





















SPECIFIER GUIDE

INCLUDES AJS® 140 / 20 / 25







CCMC Report Number 12787-R ALLJOIST®



















High Performance Floor & Roof Systems







ASG CANADA March 2014

The SIMPLE FRAMING SYSTEM®



Architects, engineers, and designers trust Boise Cascade's engineered wood products to provide a better system for framing floors and roofs.

It's the SIMPLE FRAMING SYSTEM®,

featuring beams, joists and rim boards that work together as a system, so you spend less time cutting and fitting. In fact, the SIMPLE FRAMING SYSTEM[®] uses fewer pieces and longer lengths than conventional framing, so you'll complete jobs in less time.

You'll Build Better Homes with the SIMPLE FRAMING SYSTEM[®]

Now it's easier than ever to design and build better floor systems. When you specify the SIMPLE FRAMING SYSTEM[®], your clients will have fewer problems with squeaky floors and ceiling gypsum board cracks. The SIMPLE FRAMING SYSTEM[®] also means overall better floor and roof framing than dimension lumber allows.

Better Framing Doesn't Have to Cost More

Boise Cascade Engineered Wood Products' SIMPLE FRAMING SYSTEM[®] often costs less than conventional framing methods when the resulting reduced labor and materials waste are considered. There's less sorting and cost associated with disposing of waste because you order only what you need. Although our longer lengths help your clients get the job done faster, they cost no more.

Environmentally Sound

As an added bonus, floor and roof systems built with AJS[®] Joists require about half the number of trees as those built with dimension lumber. This helps you design a home both you and future generations will be proud to own.

What Makes the SIMPLE FRAMING SYSTEM[®] So Simple?

☑ Floor and Roof Framing with ALLJOIST[®] Product

Light in weight, but heavy-duty, ALLJOIST[®] Product (AJS[®] Joists) have a better strength / weight ratio than dimension lumber. Knockouts can be removed for cross-ventilation and wiring.

Ceilings Framed with AJS[®] Joists

The consistent size of AJS[®] Joists helps keep gypsum board flat and free of unsightly nail pops and ugly shadows, while keeping finish work to a minimum.

☑ VERSA-LAM[®] Beams for Floor and Roof Framing

✓ VERSA-STUD[®] and VERSA-

LAM[®] Columns for wall Framing These highly-stable beams are free of the large-scale defects that plague dimension beams. The result is quieter, flatter floors (no camber) and no shrinkage-related call-backs.

☑ Boise Cascade Rimboard

Boise Cascade Engineered Wood Products offer several engineered rimboard products regionally, including BC RIM BOARD® OSB, BC RIM BOARD® and VERSA-RIM® (check supplier or Boise Cascade EWP representative for availability). These products work with AJS® Joists to provide a solid connection at the critical floor/wall intersection.

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ALLJOIST® ARCHITECTURAL SPECIFICATIONS

Scope: This work includes the complete furnishing and installation of all AJS[®] Joists as shown on the drawings, herein specified and necessary to complete the work.

Materials: AJS[®] Joists shall be manufactured by Boise Cascade Engineered Wood Products with oriented strand board webs, machine stress rated (MSR) lumber flanges and waterproof, structural adhesives.

Joist webs shall be rated Structural I Exposure 1 by an agency listed by a model code evaluation service. The web panels shall be glued together to form a continuous web member. The web panels shall be machined to fit into a groove in the center of the wide face of the flange members so as to form a pressed glue joint at that junction.

Design: The AJS[®] Joists shall be sized and detailed to fit the dimensions and loads indicated on the plans. All designs shall be in accordance with allowable values and section properties developed in accordance with ASTM D5055, CSA O86-09, and listed under a CCMC product evaluation.

Drawing: Additional drawings showing layout and detail necessary for determining fit and placement in the building are (are not) to be provided by the supplier.

Fabrication: The AJS[®] Joists and section properties shall be manufactured in a plant evaluated for fabrication by the governing code evaluation service and under the supervision of a third-party inspection agency listed by the corresponding evaluation service.

Storage and Installation: The AJS[®] Joists, if stored prior to erection, shall be stored in a vertical and level position and protected from the weather. They shall be handled with care so they are not damaged.

The AJS[®] Joists are to be installed in accordance with the plans and the Boise Cascade Engineered Wood Products Installation Guide. Temporary construction loads which cause stresses beyond design limits are not



permitted. Erection bracing shall be provided to keep the AJS[®] Joists straight and plumb as required and to assure adequate lateral support for the individual AJS[®] Joists and the entire system until the sheathing material has been applied.

Codes: The AJS[®] Joists shall be evaluated by the CCMC evaluation service.

Lifetime Guaranteed Quality and Performance

Boise Cascade warrants its BCI[®] Joist, VERSA-LAM[®], and ALLJOIST[®] products to comply with our specifications, to be free from defects in material and workmanship, and to meet or exceed our performance specifications for the normal and expected life of the structure when correctly stored, installed and used according to our Installation Guide.

For information about Boise Cascade's engineered wood products, including sales terms and conditions, warranties and disclaimers, visit our website at www.BC.com/ewp

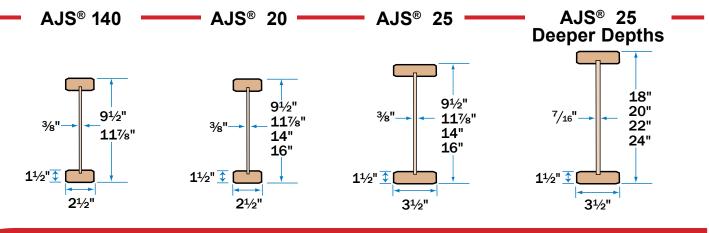
Boise Cascade Chain-of-Custody Certifications

Boise Cascade Engineered Wood Products has a proven track record of providing quality wood products and a nationwide building materials distribution network for our customers, helping them to enhance their own businesses.

Boise Cascade Engineered Wood Products build better homes with stronger, stiffer floors using only wood purchased in compliance with a number of green building programs. Take a moment to view our sustainability certification site at http://www.bc.com/sustainability/ certification.html or view our green brochure at http:// www.bc.com/wood/ewp/Boise_EWP_Green.html.

Boise Cascade Engineered Wood Products throughout North America can now be ordered FSC[®] Chain-of-Custody (COC) certified, enabling homebuilders to achieve LEED[®] points residential and commercial green building programs including LEED for Homes and LEED for New Construction. Boise Cascade Engineered Wood Products are available as PEFC[®] Chain-of-Custody certified, SFI[®] Chain-of-Custody certified and SFI[®] Fiber-Sourcing certified, as well as NAHB Research Center Green Approved, enabling homebuilders to also obtain green building points through the Green Building Standards.

ALLJOIST[®] PRODUCT PROFILES



FACTORED RESISTANCES

Limit States Design (CANADA)

	la i at	Factored	Factored	laint Oliffanna	Shear		Factored E Resistar	nd Bearing nce (lbs)	Factored Intermediate Bearing Resistance (lbs)		
AJS [®] Joist	Joist Depth	Moment Resistance	Shear Resistance	Joist Stiffness El	Deformation Coefficient, K	Joist Weight	1½" Min. Bea	ring Length ⁽²⁾	3 ¹ / ₂ " Min. Bearing Length		
Series							No Web Stiffeners	WITH Web Stiffeners	No Web Stiffeners	WITH Web Stiffeners	
	[in]	[lbs-ft]	[lbs]	[x10 ⁶ lbs-in ²]	[x10 ⁶ lbs]	[lbs/ft]	[lbs]	[lbs]	[lbs]	[lbs]	
AJS®	9½	4095	1830	182	5.2	2.2	1500	1955	3705	3865	
140	111⁄8	5305	2350	311	6.6	2.5	1505	2105	3770	4415	
	91⁄2	5675	1830	232	5.2	2.5	1500	1955	3705	3865	
AJS®	111⁄8	7350	2350	394	6.6	2.8	1505	2105	3770	4415	
20	14	8850	2825	579	7.8	3.0	1515	2240	3835	4940	
	16	10265	3255	789	8.9	3.3	1530	2365	3890	5420	
	91⁄2	8935	1830	322	5.3	3.1	1500	1955	4100	4495	
	111⁄8	11575	2350	545	6.7	3.4	1505	2105	4245	5035	
	14	13940	2825	798	7.9	3.7	1515	2240	4370	5520	
AJS®	16	16165	3255	1082	9.1	3.9	1530	2365	4495	5995	
25	18 ⁽³⁾	18260	4750	1427	12.3	4.6		3535		7450	
	20 ⁽³⁾	20405	5110	1813	13.7	4.9		3930		8065	
	22 ⁽³⁾	22375	5475	2249	15.0	5.1		3930		8255	
	24 ⁽³⁾	24325	5820	2738	16.5	5.4		3930		8435	

NOTES:

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(1) All resistance factors, as per CSA O86-09 have been applied.

(2) Minimum end bearing length is 1 % for 9 % to 16" depths and 1 % for 18" and deeper.

(3) $\rm AJS^{\otimes}$ Joists deeper than 16" require web stiffeners at all bearing locations.

(4) The AJS^{\odot} Joist deflection under uniform load may be calculated with the equation to the right:

BUILDING CODE EVALUATION REPORTS - CCMC Report Number 12787-R

$$\Delta = \frac{5wl^4}{384 EI} + \frac{wl^2}{K}$$

 Δ = Deflection [in]

w

l

ΕI

Κ

Uniform load [lb/in]

= Centerline to centerline [in]

Stiffness value from table [lb-in²]

= Shear deflection factor from table [lb]

About Floor Performance

Homeowner's expectations and opinions vary greatly due to the subjective nature of rating a new floor. Communication with the ultimate end user to determine their expectation is critical. Vibration is usually the cause of most complaints. Installing lateral bridging may help; however, squeaks may occur if not installed properly. Spacing the joists closer together does little to affect the perception of the

floor's performance. The most common methods used to increase the performance and reduce vibration of wood floor systems is to increase the joist depth, limit joist deflections, glue and screw a thicker tongue-and-groove subfloor, install the joists vertically plumb with level-bearing supports, and install a direct-attached ceiling to the bottom flange of the joists.

	⁵ ∕ ₈ " or ¹⁹ ∕ ₃₂ " Subfloor (Nailed)																						
Live Loa	Live Load: 40 psf SIMPLE SPAN											С	ONTINU	OUS SPA	N								
Dead Load: 15 psf Without Gypsum Ceiling Attached 1/2" Gypsum Ceiling					eiling Atta	ached	Without Gypsum Ceiling Attached 1/2" Gypsum Ceiling Attached																
Joist Series	Depth [in]	12"	16"	19.2"	24"	12"	16"	19.2"	24"	12"	16"	19.2"	24"	12"	16"	19.2"	24"						
AJS®	91⁄2	14'-7"	13'-7"	13'-1"	N/A	15'-0"	14'-0"	13'-5"	N/A	15'-10"	14'-9"	14'-2"	N/A	16'-3"	15'-2"	14'-7"	N/A						
140	111/8	16'-5"	15'-4"	14'-9"	N/A	16'-11"	15'-9"	15'-2"	N/A	17'-10"	16'-8"	16'-0"	N/A	18'-5"	17'-2"	16'-5"	N/A						
	91⁄2	15'-4"	14'-4"	13'-9"	N/A	15'-9"	14'-8"	14'-1"	N/A	16'-8"	15'-6"	14'-11"	N/A	17'-1"	15'-11"	15'-3"	N/A						
AJS®	111/8	17'-4"	16'-2"	15'-6"	N/A	17'-9"	16'-6"	15'-10"	N/A	18'-11"	17'-6"	16'-10"	N/A	19'-7"	17'-11"	17'-3"	N/A						
20	14	19'-1"	17'-7"	16'-10"	N/A	19'-8"	18'-1"	17'-4"	N/A	21'-2"	19'-4"	18'-5"	N/A	21'-10"	20'-0"	19'-0"	N/A						
	16	20'-10"	19'-1"	18'-1"	N/A	21'-6"	19'-8"	18'-8"	N/A	23'-1"	21'-2"	20'-1"	N/A	23'-10"	21'-11"	20'-9"	N/A						
	91⁄2	16'-5"	15'-4"	14'-9"	N/A	16'-10"	15'-8"	15'-0"	N/A	17'-10"	16'-8"	16'-0"	N/A	18'-3"	17'-0"	16'-4"	N/A						
	111/8	18'-8"	17'-3"	16'-7"	N/A	19'-2"	17'-8"	16'-11"	N/A	20'-8"	18'-11"	18'-0"	N/A	21'-3"	19'-6"	18'-6"	N/A						
	14	20'-9"	19'-0"	18'-0"	N/A	21'-4"	19'-6"	18'-6"	N/A	23'-1"	21'-1"	20'-0"	N/A	23'-9"	21'-9"	20'-7"	N/A						
AJS® 25	16	22'-8"	20'-8"	19'-8"	N/A	23'-4"	21'-4"	20'-3"	N/A	25'-2"	23'-0"	21'-10"	N/A	25'-11"	23'-9"	22'-6"	N/A						
25	18	24'-9"	22'-7"	21'-5"	N/A	25'-5"	23'-3"	22'-1"	N/A	27'-4"	25'-1"	23'-9"	N/A	28'-2"	25'-10"	24'-6"	N/A						
	20	26'-6"	24'-2"	22'-11"	N/A	27'-2"	24'-11"	23'-8"	N/A	29'-4"	26'-10"	25'-5"	N/A	30'-2"	27'-8"	26'-3"	N/A						
	22	28'-2"	25'-9"	24'-5"	N/A	28'-11"	26'-6"	25'-2"	N/A	31'-2"	28'-6"	27'-1"	N/A	32'-1"	29'-5"	27'-11"	N/A						
	24	29'-9"	27'-2"	25'-10"	N/A	30'-7"	28'-1"	26'-8"	N/A	33'-2"	30'-2"	28'-8"	N/A	34'-5"	31'-2"	29'-7"	N/A						

³/₄" or ²³/₃₂" Subfloor (Nailed)

						/4	01 /3/			uniou,								
Live Loa	ad: 40 psf				SIMPLE	E SPAN				CONTINUOUS SPAN								
Dead Lo	ad: 15 psf	Without	Gypsum	Ceiling A	Attached	½" G	ypsum C	eiling Att	ached	Without	Gypsum	Ceiling A	Attached	½" G	ypsum Ce	eiling Attached		
Joist Series	Depth [in]	12"	16"	19.2"	24"	12"	16"	19.2"	24"	12"	16"	19.2"	24"	12"	16"	19.2"	24"	
AJS®	91⁄2	15'-3"	14'-3"	13'-7"	12'-11"	15'-7"	14'-7"	13'-11"	13'-3"	16'-6"	15'-5"	14'-9"	14'-1"	16'-11"	15'-10"	15'-2"	14'-2"	
140	111/8	17'-2"	16'-0"	15'-4"	14'-7"	17'-7"	16'-5"	15'-9"	15'-0"	18'-9"	17'-4"	16'-8"	15'-10"	19'-5"	17'-10"	17'-1"	16'-3"	
	91⁄2	16'-1"	15'-0"	14'-4"	13'-7"	16'-5"	15'-3"	14'-8"	13'-11"	17'-5"	16'-3"	15'-6"	14'-9"	17'-9"	16'-7"	15'-11"	15'-1"	
AJS®	111⁄8	18'-1"	16'-10"	16'-1"	15'-4"	18'-7"	17'-3"	16'-6"	15'-8"	20'-0"	18'-4"	17'-6"	16'-8"	20'-7"	18'-11"	17'-11"	17'-1"	
20	14	20'-2"	18'-5"	17'-7"	16'-9"	20'-9"	19'-0"	18'-0"	17'-1"	22'-4"	20'-5"	19'-4"	18'-2"	23'-0"	21'-1"	20'-0"	18'-9"	
	16	22'-1"	20'-2"	19'-0"	17'-11"	22'-8"	20'-9"	19'-8"	18'-5"	24'-5"	22'-4"	21'-2"	19'-9"	25'-1"	23'-1"	21'-10"	19'-9"	
	91⁄2	17'-2"	16'-0"	15'-4"	14'-7"	17'-6"	16'-4"	15'-7"	14'-10"	18'-10"	17'-5"	16'-8"	15'-10"	19'-3"	17'-9"	17'-0"	16'-2"	
	111/8	19'-9"	18'-0"	17'-3"	16'-5"	20'-2"	18'-5"	17'-7"	16'-9"	21'-10"	20'-0"	18'-11"	17'-10"	22'-5"	20'-6"	19'-5"	18'-3"	
	14	22'-0"	20'-1"	19'-0"	17'-10"	22'-6"	20'-7"	19'-6"	18'-3"	24'-4"	22'-3"	21'-1"	19'-5"	25'-0"	22'-11"	21'-8"	19'-5"	
AJS®	16	24'-0"	21'-11"	20'-8"	19'-5"	24'-7"	22'-6"	21'-3"	19'-11"	26'-7"	24'-4"	23'-0"	19'-9"	27'-3"	25'-0"	23'-8"	19'-9"	
25	18	26'-2"	23'-11"	22'-7"	21'-2"	26'-9"	24'-6"	23'-2"	21'-9"	28'-11"	26'-6"	25'-1"	23'-6"	29'-7"	27'-2"	25'-9"	24'-2"	
	20	28'-0"	25'-7"	24'-2"	22'-8"	28'-7"	26'-3"	24'-10"	23'-4"	31'-0"	28'-4"	26'-10"	25'-2"	31'-9"	29'-2"	27'-7"	25'-11"	
	22	29'-9"	27'-2"	25'-8"	24'-1"	30'-5"	27'-11"	26'-5"	24'-10"	33'-1"	30'-2"	28'-6"	26'-9"	34'-2"	31'-0"	29'-4"	27'-7"	
	24	31'-6"	28'-9"	27'-2"	25'-6"	32'-2"	29'-7"	28'-0"	26'-3"	35'-8"	31'-11"	30'-2"	28'-4"	36'-10"	32'-11"	31'-1"	29'-2"	

NOTES:

- Spans shown are in accordance with NBCC 2010.
- Tables are based on the uniform standard loads of 40 psf live load and 15 psf dead load (for Standard Term Load Duration). 2
- 3. Floor tile will increase the dead load and specific deflection limits may apply.
- 4 Maximum spans listed are the clear spans between supports. Minimum end bearing length is $1\frac{1}{2}$ " for $9\frac{1}{2}$ " to 16" depths, and $1\frac{3}{4}$ " for 18" to 24" 5.
- 6
- depths. Bold spans have a minimum end bearing length of $3\frac{1}{2}$ ". Minimum interior bearing length is $3\frac{1}{2}$ ". Joists deeper than 16" require web stiffeners at all bearing locations.
- The subfloor shall be CSA rated Oriented Strand Board (OSB), Canadian Softwood Plywood (CSP) or Douglas Fir Plywood (DFP). 8.
- Total load deflection is limited to L/240. 10 Live load deflection is limited to L/360.
- Lateral support must be provided for the compression edge and also at the bearings 11. to prevent lateral displacement or rotation.
- It may be possible to exceed the limitations of these tables by analyzing a specific application with Boise Cascade BC CALC® or BC FRAMER® Software. 12.
- 13. When using continuous spans over an intermediate bearing, the shortest span shall not be less than 50% of the longest adjacent span. The end of the short span should be anchored to resist the uplift equal to:

Uplift = L_2^* (factor₁*W_{FD}-W_{FL})/factor₂ where: W_{FD} = Factored dead load (lb/ft)

ere:	$W_{FD} = Factored dead load (ID/II)$
	W _{FL} = Factored live load (lb/ft)
	L_2 = Length of longer span (ft)
	$factor_{a} = 8a(1+a)$

 L_1 = Length of shorter span (ft) $factor_1 = 4a^2 + 3a^3 - 1$

$factor_2 = 8a(1+a)$			a =	Snort	span/Lo	ng spar
Short/Long span ratio = a	0.5	0.6	0.7	0.8	0.9	1
factor ₁	0.38	1.09	1.99	3.10	4.43	6.00
factor ₂	6	7.68	9.52	11.52	13.68	16

Actual deflection based on span and deflection limit 14

•	Actual de	icitial dellection based on span and dellection limit.												
				Actu	al deflection	n (in)								
	Deflection limit		Span (ft)											
		15	15 17 19 21 25 30 35											
	L/360	0.50	0.57	0.63	0.70	0.83	1.00	1.17						
	L/240	0.75	0.85	0.95	1.05	1.25	1.50	1.75						

15. WARNING: Use of Span Tables for Commercial Projects (NBCC2010: Part 4) All projects within the scope of Part 4 of the National Building Code of Canada (NBCC) must consider the effects of concentrated loads, as stipulated in article 4.1.5.9. The designer of record must verify the effects of a concentrated load on the joists on all projects within the scope of Part 4 of NBCC (2010). Table 4.1.5.9 in NBCC (2010) lists concentrated loads that shall be analyzed with respect to the intended use of the floor. Given the numerous possible permutations, the span tables listed above do not take the effects of concentrated loads into consideration

		•			5	's" or 1	%₃₂" Su	bfloor	(Glue	d & Na	iled)								
Live Loa	Live Load: 40 psf SIMPLE SPAN										CONTINUOUS SPAN								
Dead Lo	ad: 15 psf	Without	Gypsum	Ceiling A	ttached	½" G	ypsum Ce	eiling Atta	ached	Without	Without Gypsum Ceiling Attached 1/2" Gypsum Ceiling Attached						ached		
Joist Series	Depth [in]	12"	12" 16" 19.2" 24" 12" 16" 19.2" 24"			12"	16"	19.2"	24"	12"	16"	19.2"	24"						
AJS®	91/2	15'-7"	14'-9"	14'-3"	N/A	16'-0"	15'-2"	14'-8"	N/A	16'-10"	15'-11"	15'-5"	N/A	17'-4"	16'-5"	15'-11"	N/A		
140	111/8	17'-6"	16'-6"	16'-0"	N/A	18'-0"	17'-0"	16'-5"	N/A	19'-2"	17'-10"	17'-3"	N/A	19'-11"	18'-6"	17'-10"	N/A		
	91⁄2	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-2"	N/A	17'-7"	16'-7"	16'-0"	N/A	18'-1"	17'-1"	16'-6"	N/A		
AJS®	111/8	18'-3"	17'-2"	16'-7"	N/A	18'-10"	17'-8"	17'-0"	N/A	20'-2"	18'-9"	18'-0"	N/A	20'-11"	19'-5"	18'-7"	N/A		
20	14	20'-3"	18'-10"	18'-0"	N/A	21'-0"	19'-6"	18'-8"	N/A	22'-6"	20'-10"	19'-11"	N/A	23'-3"	21'-7"	20'-8"	N/A		
	16	22'-1"	20'-6"	19'-7"	N/A	22'-10"	21'-3"	20'-4"	N/A	24'-6"	22'-8"	21'-8"	N/A	25'-4"	23'-7"	22'-7"	N/A		
	91⁄2	17'-3"	16'-2"	15'-8"	N/A	17'-7"	16'-7"	16'-0"	N/A	18'-10"	17'-7"	16'-11"	N/A	19'-4"	18'-0"	17'-4"	N/A		
	111/8	19'-8"	18'-2"	17'-6"	N/A	20'-2"	18'-8"	17'-11"	N/A	21'-9"	20'-2"	19'-3"	N/A	22'-5"	20'-9"	19'-10"	N/A		
	14	21'-10"	20'-2"	19'-3"	N/A	22'-6"	20'-10"	19'-10"	N/A	24'-2"	22'-4"	21'-4"	N/A	24'-11"	23'-1"	22'-1"	N/A		
AJS®	16	23'-9"	21'-11"	21'-0"	N/A	24'-6"	22'-8"	21'-8"	N/A	26'-4"	24'-4"	23'-3"	N/A	27'-2"	25'-2"	24'-0"	N/A		
25	18	25'-10"	23'-10"	22'-9"	N/A	26'-7"	24'-7"	23'-6"	N/A	28'-7"	26'-5"	25'-3"	N/A	29'-5"	27'-3"	26'-1"	N/A		
	20	27'-7"	25'-6"	24'-4"	N/A	28'-5"	26'-4"	25'-2"	N/A	30'-7"	28'-3"	26'-11"	N/A	31'-6"	29'-2"	27'-11"	N/A		
	22	29'-4"	27'-1"	25'-10"	N/A	30'-2"	27'-11"	26'-8"	N/A	32'-6"	30'-0"	28'-7"	N/A	33'-9"	31'-0"	29'-7"	N/A		
	24	31'-0"	28'-7"	27'-4"	N/A	31'-11"	29'-7"	28'-3"	N/A	34'-11"	31'-8"	30'-3"	N/A	36'-4"	32'-10"	31'-4"	N/A		

³/₄" or ²³/₃₂" Subfloor (Glued & Nailed)

Livele	Id: 40 psf CONTINUOUS SPAN																
	Live Load: 40 psf SIMPLE SPAN Dead Load: 15 psf Without Gypsum Ceiling Attached ½" Gypsum Ceiling Attached							Without	Gyneum	Ceiling A			N Vpsum Ce	iling Att	achod		
Joist Series	Depth [in]	12"	16"	19.2"	24"	12"	16"	19.2"	24"	12"	16"	19.2"	24"	12"	16"	19.2"	24"
AJS®	91⁄2	16'-7"	15'-7"	15'-1"	14'-3"	17'-0"	16'-0"	15'-6"	14'-3"	17'-11"	16'-11"	15'-10"	14'-2"	18'-6"	17'-5"	15'-10"	14'-2"
140	111/8	18'-8"	17'-6"	16'-10"	16'-2"	19'-3"	18'-0"	17'-4"	16'-3"	20'-7"	19'-2"	18'-3"	16'-2"	21'-4"	19'-11"	18'-1"	16'-2"
	91⁄2	17'-3"	16'-3"	15'-8"	15'-0"	17'-7"	16'-7"	16'-0"	15'-4"	18'-9"	17'-7"	16'-11"	16'-3"	19'-4"	18'-0"	17'-4"	16'-8"
AJS®	111/8	19'-7"	18'-2"	17'-6"	16'-10"	20'-2"	18'-9"	17'-11"	17'-3"	21'-8"	20'-1"	19'-2"	18'-3"	22'-4"	20'-10"	19'-10"	18'-10"
20	14	21'-10"	20'-2"	19'-3"	18'-3"	22'-5"	20'-10"	19'-11"	18'-11"	24'-1"	22'-4"	21'-4"	19'-5"	24'-10"	23'-1"	22'-1"	19'-5"
	16	23'-9"	22'-0"	20'-11"	19'-10"	24'-5"	22'-9"	21'-8"	20'-7"	26'-3"	24'-4"	23'-2"	19'-9"	27'-1"	25'-2"	24'-1"	19'-9"
	91⁄2	18'-3"	17'-2"	16'-6"	15'-10"	18'-8"	17'-6"	16'-10"	16'-2"	20'-2"	18'-8"	17'-11"	17'-2"	20'-8"	19'-2"	18'-4"	17'-6"
	111/8	21'-1"	19'-6"	18'-7"	17'-8"	21'-7"	20'-0"	19'-1"	18'-1"	23'-4"	21'-7"	20'-7"	19'-1"	23'-11"	22'-2"	21'-2"	19'-1"
	14	23'-5"	21'-8"	20'-7"	19'-6"	24'-0"	22'-3"	21'-2"	20'-1"	25'-11"	23'-11"	22'-10"	19'-5"	26'-7"	24'-8"	23'-6"	19'-5"
AJS® 25	16	25'-6"	23'-6"	22'-5"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"	28'-2"	26'-1"	24'-8"	19'-9"	28'-11"	26'-10"	24'-8"	19'-9"
25	18	27'-7"	25'-6"	24'-4"	23'-0"	28'-4"	26'-3"	25'-0"	23'-8"	30'-7"	28'-3"	26'-11"	25'-5"	31'-4"	29'-1"	27'-9"	26'-3"
	20	29'-6"	27'-3"	26'-0"	24'-7"	30'-3"	28'-1"	26'-9"	25'-4"	32'-8"	30'-2"	28'-9"	27'-2"	33'-10"	31'-1"	29'-8"	28'-1"
	22	31'-4"	29'-0"	27'-7"	26'-1"	32'-2"	29'-10"	28'-5"	26'-11"	35'-5"	32'-1"	30'-6"	28'-10"	36'-7"	33'-2"	31'-6"	29'-10"
	24	33'-4"	30'-7"	29'-1"	27'-6"	34'-5"	31'-6"	30'-0"	28'-5"	38'-1"	34'-4"	32'-3"	30'-6"	39'-5"	35'-9"	33'-6"	31'-6"

NOTES:

Spans shown are in accordance with NBCC 2010.

Tables are based on the uniform standard loads of 40 psf live load and 15 psf dead load (for Standard Term Load Duration).

Floor tile will increase the dead load and specific deflection limits may apply. Maximum spans listed are the clear spans between supports. 3

Minimum end bearing length is 1½" for 9½" to 16" depths, and 1¼" for 18" to 24" depths. Bold spans have a minimum end bearing length of 3½". Minimum interior bearing length is 3½". 5

6

Joists deeper than 16" require web stiffeners at all bearing locations.

Tabulated values shown assume a glued and nailed subfloor and are in compliance with the CCMC Vibration Criteria "Concluding " Report (dated September 4, 1997). The subfloor shall be CSA rated Oriented Strand Board (OSB), Canadian Softwood Plywood (CSP) or Douglas Fir Plywood (DFP). 8

10. Total load deflection is limited to L/240.

11

Live load deflection is limited to L/360. Lateral support must be provided for the compression edge and also at the bearings to prevent lateral displacement or rotation. 12

13

- It may be possible to exceed the limitations of these tables by analyzing a specific application with Boise Cascade BC CALC® or BC FRAMER® Software. Subfloor adhesive shall comply with CGSB standard CAN-CGSB 71.26-M88 "Adhesive for Field-gluing Plywood to Lumber Framing for Floor Systems" or APA Performance Specification AFG-01. 14
- 15. When using continuous spans over an intermediate bearing, the shortest span shall not be less than 50% of the longest adjacent span. The end of the short span should be anchored to resist the uplift equal to: Uplift = $L_2^*(factor_1^*W_{FD}-W_{FL})/factor_2$

Short/Long span ratio = a	0.5	0.6	0.7	0.8	0.9	1	
factor ₁	0.38	1.09	1.99	3.10	4.43	6.00	
factor ₂	6	7.68	9.52	11.52	13.68	16	

$C_{PIII} = L_2 (Iactor_1 V_{FD} V_{FL})/Iactor_2$
where: W_{FD} = Factored dead load (lb/ft)
W _{FL} = Factored live load (lb/ft)
L ₁ = Length of shorter span (ft)
L ₂ = Length of longer span (ft)

 $factor_1 = 4a^2 + 3a^3 - 1$ $factor_2 = 8a(1+a)$

a = Short span/Long span 16. Actual deflection based on span and deflection limit.

			Actu	al deflection	n (in)									
Deflection limit		Span (ft)												
	15	17	19	21	25	30	35							
L/360	0.50	0.57	0.63	0.70	0.83	1.00	1.17							
L/240	0.75													

 WARNING: Use of Span Tables for Commercial Projects (NBCC2010: Part 4) All projects within the scope of Part 4 of the National Building Code of Canada (NBCC) must consider the effects of concentrated loads, as stipulated in article 4.1.5.9. The designer of record must verify the effects of a concentrated load on the joists on all projects within the scope of Part 4 of NBCC (2010). Table 4.1.5.9 in NBCC (2010) lists concentrated loads that shall be analyzed with respect to the intended use of the floor. Given the numerous possible permutations, the span tables listed above do not take the effects of concentrated loads into consideration.

ALLOWABLE UNIFORM FLOOR LOAD (PLF)

Design	Joist Series	AJS	® 140		AJS	s® 20					AJS	® 25			
Span (ft)	Joist Depth (in)	9 ½	111%	9 ½	111/8	14	16	9 ½	111%	14	16	18	20	22	24
6	Unfactored Live Load for L / 360 [plf] Unfactored Total Load for L / 240 [plf] Factored Total Load [plf]	- - 488	- - 501	- - 488	- - 501	- - 505	- - 510	- - 488	- - 501	- - 505	- - 510	- - 993	- - 1075	- - 1100	- - 1124
8	Unfactored Live Load for L / 360 [plf] Unfactored Total Load for L / 240 [plf] Factored Total Load [plf]	- - 366	- - 376	- - 366	- - 376	- - 378	- - 382	- - 366	- - 376	- - 378	- - 382	- - 745	- - 806	- - 825	- - 843
10	Unfactored Live Load for L / 360 [plf] Unfactored Total Load for L / 240 [plf] Factored Total Load [plf]	227 - 292	- - 301	277 - 292	- - 301	- - 303	- - 306	- - 292	- - 301	- - 303	- - 306	- - 596	- - 645	- - 660	- - 674
12	Unfactored Live Load for L / 360 [plf] Unfactored Total Load for L / 240 [plf] Factored Total Load [plf]	138 207 244	227 - 250	170 - 244	- - 250	- - 252	- - 255	225 - 244	- - 250	- - 252	- - 255	- - 496	- 537	- - 550	- - 562
14	Unfactored Live Load for L / 360 [plf] Unfactored Total Load for L / 240 [plf] Factored Total Load [plf]	89 134 209	148 - 215	111 167 209	182 - 215	- - 216	- - 218	149 - 209	- - 215	- - 216	- - 218	- - 425	- - 460	- - 471	- - 482
16	Unfactored Live Load for L / 360 [plf] Unfactored Total Load for L / 240 [plf] Factored Total Load [plf]	61 92 183	102 153 188	76 115 183	126 - 188	181 - 189	- - 191	103 155 183	168 - 188	- - 189	- - 191	- - 372	- - 403	- - 412	- - 421
18	Unfactored Live Load for L / 360 [plf] Unfactored Total Load for L / 240 [plf] Factored Total Load [plf]	43 65 162	73 109 167	54 82 162	91 136 167	131 - 168	- - 170	74 111 162	122 - 167	- - 168	- - 170	304 - 331	- - 358	- - 366	- - 374
20	Unfactored Live Load for L / 360 [plf] Unfactored Total Load for L / 240 [plf] Factored Total Load [plf]		54 81 150	40 60 146	67 101 150	97 146 151	130 - 153	55 82 146	91 136 150	130 - 151	- - 153	228 - 298	285 - 322	- - 330	- - 337
22	Unfactored Live Load for L / 360 [plf] Unfactored Total Load for L / 240 [plf] Factored Total Load [plf]		41 61 136		51 77 136	74 111 137	100 - 139	41 62 133	69 104 136	99 - 137	133 - 139	176 264 270	220 - 293	268 - 300	- - 306
24	Unfactored Live Load for L / 360 [plf] Unfactored Total Load for L / 240 [plf] Factored Total Load [plf]				40 60 125	58 87 126	78 117 127		54 81 125	78 117 126	104 - 127	138 207 248	173 259 268	211 - 275	254 - 281
26	Unfactored Live Load for L / 360 [plf] Unfactored Total Load for L / 240 [plf] Factored Total Load [plf]						62 93 117		43 64 115	62 93 116	83 - 117	110 165 229	138 207 248	169 - 254	204 - 259
28	Unfactored Live Load for L / 360 [plf] Unfactored Total Load for L / 240 [plf] Factored Total Load [plf]						50 75 109			50 75 108	67 101 109	89 133 212	112 168 230	137 206 235	166 - 241
30	Unfactored Live Load for L / 360 [plf] Unfactored Total Load for L / 240 [plf] Factored Total Load [plf]						41 61 102			41 61 101	55 83 102	73 109 198	92 138 215	113 169 220	136 205 224

NOTES:

1. Total Factored Load values are limited by shear, end/interior reactions or bending moment.

2. Unfactored Live Load values are limited by deflection equal to L / 360. For deflections limited to L / 480, multiply live load values by 0.75 (recommended for less vibration).

3. Unfactored Total Load values are limited by deflection equal to L / 240.

4. All three loading cases must be checked. Where a Live Load value is not shown, the Factored Total Load value will control.

5. Joists deeper than 16" require web stiffeners at all bearing locations.

6. Table values represent the most restrictive of simple or continuous span beams applications and assume an uniform loading. Span is measured center to center of the supports. Analyze continuous span beams with the BC CALC[®] software if the length of any span is less than half the length of an adjacent span.

7. Table values assume that lateral support is provided at each support and continuously along the compression edge of the beam.

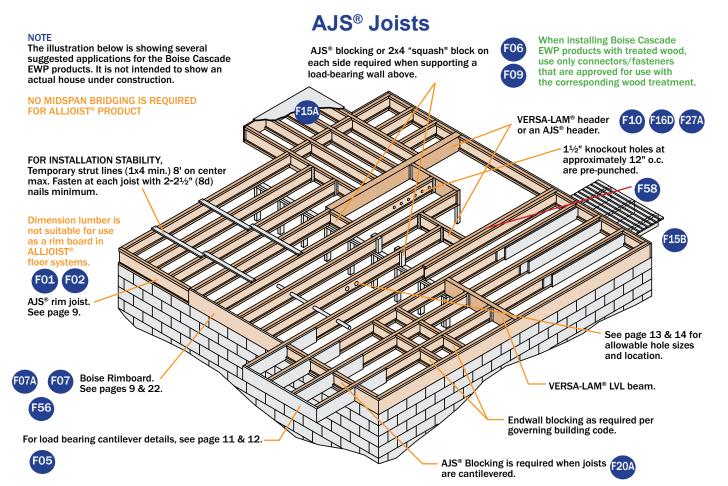
8. Table values do not consider composite action from gluing and nailing floor sheathing.

9. For 2-ply, double the Factored Total Load, Unfactored Live and Total Load values.

10. This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC[®] software.

		Actual deflection based on Span and Limit (in)														
Deflection limit		Span (ft)														
	6	8 10 12 14 16 18 20 22 24 26 28 30														
L/480	0.15	0.20	0.25	0.30	0.35	0.40	0.45	0.50	0.55	0.60	0.65	0.70	0.75			
L/360	0.20	0.27	0.33	0.40	0.47	0.53	0.60	0.67	0.73	0.80	0.87	0.93	1.00			
L/240	0.30	0.40	0.50	0.60	0.70	0.80	0.90	1.00	1.10	1.20	1.30	1.40	1.50			

FLOOR FRAMING



BCI® Joists, VERSA-LAM® and ALLJOIST® must be stored, installed and used in accordance with the Boise Cascade EWP Installation Guide, building codes, and to the extent not inconsistent with the Boise Cascade EWP Installation Guide, usual and customary building practices and standards. VERSA-LAM®, ALLJOIST®, and BCI® Joists must be wrapped, covered, and stored off of the ground on stickers at all times prior to installation. VERSA-LAM®, ALLJOIST® and BCI® Joists are intended only for applications that assure no exposure

SAFETY WARNING

DO NOT ALLOW WORKERS ON AJS® JOISTS UNTIL ALL HANGERS, AJS® RIM JOISTS, RIM BOARDS, AJS® BLOCKING PANELS, X-BRACING AND TEMPORARY 1x4 STRUT LINES ARE INSTALLED AS SPECIFIED BELOW. SERIOUS ACCIDENTS CAN RESULT FROM INSUFFICIENT ATTENTION TO PROPER BRACING DURING CONSTRUCTION. ACCIDENTS CAN BE AVOIDED UNDER NORMAL CONDITIONS BY FOLLOWING THESE GUIDELINES:

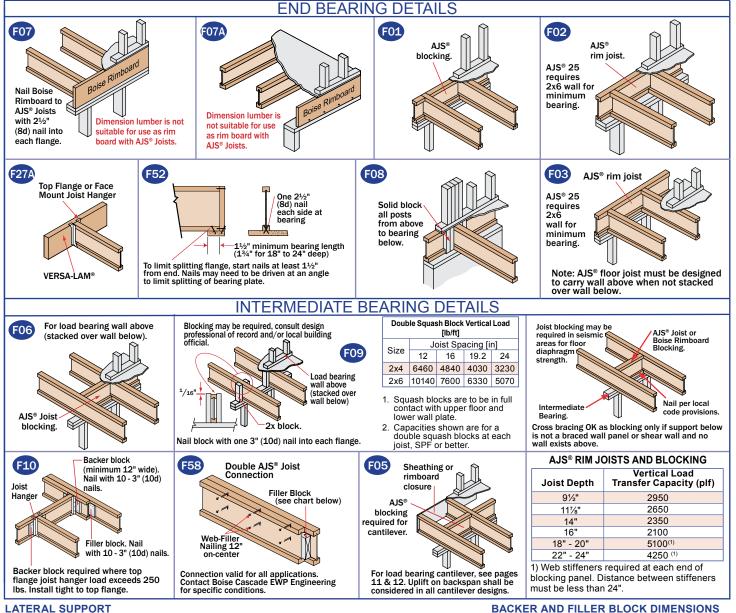
- Build a braced end wall at the end of the bay, or permanently install the first eight feet of AJS[®] Joist and the first course of sheathing. As an alternate, temporary sheathing may be nailed to the first four feet of AJS[®] Joist at the end of the bay.
- All hangers, AJS[®] rim joists, rim boards, AJS[®] blocking panels, and x-bracing must be completely installed and properly nailed as each AJS[®] Joist is set.
- Install temporary 1x4 strut lines at no more than eight feet on center as additional AJS[®] Joists are set. Nail the strut lines to the sheathed area, or braced end wall, and to each AJS[®] Joist with 2-2¹/₂" (8d) nails.
- The ends of cantilevers must be temporarily secured by strut lines on both the top and bottom flanges.
- Straighten the AJS[®] Joist to within ¹/₂ inch of true alignment before attaching strut lines and sheathing.
- Remove the temporary strut lines only as required to install the permanent sheathing.
- Failure to install temporary bracing may result in sideways buckling or roll-over under light construction loads.
- Do not stack construction materials (sheathing, drywall, etc) in the middle of AJS[®] Joist spans, contact Boise Cascade EWP Engineering for proper storage and shoring information.

to weather or the elements and an environment that is free from moisture from any source, or any pest, organism or substance which degrades or damages wood or glue bonds. Failure to correctly store, use or install VERSA-LAM®, ALLJOIST®, and BCI® Joist in accordance with the Boise Cascade EWP Installation Guide will void the limited warranty.



FLOOR FRAMING DETAILS

Additional floor framing details available with BC FRAMER® software



- LATERAL SUPPORT
- AJS[®] Joists must be laterally supported at the ends with hangers, AJS[®] rim joists, rim boards, AJS[®] blocking panels or x-bracing. AJS[®] blocking panels or x-bracing are required at cantilever supports. Blocking may be required at intermediate
- bearings for floor diaphragm as per Code, consult local building official.

MINIMUM BEARING LENGTH FOR AJS® JOISTS

- 11/2 inches is required at end supports (13/4 inches for 18" to 24" deep). 3½ inches is required at cantilever and intermediate supports.
- Longer bearing lengths allow higher reaction values. Refer to the building Code evaluation report or the BC CALC® software.

NAILING REQUIREMENTS

ALLJOIST® Specifier Guide - CANADA

- AJS® rim joist, rim board or closure panel to AJS[®] Joist:
 - Rims or closure panel 11/4 inches thick and less: 2-21/2" (8d) nails, one each in the top and
 - bottom flange. AJS[®] 140/20 rim joist: 2-31/2" (16d) box
 - nails, one each in the top and bottom flange.

- AJS^{\circledast} 25 rim joist: Toe-nail top flange to rim joist with 2-10d box nails, one each side of flange
- AJS® rim joist, rim board or AJS® blocking panel to support
- 21/2" (8d) nails at 6 inches on center.
- When used for shear transfer, follow the building designer's specification.
- AJS[®] Joist to support:
 - 2-21/2" (8d) nails, one on each side of the web, placed $1\frac{1}{2}$ inches minimum from the end of the AJS[®] Joist to limit splitting.
- Sheathing to AJS[®] Joist:
- Prescriptive residential floor sheathing nailing requires 2½" (8d) common nails @ 6" o.c. on edges and @ 12" o.c. in the field as per Code.
- Maximum nail spacing for minimum lateral stability = 24".
- 14 gauge staples may be substituted for 21/2" (8d) nails if the staples penetrate at least 1 inch into the joist.
- Wood screws may be acceptable, contact local building official and/or Boise Cascade EWP Engineering for further information.

Joist Series	Backer Block Thickness	Filler Block Thickness
AJS® 140	1 ¹ / ₈ " or two ¹ / ₂ " wood panels	2 x + %" wood panel
AJS® 20	1 ¹ / ₈ " or two ¹ / ₂ " wood panels	2 x + %" wood panel
AJS [®] 25	2 x _ lumber	Double 2 x lumber

- Cut backer and filler blocks to a maximum depth equal to the web depth minus 1/4" to avoid a forced fit.
- For deeper AJS® 25 Joists, stack 2x lumber or use multiple pieces of 3/4" wood panels.

WEB STIFFENER REQUIREMENTS

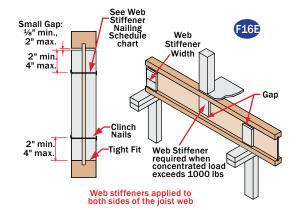
See Web Stiffener Requirements on page 10.

PROTECT AJS® JOISTS FROM THE WEATHER

AJS® Joists is intended only for applications that provide permanent protection from the weather. Bundles of product should be covered and stored off of the ground on stickers.

10

WEB STIFFENER REQUIREMENTS



W	/eb Stiffener S	Specificatior	າຣ
Series	For Structral Capacity (Min. Thick)	Lateral Restraint in Hanger	Minimum Width
AJS [®] 140/20	1"	1"	2 ⁵ / ₁₆ "
AJS [®] 25	2x4	umber (vertic	al)

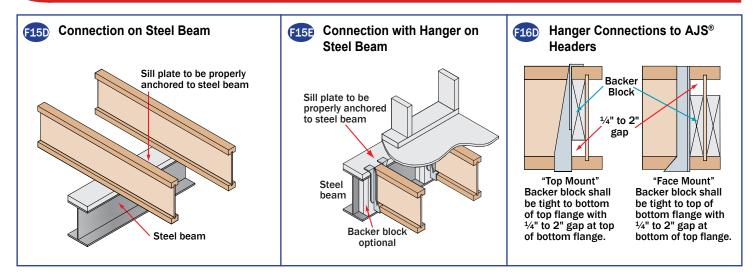
W	eb Stiffener Nailing Sch	edule
Joist Series	Joist Depth	Nailing
AJS® 140 / 20 / 25	91⁄2" – 117⁄8"	3-3" (10d)
AJS- 1407 207 25	14" – 24"	5-3" (10d)

NOTES:

Web stiffeners are optional except as noted below:

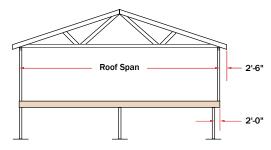
- 1. Stiffeners required at ALL bearing locations for all 18" to 24" deep joists.
- 2. Web stiffeners are always required in hangers that do not extend up to support the top flange of the AJS[®] Joist. Web stiffeners may be required with certain sloped or skewed hangers or to achieve uplift values. Refer to the hanger manufacturer's installation requirements.
- 3. Web stiffeners may be cut from structural rated wood panels, engineered rimboard or 2x lumber (AJS $^{\circ}$ 25 only).
- For Structural Capacity: Web stiffeners needed to increase the AJS[®] Joist's reaction capacity at a specific bearing location.
- 5. Web stiffeners are always required in certain roof applications. See Roof Framing Details on pages 15 & 16.
- 6. Web stiffeners are always required under concentrated loads that exceed 1000 pounds. Install the web stiffeners snug to the top flange in this situation. Follow the nailing schedule for intermediate bearings.
- 7. Web stiffeners may be used to increase allowable reaction values. See *Factored Resistances Limit States Design (CANADA)* on page 4 of this guide or the BC CALC[®] software.

CONNECTION DETAILS





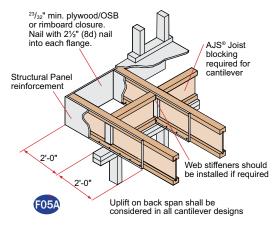
REINFORCED LOAD BEARING CANTILEVER DETAIL 11



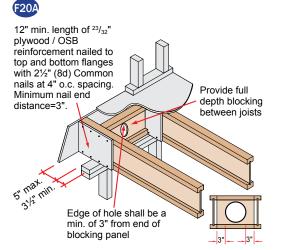
 The tables and details on pages 11 and 12 indicate the type of reinforcements, if any, that are required for load-bearing cantilevers up to a maximum length of 2'-0". Cantilevers longer than 2'-0" cannot be reinforced.
 However, longer cantilevers with lower loads may be allowable without reinforcement. Analyze specific applications with the BC CALC[®] software.

PLYWOOD / OSB REINFORCEMENT (If Required per Table on page 12)

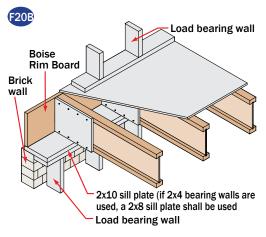
- ²³/₃₂" Min. x 48" long plywood / OSB rated sheathing must match the full depth of the AJS® Joist. Nail to the AJS® Joist with 2½" (8d) nails at 6" o.c. and nail with 4-2½" (8d) nails into backer block. When reinforcing both sides, stagger nails to limit splitting. Install with horizontal face grain.
- These requirements assume a 100 PLF wall load and applied to the AJS[®] Joists series. Additional support may be required for other loadings. See BC CALC[®] software.
- Contact Boise Cascade EWP Engineering for reinforcement requirements on AJS[®] Joist depths greater than 16".



BRICK LEDGE LOAD BEARING CANTILEVER DETAILS

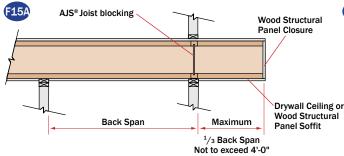


Joist depth	Maximum Factored Vertical load for the off-set bearing wall [plf]													
[in]	Joi	Joist o.c. spacing [in]												
	12	16	19.2	24										
9½	2346	1759	1466	1173										
111⁄%	2912	2184	1820	1456										
14	3420	2565	2138	1710										
16 or higher	3465	2599	2166	1733										



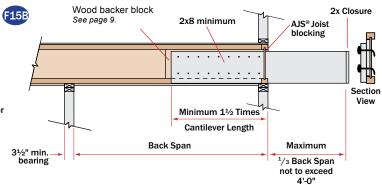
NON-LOAD BEARING WALL CANTILEVER DETAILS

AJS[®] Joists are intended only for applications that provide permanent protection from the weather.



- · These details apply to cantilevers with uniform loads only.
- It may be possible to exceed the limitations of these details by analysing a specific application with the BC CALC[®] software.

Fasten the 2x8 minimum to the AJS[®] Joist by nailing through the backer block and joist web with 2 rows of 3" (10d) nails at 6" on center. Use $3\frac{1}{2}$ " (16d) nails with AJS[®] 25 joists. Clinch all nails.



¹² REINFORCED LOAD BEARING CANTILEVER TABLES

Specified Snow Load [psf] Roof 30 50 Joist Truss 40 Joist Depth Span Joist Spacing [in] [in] Series [ft] 19.2 16 24 19.2 24 16 16 19.2 24 24 0 0 2 0 2 Х Х Х 26 2 0 1 Х х Х Х Х 1 28 Х Х Х Х х 0 1 Х 1 30 0 Х Х Х Х Х Х 1 1 32 2 0 Х Х Х AJS® 140 Х Х Х Х 34 1 Х Х Х Х Х Х Х х 36 1 Х Х Х Х Х Х Х Х 38 1 Х Х Х Х Х Х Х Х 40 1 Х Х Х Х Х Х Х Х 2 Х Х Х 42 Х Х Х Х Х 24 0 0 2 0 2 Х 1 Х Х 26 0 1 Х Х Х 2 Х Х 1 28 Х х Х х 0 1 Х 1 Х 30 0 1 Х 2 Х Х Х Х Х 32 2 AJS® 20 0 2 Х Х Х 91⁄2" Х Х х 34 1 2 Х Х Х Х Х Х Х 36 1 Х Х Х Х Х Х Х Х 38 1 Х Х Х Х Х Х Х Х 40 2 Х Х Х Х Х Х Х Х 42 2 Х Х Х Х Х Х Х Х 24 0 0 Х 0 Х Х 2 Х Х 26 0 Х Х Х 2 Х 1 Х 1 28 0 1 Х 2 Х Х Х Х Х 30 0 2 Х 2 Х Х Х Х Х 2 32 0 Х AJS® Х Х Х Х Х х 25 34 1 Х Х Х Х Х Х Х Х 36 Х Х Х Х Х Х Х Х 1 38 2 Х Х Х Х Х Х Х Х 40 2 Х Х Х Х Х Х Х Х Х Х Х Х 42 Х Х Х Х Х 24 0 0 0 0 0 1 0 1 Х 26 0 0 0 0 WS 0 Х 1 х 28 0 0 1 0 1 Х 0 1 х 30 0 0 1 0 1 Х Х Х 1 32 0 0 1 0 1 Х 1 Х Х AJS® 140 34 0 0 2 0 2 Х Х 1 Х 36 0 0 Х Х Х Х Х 1 1 38 0 Х 1 Х Х Х Х Х 1 40 0 Х 1 Х Х Х Х Х 1 42 Х 0 1 Х 1 Х Х Х х 24 0 0 ws 0 0 0 Х Х 1 26 0 0 WS 0 WS Х 0 1 Х 28 0 0 0 Х 0 2 Х 1 1 30 0 0 1 0 1 Х Х Х 1 AJS® 32 0 0 Х 0 1 Х 1 Х Х 11% 20 34 0 0 0 2 Х Х 1 Х Х 36 WS Х 2 0 Х х Х х 1 38 0 Х Х Х 2 Х Х 1 1 40 0 Х 1 Х Х Х Х Х 1 42 0 1 Х 2 Х Х Х Х х 0 0 0 2 0 24 0 0 1 Х 26 0 0 0 0 0 0 Х 1 Х 28 0 0 1 0 1 Х 0 2 Х 30 0 0 0 1 Х Х Х 1 1 32 0 0 2 0 2 Х Х Х AJS® 1 25 34 0 0 2 0 2 Х 2 Х Х 36 х Х Х 2 Х х 0 0 1 38 0 Х Х Х Х Х Х 1 1 40 2 Х 0 Х Х Х Х 1 х Х 42 0 1 2 Х Х Х Х х

AJS[®] Joists

- 0 No Reinforcement Required
- WS Web Stiffeners at Support
 - 1.... Web Stiffeners Plus One Reinforcer
 - 2.... Web Stiffeners Plus Two Reinforcers

KEY TO TABLE

 $\mathbf{X} \dots$ Use Deeper Joists or Closer Spacing

Tables are based on the following loads: 15 psf specified floor dead load, 40 psf specified floor live load, 100 plf specified wall dead load, 10 psf specified roof dead load and the listed specified snow load (Standard Term Load Duration).

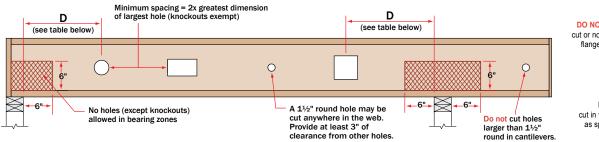
OISIS											
Joist		Roof Truss		20	Sp	ecified	Snow 40	Load [p	osf]	E0	
Depth	Joist	Span		30		Joist	Spacir	na [in]		50	
[i'n]	Series	[ft]	16	19.2	24	16	19.2	24	16	19.2	24
		24	0	0	WS	0	0	WS	0	WS	1
		26	0	0	WS	0	WS	1	0	WS	Х
		28	0	0	WS	0	WS	1	0	WS	Х
		30	0	0	WS	0	WS	Х	WS	1	Х
	AJS®	32	0	0	WS	0	WS	Х	WS	1	Х
	20	34	0	WS	WS	0	WS	Х	WS	1	Х
		36	0	WS	1	WS	1	Х	WS	Х	Х
		38	0	WS	1	WS	1	Х	1	Х	Х
		40	0	WS	1	WS	1	Х	1	Х	х
		42	0	WS	Х	WS	Х	Х	1	Х	Х
4		24	0	0	0	0	0	WS	0	0	1
		26	0	0	0	0	0	1	0	0	2
		28	0	0	0	0	0	1	0	WS	2
		30	0	0	0	0	0	2	0	1	X
	AJS®	32	0	0	WS	0	0	2	0	1	X
	AJS- 25	34	0	0	WS	0	WS	X	0	1	X
		36	0	0	1	0	1	X	0	2	X
		38	0	0	1	0	1	X	1	2	X
		40	0	0	1	0	1	X	1	X	X
		42	0	0	2	0	2	X	1	X	X
		24	0	0	WS	0	0	WS	0	WS	WS
		26	0	0	WS	0	WS	WS	0	WS	WS
		28	0	0	WS	0	WS	WS	0	WS	1
		30	0	0	WS	0	WS	WS	WS	WS	X
		30	0	0	WS	0	WS	1	WS	WS	X
	AJS® 20	34	0	WS	WS	WS	WS	1	WS	WS	X
		36	0	WS	WS	WS	WS	X	WS	WS	X
		38	0	WS	WS	WS	WS	X	WS	1	X
		40	0	WS	WS	WS	WS	X	WS	1	X
		40	0	WS	WS	WS	WS	X	WS	X	X
16"								0			
		24	0	0	0	0	0	WS	0	0	WS WS
		26	-	-	-		0		0	0	
		28	0	0	0	0	0	WS	0	-	1
		30 32	0	0	0	0	0	WS 1	0	WS WS	1 2
	AJS® 25	32	0	0	0	0			0		
	20	34	0	0	WS	0	WS	1	0	WS	2
		36	0	0	WS	0	WS	1	0	WS	X
		38	0	0	WS	0	WS	2	WS	1	X
		40	0	0	WS	0	WS	2	WS	1	X
		42	0	0	WS	0	WS	X	WS	2	X
		24	WS	WS	WS	WS	WS	WS	WS	WS	WS
		26	WS	WS	WS	WS	WS	WS	WS	WS	WS
	"8" "81 to "82 "81 "10" "10" "10" "10" "10" "10" "10"	28	WS	WS	WS	WS	WS	WS	WS	WS	WS
.4"		30	WS	WS	WS	WS	WS	WS	WS	WS	WS
to		32	WS	WS	WS	WS	WS	WS	WS	WS	WS
18"	25	34	WS	WS	WS	WS	WS	WS	WS	WS	WS
·		36	WS	WS	WS	WS	WS	WS	WS	WS	WS
		38	WS	WS	WS	WS	WS	WS	WS	WS	WS
		40	WS	WS	WS	WS	WS	WS	WS	WS	WS
		42	WS	WS	WS	WS	WS	WS	WS	WS	WS

NOTES: 1. Cut 48" long reinforc

 Cut 48" long reinforcers to match the joist depth. Use min.²³/₃₂" APA Rated Sheathing, Exposure 1, 48/24 Span Rating panels. The face grain must be horizontal (measure the 48" dimension along the long edge of the panel).

- 2. Minimum bearing length is 3¹/₂".
- Fasten the reinforcer to the joist flanges with 2½" (8d) nails at 6" o.c. When reinforcing both sides, stagger the nails to limit splitting the joist flanges.
- Attach web stiffeners per intermediate Web Stiffener Nailing Schedule on page 10.
- Use the BC CALC[®] software to analyze conditions that are not covered by this table. It may be possible to exceed the limitations of this table by analyzing a specific application with BC CALC[®] software.

HOLE CUTTING CHARTS FOR RESIDENTIAL APPLICATIONS (40/15) 13



AJS[®] Joists are manufactured with 1¹/₂" round perforated knockouts in the web at approximately 12" on center Minimum distance from support, listed in table below, is required for all holes greater than 1¹/₂"

TAB	LE 1						F	NOS	ND H	OLE	S						I
Mir	nimum	distance	e from i	inside f	ace of a	any sup	port to	the cer	nterline	of hole	J	OISTE	EPTH	• HOLE	SIZE	[IN]	1
Span		91⁄	2"			11	7⁄8"			14	4"			10	6"		
[ft]	3"	6"	9"	12"	3"	6"	9"	12"	3"	6"	9"	12"	3"	6"	9"	12"	
8'	1' - 0''	1' - 6''	-	-	1' - 0''	1' - 0''	-	-	1' - 0''	1' - 0''	1' - 0''	-	1' - 0''	1' - 0''	1' - 0''	1' - 0''	
10'	1' - 0''	2' - 6"	-	-	1' - 0''	1' - 0''	-	-	1' - 0''	1' - 0''	1' - 0''	-	1' - 0''	1' - 0''	1' - 0''	1' - 0''	
12'	1' - 0''	4' - 0''	-	-	1' - 0''	1' - 0''	-	-	1' - 0''	1' - 0''	1' - 0''	-	1' - 0''	1' - 0''	1' - 0''	1' - 6''	
14'	1' - 0''	5' - 0''	-	-	1' - 0''	1' - 0''	-	-	1' - 0''	1' - 0''	1' - 6''	-	1' - 0''	1' - 0''	1' - 0''	2' - 6''	
16'	2' - 0''	6' - 6''	-	-	1' - 0''	2' - 0''	-	-	1' - 0''	1' - 0''	2' - 6"	-	1' - 0''	1' - 0''	1' - 0''	3' - 6''	
18'	3' - 0''	7' - 6''	-	-	1' - 0''	3' - 6''	-	-	1' - 0''	1' - 0''	4' - 0''	-	1' - 0''	1' - 0''	1' - 0''	4' - 6''	
20'	4' - 0''	9' - 0''	-	-	1' - 0''	4' - 6''	-	-	1' - 0''	1' - 0''	5' - 0''	-	1' - 0''	1' - 0''	2' - 0''	6' - 0''	
22'	5' - 0''	10' - 0''	-	-	1' - 6''	5' - 6''	-	-	1' - 0''	2' - 6''	6' - 0''	-	1' - 0''	1' - 0''	3' - 0''	7' - 0''	
24'	6' - 6''	11' - 6''	-	-	2' - 6''	7' - 0''	-	-	1' - 0''	3' - 6''	7' - 6''	-	1' - 0''	1' - 0''	4' - 0''	8' - 0''	
26'	-	-	-	-	4' - 0''	8' - 0''	-	-	1' - 0''	4' - 6''	8' - 6''	-	1' - 0''	1' - 6''	5' - 6''	9' - 6''	
28'	-	-	-	-	5' - 0''	9' - 0''	-	-	2' - 0''	5' - 6''	10' - 0''	-	1' - 0''	2' - 6''	6' - 6''	10' - 6''	
30'	-	-	-	-	-	-	-	-	3' - 0''	7' - 0''	11' - 0''	-	1' - 0''	4' - 0''	7' - 6''	12' - 0''	
32'	-	-	-	-	-	-	-	-	4' - 0''	8' - 0''	12' - 6''	-	1' - 6''	5' - 0''	9' - 0''	13' - 0''	
34'	-	-	-	-	-	-	-	-	-	-	-	-	2' - 6''	6' - 0''	10' - 0''	14' - 6''	

SQUARE HOLES

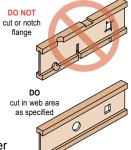
Mir	Minimum distance from inside face of any support to the centerline of hole JOIST DEPTH • HOLE SIZE [IN]																
Span		91⁄	2"			11	/8"			14	4"		16"				
[ft]	3"	6"	9"	12"	3"	6"	9"	12"	3"	6"	9"	12"	3"	6"	9"	12"	
8'	1' - 0''	1' - 6''	-	-	1' - 0''	1' - 0''	-	-	1' - 0''	1' - 0''	1' - 0''	-	1' - 0''	1' - 0''	1' - 0''	1' - 0''	
10'	1' - 0''	2' - 6''	-	-	1' - 0''	1' - 0''	-	-	1' - 0''	1' - 0''	1' - 0''	-	1' - 0''	1' - 0''	1' - 0''	2' - 6''	
12'	1' - 6''	4' - 0''	-	-	1' - 0''	1' - 6''	-	-	1' - 0''	1' - 0''	2' - 6''	-	1' - 0''	1' - 0''	1' - 0''	3' - 6''	
14'	2' - 6''	5' - 0''	-	-	1' - 0''	2' - 6''	-	-	1' - 0''	1' - 0''	3' - 6''	-	1' - 0''	1' - 0''	2' - 0''	4' - 6''	
16'	3' - 6''	6' - 6''	-	-	1' - 6''	4' - 0''	-	-	1' - 0''	2' - 0''	4' - 6''	-	1' - 0''	1' - 0''	3' - 0''	6' - 0''	
18'	5' - 0''	7' - 6''	-	-	2' - 6''	5' - 0''	-	-	1' - 0''	3' - 0''	6' - 0''	-	1' - 0''	1' - 6''	4' - 0''	7' - 0''	
20'	6' - 0''	9' - 0''	-	-	3' - 6''	6' - 6''	-	-	2' - 0''	4' - 6''	7' - 0''	-	1' - 0''	2' - 6''	5' - 6''	8' - 6''	
22'	7' - 0''	10' - 0''	-	-	5' - 0''	7' - 6''	-	-	3' - 0''	5' - 6''	8' - 6''	-	1' - 0''	3' - 6''	6' - 6''	10' - 0''	
24'	8' - 6''	11' - 6''	-	-	6' - 0''	8' - 6''	-	-	4' - 0''	6' - 6''	9' - 6''	-	2' - 0''	5' - 0''	7' - 6''	11' - 0''	
26'	-	-	-	-	7' - 0''	10' - 0''	-	-	5' - 0''	8' - 0''	11' - 0''	-	3' - 6''	6' - 0''	9' - 0''	12' - 6''	
28'	-	-	-	-	8' - 6''	11' - 0''	-	-	6' - 0''	9' - 0''	12' - 0''	-	4' - 6''	7' - 0''	10' - 0''	13' - 6''	
30'	-	-	-	-	-	-	-	-	7' - 6''	10' - 0''	13' - 6''	-	5' - 6''	8' - 6''	11' - 6''	*	
32'	-	-	-	-	-	-	-	-	8' - 6''	11' - 6''	14' - 6''	-	6' - 6''	9' - 6''	12' - 6''	*	
34'	-	-	-	-	-	-	-	-	-	-	-	-	8' - 0''	11' - 0''	14' - 0''	*	

TA	Ы	\mathbf{r}
18	DL	 0

TABLE 2

RECTANGULAR HOLES

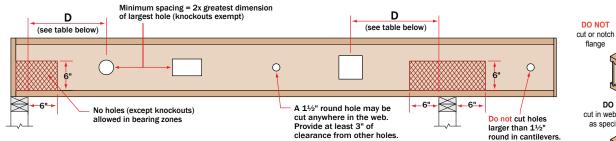
Mir	Minimum distance from inside face of any support to the centerline of hole JOIST DEPTH • HOLE SIZE [IN]																
Span		91,	⁄2"			11	7⁄8"			14	4"		16"				
[ft]	5"x8"	5"x10"	5"x12"	5"x14"	7"x10"	7"x12"	7"x14"	7"x16"	10"x12"	10"x14"	10"x16"	10"x18"	10"x16"	10"x18"	12''x14''	12"x16"	
8'	1' - 6"	2' - 0"	2' - 0"	2' - 6"	1' - 0''	1' - 6''	2' - 0''	2' - 6''	1' - 6''	2' - 6''	3' - 0''	*	1' - 6''	2' - 6''	2' - 0''	3' - 0''	
10'	2' - 6"	3' - 0"	3' - 6"	4' - 0"	2' - 0''	2' - 6"	3' - 6''	4' - 0''	3' - 0''	3' - 6''	4' - 6''	*	3' - 0''	4' - 0''	3' - 0''	4' - 0''	
12'	3' - 6"	4' - 0"	4' - 6"	5' - 0"	3' - 6''	4' - 0''	4' - 6''	5' - 0''	4' - 0''	4' - 6''	5' - 6''	*	4' - 0''	5' - 0''	4' - 6''	5' - 6''	
14'	5' - 0"	5' - 6"	6' - 0"	6' - 6"	4' - 6''	5' - 0''	6' - 0''	6' - 6''	5' - 0''	6' - 0''	*	*	5' - 6''	6' - 6''	5' - 6''	6' - 6''	
16'	6' - 0"	6' - 6"	7' - 0"	7' - 6"	5' - 6''	6' - 6''	7' - 0''	*	6' - 6''	7' - 6''	*	*	6' - 6''	7' - 6''	7' - 0''	*	
18'	7' - 6"	8' - 0"	8' - 6"	*	7' - 0''	7' - 6''	8' - 6''	*	7' - 6''	8' - 6''	*	*	8' - 0''	*	8' - 0''	*	
20'	8' - 6"	9' - 0"	9' - 6"	*	8' - 0''	9' - 0''	9' - 6''	*	9' - 0''	*	*	*	9' - 0''	*	9' - 6''	*	
22'	10' - 0"	10' - 6"	*	*	9' - 6''	10' - 0''	*	*	10' - 6''	*	*	*	10' - 6''	*	*	*	
24'	11' - 0"	*	*	*	10' - 6''	11' - 6''	*	*	11' - 6''	*	*	*	11' - 6''	*	*	*	
26'	-	-	-	-	12' - 0''	*	*	*	*	*	*	*	*	*	*	*	
28'	-	-	-	-	13' - 6''	*	*	*	*	*	*	*	*	*	*	*	
30'	-	-	-	-	-	-	-	-	*	*	*	*	*	*	*	*	
32'	-	-	-	-	-	-	-	-	*	*	*	*	*	*	*	*	
34'	-	-	-	-	-	-	-	-	-	-	-	-	*	*	*	*	



NOTES:

- 1. Hole may be positioned vertically anywhere in the web.
- 2. Tables 1-6 are for uniformly loaded maximum loads of 40 psf live loads and 15 psf dead loads on simple span application.
- 3. For other load conditions or hole sizes, contact your local distributor.
- It may be possible to exceed the limitations of those tables by analysing a specific situation with the BC CALC[®] Software.
- 5. * = Holes may be acceptable, contact your local distributor.

14 HOLE CUTTING CHARTS FOR RESIDENTIAL APPLICATIONS (40/15)



AJS[®] Joists are manufactured with 1¹/₂" round perforated knockouts in the web at approximately 12" on center Minimum distance from support, listed in table below, is required for all holes greater than 1¹/₂"

TAB	LE 4		ROUND HOLES													
Mir	nimum (mum distance from inside face of any support to the centerline of hole JOIST DEPTH • HOLE SIZE [IN]										[IN]				
Span		18	18" 20"						22"				24"			
[ft]	3''	6''	9''	12''	6''	9"	12''	15''	6''	9"	12''	15''	9''	12''	15''	18''
8'	1' - 0''	1' - 0''	1' - 0''	1' - 0''	1' - 0''	1' - 0''	1' - 0''	2' - 0''	1' - 0''	1' - 0''	1' - 0''	1' - 0''	1' - 0''	1' - 0''	1' - 0''	1' - 0''
10'	1' - 0''	1' - 0''	1' - 0''	1' - 0''	1' - 0''	1' - 0''	1' - 0''	3' - 6''	1' - 0''	1' - 0''	1' - 0''	1' - 0''	1' - 0''	1' - 0''	1' - 0''	1' - 0''
12'	1' - 0''	1' - 0''	1' - 0''	2' - 6''	1' - 0''	1' - 0''	1' - 0''	4' - 6''	1' - 0''	1' - 0''	1' - 0''	1' - 0''	1' - 0''	1' - 0''	1' - 0''	2' - 0''
14'	1' - 0''	1' - 0''	1' - 0''	3' - 6''	1' - 0''	1' - 0''	1' - 0''	6' - 0''	1' - 0''	1' - 0''	1' - 0''	1' - 6''	1' - 0''	1' - 0''	1' - 0''	3' - 6''
16'	1' - 0''	1' - 0''	1' - 0''	4' - 6''	1' - 0''	1' - 0''	1' - 0''	7' - 0''	1' - 0''	1' - 0''	1' - 0''	2' - 6''	1' - 0''	1' - 0''	1' - 0''	4' - 6''
18'	1' - 0''	1' - 0''	1' - 0''	6' - 0''	1' - 0''	1' - 0''	1' - 6''	8' - 6''	1' - 0''	1' - 0''	1' - 0''	3' - 6''	1' - 0''	1' - 0''	1' - 0''	5' - 6''
20'	1' - 0''	1' - 0''	1' - 0''	7' - 0''	1' - 0''	1' - 0''	2' - 6''	9' - 6''	1' - 0''	1' - 0''	1' - 0''	5' - 0''	1' - 0''	1' - 0''	1' - 0''	7' - 0''
22'	1' - 0''	1' - 0''	1' - 6''	8' - 6''	1' - 0''	1' - 0''	3' - 6''	*	1' - 0''	1' - 0''	1' - 0''	6' - 0''	1' - 0''	1' - 0''	2' - 0''	8' - 0''
24'	1' - 0''	1' - 0''	2' - 6''	9' - 6''	1' - 0''	1' - 0''	5' - 0''	*	1' - 0''	1' - 0''	1' - 0''	7' - 0''	1' - 0''	1' - 0''	3' - 6''	9' - 6''
26'	1' - 0''	1' - 0''	3' - 6''	11' - 0''	1' - 0''	1' - 0''	6' - 0''	*	1' - 0''	1' - 0''	2' - 6''	8' - 6''	1' - 0''	1' - 0''	4' - 6''	10' - 6''
28'	1' - 0''	1' - 0''	4' - 6''	12' - 0''	1' - 0''	1' - 0''	7' - 0''	*	1' - 0''	1' - 0''	3' - 6''	9' - 6''	1' - 0''	1' - 0''	5' - 6''	12' - 0''
30'	1' - 0''	1' - 0''	5' - 6''	13' - 6''	1' - 0''	2' - 0''	8' - 6''	*	1' - 0''	1' - 0''	4' - 6''	11' - 0''	1' - 0''	1' - 0''	6' - 6''	13' - 0''
32'	1' - 0''	1' - 0''	7' - 0''	14' - 6''	1' - 0''	3' - 0''	9' - 6''	*	1' - 0''	1' - 0''	5' - 6''	12' - 0''	1' - 0''	2' - 6''	8' - 0''	14' - 6''
34'	1' - 0''	1' - 6''	- 6" 8' - 0" 16' - 0" 1' - 0" 4' - 6" 11' - 0" * 1' - 0" 1' - 0" 6' - 6" 13' - 6" 1' - 0" 3' - 6" 9' - 0" 15' - 6									15' - 6''				

SQUARE HOLES

Mir	Minimum distance from inside face of any support to the centerline of hole JOIST DEPTH • HOLE SIZE [IN]											IN]				
Span		18	3"			20)"			2	2"		24"			
[ft]	3''	6''	9''	12"	6"	9''	12''	15''	6''	9''	12''	15''	9''	12''	15''	18''
8'	1' - 0"	1' - 0"	1' - 0"	1' - 0"	1' - 0"	1' - 0"	1' - 0"	1' - 6"	1' - 0"	1' - 0"	1' - 0"	1' - 0"	1' - 0"	1' - 0"	1' - 0"	3' - 0"
10'	1' - 0"	1' - 0"	1' - 0"	1' - 0"	1' - 0"	1' - 0"	1' - 0"	3' - 0"	1' - 0"	1' - 0"	1' - 0"	1' - 6"	1' - 0"	1' - 0"	1' - 0"	4' - 6"
12'	1' - 0"	1' - 0"	1' - 0"	1' - 6"	1' - 0"	1' - 0"	1' - 0"	4' - 0"	1' - 0"	1' - 0"	1' - 0"	2' - 6"	1' - 0"	1' - 0"	1' - 6"	5' - 6"
14'	1' - 0''	1' - 0"	1' - 0"	3' - 0"	1' - 0"	1' - 0"	1' - 6"	5' - 6"	1' - 0"	1' - 0"	1' - 0"	4' - 0"	1' - 0"	1' - 0"	2' - 6"	*
16'	1' - 0"	1' - 0"	1' - 0"	4' - 0"	1' - 0"	1' - 0"	2' - 6"	6' - 6"	1' - 0"	1' - 0"	1' - 6"	5' - 0"	1' - 0"	1' - 0"	4' - 0"	*
18'	1' - 0"	1' - 0"	1' - 6"	5' - 0"	1' - 0"	1' - 0"	3' - 6"	8' - 0"	1' - 0"	1' - 0"	2' - 6"	6' - 6"	1' - 0"	1' - 6"	5' - 0"	*
20'	1' - 0"	1' - 0"	2' - 6"	6' - 6"	1' - 0"	1' - 6"	5' - 0"	9' - 0"	1' - 0"	1' - 0"	3' - 6"	7' - 6"	1' - 0"	2' - 6"	6' - 0"	*
22'	1' - 0"	1' - 0"	3' - 6"	7' - 6"	1' - 0"	2' - 6"	6' - 0"	10' - 6"	1' - 0"	1' - 0"	4' - 6"	9' - 0"	1' - 0"	3' - 6"	7' - 6"	*
24'	1' - 0"	1' - 6"	5' - 0''	9' - 0"	1' - 0"	3' - 6"	7' - 0"	11' - 6"	1' - 0''	2' - 0"	6' - 0"	10' - 0"	1' - 0''	4' - 6"	8' - 6"	*
26'	1' - 0"	2' - 6"	6' - 0''	10' - 0"	1' - 0"	4' - 6"	8' - 6"	*	1' - 0''	3' - 0"	7' - 0"	11' - 6"	2' - 0"	5' - 6"	10' - 0"	*
28'	1' - 0"	3' - 6"	7' - 0"	11' - 6"	2' - 0"	5' - 6"	9' - 6"	*	1' - 0"	4' - 6"	8' - 0"	12' - 6"	3' - 0"	7' - 0"	11' - 0"	*
30'	1' - 0"	4' - 6"	8' - 6"	12' - 6"	3' - 0"	7' - 0"	11' - 0"	*	2' - 0"	5' - 6"	9' - 6"	14' - 0"	4' - 0"	8' - 0"	12' - 6"	*
32'	2' - 6"	5' - 6"	9' - 6"	14' - 0"	4' - 6"	8' - 0"	12' - 0"	*	3' - 0"	6' - 6"	10' - 6"	15' - 0"	5' - 6"	9' - 0"	13' - 6"	*
34'	3' - 6"	7' - 0"	10' - 6"	15' - 0"	5' - 6"	9' - 0"	13' - 6"	*	4' - 0"	7' - 6"	12' - 0"	16' - 6"	6' - 6"	10' - 6"	15' - 0"	*

RECTANGULAR HOLES

Mir	inimum distance from inside face of any support to the centerline of hole JOIST DEPTH • HOLE SIZE [IN]															
Span		18	3"			2	0"			2	2"			24	4"	
[ft]	10"x18"	12"x14"	12"x16"	12"x18"	12''x16''	12''x18''	14''x16''	14''x18''	12''x18"	14''x16''	14"x18"	16"x18"	14''x18''	14''x20''	16"x18"	16''x20''
8'	1' - 6"	1' - 0"	1' - 6"	3' - 0"	1' - 0"	1' - 6"	1' - 6"	3' - 0"	1' - 0"	1' - 0"	2' - 0"	3' - 0"	1' - 0"	2' - 0"	2' - 0"	3' - 6"
10'	2' - 6"	1' - 6"	3' - 0"	4' - 0"	1' - 6"	3' - 0"	3' - 0"	4' - 6"	2' - 0"	1' - 6"	3' - 0"	4' - 6"	2' - 0"	3' - 6"	3' - 0"	*
12'	4' - 0"	3' - 0"	4' - 0"	5' - 6"	2' - 6"	4' - 0"	4' - 0"	5' - 6"	3' - 0"	3' - 0"	4' - 0"	5' - 6"	3' - 0"	4' - 6"	4' - 6"	*
14'	5' - 0"	4' - 0"	5' - 6"	6' - 6"	4' - 0"	5' - 6"	5' - 6"	*	4' - 0"	4' - 0"	5' - 6"	*	4' - 0"	6' - 0"	5' - 6"	*
16'	6' - 6"	5' - 0"	6' - 6"	*	5' - 0"	6' - 6"	6' - 6"	*	5' - 6"	5' - 0"	6' - 6"	*	5' - 6"	7' - 0"	7' - 0"	*
18'	7' - 6"	6' - 6"	8' - 0"	*	6' - 6"	8' - 0"	8' - 0"	*	6' - 6"	6' - 6"	8' - 0"	*	6' - 6"	8' - 6"	8' - 0"	*
20'	9' - 0"	7' - 6"	9' - 0"	*	7' - 6"	9' - 0"	9' - 0"	*	7' - 6"	7' - 6"	9' - 0"	*	8' - 0"	9' - 6"	9' - 6"	*
22'	10' - 0"	9' - 0"	10' - 6"	*	9' - 0"	10' - 6"	10' - 6"	*	9' - 0"	9' - 0"	10' - 6"	*	9' - 0"	*	10' - 6"	*
24'	11' - 6"	10' - 0"	11' - 6"	*	10' - 0"	11' - 6"	11' - 6"	*	10' - 6"	10' - 0"	*	*	10' - 6"	*	*	*
26'	12' - 6"	11' - 6"	*	*	11' - 6"	*	*	*	11' - 6"	11' - 6"	*	*	11' - 6"	*	*	*
28'	*	12' - 6"	*	*	12' - 6"	*	*	*	13' - 0"	12' - 6"	*	*	13' - 0"	*	*	*
30'	*	14' - 0"	*	*	14' - 0"	*	*	*	14' - 0"		*	*	14' - 6"	*	*	*
32'	*	15' - 6"	*	*	15' - 0"	*	*	*	15' - 6"	15' - 6"	*	*	15' - 6"	*	*	*
34'	*	16' - 6"	*	*	16' - 6"	*	*	*	16' - 6"	16' - 6"	*	*	*	*	*	*

DO cut in web area as specified

NOTES:

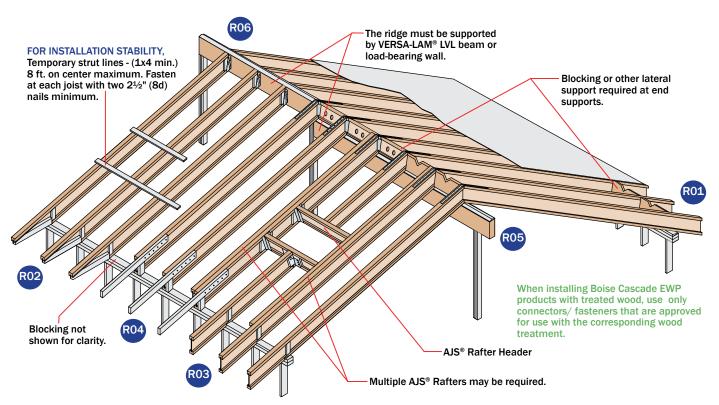
- Hole may be positioned vertically anywhere in the web.
- Tables 1-6 are for uniformly loaded maximum loads of 40 psf live loads and 15 psf dead loads on simple span application.
- For other load conditions or hole sizes, contact your local distributor.
- It may be possible to exceed the limitations of those tables by analysing a specific situation with the BC CALC[®] Software.
- 5. * = Holes may be acceptable, contact your local distributor.

TABLE 5

TABLE 6

ROOF FRAMING

AJS[®] Rafters



SAFETY WARNING

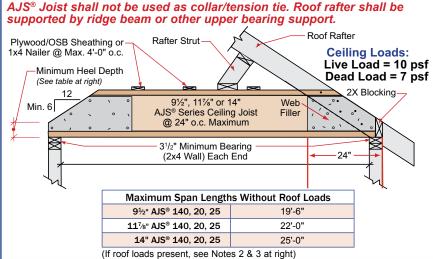
DO NOT ALLOW WORKERS ON AJS® JOISTS UNTIL ALL HANGERS, AJS® RIM JOISTS, RIM BOARDS, AJS® BLOCKING PANELS, X-BRACING AND TEMPORARY 1x4 STRUT LINES ARE INSTALLED AS SPECIFIED BELOW.

- Build a braced end wall at the end of the bay, or permanently install the first eight feet of AJS[®] Joists and the first course of sheathing. As an alternate, temporary sheathing may be nailed to the first four feet of AJS[®] Joists at the end of the bay.
- All hangers, AJS[®] rim joists, rim boards, AJS[®] blocking panels, and x-bracing must be completely installed and properly nailed as each AJS[®] Joist is set.
- Install temporary 1x4 strut lines at no more than eight feet on center as additional AJS[®] Joists are set. Nail the strut lines to the sheathed area, or braced end wall, and to each AJS[®] Joist with two 2½" (8d) nails.

SERIOUS ACCIDENTS CAN RESULT FROM INSUFFICIENT ATTENTION TO PROPER BRACING DURING CONSTRUCTION. ACCIDENTS CAN BE AVOIDED UNDER NORMAL CONDITIONS BY FOLLOWING THESE GUIDELINES:

- The ends of cantilevers must be temporarily secured by strut lines on both the top and bottom flanges.
- Straighten the AJS[®] Joist to within ½ inch of true alignment before attaching strut lines and sheathing.
- Remove the temporary strut lines only as required to install the permanent sheathing.
- Failure to install temporary bracing may result in sideways buckling or roll-over under light construction loads.

Joist



AJS® Ceiling Joist with Bevel End Cut (For Limited-Access Attics Only)

Depth 2 x 4 2 x 6 Minimum Heel 9½" 2½" 1½" Depths 11½" 3½" 2½" 11½" 3½" 2½" 1½" 14" 4½" 3½" 3½"

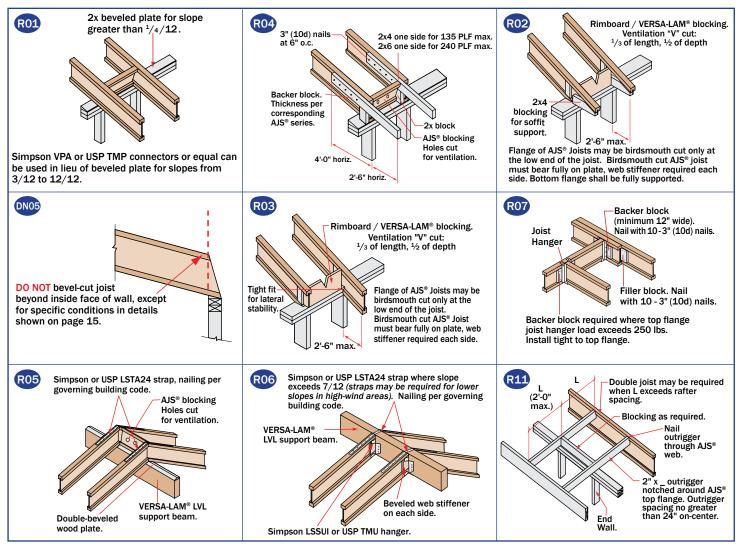
End Wall

osf NOTES:

- Detail is to be used only for ceiling joists with no access to attic space.
- Ceiling joist must be designed to carry all roof load transferred through rafter struts as shown.
- 3) AJS $^{\circ}$ ceiling joist end reaction may not exceed 550 pounds.
- 4) Minimum roof slope is 6/12.
- 5) Nail roof rafter to AJS $^{\otimes}$ top flange with 1-3½" (16d) sinker or box nail.
- 6) 1x4 nailers must be continuous and nailed to a braced end wall.
- Install a web stiffener on each side of AJS[®] Joist at beveled ends. Nail roof rafter to AJS[®] Joist per building code requirements for ceiling joist to roof rafter connection.

ROOF FRAMING DETAILS

Additional roof framing details available with BC FRAMER® software



LATERAL SUPPORT

- AJS[®] Joists must be laterally supported at the ends with hangers, AJS[®] rim joists, rim boards, AJS[®] blocking panels or x-bracing. AJS[®] blocking panels or x-bracing are required at cantilever supports.
- Blocking may be required at intermediate bearings for floor diaphragm as per Code, consult local building official.

MINIMUM BEARING LENGTH FOR AJS® JOISTS

- 1½ inches is required at end supports (1¼ inches for 18" to 24" deep). 3½ inches is required at cantilever and intermediate supports.
- Longer bearing lengths allow higher reaction values. Refer to the building code evaluation report or the BC CALC[®] software.

NAILING REQUIREMENTS

- AJS[®] rim joist, rim board or closure panel to AJS[®] Joist:
 - Rims or closure panel 1³/₄ inches thick and less: 2-2¹/₂" (8d) nails, one each in the top and
 - AJS[®] 140 / 20 rim joist: 2-3½" (16d) box
 - nails, one each in the top and bottom flange.
 - AJS[®] 25 rim joist: Toe-nail top flange to rim joist with 2-3" (10d) box nails, one each side of flange.
- AJS[®] rim joist, rim board or AJS[®] blocking panel to support:
- 2½" (8d) nails at 6 inches on center.
 ALLJOIST[®] Specifier Guide CANADA

- When used for shear transfer, follow the building designer's specification.
- AJS[®] Joist to support:
 - 2-2½" (8d) nails, one on each side of the web, placed ½ inches minimum from the end of the AJS® Joist to limit splitting.
 Sheathing to AJS® Joist:
 - Sneathing to AJS[®] Joist:
 Prescriptive residential roof sheathing nailing requires 2½" (8d) common nails @ 6" o.c. on edges and @ 12" o.c. in the field as per Code.
 - Maximum nail spacing for minimum lateral stability = 24".
 - 14 gauge staples may be substituted for 2½" (8d) nails if the staples penetrate at least 1 inch into the joist.
 - Wood screws may be acceptable, contact local building official and/or Boise Cascade EWP Engineering for further information.

BACKER AND FILLER BLOCK DIMENSIONS

Series	Backer Block Thickness	Filler Block Thickness
AJS® 140	1 ¹ ⁄s" or two ¹ ⁄2" wood panels	2 x + %" wood panel
AJS® 20	1 ¹ ⁄s" or two ½" wood panels	2 x + %" wood panel
AJS® 25	2 x _ lumber	Double 2 x lumber

- Cut backer and filler blocks to a maximum depth equal to the web depth minus 1/4" to avoid a forced fit.
- For deeper AJS[®] 25 Joists, stack 2x lumber or use multiple pieces of ³/₄" wood panels.

WEB STIFFENER REQUIREMENTS

See Web Stiffener Requirements on page 10.

MAXIMUM SLOPE

• Unless otherwise noted, all roof details are valid for slopes of 12 in 12 or less.

VENTILATION

The 1½ inch, pre-stamped knock-out holes spaced at 12 inches on center along the AJS[®] Joist may all be knocked out and used for cross ventilation. Deeper joists than what is structurally needed may be advantageous in ventilation design. Consult local building official and/or ventilation specialist for specific ventilation requirements.

BIRDSMOUTH CUTS

 AJS[®] Joists may be birdsmouth cut only at the low end support. AJS[®] Joists with birdsmouth cuts may cantilever up to 2'-6" past the low end support. The bottom flange must sit fully on the support and may not overhang the inside face of the support. High end supports and intermediate supports may not be birdsmouth cut.

PROTECT AJS® JOISTS FROM THE WEATHER

AJS[®] Joists are intended only for applications that provide permanent protection from the weather. Bundles of AJS[®] Joists should be covered and stored off of the ground on stickers.

		_	Low	Roof Slope	(0.25/12 to	6/12)	High Roof Slope (< 6/12 to 12/12)					
Loads	Joist Series	Depth (in)		o.c. s	bacing			o.c. s	pacing			
	Oches	(11)	12"	16"	19.2"	24"	12"	16"	19.2"	24"		
	AJS®	9½"	24'-8"	22'-4"	21'-0"	19'-0"	22'-1"	20'-0"	18'-10"	17'-5"		
	140	117⁄8"	29'-6"	26'-9"	24'-4"	21'-9"	26'-5"	23'-11"	22'-6"	20'-10"		
<u>م</u> بر		9½"	26'-8"	24'-2"	22'-9"	21'-1"	23'-11"	21'-8"	20'-5"	18'-10"		
sd (AJS®	117⁄8"	31'-11"	28'-11"	27'-2"	25'-2"	28'-7"	25'-11"	24'-4"	22'-6"		
	20	14"	36'-3"	32'-11"	30'-11"	28'-2"	32'-6"	29'-6"	27'-8"	25'-8"		
ad : oad		16"	40'-3"	36'-6"	33'-11"	30'-3"	36'-1"	32'-8"	30'-9"	28'-5"		
ν ΓC		9½"	29'-9"	27'-0"	25'-4"	23'-6"	26'-8"	24'-2"	22'-8"	21'-0"		
ead nov		117⁄8"	35'-6"	32'-2"	30'-3"	28'-0"	31'-10"	28'-10"	27'-1"	25'-1"		
Specified Dead Load = 10 psf Specified Snow Load = 20 psf		14"	40'-4"	36'-7"	34'-4"	31'-10"	36'-2"	32'-9"	30'-9"	28'-6"		
ifie	AJS®	16"	44'-8"	40'-6"	38'-1"	35'-3"	40'-0"	36'-4"	34'-1"	31'-7"		
pec	25	18"	49'-1"	44'-6"	41'-10"	38'-9"	44'-0"	39'-11"	37'-6"	34'-9"		
ິດທ		20"	53'-2"	48'-3"	45'-4"	42'-0"	47'-8"	43'-3"	40'-7"	37'-8"		
		20"	57'-2"	51'-10"	48'-8"	44'-10"	51'-3"	46'-5"	43'-8"	40'-5"		
		24"	60'-0"	55'-4"	52'-0"	46'-10"	54'-9"	49'-7"	46'-8"	43'-3"		
	AJS®	91⁄2"	22'-5"	20'-4"	18'-4"	16'-5"	20'-3"	18'-4"	17'-2"	15'-11"		
	140	117⁄8"	26'-7"	23'-0"	21'-0"	18'-9"	24'-2"	21'-11"	20'-4"	18'-2"		
<u>م</u> 1		91⁄2"	24'-3"	22'-0"	20'-8"	19'-1"	21'-11"	19'-10"	18'-8"	17'-3"		
sd (AJS®	117⁄8"	29'-0"	26'-3"	24'-8"	22'-1"	26'-2"	23'-8"	22'-3"	20'-7"		
°, 10, 10, 10, 10, 10, 10, 10, 10, 10, 10,	20	14"	33'-0"	29'-11"	27'-2"	24'-3"	29'-9"	27'-0"	25'-4"	23'-5"		
ad		16"	36'-7"	32'-0"	29'-3"	26'-0"	33'-0"	29'-11"	28'-1"	24'-9"		
Ľ Č		91⁄2"	27'-1"	24'-6"	23'-0"	21'-3"	24'-5"	22'-1"	20'-9"	19'-2"		
ead nov		117⁄8"	32'-3"	29'-3"	27'-6"	25'-5"	29'-1"	26'-5"	24'-9"	22'-11"		
d d V D		14"	36'-8"	33'-3"	31'-2"	26'-0"	33'-1"	30'-0"	28'-2"	24'-5"		
Specified Dead Load = 10 psf Specified Snow Load = 30 psf	AJS®	16"	40'-7"	36'-10"	32'-11"	26'-4"	36'-8"	33'-2"	31'-0"	24'-9"		
bed	25	18"	44'-8"	40'-6"	38'-0"	34'-11"	40'-4"	36'-6"	34'-4"	31'-9"		
ທິທ		20"	48'-4"	43'-10"	41'-2"	36'-11"	43'-8"	39'-7"	37'-2"	34'-5"		
		22"	52'-0"	47'-2"	43'-4"	38'-8"	46'-11"	42'-6"	40'-0"	37'-0"		
		24"	55'-6"	49'-6"	45'-2"	40'-4"	50'-1"	45'-5"	42'-8"	39'-2"		

NOTES:

- Spans shown are in accordance with NBCC 2010. 1.
- Maximum spans listed are the clear horizontal spans between supports (simple/ multiple spans, or one span + 2ft overhang). Minimum end bearing length is $1\frac{1}{2}$ " for $9\frac{1}{2}$ " to 16" depths, and $1\frac{3}{4}$ " for 18" to 24" 2. 3.
- 4.
- depths. Bold spans have a minimum end bearing length of $3\frac{1}{2}$ ". Minimum interior bearing length is $3\frac{1}{2}$ ". Joists deeper than 16" require web stiffeners at all bearing locations. 5.
- Total load deflection is limited to L/180. Live load deflection is limited to L/240. 6. 7.
- 8.
- Check the local building code for other deflection limits that may apply.
- Spans shown have not been evaluated for snow drift. Lateral support must be provided for the compression edge and also at the bearings to 9. 10. Slope roof joists at least ¼" over 12" to minimize ponding. Allowable spans and loads shall be adjusted and checked for wind loads as required
- 11. 12.
- by local building code.
- It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC® or BC FRAMER® software. 13.
- 14. When using continuous spans over an intermediate bearing, the shortest span shall not be less than 50% of the longest adjacent span.

15. Uplift = L_2^* (factor₁*W_{FD}-W_{FL})/factor₂ where: W_{FD} = Factored dead load (lb/ft) W_{FL} = Factored live load (lb/ft) L₂ = Length of longer span (ft) factor = 8-0(4=0)

W_{FL} = Factored II L ₂ = Length of lo factor ₂ = 8a(1+a)	L_1 = Length of shorter span (ft) factor ₁ = 4a ² + 3a ³ - 1 a = Short span/Long span						
Short/Long span ratio = a	0.5	0.6	0.7	0.8	0.9	1	
factor ₁	0.38	1.09	1.99	3.10	4.43	6.00	
factor ₂	6	7.68	9.52	11.52	13.68	16	

16. Actual deflection based on span and deflection limit.

	Actual deflection (in)											
Deflection limit		Span (ft)										
	15	17	20	25	30	35	40					
L/240	0.75	0.85	1.00	1.25	1.50	1.75	2.00					
L/180	1.00	1.13	1.33	1.67	2.00	2.33	2.67					

17. WARNING: Use of Span Tables for Commercial Projects (NBCC2010: Part 4) All projects within the scope of Part 4 of the National Building Code of Canada (NBCC) must consider the effects of concentrated loads, as stipulated in article 4.1.5.9. The designer of record must verify the effects of a concentrated load on the joists on all projects within the scope of Part 4 of NBCC (2010). Table 4.1.5.9 in NBCC (2010) lists concentrated loads that shall be analyzed with respect to the intended use of the floor. Given the numerous possible permutations, the span tables listed above do not take the offent of concentrated load into concidentiated. take the effects of concentrated loads into consideration

		_	Low	Roof Slope	(0.25/12 to	6/12)	High Roof Slope (< 6/12 to 12/12)					
Loads	Joist Series	Depth (in)		o.c. s	pacing			o.c. sp	bacing			
	Series	(11)	12"	16"	19.2"	24"	12"	16"	19.2"	2/12) 24" 14'-3" 16'-0" 19'-2" 19'-9" 20'-0" 17'-10" 19'-5" 19'-9" 20'-0" 19'-5" 19'-9" 20'-0" 19'-5" 19'-9" 20'-0" 19'-9" 10'-10" 19'-9" 20'-0" 19'-9" 10'-5" 10'-6" 16'-6" 16'-9" 16'-9" 16'-9" 16'-9" 16'-9" 27'-5" 29'-5" 30'-10"		
	AJS®	91⁄2"	20'-6"	17'-11"	16'-4"	14'-7"	18'-10"	17'-1"	16'-0"	14'-3"		
	140	117⁄8"	23'-9"	20'-6"	18'-8"	16'-8"	22'-7"	20'-0"	18'-3"	16'-3"		
ور شر		91⁄2"	22'-2"	20'-1"	18'-10"	17'-5"	20'-5"	18'-6"	17'-4"	16'-0"		
10 psf = 40 psf	AJS®	117⁄8"	26'-6"	24'-0"	22'-1"	19'-8"	24'-5"	22'-1"	20'-9"	19'-2"		
= <u>-</u> 5 4	20	14"	30'-2"	26'-7"	24'-3"	20'-8"	27'-9"	25'-2"	23'-7"	19'-9"		
ad		16"	33'-0"	28'-7"	26'-3"	21'-0"	30'-9"	27'-11"	25'-0"	20'-0"		
v Ľo		91⁄2"	24'-9"	22'-5"	21'-0"	19'-5"	22'-9"	20'-7"	19'-4"	17'-10"		
Dead Load Snow Load		117⁄8"	29'-6"	26'-9"	25'-1"	20'-4"	27'-2"	24'-7"	23'-1"	19'-5"		
Specified Dead Load Specified Snow Load		14"	33'-6"	30'-4"	25'-11"	20'-8"	30'-10"	27'-11"	24'-8"	19'-9"		
ifie	AJS®	16"	37'-2"	31'-6"	26'-3"	21'-0"	34'-2"	30'-0"	25'-0"	20'-0"		
bed	25	18"	40'-10"	37'-0"	34'-9"	31'-2"	37'-7"	34'-0"	32'-0"	29'-7"		
ິດທ		20"	44'-3"	40'-1"	36'-11"	32'-11"	40'-8"	36'-11"	34'-8"	32'-1"		
		20"	47'-7"	42'-4"	38'-8"	34'-6"	43'-9"	39'-8"	37'-3"	33'-8"		
		24"	50'-10"	44'-2"	40'-4"	36'-0"	46'-9"	42'-4"	39'-4"	35'-2"		
	AJS®	9½"	19'-0"	16'-4"	14'-11"	13'-4"	17'-6"	15'-10"	14'-7"	13'-0"		
	140	117⁄8"	21'-7"	18'-8"	17'-0"	15'-2"	20'-11"	18'-3"	16'-8"	14'-11"		
s, "t		91⁄2"	20'-7"	18'-7"	17'-5"	15'-8"	18'-11"	17'-1"	16'-1"	14'-10"		
10 psf : 50 psf	AJS®	117⁄8"	24'-7"	22'-3"	20'-1"	16'-11"	22'-8"	20'-6"	19'-2"	16'-3"		
	20	14"	27'-11"	24'-2"	21'-6"	17'-2"	25'-9"	23'-4"	20'-8"	16'-6"		
ad oad		16"	30'-1"	26'-2"	21'-10"	17'-5"	28'-7"	25'-2"	20'-11"	16'-9"		
Υ Υ Γ		9½"	22'-11"	20'-8"	19'-5"	16'-2"	21'-1"	19'-1"	17'-10"	15'-7"		
eac		117⁄8"	27'-4"	24'-9"	21'-2"	16'-11"	25'-2"	22'-9"	20'-4"	16'-3"		
о С О О		14"	31'-0"	25'-10"	21'-6"	17'-2"	28'-7"	24'-9"	20'-8"	16'-6"		
Specified Dead Load = Specified Snow Load =	AJS®	16"	34'-5"	26'-2"	21'-10"	17'-5"	31'-8"	25'-2"	20'-11"	16'-9"		
bec	25	18"	37'-10"	34'-3"	31'-9"	28'-5"	34'-11"	31'-7"	29'-8"	27'-5"		
S S		20"	41'-0"	36'-10"	33'-7"	30'-0"	37'-10"	34'-3"	32'-2"	29'-5"		
		22"	44'-1"	38'-7"	35'-2"	31'-6"	40'-8"	36'-10"	34'-7"	30'-10"		
		24"	46'-6"	40'-3"	36'-9"	32'-10"	43'-5"	39'-4"	36'-0"	32'-2"		

NOTES:

- Spans shown are in accordance with NBCC 2010.
- Maximum spans listed are the clear horizontal spans between supports (simple/multiple spans, or one span + 2ft overhang). Minimum end bearing length is $1\frac{1}{2}$ " for $9\frac{1}{2}$ " to 16" depths, and $1\frac{3}{4}$ " for 18" to 24" 2.
- 3.
- 4
- depths. Bold spans have a minimum end bearing length of $3\frac{1}{2}$ ". Minimum interior bearing length is $3\frac{1}{2}$ ". Joists deeper than 16" require web stiffeners at all bearing locations. 5
- Total load deflection is limited to L/180. Live load deflection is limited to L/240. 6.
- 7. 8
- Check the local building code for other deflection limits that may apply.
- Spans shown have not been evaluated for snow drift. Lateral support must be provided for the compression edge and also at the bearings to 10. prevent lateral displacement or rotation.
- 11
- Slope roof joists at least $\frac{1}{2}$ voer 12" to minimize ponding. Allowable spans and loads shall be adjusted and checked for wind loads as required 12. by local building code.
- It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC® or BC FRAMER® software. 13.
- 14. When using continuous spans over an intermediate bearing, the shortest span shall not be less than 50% of the longest adjacent span.

15. Uplift = L₂*(factor₁*W_{FD}-W_{FL})/factor₂

where: W_{FD} = Factored of W_{FL} = Factored II L_2 = Length of Io factor ₂ = 8a(1+a)	L₁ fac	L₁ = Length of shorter span (ft) factor₁ = 4a² + 3a³ - 1 a = Short span/Long span					
Short/Long span ratio = a	0.7	0.8	0.9	1			
factor ₁	0.38	1.09	1.99	3.10	4.43	6.00	
factor ₂	6	7.68	9.52	11.52	13.68	16	

16. Actual deflection based on span and deflection limit.

Deflection		Actual deflection (in)											
Deflection		Span (ft)											
mm	15 17 20 25 30 35												
L/240	0.75	0.85	1.00	1.25	1.50	1.75	2.00						
L/180	1.00	1.13	1.33	1.67	2.00	2.33	2.67						

17. WARNING: Use of Span Tables for Commercial Projects (NBCC2010: Part 4) All warkinks, base of span tables for Confinencial Projects (NBCC201), Part 4) All projects within the scope of Part 4 of the National Building Code of Canada (NBCC) must consider the effects of concentrated loads, as stipulated in article 4.1.5.9. The designer of record must verify the effects of a concentrated load on the joists on all projects within the scope of Part 4 of NBCC (2010). Table 4.1.5.9 in NBCC (2010) lists concentrated loads that shall be analyzed with respect to the intended use of the floor. Given the numerous possible permutations, the span tables listed above do not take the effects of concentrated loads into consideration. take the effects of concentrated loads into consideration.

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		_	Low	Roof Slope	(0.25/12 to	6/12)	High Roof Slope (< 6/12 to 12/12)					
Loads	Joist Series	Depth (in)		o.c. s	pacing			o.c. s	bacing			
	Genes	(11)	12"	16"	19.2"	24"	12"	16"	19.2"	24"		
	AJS®	9½"	23'-4"	21'-1"	19'-10"	17'-8"	20'-9"	18'-9"	17'-8"	16'-4"		
	140	117⁄8"	27'-10"	24'-9"	22'-7"	20'-2"	24'-10"	22'-6"	21'-1"	19'-2"		
<u>م</u> بر		9½"	25'-3"	22'-11"	21'-6"	19'-11"	22'-6"	20'-4"	19'-1"	17'-8"		
sd g	AJS®	117⁄8"	30'-2"	27'-4"	25'-8"	23'-9"	26'-10"	24'-4"	22'-10"	21'-2"		
14	20	14"	34'-4"	31'-1"	29'-3"	26'-1"	30'-6"	27'-8"	26'-0"	24'-1"		
ad : bad		16"	38'-0"	34'-6"	31'-5"	28'-1"	33'-10"	30'-8"	28'-10"	26'-8"		
v Lo		9½"	28'-2"	25'-6"	23'-11"	22'-2"	25'-0"	22'-8"	21'-4"	19'-9"		
ead nov		117⁄8"	33'-7"	30'-5"	28'-7"	26'-5"	29'-10"	27'-1"	25'-5"	23'-6"		
d N d N		14"	38'-2"	34'-7"	32'-6"	30'-1"	33'-11"	30'-9"	28'-11"	26'-9"		
lifie Lifie	AJS®	16"	42'-3"	38'-3"	36'-0"	30'-6"	37'-7"	34'-1"	32'-0"	27'-6"		
Specified Dead Load = 15 psf Specified Snow Load = 20 psf	25	18"	46'-5"	42'-1"	39'-7"	36'-8"	41'-4"	37'-5"	35'-2"	32'-7"		
ິດເບ		20"	50'-3"	45'-7"	42'-10"	39'-8"	44'-9"	40'-7"	38'-1"	35'-4"		
		20"	54'-0"	49'-0"	46'-1"	41'-8"	48'-1"	43'-7"	41'-0"	38'-0"		
		24"	57'-9"	52'-4"	48'-7"	43'-5"	51'-4"	46'-7"	43'-9"	40'-7"		
	AJS®	91⁄2"	21'-5"	19'-0"	17'-4"	15'-6"	19'-3"	17'-5"	16'-4"	14'-10"		
	140	117⁄8"	25'-2"	21'-9"	19'-10"	17'-8"	23'-0"	20'-10"	19'-0"	17'-0"		
ی در		91⁄2"	23'-3"	21'-1"	19'-9"	18'-4"	20'-10"	18'-10"	17'-9"	16'-5"		
sd g	AJS®	117⁄8"	27'-9"	25'-2"	23'-4"	20'-10"	24'-11"	22'-6"	21'-2"	19'-7"		
11	20	14"	31'-7"	28'-2"	25'-8"	22'-10"	28'-4"	25'-8"	24'-1"	21'-5"		
ad		16"	35'-0"	30'-3"	27'-7"	23'-7"	31'-5"	28'-5"	26'-9"	21'-9"		
ς Γο Γ		91⁄2"	25'-11"	23'-6"	22'-0"	20'-4"	23'-3"	21'-0"	19'-9"	18'-3"		
ead nov		117⁄8"	30'-11"	28'-0"	26'-3"	22'-10"	27'-8"	25'-1"	23'-7"	21'-1"		
d N D N		14"	35'-1"	31'-10"	29'-0"	23'-3"	31'-6"	28'-6"	26'-9"	21'-5"		
Specified Dead Load = 15 psf Specified Snow Load = 30 psf	AJS®	16"	38'-11"	35'-3"	29'-5"	23'-7"	34'-10"	31'-7"	27'-2"	21'-9"		
bed	25	18"	42'-9"	38'-9"	36'-5"	33'-0"	38'-4"	34'-9"	32'-8"	30'-3"		
ິດທ		20"	46'-4"	42'-0"	39'-1"	34'-11"	41'-6"	37'-8"	35'-4"	32'-9"		
		22"	49'-9"	44'-10"	40'-11"	36'-7"	44'-8"	40'-5"	38'-0"	35'-2"		
		24"	53'-2"	46'-10"	42'-8"	38'-2"	47'-8"	43'-3"	40'-7"	36'-8"		

NOTES:

- Spans shown are in accordance with NBCC 2010. 1.
- Maximum spans listed are the clear horizontal spans between supports (simple/multiple spans, or one span + 2ft overhang). Minimum end bearing length is $1/2^{"}$ for $9/2^{"}$ to 16" depths, and $1/4^{"}$ for 18" to 24" 2. 3.
- depths. Bold spans have a minimum end bearing length of $3\frac{1}{2}$ ". Minimum interior bearing length is $3\frac{1}{2}$ ". Joists deeper than 16" require web stiffeners at all bearing locations.
- 4. 5.
- 6.
- Total load deflection is limited to L/180. Live load deflection is limited to L/240. 7.
- 8. Check the local building code for other deflection limits that may apply.
- 9. 10. Spans shown have not been evaluated for snow drift. Lateral support must be provided for the compression edge and also at the bearings to Prevent lateral displacement or rotation. Slope roof joists at least ¼" over 12" to minimize ponding. Allowable spans and loads shall be adjusted and checked for wind loads as required
- 11
- 12. by local building code.
- It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC® or BC FRAMER® software. 13.
- 14. When using continuous spans over an intermediate bearing, the shortest span shall not be less than 50% of the longest adjacent span.

Short/Long span ratio = a

Uplift = L₂*(factor₁*W_{FD}-W_{FL})/factor₂ where: W_{FD} = Factored dead load (lb/ft)

iere:	W_{FD} = Factored dead load (ID/III)	
	W _{FI} = Factored live load (lb/ft)	
	L_2 = Length of longer span (ft)	

W_{FL} = Factored II L ₂ = Length of Io factor ₂ = 8a(1+a)	nger sp		L ₁ = Length of shorter spar factor ₁ = 4a ² + 3a ³ - 1 a = Short span/Long span						
'Long span ratio = a	0.5	0.6	0.7	0.8	0.9	1			
factor ₁	0.38	1.09	1.99	3.10	4.43	6.00			
factor ₂	6	7.68	9.52	11.52	13.68	16			

factor ₁	0.38	1.09	1.99	3.10	4.43
factor ₂	6	7.68	9.52	11.52	13.68

16. Actual deflection based on span and deflection limit.

			Actu	ual deflection	(in)		
Deflection limit				Span (ft)			
	15	17	20	25	30	35	40
L/240	0.75	0.85	1.00	1.25	1.50	1.75	2.00
L/180	1.00	1.13	1.33	1.67	2.00	2.33	2.67

17. WARNING: Use of Span Tables for Commercial Projects (NBCC2010: Part 4) All projects within the scope of Part 4 of the National Building Code of Canada (NBCC) must consider the effects of concentrated loads, as stipulated in article 4.1.5.9. The designer of record must verify the effects of a concentrated load on the joists on all projects within the scope of Part 4 of NBCC (2010). Table 4.1.5.9 in NBCC (2010) lists concentrated loads that shall be analyzed with respect to the intended use of the floor. Given the numerous possible permutations, the span tables listed above do not take the effects of concentrated loads into consideration.

			Low	Roof Slope	(0.25/12 to	6/12)	High	Roof Slope	(< 6/12 to [,]	12/12)
Loads	Joist Series	Depth (in)		o.c. s	pacing			o.c. sp	bacing	
	Series	(11)	12"	16"	19.2"	24"	12"	16"	19.2"	24"
	AJS®	9½"	19'-10"	17'-2"	15'-8"	13'-11"	18'-1"	16'-4"	15'-1"	13'-6"
	140	117⁄8"	22'-8"	19'-7"	17'-10"	15'-11"	21'-8"	18'-11"	17'-3"	15'-5"
ور شر		91⁄2"	21'-9"	19'-8"	18'-6"	16'-6"	19'-7"	17'-9"	16'-8"	15'-5"
= 15 psf = 40 psf	AJS®	117⁄8"	26'-0"	23'-1"	21'-1"	18'-7"	23'-5"	21'-2"	19'-11"	17'-5"
= 15 psf = 40 psi	20	14"	29'-4"	25'-4"	23'-1"	18'-11"	26'-7"	24'-1"	22'-1"	17'-8"
ad		16"	31'-7"	27'-4"	24'-0"	19'-2"	29'-6"	26'-4"	22'-5"	17'-11"
Dead Load Snow Load		91⁄2"	24'-3"	21'-11"	20'-7"	17'-10"	21'-10"	19'-9"	18'-6"	16'-8"
eac		117⁄8"	28'-11"	26'-2"	23'-3"	18'-7"	26'-0"	23'-7"	21'-9"	17'-5"
а а о о		14"	32'-10"	28'-5"	23'-8"	18'-11"	29'-7"	26'-7"	22'-1"	17'-8"
ifie	AJS® 25	16"	36'-5"	28'-9"	24'-0"	19'-2"	32'-9"	26'-11"	22'-5"	17'-11"
Specified Dead Load Specified Snow Load		18"	40'-0"	36'-3"	33'-4"	29'-9"	36'-1"	32'-8"	30'-8"	28'-5"
ິດທ		20"	43'-4"	38'-8"	35'-3"	31'-6"	39'-1"	35'-5"	33'-3"	30'-5"
		20"	46'-7"	40'-6"	36'-11"	33'-0"	42'-0"	38'-0"	35'-9"	31'-11"
		24"	48'-9"	42'-3"	38'-6"	34'-5"	44'-10"	40'-8"	37'-3"	33'-3"
	AJS®	91⁄2"	18'-2"	15'-9"	14'-4"	12'-10"	17'-2"	15'-3"	13'-11"	12'-5"
	140	117⁄8"	20'-9"	18'-0"	16'-5"	14'-7"	20'-2"	17'-6"	15'-11"	14'-2"
s, "		91⁄2"	20'-7"	18'-7"	16'-11"	15'-0"	18'-7"	16'-10"	15'-9"	14'-2"
р 5 0 2 0	AJS®	117⁄8"	24'-7"	21'-2"	19'-4"	15'-8"	22'-2"	20'-1"	18'-6"	14'-10"
1 1 1	20	14"	26'-11"	23'-3"	19'-11"	15'-11"	25'-3"	22'-8"	18'-10"	15'-1"
ad oad		16"	29'-0"	24'-3"	20'-3"	16'-2"	28'-0"	22'-11"	19'-1"	15'-3"
ν Γ Γ		91⁄2"	22'-11"	20'-8"	18'-10"	15'-0"	20'-8"	18'-8"	17'-6"	14'-2"
eac		117⁄8"	27'-4"	23'-6"	19'-7"	15'-8"	24'-8"	22'-3"	18'-6"	14'-10"
D S S Q D		14"	31'-0"	23'-11"	19'-11"	15'-11"	28'-1"	22'-8"	18'-10"	15'-1"
Specified Dead Load = 15 psf Specified Snow Load = 50 psf	AJS®	16"	32'-5"	24'-3"	20'-3"	16'-2"	30'-7"	22'-11"	19'-1"	15'-3"
spe(25	18"	37'-10"	33'-6"	30'-7"	27'-4"	34'-2"	31'-0"	29'-1"	26'-7"
00		20"	41'-0"	35'-5"	32'-4"	28'-11"	37'-1"	33'-7"	31'-6"	28'-1"
		22"	42'-11"	37'-2"	33'-11"	30'-3"	39'-10"	36'-1"	32'-11"	29'-5"
		24"	44'-9"	38'-9"	35'-4"	31'-7"	42'-7"	37'-8"	34'-4"	30'-8"

NOTES:

- Spans shown are in accordance with NBCC 2010. 1.
- Maximum spans listed are the clear horizontal spans between supports (simple/multiple spans, or one span + 2ft overhang). Minimum end bearing length is $1/2^{\circ}$ for $9/2^{\circ}$ to 16" depths, and 1°_{4} " for 18" to 24" 2.
- 3. depths.
- 4.
- Minimum interior bearing length is 3½". Joists deeper than 16" require web stiffeners at all bearing locations. 5.
- Total load deflection is limited to L/180. Live load deflection is limited to L/240. 6.
- 7. 8.
- Check the local building code for other deflection limits that may apply.
- Spans shown have not been evaluated for snow drift. Lateral support must be provided for the compression edge and also at the bearings to prevent lateral displacement or rotation. 10.
- 11
- Slope roof joists at least $\frac{1}{2}$ " over 12" to minimize ponding. Allowable spans and loads shall be adjusted and checked for wind loads as required 12. by local building code.
- It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC® or BC FRAMER® software. When using continuous spans over an intermediate bearing, the shortest span shall 13.
- 14. not be less than 50% of the longest adjacent span.

15. Uplift = $L_2^*(factor_1^*W_{FD}-W_{FL})/factor_2$ where: W_{FD} = Factored dead load (lb/ft) W_{FL} = Factored live load (lb/ft) L_2 = Length of longer span (ft) factor = $2e_1(4=0)$ factor2 = 8a(1-

L₁ = Length of shorter span (ft) factor1 = 4a2 + 3a3 - 1

1+a)			a =	= Short	span/Lo	ong spar
a	0.5	0.6	0.7	0.8	0.9	1

Short/Long span ratio = a	0.5	0.6	0.7	0.8	0.9	1
factor ₁	0.38	1.09	1.99	3.10	4.43	6.00
factor ₂	6	7.68	9.52	11.52	13.68	16

16. Actual deflection based on span and deflection limit.

			Actu	ual deflection	(in)		
Deflection limit				Span (ft)			
	15	17	20	25	30	35	40
L/240	0.75	0.85	1.00	1.25	1.50	1.75	2.00
L/180	1.00	1.13	1.33	1.67	2.00	2.33	2.67

17. WARNING: Use of Span Tables for Commercial Projects (NBCC2010: Part 4) All WARNING. Use of Span Tables for Confinencial Projects (NBCC201), Part 4) All projects within the scope of Part 4 of the National Building Code of Canada (NBCC) must consider the effects of concentrated loads, as stipulated in article 4.1.5.9. The designer of record must verify the effects of a concentrated load on the joists on all projects within the scope of Part 4 of NBCC (2010). Table 4.1.5.9 in NBCC (2010) lists concentrated loads that shall be analyzed with respect to the intended use of the floor. Given the numerous possible permutations, the span tables listed above do not take the effects of concentrated load in the consideration. take the effects of concentrated loads into consideration.

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ROOF ALLOWABLE UNIFORM LOADS, PLF

Design	Joist Series	AJS	◎ 140		AJS	® 20					AJS	® 25			
Span (ft)	Joist Depth (in)	9 ½	111⁄8	9 ½	11 1⁄%	14	16	9 ½	111%	14	16	18	20	22	24
6	Unfactored Live Load for L / 360 [plf]	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	Unfactored Total Load for L / 240 [plf]	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	Factored Total Load [plf]	488	501	488	501	505	510	488	501	505	510	993	1075	1100	1124
8	Unfactored Live Load for L / 360 [plf]	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	Unfactored Total Load for L / 240 [plf]	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	Factored Total Load [plf]	366	376	366	376	378	382	366	376	378	382	745	806	825	843
10	Unfactored Live Load for L / 360 [plf]	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	Unfactored Total Load for L / 240 [plf]	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	Factored Total Load [plf]	292	301	292	301	303	306	292	301	303	306	596	645	660	674
12	Unfactored Live Load for L / 360 [plf] Unfactored Total Load for L / 240 [plf] Factored Total Load [plf]	207 - 244	- - 250	- - 244	- 250	- - 252	- 255	- - 244	- - 250	- - 252	- - 255	- - 496	- 537	- - 550	- - 562
14	Unfactored Live Load for L / 360 [plf]	134	-	167	-	-	-	-	-	-	-	-	-	-	-
	Unfactored Total Load for L / 240 [plf]	179	-	-	-	-	-	-	-	-	-	-	-	-	-
	Factored Total Load [plf]	209	215	209	215	216	218	209	215	216	218	425	460	471	482
16	Unfactored Live Load for L / 360 [plf]	92	153	115	-	-	-	155	-	-	-	-	-	-	-
	Unfactored Total Load for L / 240 [plf]	122	-	153	-	-	-	-	-	-	-	-	-	-	-
	Factored Total Load [plf]	183	188	183	188	189	191	183	188	189	191	372	403	412	421
18	Unfactored Live Load for L / 360 [plf]	65	109	82	136	-	-	111	-	-	-	-	-	-	-
	Unfactored Total Load for L / 240 [plf]	87	146	109	-	-	-	148	-	-	-	-	-	-	-
	Factored Total Load [plf]	162	167	162	167	168	170	162	167	168	170	331	358	366	374
20	Unfactored Live Load for L / 360 [plf]	48	81	60	101	146	-	82	136	-	-	-	-	-	-
	Unfactored Total Load for L / 240 [plf]	64	108	81	135	-	-	110	-	-	-	-	-	-	-
	Factored Total Load [plf]	146	150	146	150	151	153	146	150	151	153	298	322	330	337
22	Unfactored Live Load for L / 360 [plf]	36	61	46	77	111	-	62	104	-	-	264	-	-	-
	Unfactored Total Load for L / 240 [plf]	48	82	61	102	-	-	83	-	-	-	-	-	-	-
	Factored Total Load [plf]	133	136	133	136	137	139	133	136	137	139	270	293	300	306
24	Unfactored Live Load for L / 360 [plf]	28	47	35	60	87	117	49	81	117	-	207	259	-	-
	Unfactored Total Load for L / 240 [plf]	37	63	47	80	116	-	65	108	-	-	-	-	-	-
	Factored Total Load [plf]	122	125	122	125	126	127	122	125	126	127	248	268	275	281
26	Unfactored Live Load for L / 360 [plf] Unfactored Total Load for L / 240 [plf] Factored Total Load [plf]		37 50 115	28 37 112	47 63 115	69 92 116	93 - 117	38 51 112	64 86 115	93 - 116	- - 117	165 220 229	207 - 248	- - 254	- - 259
28	Unfactored Live Load for L / 360 [plf] Unfactored Total Load for L / 240 [plf] Factored Total Load [plf]		30 40 107		38 51 107	55 74 108	75 100 109	31 41 104	52 69 107	75 100 108	101 - 109	133 178 212	168 224 230	206 - 235	- - 241
30	Unfactored Live Load for L / 360 [plf] Unfactored Total Load for L / 240 [plf] Factored Total Load [plf]				31 41 100	45 60 101	61 82 102	25 34 97	42 57 100	61 82 101	83 - 102	109 146 198	138 184 215	169 - 220	205 - 224

NOTES:

- 1. Total Factored Load values are limited by shear, end/interior reactions or bending moment.
- Unfactored Live Load values are limited by deflection equal to L / 360. For deflections limited to L / 480, multiply live load values by 0.75 (recommended for less vibration).
- 3. Unfactored Total Load values are limited by deflection equal to L / 240.
- 4. All three loading cases must be checked. Where a Live Load value is not shown, the Factored Total Load value will control.
- 5. Joists deeper than 16" require web stiffeners at all bearing locations.
- 6. Table values represent the most restrictive of simple or continuous span beams applications and assume an uniform loading. Span is measured center to center of the supports. Analyze continuous span beams with the BC Calc software if the length of any span is less than half the length of an adjacent span.
- 7. Table values assume that lateral support is provided at each support and continuously along the compression edge of the beam.
- 8. Table values do not consider composite action from gluing and nailing floor sheathing.
- 9. For 2-ply, double the Factored Total Load, Unfactored Live and Total Load values.
- This table was designed to apply to a broad range of applications. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC[®] software.

	Deflection			Ac	tual d	eflecti	on ba	sed or	n Spar	n and	Limit (in)		
	limit						S	pan (f	t)					
	mmu	6	8	10	12	14	16	18	20	22	24	26	28	30
Γ	L/360	0.20	0.27	0.33	0.40	0.47	0.53	0.60	0.67	0.73	0.80	0.87	0.93	1.00
	L/240	0.30	0.40	0.50	0.60	0.70	0.80	0.90	1.00	1.10	1.20	1.30	1.40	1.50
	L/180	0.40	0.53	0.67	0.80	0.93	1.07	1.20	1.33	1.47	1.60	1.73	1.87	2.00

Joist Pitch 2/12 3/12 4/12 5/12 6/12 7/12 8/12 9/12 10/12 11/12 12/12		DEAD LOAD SLOPE FACTOR											
	Joist Pitch	2/12	3/12	4/12	5/12	6/12	7/12	8/12	9/12	10/12	11/12	12/12	
Slope Factor 1.014 1.031 1.054 1.083 1.118 1.158 1.202 1.250 1.302 1.357 1.414	Slope Factor	1.014	1.031	1.054	1.083	1.118	1.158	1.202	1.250	1.302	1.357	1.414	

		PSF	to PLF C	ONVERSI	ON TABL	E		
Joist				LOAD) (psf)			
Spacing	20	25	30	35	40	45	50	60
12"	20	25	30	35	40	45	50	60
16"	27	33	40	47	53	60	67	80
19.2"	32	40	48	56	64	72	80	96
24"	40	50	60	70	80	90	100	120

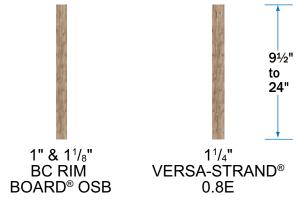
TO CONVERT FROM SPECIFIED LOAD (PLF) TO FACTORED LOAD (PLF)

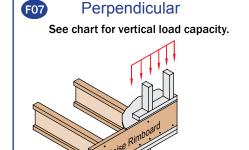
- Factored (PLF) = 1.25 x Specified Dead Load (PLF) + 1.50 x Specified Live/Snow Load (PLF)

21

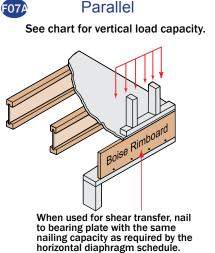
BOISE CASCADE RIMBOARD

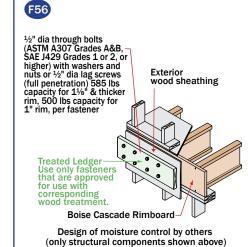
Boise Cascade Rimboard Product Profiles





When used for shear transfer, nail to bearing plate with the same nailing capacity as required by the horizontal diaphragm schedule.





BOISE CASCADE RIMBOARD PROPERTIES

Dim Boord Type	Thickness	φH ⁽¹⁾	φV [II	b/ft] ⁽²⁾	φZ ⁽³⁾	φP ⁽⁴⁾
Rim Board Type	[in]	[lb/ft]	d ≤ 16"	d > 16"	[lb]	[lb]
Boise Cascade Rimboard	1"	235	5500	2750	495	5840
Boise Cascade Rimboard	1½"	235	7340	5000	585	5840
Boise Cascade Rimboard Plus	1½"	260	8090	5340	585	5840
Boise Cascade VERSA-STRAND [®] 0.8E	1¼"	310	9460	5820	830	8990

NOTES:

22

1. ϕ H = Factored horizontal (shear) load transfer capacity is based on the minimum nailing attachment schedule specified in NBCC 2010 and APA document D340CA.

2. ϕ V = Factored uniform bearing (vertical) load resistance. The uniform bearing load shall be simultaneously satisfied with the concentrated vertical load resistance, when applicable.

3. ϕZ = Factored lateral resistance of a $\frac{1}{2}$ inch (12.7 mm) diameter lag screw.

- 4. φP = Factored concentrated vertical load resistance based on 4½ inch (114 mm) bearing length. The concentrated vertical load shall be simultaneously satisfied with the uniform bearing load capacity, when applicable.
- 5. All tabulated values are applicable to the standard-term load duration and permitted to be adjusted for other load durations in accordance with CSA 086-09.

6. See CCMC Evaluation Report No. 13143-R for further product information on Boise Cascade VERSA-STRAND 0.8E.

FRAMING CONNECTORS - SIMPSON STRONG-TIE 23

SINGLE I-JOISTS – Canadian/Factored Resistance (lbs)

SIMPSON

· · · · · ·		
troi	10-	l'1e
	- E - I	

	Top Flange								-	Sna	ap -In					Face Mount								
Joist Height		В	Fa	astener		Uplift	Down	Load		в	Fasten	-	Uplift	Down	Load		в	Faste	ener Type	Uplift	Down	Load		
Height	Model	Dim	Heade	r		(115)	DF	SPF	Model	Dim	Header	Joist	(115)	DF	SPF	Model	Dim	Header	Joist	(115)	DF	SPF		
AJS 1	40, 20											J	oist Wid	th 2½"										
91⁄2	LT259	2	6-100	l 1-#	#8x1¼ws	105	2625	1725	IUS2.56/9.5	2	8-10d		145	2385	1690	LF259	2	10-10d	1-#8x1¼ws	105	2525	2155		
111/8	LT251188	2	6-100	1-#	#8x1¼ws	105	2625	1725	IUS2.56/11.88	2	10-10d		145	2565	1820	LF2511	2	12-10d	1-#8x1¼ws	105	2880	2270		
14	LT2514	2	6-100	1-#	#8x1¼ws	105	2625	1725	IUS2.56/14	2	12-10d		145	2565	1820	LF2514	2	14-10d	1-#8x1¼ws	105	3235	2385		
16	LT2516	2	6-100	1-#	#8x1¼ws	105	2625	1725	IUS2.56/16	2	14-10d		145	2725	1935	MIU2.56/16	21⁄2	24-16d	2-10dx1½	450	4930	3485		
AJS 2	5											J	oist Wid	th 3½"										
91⁄2	LT359	2	6-100	2-#	#8x1¼ws	105	2625	1725	IUS3.56/9.5	2	10-10d		145	2370	1685	LF359	2	10-10d	2-#8x1¼ws	105	2525	2155		
111/8	LT351188	2	6-100	2-#	#8x1¼ws	105	2625	1725	IUS3.56/11.88	2	12-10d		145	2370	1685	LF3511	2	12-10d	2-#8x1¼ws	105	2880	2270		
14	LT3514	2	6-100	1 2-#	#8x1¼ws	105	2625	1725	IUS3.56/14	2	12-10d		145	2370	1685	LF3514	2	14-10d	2-#8x1¼ws	105	3235	2385		
16	LT3516	2	6-100	1 2-#	#8x1¼ws	105	2625	1725	IUS3.56/16	2	14-10d		145	2370	1685	MIU3.56/16	21⁄2	24-16d	2-10dx1½	450	4930	3485		
18	MIT418	21⁄2	6-160	2-	10dx1½	320	3490	2420		No I	US Hange	r for these	e sizes			MIU3.56/18	21⁄2	26-16d	2-10dx1½	450	4930	3485		
				45°	Skew					Adjustable Height							Field Slope & Skew							
Joist Height	Model		в	Faste	ner Type	Uplift	Dow	nLoad	Model	В	Faster	ner Type	Uplift		vnLoad	Model	В	Fas	tener Type	Uplift	Down	Load		
	Model		Dim H	eader	Joist	(115)	DF	SPF	Model	Dim	Header	Joist	(115)	DF	SPF	Model	Dim	Header	Joist	(115)	DF	SPF		
AJS 1						-						J	oist Wid	th 2½"	- T	-	-	- <u>1</u>	- T	Ť				
91⁄2	SUR/L2.56			4-16d	2-10dx1½	385	3950	2805	THAI322	2¼		2-10dx11	-	3000	2385	LSSUH310	31⁄2	14-16d		1625	2620	1860		
111/8	SUR/L2.56			6-16d	2-10dx1½	385	3950	2805	THAI322	21⁄4		2-10dx11	-	3000	2385	LSSUH310	31⁄2	14-16d		1625	2620	1860		
14	SUR/L2.56	/14	3 ³ ⁄ ₁₆ 1	8-16d	2-10dx1½	385	3950	2805	THAI322	2¼		2-10dx11		3000	2385	LSSUH310	31⁄2	14-16d		1625	2620	1860		
16	SUR/L2.56	/14	3¾ ₁₆ 1	8-16d	2-10dx1½	385	3950	2805	See		lian Wood alogue for					S			od Constructior for hanger sele		tors			
AJS 2	5				1			1	Joist Width 3½"						1									
91⁄2	SUR/L41	0	2% 1	4-16d	6-16d	1695	4065	2875	THAI422	2¼	6-10d	2-10dx1½	ź -	3000	2385	LSSU410	31/2	14-16d	12-10dx1½	1625	3055	2170		
111/8	SUR/L41	0	25% 1	4-16d	6-16d	1695	4065	2875	THAI422	21⁄4	6-10d	2-10dx11	2 -	3000	2385	LSSU410	31/2	14-16d	12-10dx1½	1625	3055	2170		
14	SUR/L41	4	25% 1	8-16d	8-16d	2265	4095	2895	THAI422	21⁄4	6-10d	2-10dx11	2 -	3000	2385	LSSU410	31/2	14-16d	12-10dx1½	1625	3055	2170		
16	SUR/L41	4	25% 1	8-16d	8-16d	2265	4095	2895	See	Canad	lian Wood	Construc	ion Cor	nectors		s	See Canadian Wood Construction Connectors							
18	SUR/L41	4	25⁄8 1	8-16d	8-16d	2265	4095	2895			alogue for								for hanger sele		-			

1. Shaded hangers require web stiffeners at joist ends. Web stiffeners may be required for non-shaded hangers by others.

2. The B Dim is the depth of the hanger seat.

4. WS = wood screw

For more information, call Simpson Strong-Tie at 800-999-5099 or visit their website at www.strongtie.com DOUBLE I-JOISTS – Canadian/Factored Resistance (Ibs)

			1	op Flange				Face Mount								45	° Skew				
Joist Height	Model	В	Faste	ener Type	Uplift	Dowr	Load	Model	в	Faste	ner Type	Uplift	Down	Load	Model	В	Fastener Type		Uplift	Down	Load
g	woder	Dim	Header	Joist	(115)	DF	SPF	widdei	Dim	Header	Joist	(115)	DF	SPF	woder	Dim		Joist	(115)	DF	SPF
Doubl	e AJS 140, 20							Joist Width 5"				h 5"									
91⁄2	MIT39.5-2	21⁄2	8-16d	2-10dx11/2	450	3490	2420	MIU5.12/9	21⁄2	16-16d	2-10dx1½	450	4550	3230	HSUR/5.12/9	2 ¹³ / ₁₆	12-16d	2-10dx1½	275	2995	2350
111/8	MIT311.88-2	21⁄2	8-16d	2-10dx11/2	450	3490	2420	MIU5.12/11	21⁄2	20-16d	2-10dx1½	450	4550	3230	HSUR/5.12/11	2 ¹³ / ₁₆	16-16d	2-10dx1½	275	4195	2965
14	MIT314-2	21/2	8-16d	2-10dx11/2	450	3490	2420	MIU5.12/14	21/2	22-16d	2-10dx1½	450	4930	3485	HSUR/5.12/14	2 ¹³ / ₁₆	20-16d	2-10dx1½	275	4195	2965
16	MIT5.12/16	21⁄2	8-16d	2-10dx11/2	450	3490	2420	MIU5.12/16	21⁄2	24-16d	2-10dx1½	450	4930	3485	HSUR/5.12/16	2 ¹³ / ₁₆	24-16d	2-10dx1½	275	4195	2965
Doub	le AJS 25							Joist Width 7"													
91⁄2	B7.12/9.5	21⁄2	14-16d	6-16d	1650	5940	3910	HU410-2	21⁄2	18-16d	8-16d	2455	5780	4690	HU410-2x4	21⁄2	18-16d	8-16d	1840	3755	3045
111⁄8	B7.12/11.88	21⁄2	14-16d	6-16d	1650	5940	3910	HU412-2	21⁄2	22-16d	8-16d	2455	5780	4690	HU412-2x4	21⁄2	22-16d	8-16d	1840	3755	3045
14	B7.12/14	21⁄2	14-16d	6-16d	1650	5940	3910	HU414-2	21⁄2	26-16d	12-16d	3685	7025	5780	HU414-2x4	21⁄2	26-16d	12-16d	2760	4565	3755
16	B7.12/16	21⁄2	14-16d	6-16d	1650	5940	3910	HU414-2	21⁄2	26-16d	12-16d	3685	7025	5780	HU414-2x4	21⁄2	26-16d	12-16d	2760	4565	3755
18	B7.12/18	21⁄2	14-16d	6-16d	1650	5940	3910	HU414-2	21⁄2	26-16d	12-16d	3685	7025	5780	HU414-2x4	21⁄2	26-16d	12-16d	2760	4565	3755
			Adjus	table Height				Field Slope & Skew							1. Shaded h	anger	s require	web stiffen	iers at j	oist end	ls.
Joist Height		в	Fasten	er Type	Uplift	DownL	oad	B Fastener Type Uplift DownLoad						Web stiffeners may be required for non-shaded hange by others.					ingers		
meight	Model [Dim 📊	Header	Joist	(115)	DF	SPF	Model	Dim 🗌	Header	Joist	(115)	DF	SPF	.,		enecial	ordered, sp	ocify b	anger ei	oat
Double	e AJS 140, 20			·			Joist	t Width 5"					width bet				Conyine	inger 5	Jai		
91⁄2	THAI-2 ²	21⁄2	6-10d	2-10dx1½	-	2800	2800	LSU5.123	31⁄2	24-16d	16-10dx1½	1285	2600	1845	3. LSU5.12	skew	options m	nust be facto	ory orde	ered.	
111/8	THAI-2 ²	21/2	6-10d	2-10dx1½	-	2800	2800	LSU5.12 ³	31⁄2	24-16d	16-10dx1½	1285	2600	1845	4. Skewed o	ption	must be	special orde	ered. Sp	becify s	kew
14	THAI-2 ²	21/2	6-10d	2-10dx1½	-	2800	2800	LSU5.12 ³	31⁄2	24-16d	16-10dx1½	1285	2600	1845	angle and direction (i.e. HU410-2X, SKR 45°).						
16	See			d Construction		ectors		See			d Construction		ectors		5. The B Din	n is the	e depth o	f the hange	r seat.		

See Canadian Wood Construction Connectors

catalogue for hanger selection.

 Double AJS[®] 25 (Joist Width = 7") Adjustable Height and Field Slope & Skew Hangers - See Wood Construction Connectors Catalogue for hanger selection.

See Canadian Wood Construction Connectors

catalogue for hanger selection.

9½ 11%

14

16 18

²⁴ FRAMING CONNECTORS - SIMPSON STRONG-TIE

General Notes

- See current Canadian Wood Construction Connectors catalogue for Important Information and General Notes section and for hanger models, joist sizes, and header situations not shown. See pages 10-11 of the Simpson Strong-Tie Publication CSG-BCCANAJS12 " AJS I-JOISTS Connector Selection Guide, Limit States Design" version for installation information.
- Unless otherwise noted, factored resistances (downloads) listed address hanger/header/fastener limitations assuming header material is Douglas Fir-Larch or Spruce Pine Fir or LVL. Joist reaction should be checked by a qualified designer to ensure proper hanger selection.
- Factored uplift resistances (uplift) listed assume SPF joist and header and have been increased by 15% for earthquake and wind loading with no further increase allowed. Reduce loads according to code for normal duration loading such as cantilever construction.
- 4. If hanger height is less than 60% of joist height, joist rotation may occur; see Prevent Rotation information on page 2 of the Simpson guide referenced in Note 1 above.
- 5. Top flange hanger configuration and thickness of top flange need to be considered for flush frame conditions, see page 10 of the Simpson guide referenced in Note 1 to the left.

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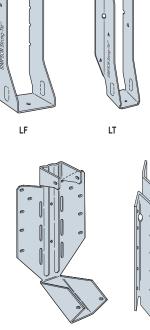
HOW TO PICK A HANGER:

- 1. Find your joist size in this guide. (Single I-Joist, Double I-Joist, Beam, etc.)
- 2. Locate your connector type in the table.
 - Face mount, top flange, skewed, sloped, etc.
- 3. Select a hanger from the table.
- 4. Confirm that your factored joist reaction is less than the factored resistance of hanger.
- Check to see if the bearing length "B dim" meets the bearing length requirement of the I-Joist. If yes, you have successfully selected your hanger.

If you did not find a suitable hanger; please see the current Canadian Wood Construction Connectors catalogue or call Simpson Strong-Tie at 800-999-5099.

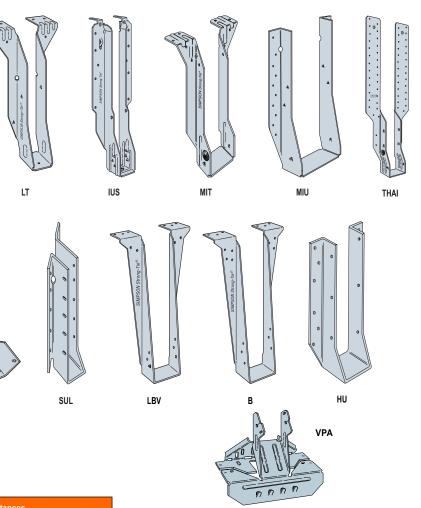
You will need the following information:

- Download
- Uplift
- Header condition
- Bearing length requirement



LSSU

- 6. For this publication, carrying members are assumed to be at least 5½ inches tall for top flange hangers. The horizontal thickness of the carrying member must be at least the length of nail being used or the hanger top flange dimension, whichever is greater. <u>Exception</u>: narrower carrying members may be used with face mount hangers but the horizontal thickness must be at least 1³/₄ inches for 3" (10d) nails; 2 inches for 3¹/₂" (16d) nails. Clinch nails on back side.
- THAI hangers in this publication are based on a "top flange" installation and require that the carrying member have a horizontal thickness of at least 2½ inches. Backer blocks are required when the header is an I-joist. Install 4 top nails and 2 face nails. THAI hangers are not rated for uplift
- NAILS: 3¹/₂" (16d) = 0.162" dia. x 3 ¹/₂" long 3" (10d) = 0.148" dia. x 3" long 3" (10d) x 1¹/₂ = 0.148" dia. x 1¹/₂" long



VPA - Variable Pitch Connectors

		Fasteners		Factored Resistances											
Joist	Model			Un		Down	beak	L	ateral L	oad [160]					
Width	No.	Top Plate	Rafter	Uplift [160]		Download [100]		D	F	SPF					
				DF	SPF	DF	SPF	F1	F ₂	F1	F ₂				
21⁄2	VPA3	9-10d	2-10dx1½	405	370	2050	1855	695	405	615	370				
31/2	VPA4	11-10d	2-10dx1½	405	370	2050	1855	695	405	615	370				

VPA Connector - 18 gauge

- This variable pitch connector allows a sloped beam to sit on a top plate without having to notch, birdmouth, bevel, or toe nail. It also provides uplift capacity. Adjustable from 3:12 to 12:12 pitch.
- VPAs are not appropriate for applications that require more than 2" of bearing, such as intermediate supports.

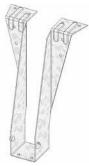
FRAMING CONNECTORS - USP STUCTURAL CONNECTORS 25

SINGLE

Or AJS ¹⁰⁰ 20 Joist With = 2412 ² (2) 104 x 1-12 2465 745 1771 530 TH2592 (2) 104 x 1-12 3310 335 2280 2310 720 2580 511 FL2516 (6) 100 (2) 100 x 1-12 2465 745 1771 530 TH25121 (4) 100 (2) 100 x 1-12 4405 720 318 511 FL2516 (6) 100 (2) 100 x 1-12 2465 745 1771 530 TH25160 (2) 100 x 1-12 4405 720 318 511 FL2516 (6) 100 (2) 100 x 1-12 2495 745 370 330 1373 730 330 1373 730 330 1383 511 1331 1310 1310 1310 1312 1300 1311 1311 1311 1311 1311 1311 1311 1311 1311 1311 1311 1311 1311 1311 1311 1311 1311 13111 13111 13111 <td< th=""><th>ile I-J</th><th>OISTS</th><th>S - Car</th><th>nadi</th><th>ian/</th><th>'Fac</th><th>tor</th><th>ed Re</th><th>sistar</th><th>nce (lbs</th><th>)</th><th>A GERAL</th><th></th><th>CTORS[®]</th></td<>	ile I-J	OISTS	S - Car	nadi	ian/	'Fac	tor	ed Re	sistar	nce (lbs)	A GERAL		CTORS [®]
USP Block No. ¹⁶ Pieader Joint Down ¹ Uptilt Down ¹ Uptilt Uptilt Uptil			Top Mount Han	gers⁵						Face Mount Ha	ngers			
Stock No. ¹¹ Header Joint 109h 1155 1155 117h		Fastene	er Schedule ⁴	DF-	/SP	S-	P-F		Faster	er Schedule ⁴	DF-I	L/SP	S-I	P-F
Op of AJS ¹⁰⁰ 20 Joist Writh = 2-1/2 Joist Writh = 2-1/2 Joist Writh = 2-1/2 FL2566 (6) 100 (2) 100 x 1-1/2 2465 746 1771 530 1122 100 x 1-1/2 230 232 230 511 FL2566 (6) 100 (2) 100 x 1-1/2 2465 746 1771 530 1122 112 <th>USP</th> <th></th> <th></th> <th>Down²</th> <th>Uplift³</th> <th>Down²</th> <th>Uplift³</th> <th>USP</th> <th></th> <th></th> <th>Down²</th> <th>Uplift³</th> <th>Down²</th> <th>Uplift</th>	USP			Down ²	Uplift ³	Down ²	Uplift ³	USP			Down ²	Uplift ³	Down ²	Uplift
FL3558 (f) (f) <t< td=""><td>Stock No.1,5</td><td>Header</td><td>Joist</td><td>100%</td><td>115%</td><td>100%</td><td>115%</td><td>Stock No.1</td><td>Header</td><td>Joist</td><td>100%</td><td>115%</td><td>100%</td><td>115%</td></t<>	Stock No.1,5	Header	Joist	100%	115%	100%	115%	Stock No.1	Header	Joist	100%	115%	100%	115%
Fig.5518 (i)	40 or AJS™	20				Joist	Width	= 2-1/2″						
Fig.3516 (i) (i	TFL2595	(6) 10d	(2) 10d x 1-1/2	2495	745	1771	530	THF25925	(12) 10d	(2) 10d x 1-1/2	3310	335	2350	238
FL2516 (6) 100 (2) 103 x 1-12 2495 745 1771 530 TH-25160 (22) 100 (2) 104 x 1-12 2405 720 3128 511 S 005350 (10) 100 (2) 104 x 1-12 2575 500 2115 55 TH-75312 (10) (10) (2) 104 x 1-12 2540 465 3720 300 H035160 (12) (10) (12) <td>TFL25118</td> <td></td> <td></td> <td>2495</td> <td>745</td> <td>1771</td> <td>530</td> <td>THF25112</td> <td>. ,</td> <td></td> <td>3310</td> <td>720</td> <td>2350</td> <td>511</td>	TFL25118			2495	745	1771	530	THF25112	. ,		3310	720	2350	511
Society Joint Wittin = 3-1/2" Joint Wit	TFL2514		. ,	2495	745	1771	530	THF25140	. ,		4405	720	3128	511
H035840 (10) 108 (2) 104 xt-12 2275 600 2115 355 THF3512 (12) 104 (12) 104 xt-12 220 445 3720 300 H035140 (12) 102 (10) 1012 (2) 104 xt-12 240 445 3720 300 H035140 (12) 102 (10) 112 (10) 112 (10) 112 445 3720 300 F1418 (6) 164 (2) 104 xt-12 4460 50 3160 355 THF35167 (22) 100 (2) 104 xt-12 680 465 4743 300 F1418 (6) 164 (2) 104 xt-12 4490 545 2975 385 THF35167 (22) 100 (2) 104 xt-112 680 465 4743 300 F1418 (6) 164 (2) 104 xt-112 1495 500rM 1497 Usp Superior Superior Superior Superior Superior 14984 Joint Joint 1497 1497 1496 120 xt-112 4200 155 3025 1390 ISN318 (6) 100 (4) 100 xt-112 1155 -00 -015	TFL2516	(6) 10d	(2) 10d x 1-1/2	2495	745	1771	530	THF25160	(22) 10d	(2) 10d x 1-1/2	4405	720	3128	511
HG35118 (10) 10d (2) 10d x 1-12 2975 500 2115 355 THF35112 (16) 10d (2) 10d x 1-12 620 426 472 300 HG35118 (10) 10d (12) 10d x 1-12 445 500 3160 355 THF35157 (22) 10d x 1-12 6480 445 4743 300 F1418 (0) 10d (2) 10d x 1-12 4450 500	25					Joist	Width	= 3-1/2″						
H035140 (12) 10d (2) 10d x1-12 (4450 500 3160 355 THF35157 (22) 10d (2) 10d x1-12 6680 465 4743 330 H035160 (12) 10d (2) 10d x1-12 4450 500 3160 355 THF35157 (22) 10d (2) 10d x1-12 6680 465 4743 330 Adjustable Height Hangers Strategy	THO35950	(10) 10d	(2) 10d x 1-1/2	2975	500	2115	355	THF35925	(12) 10d	(2) 10d x 1-1/2	5240	465	3720	330
H-035160 (12) 10d (2) 10d x1-1/2 (445) 500 3160 355 THF35157 (22) 10d (2) 10d x1-1/2 6680 465 4743 330 THF35157 (22) 10d (2) 10d x1-1/2 6680 465 4743 330 THF35157 (22) 10d (2) 10d x1-1/2 6680 465 4743 330 THF35157 (22) 10d (2) 10d x1-1/2 6680 465 4743 330 THF35157 (22) 10d (2) 10d x1-1/2 6680 465 4743 330 THF35157 (22) 10d (2) 10d x1-1/2 6680 465 4743 330 THF35157 (22) 10d (2) 10d x1-1/2 6680 465 4743 330 THF35157 (22) 10d (2) 10d x1-1/2 6680 465 4743 330 THF35157 (22) 10d (2) 10d x1-1/2 6680 465 175 100 x 1155 100 x 1150	THO35118	(10) 10d	(2) 10d x 1-1/2	2975	500	2115	355	THF35112	(16) 10d	(2) 10d x 1-1/2	5240	465	3720	330
FH18 (6) 16d (2) 10d x1-1/2 (4) 20 44 (4) 20 45 (4) 20 45 (4) 300 Adjustable Height Barges Stock No. Fastener Schedule" DP-L/SP S-P-F Stock No. Stock No.	THO35140	(12) 10d	(2) 10d x 1-1/2	4450	500	3160	355	THF35140	(20) 10d	(2) 10d x 1-1/2	6680	465	4743	330
Slope and Skew Hangers Slope and Skew Hangers Slope and Skew Hangers Slope and Skew Hangers Usp Slock No. ¹ Patter Schedule [*] DPL/SP Header Joint With = 2-1/2" Header Joint With = 2-1/2" Header Joint With = 2-1/2" Header Joint With = 2-1/2" Header Joint With = 2-1/2" Header Joint With = 2-1/2" Joint With = 2-1/2" LissH25 Fastener Schedule [*] Down [*] Uplit Header Joint With = 2-1/2" Joint With = 2-1/2" Joint With = 2-1/2" Fastener Schedule [*] Joint With = 2-1/2" Joint With = 2-1/2" Joint With = 2-1/2" Joint With = 2-1/2" Schedule [*] Joint With = 2-1/2" Joint With = 2-1/2" Joint With = 2-1/2" Schedule [*] Joint With = 2-1/2" Joint With = 2-1/2" Schedule [*] Joint With = 2-1/2" Joint With = 2-1/2" Schedule [*] Joint With = 2-1/2" Joint With = 2-1/2" Schedule [*] Joint With = 2-1/2"	THO35160	(12) 10d	(2) 10d x 1-1/2	4450	500	3160	355	THF35157	(22) 10d	(2) 10d x 1-1/2	6680	465	4743	330
USP Block No. ^{1,6} Fastener Schedule ⁴ DF-USP Down ² S.P.F. Upfit ² Usp Town ³ Fastener Schedule ⁴ DF-USP Down ³ S.P.F. Upfit ³ 0 or AUS ³²⁰ Joist 100% 115%, 100% 105%, 115%, 100% 115%, 100% 100%, 115%, 100% 115%, 100% 115%, 100% 100%, 115%, 100% 115%, 100% 100%, 115%, 100% 115%, 100% 100%, 115%, 100% 115%, 100% 100%, 115%, 100% 115%, 100% 100%, 115%, 100% 115%, 100% 100%, 115%, 100% 110%, 100%, 111%, 100% 111%, 100%, 111%, 100% 110%, 100%, 111%, 100% 110%, 100%, 111%, 100% 110%, 100%, 111%, 100% 110%, 100%, 111%, 100% 110%, 100%, 111%, 100% 110%, 100%, 111%, 100% 110%, 100%, 111%, 100%, 110%, 110%, 100%, 110%, 100%, 110%, 100%, 110%, 100%, 110%, 100%, 110%, 100%, 110%, 100%, 110%, 100%, 110%, 100%, 110%, 100%, 110%, 100%, 100%, 110%, 100%, 1	TFI418	(6) 16d	(2) 10d x 1-1/2	4190	545	2975	385	THF35157	(22) 10d	(2) 10d x 1-1/2	6680	465	4743	330
USP Image: Transmission of the second s				langers							angers			
Stock No. ¹⁴ Header Joist 100% 115% 100% 115% Stock No. ¹ Header Joist 100% 115% 100% <t< td=""><td></td><td>Fastene</td><td>er Schedule⁴</td><td></td><td></td><td></td><td></td><td>ļ</td><td>Faster</td><td>er Schedule⁴</td><td></td><td></td><td></td><td>-</td></t<>		Fastene	er Schedule ⁴					ļ	Faster	er Schedule ⁴				-
Op CAUS Could <				Down ²	Uplift ³	Down ²	· ·	1			Down ²	Uplift ³	Down ²	Uplift
Image: start in the s	Stock No.".	Header	Joist	100%	115%	100%	115%	Stock No.	Header	Joist	100%	115%	100%	115%
ISH318 (6) 10d (4) 10d x 1-1/2 1185 840 LSSH25 (14) 16d (12) 10d x 1-1/2 4200 1985 3025 1390 ISH318 (6) 10d (4) 10d x 1-1/2 1185 840 LSSH25 (14) 16d (12) 10d x 1-1/2 4200 1985 3025 1390 ISH318 (6) 10d (4) 10d x 1-1/2 1185 840 LSSH25 (14) 16d (12) 10d x 1-1/2 4200 1985 3025 1390 ISH422 (6) 10d (4) 10d 1105 785 LSSH35 (14) 16d (12) 10d x 1-1/2 5202 2695 3715 1845 ISH422 (6) 10d (4) 10d 1105 LSSH35 (14) 16d (12) 10d x 1-1/2 5202 2695 3715 1845 ISH422 (6) 10d (10) 10d x 1-1/2 3265 2910 3202 2065 1904 1105 1105 1106 1106 1107 1004 1107 1104 1102 1014 1102 101	40 or AJS™	20				Joist	Width	= 2-1/2″						
ISH318 (6) 10d (4) 10d x 1-1/2 1185 840 LSSH25 (14) 16d (12) 10d x 1-1/2 4260 1955 3025 1390 ISH318 (6) 10d (4) 10d x 1-1/2 1185 840 LSSH25 (14) 16d (12) 10d x 1-1/2 4260 1955 3025 1390 ISH422 (6) 10d (4) 10d 1105 785 LSSH35 (14) 16d (12) 10d x 1-1/2 5230 2595 3715 1845 LSSH35 (14) 16d (12) 10d x 1-1/2 5230 2595 3715 1845 LSSH35 (14) 16d (12) 10d x 1-1/2 5230 2595 3715 1845 LSSH35 (14) 16d (12) 10d x 1-1/2 5230 2595 3715 1845 USP Stewed 45 ² Hangers S-P-F See S-P-F See See 3715 1845 US									. ,	. ,	<u> </u>	<u> </u>		
ISH318 (6) 10d (4) 10d xt 1.12 1185 840 LSSH25 (14) 16d (12) 10d xt 1.12 420 195 ISH422 (6) 10d (4) 10d 1105 785 LSSH35 (14) 16d (12) 10d xt 1.12 520 250 271 1845 ISH422 (6) 10d (100 100 1105 CSSH35 (14) 16d (12) 10d xt 1.12 5230 2565 3715 1845 ISH22 (6) 10d (100 1105 LSSH35 (14) 16d (12) 10d xt 1.12 5230 2565 3715 1845 ISN Stock No.1 Fastener Schedule ⁴ DF-USP S.P-F LSSH35 (14) 16d (12) 10d xt 1.12 5230 2695 3715 1845 Stock No.1 Piate Rafer DP-USP S.P-F LSSH35 (14) 16d (12) 10d xt 1.12 5230 2695 3715 1845 Stock No.1 Piate Rafer DP-USP S.P-F LSSH35 (14) 16d (12) 10d xt 1.12 5230 2695	MSH318			<u> </u>							<u> </u>			
S Joist Width = 3-1/2" SH422 (6) 10d (4) 10d 1105 785 LSSH35 (14) 16d (12) 10d x 1-1/2 520 2595 3715 1845 LSSH35 (14) 16d (12) 10d x 1-1/2 520 2595 3715 1845 LSSH35 (14) 16d (12) 10d x 1-1/2 5200 2595 3715 1845 LSSH35 (14) 16d (12) 10d x 1-1/2 5200 2595 3715 1845 LSSH35 (14) 16d (12) 10d x 1-1/2 5200 2595 3715 1845 Stexted 45* Hangers LSSH35 (14) 16d (12) 10d x 1-1/2 502 2505 3715 1845 Stexted 45* Hangers Joist Width = 2-1/2" 100x1 110x1 100x1 101x1/12 <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td><u> </u></td><td></td><td></td><td></td><td><u> </u></td><td><u> </u></td><td></td><td></td></t<>							<u> </u>				<u> </u>	<u> </u>		
ISH422 (6) 10d (4) 10d 1105 785 LSSH35 (14) 16d (12) 10d x 1-1/2 5230 2595 3715 1845 ISH422 (6) 10d (4) 10d 1105 785 LSSH35 (14) 16d (12) 10d x 1-1/2 5230 2595 3715 1845 LSSH35 (14) 16d (12) 10d x 1-1/2 5230 2595 3715 1845 LSSH35 (14) 16d (12) 10d x 1-1/2 5230 2595 3715 1845 LSSH35 (14) 16d (12) 10d x 1-1/2 5205 3715 1845 Stack No. LSSH35 (14) 16d (12) 10d x 1-1/2 5205 3715 1845 Stack No. LSSH3		(6) 100	(4) 100 X 1-1/2	1185					(14) 160	(12) 10d x 1-1/2	4260	1955	3025	1390
ISH422 (6) 10d (4) 10d 1105 785 LSSH35 (14) 16d (12) 10d x 1-1/2 520 256 3715 1845 LSSH35 (14) 16d (12) 10d x 1-1/2 5230 2595 3715 1845 LSSH35 (14) 16d (12) 10d x 1-1/2 5230 2595 3715 1845 LSSH35 (14) 16d (12) 10d x 1-1/2 5230 2595 3715 1845 LSSH35 (14) 16d (12) 10d x 1-1/2 5230 2595 3715 1845 LSSH35 (14) 16d (12) 10d x 1-1/2 5230 2565 3715 1845 LSSH35 (14) 16d 120 dx 1-1/2 5230 2565	25 MSH422	(6) 10d	(1) 10d	1105		_			(14) 16d	(12) 104 - 1 1/2	E220	2505	2715	1045
LSSH35 (14) 16d (12) 10d x 1-1/2 520 256 3715 1845 LSSH35 (14) 16d (12) 10d x 1-1/2 5230 2595 3715 1845 LSSH35 (14) 16d (12) 10d x 1-1/2 5230 2595 3715 1845 Stock No.1 LSSH36 (14) 16d (12) 10d x 1-1/2 5200 2595 3715 1845 Stock No.1 Plate Rafter Dorw1 Uplift Dorw1 Uplif									. ,	. ,	<u> </u>	<u> </u>		
LSRH35* (14) 16d (12) 10d x 1-1/2 520 2595 3715 1845 Skewed 45* Hangers USP Fastener Schedule* DF-LSP Colspan="5">Colspan="5" <colspan="5">Colspan="5"<colspan="5"colspan="5"< td=""> Colspan="5" Colspan</colspan="5"colspan="5"<></colspan="5">		. ,					<u> </u>		. ,		<u> </u>			
LSRH35 ^a (14) 16d (12) 10d x 1-1/2 5230 2595 3715 1845 Skewed 45 ^a Hangers Colspan="4">Skewed 45 ^a Hangers Skewed 45 ^a Hangers Skewed 45 ^a Hangers Skewed 45 ^a Hangers Skewed 45 ^b Hangers Skewed 45 ^a Hangers Skewed 45 ^a Hangers Skewed 45 ^b Hangers Skeide hangers re							<u> </u>		. ,					
Skewed 45° Hargers I Skewed 45° Hargers I Species Solid Save Design of Colspan="2">I Shaded hangers require web stiffeners at joist ends. Web stiffeners may be required for non-shaded hangers by Ljoist manufacturers. USP I Shaded hangers require web stiffeners at joist ends. Web stiffeners may be required for non-shaded hangers by Ljoist manufacturers. I Shock No. ¹ Down ² Uplift ¹ Down ² Uplift ¹ 100% 115% 100% 115% 100% 115% 100% 115% 100% 115% 100% 115% 100% 115% 100% 115% 100% 115% 100% 115% 100% 115% 100% 115% 100% 115% 100% 115% 100% 115% 100% 115% 100% 100 100 x 1-1/2 100 100 100 x				<u> </u>					. ,	. ,				
USP Stock No. ¹ Down ² Uplift ¹ Down ² Uplift ¹ Down ² Uplift ² 40 or AJS ^{1M} 20 Joist Width = 2-1/2" equired for non-shaded hangers by Hjoist manufacturers. 2) Loads listed are based on 2001 NDS® and hanger attachment to a DF-L, SP, or S-P-F species solid sawn or LVL header. Some loads may be increased for duration of load adjustments. Refer to USP Full Line Catalog for details. KH252QLUR (14) 10d (10) 10d x 1-1/2 3265 2910 2320 2065 3) Uplift loads have been increased 15% for wind and seismic loading; no further increase shall be permitted. KH252QLUR (16) 10d (10) 10d x 1-1/2 3265 2910 2320 2065 3) Uplift loads have been increased 15% for wind and seismic loading; no further increase shall be permitted. KH2524LUR (16) 10d (10) 10d x 1-1/2 3265 2910 2320 2065 3) KH410LIR ⁷ (16) 16d (10) 16d 3690 3560 2620 2530 Minimum nail penetration shall be 1-1/2" for 10d nails. KH410LIR ⁷ (16) 16d (10) 16d 7405 3560 2520 2530 5) Top Mount Hangers require minimum 3" header width for THO series hanger <td></td> <td></td> <td>Skewed 45° Ha</td> <td>ngers</td> <td></td> <td></td> <td>1</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>			Skewed 45° Ha	ngers			1							
Stock No. ¹ Plate Refter 100% 115% 100% 115% 100% 115% 100% 115% 100% 115% 100% 115% 100% 115% 100% 115% 100% 115% 100% 115% 100% 115% 100% 115% 100% 115% 2) Loads listed are based on 2001 NDS® and hanger attachment to a DF-L, SP, or S-P-F species solid sawn or LVL header. Some loads may be increased for duration of load adjustments. Refer to USP Full Line Catalog for details. 3) Uplit loads have been increased 15% for wind and seismic loading; no further increase shall be permitted. 3) Uplit loads have been increased 15% for wind and seismic loading; no further increase shall be primitted. 3) Uplit loads have been increased 15% for wind and seismic loading; no further increase shall be primited. 3) Uplit loads have been increased 15% for wind and seismic loading; no further increase shall be primited. 3) Uplit loads have been increased 15% for wind and seismic loading; no further increase shall be primited. 3) Uplit loads have been increased 15% for wind and seismic loading; no further increase shall be primited. 3) Uplit loads have been increased 15% for wind and seismic loading; no further increase shall be primited. 4) Udx 1-1/2' long. Minimum hail penetration shall be 1-1/2' for 10d nails. 10d shares (0.148' diameter) by 3-1/4' long may be substituted for 10d common with no load reduction. 5) Top Mount Hangers requ		Fastene	er Schedule ⁴	DF-	_/SP	S-	P-F	1) Shaded ha	ngers require	web stiffeners at joi	st ends. V	Veb stiffe	ners may	be
40 or AJS TM 20 Joist Width = 2-1/2" or S-P-F species solid sawn or LVL beader. Some loads may be increased for duration of load adjustments. Refer to USP Full Line Catalog for details. 3KH2520L/R (14) 10d (10) 10d x 1-1/2 3265 2910 2320 2065 3KH2520L/R (16) 10d (10) 10d x 1-1/2 3265 2910 2320 2065 3KH2520L/R (16) 10d (10) 10d x 1-1/2 3265 2910 2320 2065 3KH410L/R ⁷ (16) 10d (10) 10d x 1-1/2 3265 2910 2320 2065 3KH410L/R ⁷ (16) 16d (10) 16d 3690 3560 2620 2530 3KH410L/R ⁷ (22) 16d (10) 16d 7405 3560 5260 2530 3KH411L/R ⁷ (22) 16d (10) 16d 7405 3560 5260 2530 3KH4141L/R ⁷ (22) 16d (10) 16d 7405 3560 5260 2530 3KH4141L/R ⁷ (22) 16d (10) 16d 7405 3560 5260 2530 3KH4141L/R ⁷ (22) 16d (10) 16d 7405 3560 5260 2530				Down ²	Uplift ³	Down ²	Uplift ³	required for	non-shaded	hangers by I-joist m	anufactur	ers.		
KH2520L/R (14) 10d (10) 10d x 1-1/2 3265 2910 2320 2065 SKH2520L/R (14) 10d (10) 10d x 1-1/2 3265 2910 2320 2065 SKH2520L/R (16) 10d (10) 10d x 1-1/2 3265 2910 2320 2065 SKH2520L/R (16) 10d (10) 10d x 1-1/2 3265 2910 2320 2065 SKH2524L/R (16) 10d (10) 10d x 1-1/2 3265 2910 2320 2065 SKH2524L/R (16) 10d (10) 10d x 1-1/2 3265 2910 2320 2065 SKH410L/R ⁷ (16) 16d (10) 16d 3690 3560 2620 2530 SKH414L/R ⁷ (22) 16d (10) 16d 7405 3560 5260 2530 SKH414L/R ⁷ (22) 16d (10) 16d 7405 3560 5260 2530 SKH414L/R ⁷ (22) 16d (10) 16d 7405 3560 5260 2530 SKH414L/R ⁷ (22) 16d (10) 16d 7405 3560 5260 2530 SKh414L/R ⁷ Joist Down ²	Stock No.1	Plate	Rafter	100%	115%	100%	115%	2) Loads lister	d are based o	n 2001 NDS® and h	anger att	achment	to a DF-L	., SP,
KH2520L/R (14) 10d (10) 10d x 1-1/2 3265 2910 2320 2065 KH2520L/R (16) 10d (10) 10d x 1-1/2 3265 2910 2320 2065 KH2524L/R (16) 10d (10) 10d x 1-1/2 3265 2910 2320 2065 SKH2524L/R (16) 10d (10) 10d x 1-1/2 3265 2910 2320 2065 SKH410L/R' (16) 16d (10) 16d 3690 3560 2620 2530 KH410L/R' (16) 16d (10) 16d 7405 3560 5260 2530 KH414L/R' (22) 16d (10) 16d 7405 3560 5260 2530 KH414L/R' (22) 16d (10) 16d 7405 3560 5260 2530 KH414L/R' (22) 16d (10) 16d 7405 3560 5260 2530 KH414L/R' (22) 16d (10) 16d 7405 3560 5260 2530 KH414L/R' (22) 16d (10) 16d 7405 3560 5260 2530 KKH414L/R' (22) 16d (10) 16d 115%	40 or AJS™	20	Joist	Width	= 2-1/2″			or S-P-F sp	ecies solid sa	wn or LVL header.	Some loa	ds may b	e increas	ed for
KH2524L/R (16) 10d (10) 10d x 1-1/2 3265 2910 2320 2065 SKH2524L/R (16) 10d (10) 10d x 1-1/2 3265 2910 2320 2065 SKH2524L/R (16) 10d (10) 10d x 1-1/2 3265 2910 2320 2065 SKH410L/R ⁷ (16) 16d (10) 16d 3690 3560 2620 2530 SKH414L/R ⁷ (22) 16d (10) 16d 7405 3560 5260 2530 SKH414L/R ⁷ (22) 16d (10) 16d 7405 3560 5260 2530 SKH414L/R ⁷ (22) 16d (10) 16d 7405 3560 5260 2530 SKH414L/R ⁷ (22) 16d (10) 16d 7405 3560 5260 2530 SKH414L/R ⁷ (22) 16d (10) 16d 7405 3560 5260 2530 SKH414L/R ⁷ (22) 16d (10) 16d 7405 3560 5260 2530 SKH414L/R ⁷ (22) 16d (10) 16d 7405 3560 5260 2530 MSP Fastener Schedule ⁴ DF-L/SP	SKH2520L/R	(14) 10d	(10) 10d x 1-1/2	3265	2910	2320	2065	duration of	load adjustm	ents. Refer to USP I	-ull Line (Catalog fo	or details.	
KH2524L/R (16) 10d (10) 10d x 1-1/2 3265 2910 2320 2065 Joist Width = 3-1/2" Joist Width = 3-1/2" Minimum nail penetration shall be 1-1/2" for 10d nails. KH410L/R' (16) 16d (10) 16d 3690 3560 2620 2530 KH410L/R' (16) 16d (10) 16d 3690 3560 2620 2530 KH414L/R' (22) 16d (10) 16d 7405 3560 2620 2530 KH414L/R' (22) 16d (10) 16d 7405 3560 5260 2530 KH414L/R' (22) 16d (10) 16d 7405 3560 5260 2530 KH414L/R' (22) 16d (10) 16d 7405 3560 5260 2530 KH414L/R' (22) 16d (10) 16d 7405 3560 5260 2530 KH414L/R' (22) 16d (10) 16d 7405 3560 5260 2530 KKH414L/R' (22) 16d (10) 16d 7405 3560 5260 2530 Kock No.1 Header Joist 100% 115% Now <	SKH2520L/R			<u> </u>			<u> </u>	3) Uplift loads	have been in	creased 15% for wir	nd and se	ismic load	ding; no f	urther
Joist Width = 3-1/2" Minimum nail penetration shall be 1-1/2" for 10d nails. SKH410L/R' (16) 16d (10) 16d 360 2620 2530 KKH414L/R' (22) 16d (10) 16d 7 405 3560 2620 2530 KKH414L/R' (22) 16d (10) 16d 7 405 3560 2620 2530 Variable Pitch Connectors 5 Variable Pitch Connectors Yariable Pitch Connectors Yariable P		. ,	. ,											
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KKH410L/R ⁷ (16) 16d (10) 16d 3690 3560 2620 2530 KKH414L/R ⁷ (22) 16d (10) 16d 7405 3560 5260 2530 KKH414L/R ⁷ (22) 16d (10) 16d 7405 3560 5260 2530 KKH414L/R ⁷ (22) 16d (10) 16d 7405 3560 5260 2530 KH414L/R ⁷ (22) 16d (10) 16d 7405 3560 5260 2530 KH414L/R ⁷ (22) 16d (10) 16d 7405 3560 5260 2530 KH414L/R ⁷ (22) 16d (10) 16d 7405 3560 5260 2530 KH414L/R ⁷ (22) 16d (10) 16d 7405 3560 5260 2530 Variable Pitch Connectors S-P-F Second Sock No.1 Down ² Uplift ³ Down ² Uplift ³ 40 or AJS TM 20 100% 115% 100% 115% 100% 115% MP42 (6) 10d (4) 10d x 1-1/2 2835 275 2835 275 MP44 (10) 10d (8) 10d x	25 SKH410L/P ⁷	(10) 40 -				2000	0500					ubo <u>tite</u> te	for 10-	
KH414L/R' (22) 16d (10) 16d 7405 3560 5260 2530 KH414L/R' (22) 16d (10) 16d 7405 3560 5260 2530 KH414L/R' (22) 16d (10) 16d 7405 3560 5260 2530 KH414L/R' (22) 16d (10) 16d 7405 3560 5260 2530 KH414L/R' (22) 16d (10) 16d 7405 3560 5260 2530 Variable Pitch Connectors Variable Pitch Connectors Stock No.1 Header DF-L/SP S-P-F LUSP Stock No.1 Down2 Uplift ³ Down2 Uplift ³ 100% 115% 100% 115% 00% 115% 40 or AJS TM 20 Colspan="4">Concertors.com MP25 (6) 10d (4) 10d x 1-1/2 2835 275 2835 275 5 MP4 (10) 10d (8) 10d x 1-1/2 2485 250 2995 215 MP4 (6) 10d (4) 10d x 1-1/2 2485		. ,					<u> </u>			eiei) by 3-1/4 long	may be s	upstituted	LIUT IUD	Jouino
KH414L/R ⁷ (22) 16d (10) 16d 7405 3560 5260 2530 SKH414L/R ⁷ (22) 16d (10) 16d 7405 3560 5260 2530 SKH414L/R ⁷ (22) 16d (10) 16d 7405 3560 5260 2530 Variable Pitch Connectors 3-1/2" minimum header thickness for all other stock numbers. Variable Pitch Connectors Variable Pitch Connectors Stock No. ¹ Header DF-L/SP S-P-F Header Joist 100% 115% 100% 115% 40 or AJS T* 20 75 2630 275 2630 275 MP25 (6) 10d (4) 10d x 1-1/2 2435 275 2630 275 5 5 5 5 5 7 2435 250 2995 215 MP4 (6) 10d (4) 10d x 1-1/2 2435 275 2835 275 MP44 (10) 10d (8) 10d x 1-1/2 2485 250 2995 215	7		<u> </u>				<u> </u>			ire minimum 3″ hoo	der width	for THO	series ha	ndei
KH414L/R ⁷ (22) 16d (10) 16d 7405 3560 5260 2530 6) For additional sizes, stock numbers, and modifications not shown, refer to USP's Full Line Catalog. Variable Pitch Connectors Variable Pitch Connectors USP Fastener Schedule ⁴ DF-L/SP S-P-F USP Down ² Uplift ³ Down ² Uplift ³ Own ² Uplift ³ 40 or AJS T ^M 20 Own ² Uplift ³ Conformation please call 1.800.328.5934 or go to www.USPconnectors.com MP25 (6) 10d (4) 10d x 1-1/2 2435 275 2630 275 S MP4 (6) 10d (4) 10d x 1-1/2 2435 275 2835 275 MP4 (10) 10d (8) 10d x 1-1/2 2435 250 2995 215 TMP4 (10) 10d (8) 10d x 1-1/2 2435 250 2995 215 MPH4 (10) 10d (8) 10d x 1-1/2 2435 250 2995 215	-						<u> </u>						301103 110	liger
Variable Pitch Connectors refer to USP's Full Line Catalog. USP Stock No. ¹ Header Joist Obwm ² Uplift ³ Towm ² Uplift ³ Dowm ² Uplift ³ Towm ² Uplift ³	-												wn.	0
Fastener Schedule ⁴ DF-L/SP S-P-F Stock No. ¹ Down ² Uplift ³ Down ² Uplift ³ 40 or AJS TM 20 70 Niter cut required along top chord for lateral restraint. MP25 (6) 10d (4) 10d x 1-1/2 2630 275 MP42 (6) 10d (4) 10d x 1-1/2 2835 275 MP44 (10) 10d (8) 10d x 1-1/2 3485 250 2995 MP44 (10) 10d (8) 10d x 1-1/2 3485 250 2995 215 MP44 (10) 10d (8) 10d x 1-1/2 3485 250 2995 215 hangers require web stiffeners at joist ends. Web stiffeners may be required for non-shaded step l-joist manufacturers. TMP TMP			()					1 [/]					,	0
USP Stock No.1 Down ² Uplift ³ U		-		1	/SD	S.	D_E	7) Miter cut re	quired on end	l of joist to achieve o	lesign loa	ids.		0
Stock No.1 Header Joist 100% 115% 100% 115% for further information please call 1.800.328.5934 or go to www.USPconnectors.com MP25 (6) 10d (4) 10d x 1-1/2 2630 275 2630 275 MP425 (10) 10d (8) 10d x 1-1/2 2485 250 2995 215 5 Impear of the state of th								8) LSTA24 str	ap required a	long top chord for la	teral rest	raint.		0
40 or AJS TM 20 Please call 1.800.328.5934 or go to MP25 (6) 10d (4) 10d x 1-1/2 2630 275 2630 275 MPH25 (10) 10d (8) 10d x 1-1/2 3485 250 2995 215 5 5 5 5 5 100 100 x 1-1/2 3485 250 2995 215 MPH4 (10) 10d (8) 10d x 1-1/2 3485 250 2995 215 hangers require web stiffeners at joist ends. Web stiffeners may be required for non-shaded so by I-joist manufacturers. TMP TMP 100 x 1-1/2 3485 250 2995 215 100 x 1-1/2 100 x 1-1/2 <td< td=""><td>USP</td><td>I</td><td>loist</td><td></td><td>·</td><td></td><td></td><td></td><td></td><td></td><td>A</td><td></td><td></td><td>12</td></td<>	USP	I	loist		·						A			12
MP25 (6) 10d (4) 10d x 1-1/2 2630 275 2630 275 MPH25 (10) 10d (8) 10d x 1-1/2 3485 250 2995 215 5 MPH4 (6) 10d (4) 10d x 1-1/2 2835 275 2835 275 100 MWw.USPconnectors.com Image: Comparison of the		Header		//	.1370	.30 /0	1.13/0	· · · · · · · · · · · · · · · · · · ·	11.800.32	28.5934	~>			2
MPH25 (10) 10d (8) 10d x 1-1/2 3485 250 2995 215 5 5 5 5 5 5 6 6 10d x 1-1/2 2835 275 2835 275 100 100 100 x 1-1/2 2835 275 2835 275 100 100 100 x 1-1/2 3485 250 2995 215 100 100 100 x 1-1/2 3485 250 2995 215 100 100 100 x 1-1/2 3485 250 2995 215 100 100 100 x 1-1/2 3485 250 2995 215 100 100 100 x 1-1/2 3485 250 2995 215 100 100 100 x 1-1/2 3485 250 2995 215 100 100 100 x 1-1/2 3485 250 2995 215 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 <t< td=""><td>Stock No.¹</td><td></td><td></td><td></td><td>275</td><td>2630</td><td>275</td><td></td><td>onnector</td><td>s.com</td><td>11</td><td>S</td><td></td><td></td></t<>	Stock No. ¹				275	2630	275		onnector	s.com	11	S		
5 MP4 (6) 10d (4) 10d x 1-1/2 2835 275 2835 275 MPH4 (10) 10d (8) 10d x 1-1/2 3485 250 2995 215 hangers require web stiffeners at joist ends. Web stiffeners may be required for non-shaded s by I-joist manufacturers.	Stock No. ¹ 40 or AJS™	20	(4) 10d x 1-1/2	2630						1 August	1 6			
Imp4 (6) 10d (4) 10d x 1-1/2 2835 275 2835 275 ImpH4 (10) 10d (8) 10d x 1-1/2 3485 250 2995 215 hangers require web stiffeners at joist ends. Web stiffeners may be required for non-shaded s by I-joist manufacturers. Imphi Imphi Imphi	Stock No. ¹	20 (6) 10d				2995	215			~ //		S. M.		
MPH4 (10) 10d (8) 10d x 1-1/2 3485 250 2995 215 hangers require web stiffeners at joist ends. Web stiffeners may be required for non-shaded s by I-joist manufacturers. TMP Image: Comparison of the state of the	Stock No. ¹ I40 or AJS™ TMP25	20 (6) 10d				2995	215			G		27		
s by I-joist manufacturers.	Stock No. ¹ 140 or AJS™ TMP25 TMPH25 25	20 (6) 10d (10) 10d	(8) 10d x 1-1/2	3485	250						U	2		
	Stock No. ¹ 140 or AJS™ TMP25 TMPH25 25 TMP4 TMPH4	20 (6) 10d (10) 10d (6) 10d (10) 10d	 (8) 10d x 1-1/2 (4) 10d x 1-1/2 (8) 10d x 1-1/2 	3485 2835 3485	250 275 250	2835 2995	275 215			000	k	2	110	
	Stock No. ¹ 40 or AJS TM TMP25 5 5 TMP4 TMP44 hangers require	20 (6) 10d (10) 10d (6) 10d (10) 10d e web stiffene	 (8) 10d x 1-1/2 (4) 10d x 1-1/2 (8) 10d x 1-1/2 	3485 2835 3485	250 275 250	2835 2995	275 215	on-shaded		TMP			1	



TFL



THO



THF single



LSSH March 2014

2) Loads listed are or LVL header. L 3) Uplift loads have bee

4) 10d x 1-1/2" nails are 9 gauge (0.148" diameter) by 1-1/2" long.

Minimum nail penetration shall be 1-1/2" for 10d nails.

TMPH4 1) Shaded hangers hangers by I-jois

Joist

Height AJS™ 140 or . 9-1/2

11-7/8

14

16 AJS™ 25

9-1/2

11-7/8

14

16

18

Joist

Height

9-1/2

11-7/8

14

16 AJS™ 25 9-1/2

11-7/8

14

16

18

Joist

Height

11-7/8 14

16

11-7/8

14

16

18

Joist

Width AJS™ 140 or A TMP25

2-1/2

3-1/2

AJS™ 25

AJS™ 25 9-1/2

AJS™ 140 or / 9-1/2

AJS™ 140 or /

TMPH

DOUBLE I-JOISTS - Canadian/Factored Resistance (lbs)



		Т	op Mount Ha	ngers⁵					Fa	ce Mount Han	igers			
		Fastener	Schedule ⁴	DF-L	_/SP	S-F	P-F		Fastener	Schedule ⁴	DF-I	_/SP	S-I	P-F
Joist	USP			Down ²	Uplift ³	Down ²	Uplift ³	USP			Down ²	Uplift ³	Down ²	Uplift ³
Height	Stock No. 1,5	Header	Joist	100%	115%	100%	115%	Stock No.1	Header	Joist	100%	115%	100%	115%
AJS™	140 or AJS™	20				Jois	st Width	= 5″						
9-1/2	THO25950-2	(10) 16d	(6) 10d	6005	2210	4265	1570	THF25925-2	(12) 10d	(6) 10d	5240	3325	3720	2361
11-7/8	THO25118-2	(10) 16d	(6) 10d	6005	2210	4265	1570	THF25112-2	(16) 10d	(6) 10d	5240	3325	3720	2361
14	THO25140-2	(12) 16d	(6) 10d	6645	2210	4715	1570	THF25140-2	(20) 10d	(6) 10d	6680	3325	4743	2361
16	THO25160-2	(12) 16d	(6) 10d	6645	2210	4715	1570	THF25160-2	(24) 10d	(6) 10d	6680	3325	4743	2361
18	THO25180-2	(14) 16d	(6) 10d	9500	2210	6745	1570	THF25160-2	(24) 10d	(6) 10d	6680	3325	4743	2361
AJS™	25					Jois	st Width	= 7″						
9-1/2	BPH7195	(10) 16d	(6) 10d	5055	1245	4725	885	HD7100	(12) 16d	(6) 10d	7215	4435	5123	3149
11-7/8	BPH71118	(10) 16d	(6) 10d	5055	1245	4725	885	HD7120	(16) 16d	(6) 10d	7215	4435	5123	3149
14	BPH7114	(10) 16d	(6) 10d	5055	1245	4725	885	HD7120	(16) 16d	(6) 10d	7215	4435	5123	3149
16	BPH7116	(10) 16d	(6) 10d	5055	1245	4725	885	HD7160	(24) 16d	(8) 10d	7215	4435	5123	3149
18	BPH7118	(10) 16d	(6) 10d	5055	1245	4725	885	HD7160	(24) 16d	(8) 10d	7215	4435	5123	3149
		Adju	stable Height	Hangers					Sk	ewed 45° Han	igers			
		Fastener	Schedule ⁴	DF-L	_/SP	S-F	P-F		Fastener	Schedule ⁴	DF-I	_/SP	S-I	P-F
Joist	USP			Down ²	Uplift ³	Down ²	Uplift ³	USP			Down ²	Uplift ³	Down ²	Uplift ³
Height	Stock No. 1,8	Header	Joist	100%	115%	100%	115%	Stock No.1	Plate	Rafter	100%	115%	100%	115%
AJS™	140 or AJS™	20				Jois	t Width	= 5″						
9-1/2	MSH2622-2	(6) 10d	(4) 10d	1490		1055		SKH2520L/R-27	(14) 10d	(10) 10d	5430	3565	3855	2530
11-7/8	MSH2622-2	(6) 10d	(4) 10d	1490		1055		SKH2524L/R-27	(16) 10d	(10) 10d	5055	3560	3590	2530
14	MSH2622-2	(6) 10d	(4) 10d	1490		1055		SKH2524L/R-27	(16) 10d	(10) 10d	5055	3560	3590	2530
16	MSH2622-2	(6) 10d	(4) 10d	1490		1055		SKH2524L/R-27	(16) 10d	(10) 10d	5055	3560	3590	2530
AJS™	25					Jois	t Width	= 7″						
9-1/2								HD7100-SK45L/R ^{6,7}	(12) 16d	(6) 10d	7215	3325	5123	2360
11-7/8	MSH422-2	(8) 16d	(6) 16d	2295		1630		HD7120-SK45L/R ^{6,7}	(16) 16d	(6) 10d	7215	3325	5123	2360
14	MSH422-2	(8) 16d	(6) 16d	2295		1630		HD7120-SK45L/R6,7	(16) 16d	(6) 10d	7215	3325	5123	2360
16	MSH422-2	(8) 16d	(6) 16d	2295		1630		HD7160-SK45L/R ^{6,7}	(24) 16d	(8) 10d	7215	3325	5123	2360
18	MSH422-2	(8) 16d	(6) 16d	2295		1630		HD7160-SK45L/R ^{6,7}	(24) 16d	(8) 10d	7215	3325	5123	2360

1) Shaded hangers require web stiffeners at joist ends. Web stiffeners may be required for non-shaded hangers by I-joist manufacturers. 2) Loads listed are based on 2001 NDS® and hanger attachment to a DF-L, SP, or S-P-F species solid sawn or LVL header.

Some loads may be increased for duration of load adjustments. Refer to USP Full Line Catalog for details.

3) Uplift loads have been increased 15% for wind and seismic loading; no further increase shall be permitted.

4) 10d x 1-1/2" nails are 9 gauge (0.148"" diameter) by 1-1/2" long.

Minimum nail penetration shall be 1-1/2" for 10d nails and 1-5/8" for 16d nails.

16d sinkers (0.148" diameter) by 3-1/4" long may be substituted for 10d common nails with no load reduction.

5) Top Mount Hangers require minimum 3" header width for THO series hangers; 3-1/2" minimum header thickness for all other stock numbers.

6) Hangers are special order. Consult USP for pricing and lead times.

7) Miter cut required on end of joist to achieve design loads.

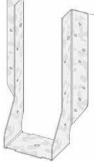
8) For additional sizes, stock numbers, and modifications not shown, refer to USP's Full Line Catalog.



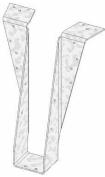




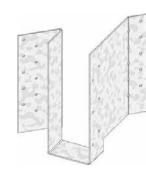
BPH



THF Double



THO Double



For further information

please call 1.800.328.5934

or go to www.USPconnectors.com

SKH_L Double left shown

WEIGHTS OF BUILDING MATERIALS

CEILING Pounds Per Squa	re Foot [PSF]
Acoustical fiber tile ⁽¹⁾	1
Suspended steel channel system (1)	2
Suspended wood channel system	2.5
2x8 ceiling joists @ 16" o.c., R-49 insulation,	7
1/2" gypsum board	
1" Plaster	8
1/2" gypsum board	2.2
5∕s" gypsum board	2.75
ROOF Pounds Per Squa	
Fiberglass shingles	3
Asphalt shingles (1)	2
Wood shingles (1)	3
Spanish clay tile (1)	19
Composition Roofing:	
Three-ply ready roofing (1)	1
Four-ply felt and gravel ⁽¹⁾	5.5
Five-ply felt and gravel ⁽¹⁾	6
20 gage metal deck ⁽¹⁾	2.5
18 gage metal deck ⁽¹⁾	3
1" fiberglass batt insulation	0.04
1" loose fiberglass insulation	0.04
1" loose cellulose insulation	0.14
1" rigid insulation ⁽¹⁾	1.5
³ /16" slate ⁽¹⁾	7
1/4" slate (1)	10
Single-ply (no ballast) ⁽¹⁾	0.7
Single-ply (ballasted)	11
Dry gravel ⁽¹⁾	8.7
2x8 rafters @ 16" o.c., fiberglass shingles, 15# felt,	
	8
3%" sheathing	0
%" sheathing Skylight: metal frame w/ %" wire glass ⁽¹⁾	8
3/6" sheathing Skylight: metal frame w/ 3/6" wire glass ⁽¹⁾ FLOOR Pounds Per Squa	8 re Foot [PSF]
3/6" sheathing Skylight: metal frame w/ 3/6" wire glass ⁽¹⁾ FLOOR Pounds Per Squa 1" reinforced regular weight concrete	8 re Foot [PSF] 12.5
3/6" sheathing Skylight: metal frame w/ 3/6" wire glass (1) FLOOR Pounds Per Squa 1" reinforced regular weight concrete 1" plain lightweight concrete (1)	o 8 re Foot [PSF] 12.5 8
3/6" sheathing Skylight: metal frame w/ 3/6" wire glass (1) FLOOR Pounds Per Squa 1" reinforced regular weight concrete 1" plain lightweight concrete (1) 7/16" cementitious backerboard	o 8 re Foot [PSF] 12.5 8 3
3/6" sheathing Skylight: metal frame w/ 3/6" wire glass (1) FLOOR Pounds Per Squa 1" reinforced regular weight concrete 1" plain lightweight concrete (1) 7/16" cementitious backerboard Ceramic or quarry tile (3/4") on 1/2" mortar bed (1)	o 8 re Foot [PSF] 12.5 8 3 16
3/6" sheathing Skylight: metal frame w/ 3/6" wire glass (1) FLOOR Pounds Per Squa 1" reinforced regular weight concrete 1" plain lightweight concrete (1) 7/16" cementitious backerboard Ceramic or quarry tile (3/4") on 1/2" mortar bed (1) Ceramic or quarry tile (3/4") on 1" mortar bed (1)	o 8 re Foot [PSF] 12.5 8 3 16 23
3/6" sheathing Skylight: metal frame w/ 3/8" wire glass (1) FLOOR Pounds Per Squa 1" reinforced regular weight concrete 1" plain lightweight concrete (1) 7/16" cementitious backerboard Ceramic or quarry tile (3/4") on 1/2" mortar bed (1) Ceramic or quarry tile (3/4") on 1" mortar bed (1) 1" mortar bed	o 8 re Foot [PSF] 12.5 8 3 16 23 12
3/6" sheathing Skylight: metal frame w/ 3/6" wire glass ⁽¹⁾ FLOOR Pounds Per Squa 1" reinforced regular weight concrete 1" plain lightweight concrete ⁽¹⁾ 7/ ₁₆ " cementitious backerboard Ceramic or quarry tile (3/4") on 1/2" mortar bed ⁽¹⁾ Ceramic or quarry tile (3/4") on 1" mortar bed ⁽¹⁾ 1" mortar bed 1" slate ⁽¹⁾	o 8 re Foot [PSF] 12.5 8 3 16 23
3/6" sheathing Skylight: metal frame w/ 3/6" wire glass ⁽¹⁾ FLOOR Pounds Per Squa 1" reinforced regular weight concrete 1" plain lightweight concrete ⁽¹⁾ 7/ ₁₆ " cementitious backerboard Ceramic or quarry tile (3/4") on 1/2" mortar bed ⁽¹⁾ Ceramic or quarry tile (3/4") on 1" mortar bed ⁽¹⁾ 1" mortar bed 1" slate ⁽¹⁾ 3/6" marble tile	o 8 re Foot [PSF] 12.5 8 3 16 23 12 15
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(1) Minimum Design Loads for Buildings and Other Structures, ASCE 7-05.

(2) National Farm Building Code (Canada) 1995. Value in pounds per cubic foot (PCF), multiply by maximum height to obtain PSF.

	er Square Foot [PSF]
¹¹ / ₃₂ " or 3/8" Plywood – OSB ⁽³⁾	1.0 – 1.2
¹⁵ / ₃₂ " or ¹ / ₂ " Plywood – OSB ⁽³⁾	1.4 – 1.7
¹⁹ / ₃₂ " or ⁵ / ₈ " Plywood – OSB ⁽³⁾	1.8 – 2.1
²³ / ₃₂ " or ³ ⁄ ₄ " Plywood – OSB ⁽³⁾	2.2 – 2.5
⁷ / ₆ " Plywood – OSB ⁽³⁾	2.6 – 2.9
1 ¹ / ₆ " Plywood – OSB ⁽³⁾	3.3 – 3.6
1/2" cementitious backerboard	3
1 ¹ / ₂ " softwood T & G decking	4.6
	er Square Foot [PSF]
2x4 @ 16" o.c.	1.1
2x6 @ 16" o.c.	1.7
2x8 @ 16" o.c.	2.2
2x10 @ 16" o.c.	2.9
2x12 @ 16" o.c.	3.5
BCI® 4500s, 5000 or 5000s @ 12" o.c.	2.1 – 2.9
BCI [®] 4500s, 5000 or 5000s @ 16" o.c.	1.6 – 2.2
BCI [®] 4500s, 5000 or 5000s @ 19.2" o.c.	1.3 – 1.8
BCI [®] 4500s, 5000 or 5000s @ 24" o.c.	1.1 – 1.5
BCI [®] 6000 or 6000s @ 12" o.c.	2.5 – 3.4
BCI [®] 6000 or 6000s @ 16" o.c.	1.9 – 2.6
BCI [®] 6000 or 6000s @19.2" o.c.	1.6 - 2.1
BCI® 6000 or 6000s @ 24" o.c.	1.3 - 1.7
BCI [®] 60, 60s, 6500 or 6500s @ 12" o.c.	2.5 – 3.8
BCI® 60, 60s, 6500 or 6500s @ 16" o.c.	1.9 – 2.9
BCI® 60, 60s, 6000 or 6500s @19.2" o.c.	1.6 – 2.4
BCI® 60, 60s, 6500 or 6500s @ 24" o.c.	1.3 – 1.9
BCI® 90, 90s or 90e @ 12" o.c.	3.9 – 5.4
BCI® 90, 90s or 90e @ 12" 0.c.	2.9 – 4.1
BCI® 90, 90s or 90e @ 19.2" o.c.	2.9 - 4.1
BCI® 90, 90s or 90e @ 24" o.c.	1.9 – 2.7
AJS [®] 140 or 20 @ 12" o.c.	2.2 - 3.3
AJS® 140 of 20 @ 12 0.c. AJS® 140 or 20 @ 16" o.c.	
AJS® 140 of 20 @ 19.2" o.c.	1.7 – 2.5 1.4 – 2.1
AJS [®] 140 of 20 @ 19.2 0.C. AJS [®] 140 or 20 @ 24" o.C.	1.4 - 2.1
AJS® 140 01 20 @ 24 0.0. AJS® 25 @ 12" o.c.	3.1 – 5.4
AJS [®] 25 @ 16" o.c.	2.3 – 4.1
AJS [®] 25 @ 19.2" o.c.	1.9 - 3.4
AJS [®] 25 @ 24" o.c.	1.6 – 2.7
	er Square Foot [PSF]
⁵ / ₁₆ " x 7 ¹ / ₂ " fiber cement lap siding	3
4" clay brick ⁽¹⁾	39
1/4" ceramic wall tile ⁽¹⁾	3.1
1 ³ / ₄ " Cultured Stone	12
2x4 studs @ 16" o.c., %" gypsum, insulation, %"	' siding ⁽¹⁾ 11
2x6 studs @ 16" o.c., 5/8" gypsum, insulation, 3/8"	
Wood or steel studs, 1/2" gypsum board each	n side ⁽¹⁾ 8
Exterior stud walls w/ brick veneer (1)	48
Stucco	10
Log Wall: 10" diameter	26
Glass Block:	
4" Thick - standard (hollow)	20
3" Thick - standard (hollow)	16
4" Thick - thin face	30
3" Thick - solid glass block	40
Windows: glass, frame and sash ⁽¹⁾	8
Include at least 1.5 psf in all dead load summ	-
Include at least 1.3 psi in all dead load Sumi	
for incidentals such as plumbing, ducts, light	t fixtures etc

(3) Approximate Engineering Dead Load Weight of Wood Structural Panels, APA EWS TT-019, 2005.



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Your local retailer

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CCMC Report Number 12787-R ALLJOIST®

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455, boulevard Fénelon, suite 102 Dorval (Québec) QC H9S 5T8 www.BC.com/ewp



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