge margins	Y	Р	IBC 1704.6	SII	PE/SE or EIT	JU
2. Load Tests for Joist Hangers: Provide evidence of manufacturer's load test in accordance with ASTM D1761 including the vertical load bearing capacity, torsional moment capacity, and deflection characteristics when there is no calculated procedure recognized by the code.	Y	S	IBC 1715 [submit ICBO reports]	SII	PE/SE or EIT	NA



			FLORENCE HOUSE	_
			FINAL REPORT - March 24, 2010	
1	1 st floor wall framing	07/27/09	For the in-fill wall panels at the first floor level along Line 1, from Grid C to the site retaining wall, it is acceptable to replace the LVL wall studs with (2) 2x6 at 24" o.c. since these walls do not serve as a backing for brick veneer at these locations. 08/13/09: Wall studs have been constructed.	CLOSED
2	2 nd floor wall framing	07/27/09	Due to inconsistencies in the levelness of the 2 nd floor concrete slab on deck, gaps exist between the bottom plate of the wall panel and the PT sill plate bolted to the slab. At all locations where gaps exist, place full width by full depth hardwood shims below all vertical studs. Contractor has stated that shims will be cut to fit the various gap widths. Apply construction adhesive to each side of each shim prior to placing to provide positive connection between the plates. 08/24/09: It appears that shims have been installed as necessary, and joints have been sealed.	CLOSED
3	2 nd & 3 rd floor wall framing	07/27/09	At the 5 ½" x 7" LVL supporting the 3H1 headers (see S1.4, 2 locations), it is acceptable to bear the LVL on (2) PT SYP #2 sill plates fastened to the concrete slab with (2) 5/8" diameter x 2 ¾" concrete embed Simpson Titen HD screws (countersink screws into top-most plate). 08/13/09: Plate and bolts have been placed, but LVL is not in place. 08/24/09: LVL & wood stud posts have been installed.	CLOSED
4	1 st floor wall framing	07/31/09	At the first floor window openings along Line A,N,7 & 16, the top of the R.O. is at 11'-11" above the first floor slab. With a floor-to-floor height of 14'-1", a clear distance of between ½" to 8 ½" remains between the bottom of the steel beam and the top of the window header. At these locations, it is not necessary to install cripple studs above the header, but the header shall be attached to the jambs using the Simpson clips at the top and bottom of the header (as specified). If the header is at least 1" into the light-gauge track, the top Simpson angles can be omitted. If top angles cannot be installed due to interference with the metal track, a 4" long max section of the inside flange of the top track can be cut to fit a palm nailer for fastening the hanger to the jamb. At cut locations, fasten a 16" long Simpson CMST-12 ga strap (or equivalent) centered across the cut and screwed to the track flange. Note: When installing plywood, ensure that joints do not occur at the metal track. Sheathing fastener spacing shall follow drawing requirements. 08/13/09: Headers and clips have been installed without needing to cut the metal track.	CLOSED
5	1 st floor wall framing	07/31/09	Along Line F, the jamb at Door 126 interferes with the HSS hangers for the canopy. At this location, increase the jamb at adjacent Window F to (6) 1 ¾" x 5 ½" LVL's, and extend the door header to this jamb. Install (2) LVL's for the door jamb extending from the slab to the underside of the header. Attach with Simpson LS50 angles, each side of jamb. 08/13/09: Item has been completed as described above.	CLOSED
6	2 nd & 3 rd floor wall framing	07/31/09	At all shear wall locations, attach the sheathing to the PT bottom plates with galvanized fasteners. 08/24/09: Fastening not complete at all areas. Confirmation is necessary that fasteners used are galvanized. Review of exterior wall sheathing attachments not yet completed. 09/25/09: Item was reviewed with LocBid during inspection of exterior shear wall plywood fastening.	CLOSED

BECKE R structural engineers, inc.

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7	2 nd & 3 rd floor	07/31/09	For floor truss strong-back attachment at wood stud walls, it is acceptable to replace the Simpson A34 angles specified on Section	CLOSED
	framing		2/S1.4 with a vertical 2x6 stud placed against the strong-back.	
			Fasten all members together.	
			8/24/09: Strong-backs are being installed as specified above.	
8	2 nd floor	07/31/09	At SW-5 locations, the Titen HD anchors are not installed to attach	CLOSED
	wall		the wall panel to the concrete slab.	
	framing		08/24/09: Titen HD anchors have been installed.	
9	2 nd floor	07/31/09	At SW-4 locations, the 5/8" through bolts are not installed to attach	CLOSED
	wall		the wall panel to the steel framing.	
	framing		08/24/09: Correct fastening was not completed as these shear	
			walls. Titen HD anchors were placed in lieu of through bolts. Areas	
			shall be corrected to meet drawing requirements.	
			09/25/09: Work is not complete. One area could not be reviewed	
			from below due to sheetrock installation.	
			10/19/09: It appears that through bolts have been placed as	
	ord a	07/04/00	specified. Bolts were noted in all areas reviewed.	0.00==
10	3 rd floor	07/31/09	At LVL attachment between third floor trusses (along Line 10), the	CLOSED
	framing		Simpson hanger flange nailing is incomplete.	
			08/24/09: Top flange hanger used, and fastening appears	
	and a ord	07/04/00	complete.	01 0055
11	2 nd & 3 rd	07/31/09	At various locations, floor trusses do not line up with wall studs.	CLOSED
	floor wall		Additional studs will be added as necessary.	
	framing		09/02/09: Item occurs at areas of the third floor also.	
			09/25/09: Studs were added where necessary at all areas	
12	2 nd & 3 rd	07/31/09	observed.	CLOSED
12	2 & 3 floor wall	07/31/09	At joints between panels, vertical studs shall be in contact and shall be attached together with (2) 10d pails at 6" a.a.	CLOSED
			be attached together with (2) 10d nails at 6" o.c. 09/25/09: Fasteners were found at areas observed.	
13	framing 3 rd floor	07/31/09	Between Lines G & H, drawing S1.4 specifies floor trusses to be	CLOSED
13	framing	07/31/09	aligned at edges of floor opening. GC shall confirm truss layout is	CLOSED
	Iraning		adequate to suit floor opening location.	
			09/25/09: GC shall coordinate as necessary.	
14	2 nd floor	07/31/09	At numerous locations, the shear wall holdown bolt (per Detail	CLOSED
17	wall	01701700	B/S4.3) does not extend above the wall bottom plates. GC shall	GLOGED
	framing		confirm locations, and submit recommendation for repair.	
	l raming		09/25/09: Threaded rods have been extended using coupler nuts,	
	•		and holdowns are installed.	
15	3 rd floor	07/31/09	The CMU wall construction at the third floor cantilever bond beam	CLOSED
. •	355.		(Section 9/S3.2) has un-mortared joints.	
			09/25/09: Work is on-going but not yet completed.	
			10/19/09: Joints have been filled in.	
16	1 st floor	08/13/09	At the first floor in-fill wall framing along Line F, the jamb attachment	CLOSED
	wall		at Door 126 is missing a Simpson angle at the base of the jamb.	
	framing			
	·	-		

$\underbrace{\frac{B\ E\ C\ K\ E\ R}{\text{structural engineers, inc.}}}_{\text{structural engineers, inc.}}$

17	3 rd floor & Roof Framing	08/24/09	PT ledgers against CMU walls that support floor and roof framing are not bolted to CMU wall as specified (see section 9/S4.2 & 4/S4& 4/S4.1). 09/25/09: Ledgers were found not bolted to the CMU walls as specified. ALL areas shall be reviewed when completed and prior to sheetrock installation. 10/19/09: PT ledgers were not visible for review during this visit due to installation of sheetrock. GC has stated that ledgers were bolted as specified. 3/24/10: Sheetrock was removed at one location at each level to confirm installation of anchors. Simpson Titen HD anchors were visible and located at spacing specified.	CLOSED
18	Roof Truss System	08/24/09	Configuration of Roof Trusses at Exterior Wall bearing end do not accommodate the 2x ribbon per Section 5/S4.1. An alternate detail to provide adequate stability of the truss shall be provided by the truss manufacturer. Also, Section 5/S4.1 will be revised to show alternate sheathing framing configuration and sheathing fastening. 09/25/09: Detail was observed from the third floor level (looking up). ** All fastening identified in CSKS-18 could not be reviewed due to roofing installation.	CLOSED
19	3 rd floor framing	08/24/09	The 3B2 & 3H1 headers are pocketed into the CMU stair walls. At 3B2, the CMU wall does not line up with the header, and (1) LVL is not properly bearing on the wall. Contractor shall review area to ensure proper layout, and header bearing will need to be improved. Additional instruction shall be provided by Engineer to address this issue. 09/25/09: Fasten a 2x8 x 8" long PT ledger into solid grouted CMU located directly below the unsupported section of the header (ensure contact between ledger and header). Fasten the ledge to the CMU wall with (2) 5/8" x 6 5/8" embed adhesive anchors, centrally located on the plate. 10/19/09: Work was completed as instructed.	CLOSED
20	3 rd floor framing	08/24/09	At North side of Stair 1, the PT ledger against the CMU wall is not in place for floor sheathing support. 10/19/09: Work is not yet completed. Same condition exists at south side of Stair 2. 03/24/10: Areas were not specifically reviewed, but adjacent areas were reviewed and noted to be constructed per drawing requirements.	CLOSED
21	3 rd floor framing	08/24/09	Truss strong-backs are not attached at ends against CMU walls. 09/25/09: Areas were observed where strong-backs need to extend to the CMU wall for attachment. Strong-backs shall be positively attached to structure (ie. walls) at all ends. 10/19/09: Strong-backs have been attached as specified.	CLOSED
22	3 rd floor framing	08/24/09	Truss strong-back missing at north side of stair 1 09/25/09: Strong-backs are in place as specified	CLOSED
23	2 nd floor wall framing	08/24/09	Titen HD anchors are not installed at "recessed" wall locations per Section 7/S3.2. 09/25/09: Titen anchors were observed where specified.	CLOSED
24	2 nd floor wall framing	08/24/09	At exterior wall locations, sill anchors are required at 4'-0" on center. Contractor shall review areas, and add anchors as necessary. At missing locations, install Simpson Titen HD anchors. 09/25/09: Anchors were found as specified at all areas observed.	CLOSED



25	2 rd & 3 rd floor wall framing	08/24/09	At the exterior face of the CMU walls, it has been determined by the General Contractor that sufficient space exists to replace the wood strapping with 2x4 studs. The framing shall be located against the CMU wall and shall create a flush sheathing condition with the wood stud walls on either side. Attach 2x4 studs to the 2x6 bearing wall studs on either side of the CMU wall using 10d nails at 6" on center. Additional instruction on constructing the 2x4 walls and attaching to the CMU wall will be provided by the Engineer shortly. 09/25/09: Work not completed along the Valley St. side of Stair 2 per	CLOSED
			CSKS-17 10/19/09: 2x4 wall has been constructed in front of CMU wall. Most of the wall is not accessible, but at one side, the attachment of this wall to the 2x6 bearing wall was observed, and follows CSKS-17.	
26	Shear wall holdowns	09/02/09	The shear wall holdowns between the 2 nd and 3 rd floor wall panels are not in place per Det. C/S4.3. 09/25/09: Holdowns were found installed at all accessible locations. Photos were provided by the GC to confirm areas not accessible during the visit, as the top of the second floor wall panels was sheetrocked at all holdown locations, and installation of the holdown could not be verified	CLOSED
27	3 rd floor header post	09/02/09	The (5) 2x8 wood post that supports the 3H1 corridor header at the south side of the building is not in place. 09/25/09: Post is in place as specified	CLOSED
28	2 nd & 3 rd floor wall framing	09/02/09	Built-up wood posts shall be fastened with (2) 10d nails at 6" o.c. as specified on Dwg. S1.0 09/25/09: Fastening of field-constructed built-up posts was found adequate at all locations reviewed.	CLOSED
29	Roof Framing	09/02/09	The RB1 header is pocketed into the CMU at Stair 1. The header does not line up adequately with the CMU wall to provide proper bearing. Contractor shall review area to ensure proper layout, and header bearing will need to be improved. Additional instruction shall be provided by Engineer to address this issue. 09/25/09: Fasten a 2x8 x 8" long PT ledger into solid grouted CMU located directly below the unsupported section of the header (ensure contact between ledger and header). Fasten the ledge to the CMU wall with (2) 5/8" x 6 5/8" embed adhesive anchors, centrally located on the plate. 10/19/09: Work was completed as instructed.	CLOSED
30	3 rd floor - Shear Wall Plate Attachment	09/02/09	Additional fastening is necessary for the shear wall bottom plate attachments as specified in the Shear Wall Schedule on Dwg. S4.3 09/15/09: Fastening appears to be in general conformance with drawing requirements.	CLOSED
31	Roof Framing – Strong- backs	09/02/09	Truss bracing strong-backs have not yet been installed at 10'-0" on center as required. 09/25/09: Strong-backs appear to have been installed as necessary.	CLOSED

$\underbrace{B\ E\ C\ K\ E\ R}_{\text{structural engineers, inc.}}$

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32	Stair 2 – CMU wall	09/02/09	A wood stud wall is constructed above the top of the CMU wall at Stair 2 as shown in Section 10/S4.2. A roof joist has been located	CLOSED
	bracing		within 6" of the CMU wall, creating an interference with placement of	
İ	bracing		the 2x4 brace diagonal. Instead, place 2x6 x 24" long blocking tight	
			between the wall studs and the top & bottom of the truss, and install	
			both the diagonal and horizontal brace in the next truss space away	
			from the wall. Ensure roof sheathing is attached to the horizontal	
			brace as specified.	
	–	00/00/00	09/25/09: Work has been completed as specified.	01 0055
33	Roof	09/02/09	Numerous locations exist where the bearing seat at the corridor end	CLOSED
	Trusses		of the truss is cracked and split. At some locations, the seat has	
			completely failed, with a section of the bearing seat missing. GC	
			has contacted the truss manufacturer's engineer to review the in-	
			place trusses, and submit engineered repairs for all damaged	
			trusses. All repairs shall be sealed by a Professional Engineer	
			licensed in the State of Maine. Following completion of all repairs,	
			truss manufacturer's engineer shall review all work, and submit a	
			letter to the Owner and Engineer of Record certifying that the roof	
			truss system is acceptable and in accordance with all project	
			requirements.	
			09/25/09: Repair documentation is required for review.	
			10/19/09: PE stamped certification has not yet been received.	
		_	01/13/10: Certification letter provided.	
34	Roof	09/08/09	At locations where roof joists span parallel with exterior walls, and	CLOSED
	Framing		the roof joist slopes while the wall is kept level, a 2x6 ledger has	
			been used to support the edge of the roof plywood. This ledger has	
			not been detailed on the structural drawings (Section 6/S4.1 was	
			specified in these areas, which considers a sloping wall top plate).	
			Ledger shall be attached to each vertical wall stud with (4) 10d nails.	
			Also, full-depth 2x6 blocking shall be placed between studs behind	
			the ledger. Fasten blocking to the vertical studs, and fasten the	
			ledger and the wall sheathing to the blocking with 10d nails at 4" o.c.	
			09/25/09: Work has not been completed.	
			10/19/09: GC has stated that the blocking and attachment details	
			specified above have been completed, but the blocking cannot be	
			reviewed due to sheetrock installation. GC stated that photos are	
			available to certify the work was completed. It is requested that	
			these photos are submitted for review.	
		·	3/24/10: Sheetrock and insulation was removed to expose two	
			vertical studs. Ledger attachment to vertical studs was not found to	
			be as specified. The as-built configuration was reviewed, taking	
			into account the proximity of additional shear walls, and it was	
105	Deef	00/00/00	determined that the as-built capacity of the connection is acceptable.	01.0055
35	Roof	09/08/09	At all interior shear wall locations, roof plywood sheathing shall be	CLOSED
	Sheathing		fastened to wall top plates with 8d nails at 4" o.c. per Section 9/S4.1	
	Attachment		09/25/09: GC has stated that this item was completed.	
20	0	00/00/00	** Item cannot be reviewed due to roofing installation.	01.0055
36	Corridor	09/08/09	At corridor bearing walls, area between truss seats shall be blocked	CLOSED
	Bearing		per Section 3/S4.1. At roof trusses, it is acceptable to install (1)	
	Walls		2x10. At floor level, add (2) 2x10. Attach blocking to LVL rim, and	
			attach wall plates/sheathing to blocking.	
L			09/25/09: Blocking was observed at all areas reviewed.	



37	Roof ·	09/08/09	At roof screen post attachments along the north and south sides,	CLOSED
	Screen Post Attachment	33,00,00	the roof trusses are to be restrained at the posts per Section 13/S4.2. The framing specified in Section 13 is installed, but when the post locations were determined, it was necessary to move the posts slightly to either align with the east & west walls, or to have adequate room to install the bolts. It appears that the maximum clear distance between the post and the truss bracing wall is under 2" (GC shall confirm, and notify engineer is distance is greater than this). To provide adequate restraint at the post, it is necessary to add (4) 2x6 blocking between roof trusses at the top and bottom of the truss. Ensure a tight fit between the blocking and the truss chords, and attach the blocking to the 2x4's in the bracing wall with 3/8" x 8" long RSS Structural Screws (pre-drill holes in 2x4 members). Place (3) RSS Screws at each vertical 2x4 located against the roof trusses, and at 8" o.c. +/- along the top and bottom horizontal 2x4 members. 09/25/09: Work is not complete.	OLOGED
38	Wood Wall Studs Near Holdowns	09/25/09	10/19/09: Work has been completed. Wall studs have been cut for installation and fastening of shear wall holdowns. At non-load bearing walls, replace the cut section and scab a new 4'-0" long 2x6 section to the side of the full height stud, fastening with (2) 10d nails at 6" o.c. At load-bearing walls, add a new full height stud. Fasten all new sections to the plywood sheathing per drawing requirements. 10/19/09: Wall studs were found to be repaired as specified at locations observed.	CLOSED
39	Post Cap Attachment at Roof Beam	09/25/09	At the RH1 header adjacent to Stair 2, the Simpson post cap is not attached to the jamb post. 10/19/09: Post cap has been attached to jamb post as necessary.	CLOSED
40	Valley Street Canopy – North End	10/19/09	At the Valley Street canopy, the MC 18 connection to the Second Floor Beam connection plate at Line 13 is not fitting up correctly. The connection plate appears rotated slightly such that the bolt holes do not line up. The item was discussed with the steel erector, and in lieu of fully bolting the connection, three bolt holes will be elongated slightly such that a minimum of (1) bolt in each row will be placed. Then the south side and the top of the MC18 connection plate will be welded to the W14 connection plate. Use a ¼" fillet weld along the south side, and a full penetration beveled weld along the top. These field welds require visual inspection from an AWS Certified Weld Inspector, with a report submitted to Becker Structural Engineers for Record. 03/04/10: Item was tracked under separate report specific to the canopy. Final inspection report from AWS Certified Weld Inspector was submitted.	CLOSED

LETTER OF TRANSMITTAL

Ganneston Construction Corp PO Box 27, North Belfast Avenue Augusta, ME 04332-0027

Tel- (207) 621-8505

Tel: (207) Fax: (207)				GCC PROJECT NUMBER GCC-80677		
29 BLACK	(POINT R	ON ARCHITECTS ROAD MAINE 04074	Attentio	TUESDAY 3:10pm Date: February 16, 2010 Attention: WENDI RE: FLORENCE HOUSE PROJECT		
WE ARE S	SENDING	YOU X	ATTACHED	UNDER SEPARATE COVER		
COPIES	NO.		DESC	CRIPTION		
2			AS REQUESTED BY J			
REMARKS	3					
HIS E MAI	L KICKED) IT BACK THUS WE	ARE SENDING THESE	FOR RECORD		
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· 						
Signed:		£. Adams ct Manager		Date: BECKER STRUCTURAL ENGINE		

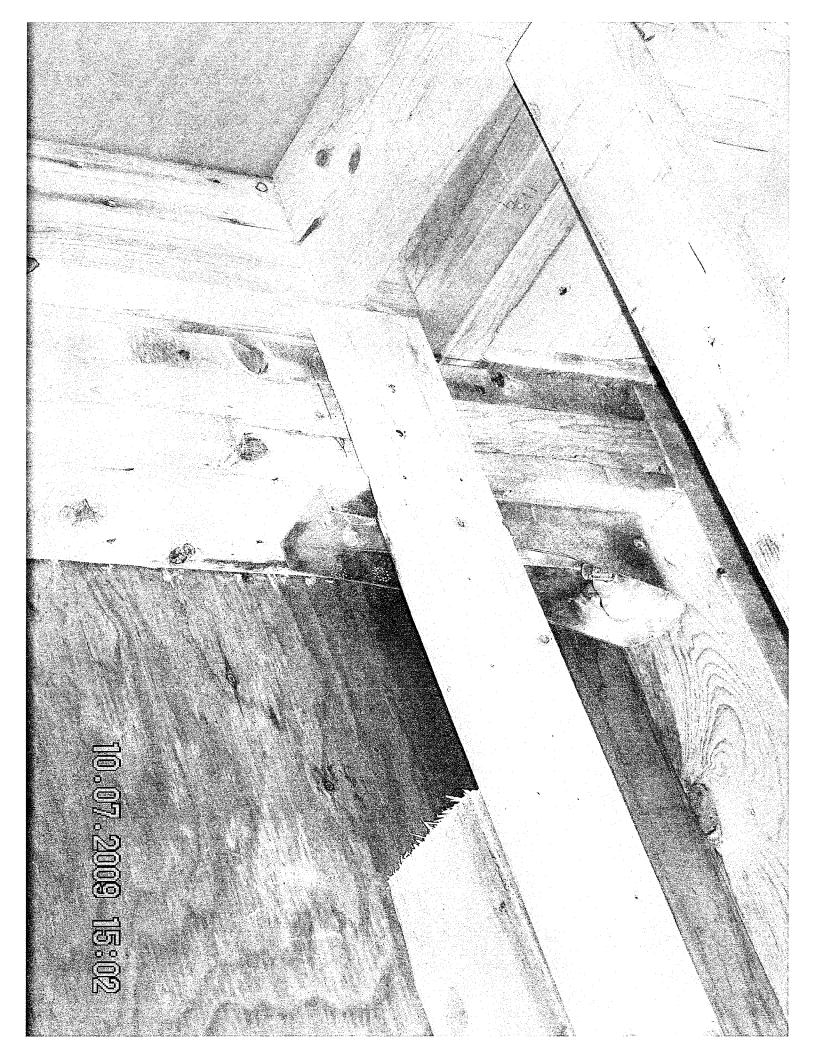
RECEIVED FEB 18 1/4

FEB 2 2 2010

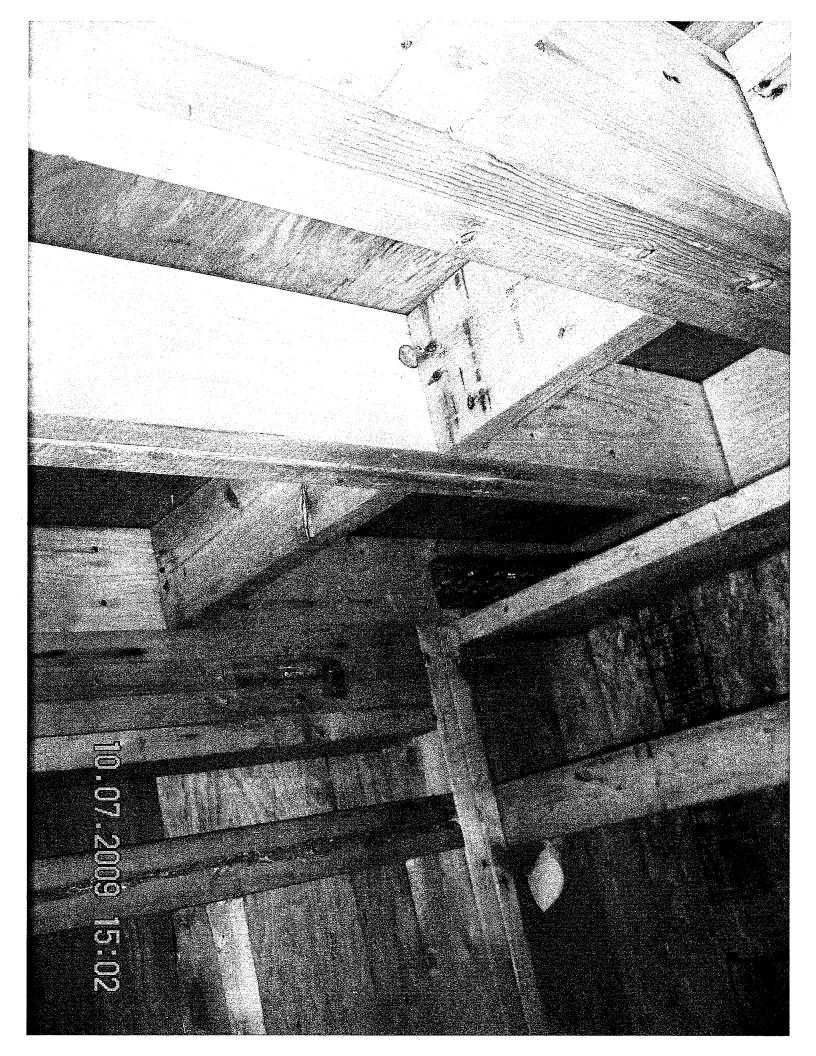
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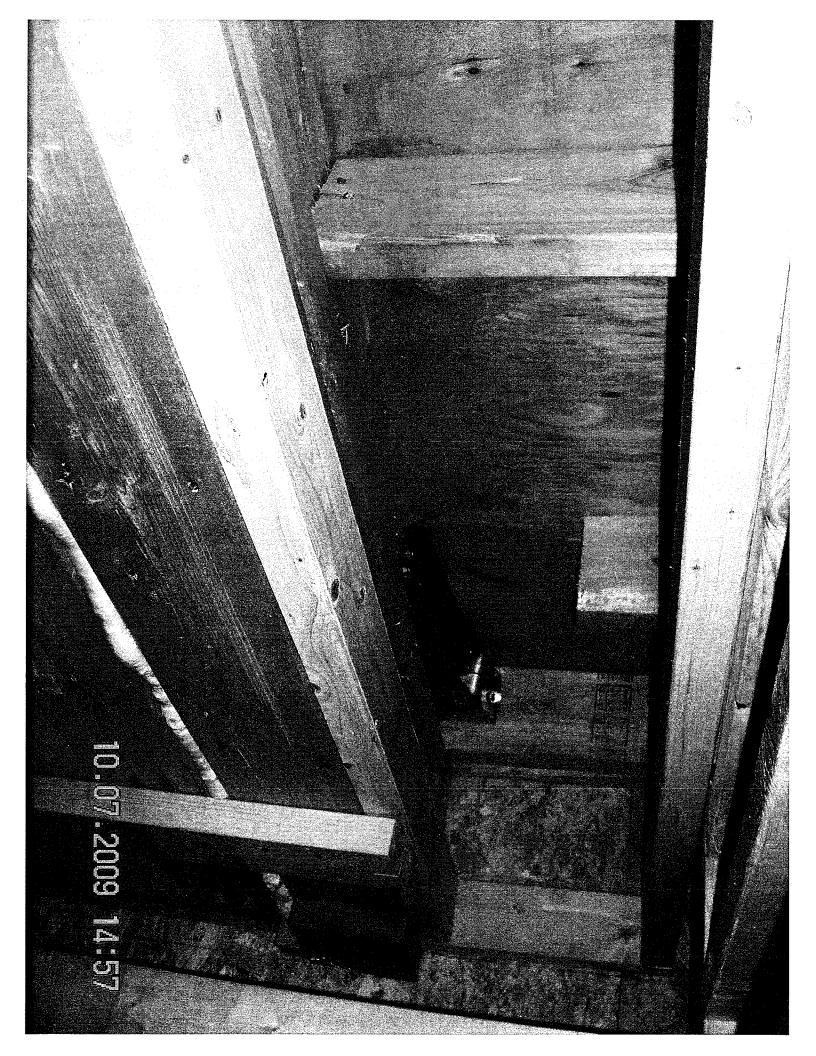












BECKER structural engineers, inc.

OBSERVATION	I REPORT			Date:	June 18, 2009
Rough Carpentry				Time:	1:30 pm-2:30 pm
		*******		Temp:	60's
•				Weather:	Cloudy / Rain Showers
Project:	Florence Hou	ICO.			
Location:				and the second s	
Becker Job No:	Portland, Mai	iiie		·····	
Decker Job No.	SI2058				
Observation Loca	ation: Review o	f panelize	ed wood v	vall sections at t	he Hancock Lumber fabrication
facility in Windham	ı, Maine.				
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· -	Satisfactory	Not Completed	Not Applicable		
	tisfa	နီ သိ	t Ap		
	Sa	5 2	2		Comments
Member Sizes					
Material Quality Bearing Condition] []			
Connections					
Nailing Pattern					
Bridging/Bracing Other:					
Other:					

Notes:					
This report shall do	ocument review o	of pre-fab	ricated we	ood wall panels	for the Florence House project. A
shipment to the sit	e. The sections	of wall p	anels acc	essible for revie	acility in Windham, Maine prior to were found to be in general
conformance with					3

BECKER structural engineers, inc.

OBSE	RVATION	REPORT
Rough	Carpentry	

Other:

Date:	July 27, 2009	
Time:	8:30 am 11:00 am	.,
Temp:	70 – 80	
Weather:	Clouds, Sun, Humid	

						Weat	her:	Clouds, Sun, Humid	,	
Project:	Flore	ence I	House		***************************************					
Location:		land,	**********************	AN ANTONOMINE SECULO CONTRACTO	er ter vincente in distributer etter en de vindere	***************************************				
Becker Job No:	İ		want							
Decker Job No:	SI20	158		***************************************						
Observation Loc	ation:	Reviev	w of w	ood co	onstruction	n complete	d to d	ate.		
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	***************************************	·	1							*************
			ory	eg	<u>e</u>					
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		tisf	တို-	Ŏ	ĮΨ					
•		Sa	ร	8	S N			Comments		
Member Sizes		\boxtimes	П	П	П			Commens		
Material Quality		$\overline{\boxtimes}$							U-191-12-14-1-1-1	***************************************
Bearing Condition		\boxtimes					***************************************			
Connections		\boxtimes	\boxtimes	\boxtimes						
Nailing Pattern		\boxtimes								
Bridging/Bracing		<u> </u>	Ц		Ц			alanananananananananananananananananana	h	
Other:	Ì	1]						

A preliminary review of the 2nd floor wood wall panels and third floor trusses was completed. The intent was to observe the framing and identify areas that were not in general conformance with the contract documents. At this time, all areas were in progress but appeared to be following the intent of the drawings. A few items were discussed with the General Contractor and framing sub-contractor. These items are provided below.

1. For the in-fill wall panels at the first floor level along Line 1, from Grid C to the site retaining wall, it is acceptable to replace the LVL wall studs with (2) 2x6 at 24" o.c. since these walls do not serve as a backing for brick veneer at these locations.

- 2. Due to inconsistencies in the levelness of the 2nd floor concrete slab on deck, gaps exist between the bottom plate of the wall panel and the PT sill plate bolted to the slab. At all locations where gaps exist, place full width by full depth hardwood shims below all vertical studs. Contractor has stated that shims will be cut to fit the various gap widths. Apply construction adhesive to each side of each shim prior to placing to provide positive connection between the plates.
- 3. At the 5 $\frac{1}{4}$ " x 7" LVL supporting the 3H1 headers (see S1.4, 2 locations), it is acceptable to bear the LVL on (2) PT SYP #2 sill plates fastened to the concrete slab with (2) 5/8" diameter x 2 $\frac{3}{4}$ " concrete embed Simpson Titen HD screws (countersink screws into top-most plate).

Please call with any questions.

BECKE R structural engineers, inc.

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Brandurating in A	Rough Ca	rpentry		ļ

Bridging/Bracing

Other:

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Date:	July 31, 2009
Time:	8:30 am-10:30 am
Temp:	
_	Sun and Clouds

					Weather:	Sun and Clouds
Project:	Florence I	House)			
Location:	Portland,	Maine)			
Becker Job No:	SI2058					
Observation Loc	ation: Revie	w of fir	rst and	l second fl	loor wood const	ruction completed to date.
	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable		
Member Sizes	\boxtimes	П				Comments
Material Quality						
Bearing Condition	X					
Connections						
Nailing Pattern	\square					The state of the s

This site visit was completed to review wood construction completed to date. Review included the 1st & 2nd floor exterior wall framing, and the third floor trusses. A few items were discussed with the General Contractor and framing sub-contractor. These items are provided below.

A. At the first floor window openings along Line A,N,7 & 16, the top of the R.O. is at 11'-11" above the first floor slab. With a floor-to-floor height of 14'-1", a clear distance of between ½" to 8 ½" remains between the bottom of the steel beam and the top of the window header. At these locations, it is not necessary to install cripple studs above the header, but the header shall be attached to the jambs using the Simpson clips at the top and bottom of the header (as specified). If the header is at least 1" into the light-gauge

track, the top Simpson angles can be omitted. If top angles cannot be installed due to interference with the metal track, a 4" long max section of the inside flange of the top track can be cut to fit a palm nailer for fastening the hanger to the jamb. At cut locations, fasten a 16" long Simpson CMST-12 ga strap (or equivalent) centered across the cut and screwed to the track flange. Note: When installing plywood, ensure that joints do not occur at the metal track. Sheathing fastener spacing shall follow drawing requirements.

- B. Along Line F, the jamb Door 126 interferes with the HSS hangers for the canopy. At this location, increase the jamb at adjacent Window F to (6) $1\frac{3}{4}$ " x $5\frac{1}{2}$ " LVL's, and extend the door header to this jamb. Install (2) LVL's for the door jamb extending from the slab to the underside of the header. Attach with Simpson LS50 angles, each side of jamb.
- C. For floor truss strong-back attachment at wood stud walls, it is acceptable to replace the Simpson A34 angles specified on Section 2/S1.4 with a vertical 2x6 stud placed against the strong-back. Fasten all members together.

The following items were noted during a walk-through of the second floor. These items were not discussed with the framing sub-contractor or general contractor prior to my departure.

- 1. At interior shear wall locations, attach the sheathing to the PT bottom plates with galvanized fasteners.
- 2. At SW-5 locations, the Titen HD anchors are not installed to attach the wall panel to the concrete slab.
- 3. At SW-4 locations, the 5/8" through bolts are not installed to attach the wall panel to the steel framing.
- 4. At LVL attachment between third floor trusses (along Line 10), the Simpson hanger flange nailing is incomplete.
- 5. At various locations, floor trusses do not line up with wall studs. Additional studs will be added as necessary.
- 6. At joints between panels, vertical studs shall be in contact and shall be attached together with (2) 10d nails at 6" o.c.
- 7. At third floor, between Lines G & H, drawing S1.4 specifies floor trusses to be aligned at edges of floor opening. GC shall confirm truss layout is adequate to suit floor opening location.
- 8. At numerous locations, the shear wall holdown bolt (per Detail B/S4.3) does not extend above the wall bottom plates. GC shall confirm locations, and submit recommendation for repair.
- 9. The CMU wall construction at the third floor cantilever bond beam (Section 9/S3.2) has un-mortared joints.

10. At locations where the radon pipe runs through the concrete floor haunches shown in CSKS-10, remove the crushed stone, wrap, and pour the concrete haunch around the pipe.

Please call with any questions.

BECKE R structural engineers, inc.

OBSERV	ATION REP	ORT
Rough Car	pentry	

Date:	August 13, 2009
Time:	8:00 am-10:30 am
Temp:	80's
Weather:	Sunny & Humid

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI2058

Observation Location:	Reviev	v of co	omplet	e wall	and floor wood construction completed to date.

	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Member Sizes	\square	П	П	П	AND CONTROL OF STATES OF CONTROL AND CONTROL OF CONTROL
Material Quality	$\overline{\boxtimes}$				The discontinues and the second of the secon
Bearing Condition	\boxtimes				engentian de demonstration of the contraction of th
Connections		\boxtimes	\boxtimes		ag ta green de same and a same and a service of the same and a service
Nailing Pattern	\boxtimes				Em transferrence contributed the contributed and the second of the second secon
Bridging/Bracing			\boxtimes		The state of the control of the cont
Other:		\boxtimes	\boxtimes		See notes (attached)
Other:					regional communitation to the community of the community

This site visit was completed to review wood construction completed to date. Review included the exterior wall and interior bearing wall framing and floor trusses. A meeting was also conducted to review the roof trusses that were exhibiting cracks and excessive curling at the bearing ends. This issue will be tracked independent of this report.

Items noted during this visit are provided below, along with previously documented items. New information, as well as updates to previously documented items, are provided in bold.

If there are any questions with the information provided, please contact our office.

Thank you -

BECKER structural engineers, inc.

OBSERVATION REP	
Barrier Control	
Payah Carpontay	
Rough Carpentry	
Rough Carpentry	
Rough Carpentry	
Rough Carpentry	
Rough Carpentry	
Rough Carpentry	

	August 24, 2009
Time:	10:00 am-12:00 pm
Temp:	80's
Weather:	Sunny & Humid

	weather: Sunny & Humid					
Project:	Florence	House)			
Location:	Portland,	Maine)		\$ 175 \(\sigma \) 1.00 \(\sigma \) 1.00 \(\sigma \)	
Becker Job No:	SI2058					
Observation Location (including prelin	ation: Revie	w of so	econd d floor	floor v wall fr	vall framing and third floor wood construction. aming and roof truss system)	
	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments	
Member Sizes						
Material Quality Bearing Condition				Н	As noted	
Connections		Ø	<u> </u>		As noted	
Nailing Pattern	\square	П	П		The second of th	

This site visit was completed to review wood construction completed to date. Review included the exterior wall and interior bearing wall framing at the second floor as well as the third floor trusses. A preliminary review of the third floor wall framing and roof trusses was completed, but additional review is necessary.

As noted

See notes (attached)

Items noted during this visit are provided below, along with previously documented items. New information, as well as updates to previously documented items, is provided in bold.

If there are any questions with the information provided, please contact our office.

Thank you -

Bridging/Bracing

Other:

Other:

BECKER R structural engineers, inc.

OBSERVATION Rough Carpenton		Date: Time:	September 2, 2009 1:00 pm-3:00 pm	
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		Weather:	Sunny	
Project:	Florence House	100 JUL 100 JU		1
Becker Job No:	Portland, Maine SI2058			
Observation Loc	ation: Review of third floor wal	l framing and roof tru	ss installation.	:

	Satisfactory	Satisfactory	Completed	Applicable	
Member Sizes	Satis	S-L	Not	_ Not ∤	Comments
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Bearing Condition		Ħ	Ħ		See notes (attached)
Connections			$\overline{\boxtimes}$		See notes (attached)
Nailing Pattern		$\overline{\boxtimes}$	$\overline{\boxtimes}$		See notes (attached)
Bridging/Bracing			Ø		See notes (attached)
Other:		$\overline{\boxtimes}$	$\overline{\boxtimes}$		See notes (attached)
Other:					and the second s

This site visit was completed to review third floor wall construction and roof truss installation completed to date. Construction appears to be progressing well. Items noted have been included in the following table. New information, as well as updates to previously documented items, is provided in bold.

The following items have not yet been reviewed. These items will be reviewed once the General Contractor states that all work is completed and ready for review. These items include:

- 1. Roof Plywood Attachment
- 2. Exterior Wall Sheathing Attachment and Plywood Fastening between wall panels
- 3. Shear wall nailing at bottom plates
- 4. Specific fastening details specified in the construction documents (Section 1/S4.1, for example)
- 5. Canopies (not yet installed)
- 6. Simpson H-clips at ends of all roof trusses.
- 7. Roof Screen Attachments to roof trusses, and associated details

If there are any questions with the information provided, please contact our office.

BECKE R structural engineers, inc.

OBS	ERVATI	ON REP	ORT
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Date:	September 8, 2009	·
	1:30 pm-3:30 pm	ì
Temp:		
Weather:	Sunny	

Project:	Florence h	House	00 00 1988/W 10 00 00 00		
Location:	Portland, i	Maine			
Becker Job No:	SI2058				
Observation Loca	ation: Review	of roo	of shea	thing	attachments, and roof truss system.
	Satisfactory	Un-Satisfactory	Not Completed	Not Applicable	Comments
Member Sizes				\boxtimes	The second secon
Material Quality	\boxtimes				
Bearing Condition		<u> </u>	Ц.	\square	
Connections		닐	X	Щ.	See notes (attached)
Nailing Pattern				<u> </u>	Only as noted below
Bridging/Bracing Other:				H	See notes (attached)
Other:		H	H	H	See notes (attached)

This site visit was completed to review the attachments of the plywood roof sheathing to the roof joists, as well as review roof framing details with Dick Beaulieu and Derik Ouellette. All plywood roof sheathing was in place during this visit, and attachments appeared satisfactory where reviewed, except for a few minor areas that were spray-painted for identification. As a result of this visit, a few additional items were noted that have been included in the attached table. These items require additional attention to meet the drawing requirements.

The following list of items is provided to identify major structural areas that have not yet been reviewed. These items will be reviewed once the General Contractor states that all work is completed and ready for review. These items include:

- Exterior Wall Sheathing Attachment and Plywood Fastening between wall panels
 Note: This inspection needs to occur prior to placement of any building wrap (ie. tyvek)
- 2. Shear wall nailing at bottom plates
- 3. Specific fastening details specified in the construction documents (Section 1/S4.1, for example)
- 4. Canopies (not yet installed)
- 5. Simpson H-clips at ends of all roof trusses.
- 6. Roof Screen Attachments to roof trusses, and associated details

Also, CSKS-18 has been completed in a majority of areas, but it is not possible to review all fastening as the area has been covered with additional blocking and plywood to construct the eave.

If there are any questions with the information provided, please contact our office.

BECKE R structural engineers, inc.

OBSER	VATION RE	PORT
Rough C	arpentry	

 Date:
 September 25, 2009

 Time:
 1:30 pm-3:30 pm

 Temp:
 80's

 Weather:
 Sunny

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI2058

Observation Location: Review of entire structure. Review completion of previously documented items.

At the request of the General Contractor, a site visit was completed to review construction and completion of previously documented items. There was concern that areas requiring re-inspection were being covered up due to sheetrock installation. The attached table has been updated based on today's review.

At this point, the following items remain that need to be reviewed when completed, and prior to installation of finishes. These items will be reviewed once the General Contractor states that all work is completed and ready for review. These items include:

- 1. Roof Screen Post Attachment to the Roof Trusses, and completion of associated details
- 2. Roof Truss Repairs at Damaged Areas
 - a. In order to review this item, it is necessary that documentation is submitted from the truss manufacturer (Clyvanor). Documentation shall identify the trusses that require repair, the engineered design of the repair, and the criteria used to accept damaged trusses that were identified by the design team.
- 3. Repair of Damaged Steel Joists (2 locations)
- 4. Canopies (not yet installed)

If there are any questions with the information provided, please contact our office.

OBSERVATION REPORT Rough Carpentry

Data	O-t-b40, 2000	
Date.	October 19, 2009	
Time:	10:30 am-12:30 pm	
Temp:	60's	
Weather:		

Project:	Florence House
Location:	Portland, Maine
Becker Job No:	SI2058

Observation Location: Review completion of previously documented items.

Following notification by the General Contractor, a site visit was completed to review previously documented items that had been addressed. A majority of open items were addressed by the General Contractor, and these items have been updated in the attached table. A few items remain that require attention by the General Contractor. Once these items are completed, a return visit can be scheduled. These items need to be reviewed prior to installation of finishes that will block access.

Also, the following items need to be reviewed when the General Contractor states that all work is complete, and prior to installation of finishes.

These items include:

- 1. Repair of Damaged Steel Joists (2 locations) (includes PE stamped design submittal)
- 2. Canopies (installation not complete)

Please note that I will be on vacation from October 23rd to November 3rd, and will not have access to e-mail. A return site visit can be scheduled once I return.

Thank you -

EXHIBIT C

01000 Quality Assurance

			House September 2, 2008
Qu	ality	Assu	rance Plan – Seismic and Wind
			SURANCE FOR SEISMIC RESISTANCE CHECK LIST [IBC 1705] n Category C
⊠ F(OR SEIS	MIC DE	SIGN CATEGORY C OR HIGHER:
Struc	tural: The seisn ⊠ Steel	nic-force Braced F	-resisting systems rames and associated connections/anchorage
		walls:	Frames and associated connections ☐ CMU ☑ Wood ☐ Concrete ☑ Diaphragms: ☑ Floor ☑ Roof
			SURANCE FOR WIND RESISTANCE CHECK LIST [IBC 1706]
-		kposulind Si	re Category C peed 100 mph
	1	1	
REQUIRED	NOT REQUIRED	NOT APPLICABLE	QUALITY ASSURANCE PLAN REQUIREMENTS (A Quality Assurance Plan is required where indicated below)
			In wind exposure Categories A and B, where the 3-second-gust basic wind speed is 120 miles per hour (mph) (52.8 m/sec) or greater.
		\boxtimes	In wind exposure Categories C and D, where the 3-second-gust basic wind speed is 110 mph (49 m/sec) or greater.

Signature

Building Code Official's Acceptance:

Date

Prepared by:

Date Prepared: September 2, 2008

Structural Schedule of Special Inspections SEISMIC RESISTANCE - STRUCTURAL

VERIFICATION AND INSPECTION	Y/N	EXTENT: CONTINUOU	COMMENTS	AGENT		TASK
IBC Section 1707		S, PERIODIC, SUBMITTAL, OR NONE			QUALIFICATION	D
Special inspections for seismic resistance. Special inspection as specified in this section is required for the following:		## (F)	Seismic Design Category: C			
a. The seismic-force-resisting systems in structures assigned to Seismic Design Category C, D, E or F	Y	P	IBC 1707.1	SII	PE/SE or EIT	JLF N/A
2. Structural steel: Continuous special inspection for structural welding in accordance with AISC 341.	Y	· P	IBC 1707.2	TA2	AWS-CWI	NA
3. Structural wood:	e de la constante de la consta					
a. Continuous special inspection during field gluing operations of elements of the seismic-force-resisting system.	Y	С	IBC 1707.3	SII	PE/SE or EIT	NA
b. Periodic special inspections for nailing, bolting, anchoring and other fastening of components within the seismic-force-resisting system, including drag struts, braces and hold-downs	Y	P	IBC 1707.3	SII	PE/SE or EIT	TH
4. Cold-formed steel framing: Periodic special inspections during welding operations of elements of the seismic-force-resisting system. Periodic special inspections for screw attachment, bolting, anchoring and other fastening of components within the seismic-force-resisting system, including struts, braces, and hold-downs	Ž.	Ŋ				
Seismic isolation system. Provide periodic special inspection during the fabrication and installation of isolator units and energy dissipation devices if used as part of the seismic isolation system	N/A	Ň	- IBC 1707.8			

EXHIBIT D

Statements of Responsibility

Date Prepared: September 2, 2008

Contractor's Statement of Responsibility

Each contractor responsible for the construction or fabrication of a system or component designated in the Quality Assurance Plan must submit a Statement of Responsibility. The Statement of Responsibility is required for Seismic Design Category C or higher. Make additional copies of this form as required.

Project: Florence House

Contractor's Name: (5000 eslav

Address: Pd Box 27, Augusta Maine 04332

License No.:

Description of designated building systems and components included in the Statement of Responsibility:

Contractor's Acknowledgment of Special Requirements

Building timsoul ywens

I hereby acknowledge that I have received, read, and understand the Quality Assurance Plan and Special Inspection program.

I hereby acknowledge that control will be exercised to obtain conformance with the construction documents approved by the Building Official.

Signature

Contractor's Provisions for Quality Control

Procedures for exercising control within the contractor's organization, the method and frequency of reporting and the distribution of reports is attached to this Statement.

Identification and qualifications of the person(s) exercising such control and their position(s) in the organization are attached to this Statement.

Date Prepared: September 2, 2008

Contractor's Statement of Responsibility

Each contractor responsible for the construction or fabrication of a system or component designated in the Quality Assurance Plan must submit a Statement of Responsibility. The Statement of Responsibility is required for Seismic Design Category C or higher. Make additional copies of this form as required.

Project: Florence House
Contractor's Name: LOCBID Construction, Inc.
Address: P.O. Box 607 Sabattus, ME
License No.:
Description of designated building systems and components included in the Statement of Responsibility:
wood shear walls & floor/roof plywood diaphragms

Contractor's Acknowledgment of Special Requirements

I hereby acknowledge that I have received, read, and understand the Quality Assurance Plan and Special Inspection program.

I hereby acknowledge that control will be exercised to obtain conformance with the construction documents approved by the Building Official.



Contractor's Provisions for Quality Control

Procedures for exercising control within the contractor's organization, the method and frequency of reporting and the distribution of reports is attached to this Statement.

Identification and qualifications of the person(s) exercising such control and their position(s) in the organization are attached to this Statement.

Date Prepared: September 2, 2008

Contractor's Statement of Responsibility

Each contractor responsible for the construction or fabrication of a system or component designated in the Quality Assurance Plan must submit a Statement of Responsibility. The Statement of Responsibility is required for Seismic Design Category C or higher. Make additional copies of this form as required.

Project: Florence House	
Contractor's Name: Precision Weiling & Fabrication	
Address: 690 Struduates of Weltbrook, me 04092	
License No.:	
Description of designated building systems and components included in the Statement of Responsil	oility:
Structural & misc steel	•

Contractor's Acknowledgment of Special Requirements

I hereby acknowledge that I have received, read, and understand the Quality Assurance Plan and Special Inspection program.

I hereby acknowledge that control will be exercised to obtain conformance with the construction documents approved by the Building Official.

Signature 7-3-10
Date

Contractor's Provisions for Quality Control

Procedures for exercising control within the contractor's organization, the method and frequency of reporting and the distribution of reports is attached to this Statement.

Identification and qualifications of the person(s) exercising such control and their position(s) in the organization are attached to this Statement.

Date Prepared: September 2, 2008

Fabricator's Certificate of Compliance

Each approved fabricator that is exempt from Special Inspection of shop fabrication and implementation procedures per section 1704.2 of the International Building Code must submit a Fabricator's Certificate of Compliance at the completion of fabrication.

Project: Florence House - Pontand Me.
Fabricator's Name: Precision Welding a toppolication Inc.

Address: 690A STROULWATER ST. WEST-brook, ME. 0409Z

Certification or Approval Agency: AISC . STATURED For STEAL BUILDING STATUTURES

Certification Number:

Date of Last Audit or Approval: October 2009

Description of structural members and assemblies that have been fabricated:

Dr. # 05500 " METAL FAMILIATIONS"

I hereby certify that items described above were fabricated in strict accordance with the approved construction documents.

Signature

Date

Title

Attach copies of fabricator's certification or building code evaluation service report and fabricator's quality control manual

CASE Form 104 • Fabricator's Certificate of Compliance • @CASE 2004

American Institute of Steel Construction

is proud to recognize

Precision Welding & Fabrication, Inc.

Westbrook, ME

for successfully meeting the quality certification requirements for

Standard for Steel Building Structures

Lager Sheet

Roger E. Ferch



Certification valid through "October 2010

End of Special Inspections Report