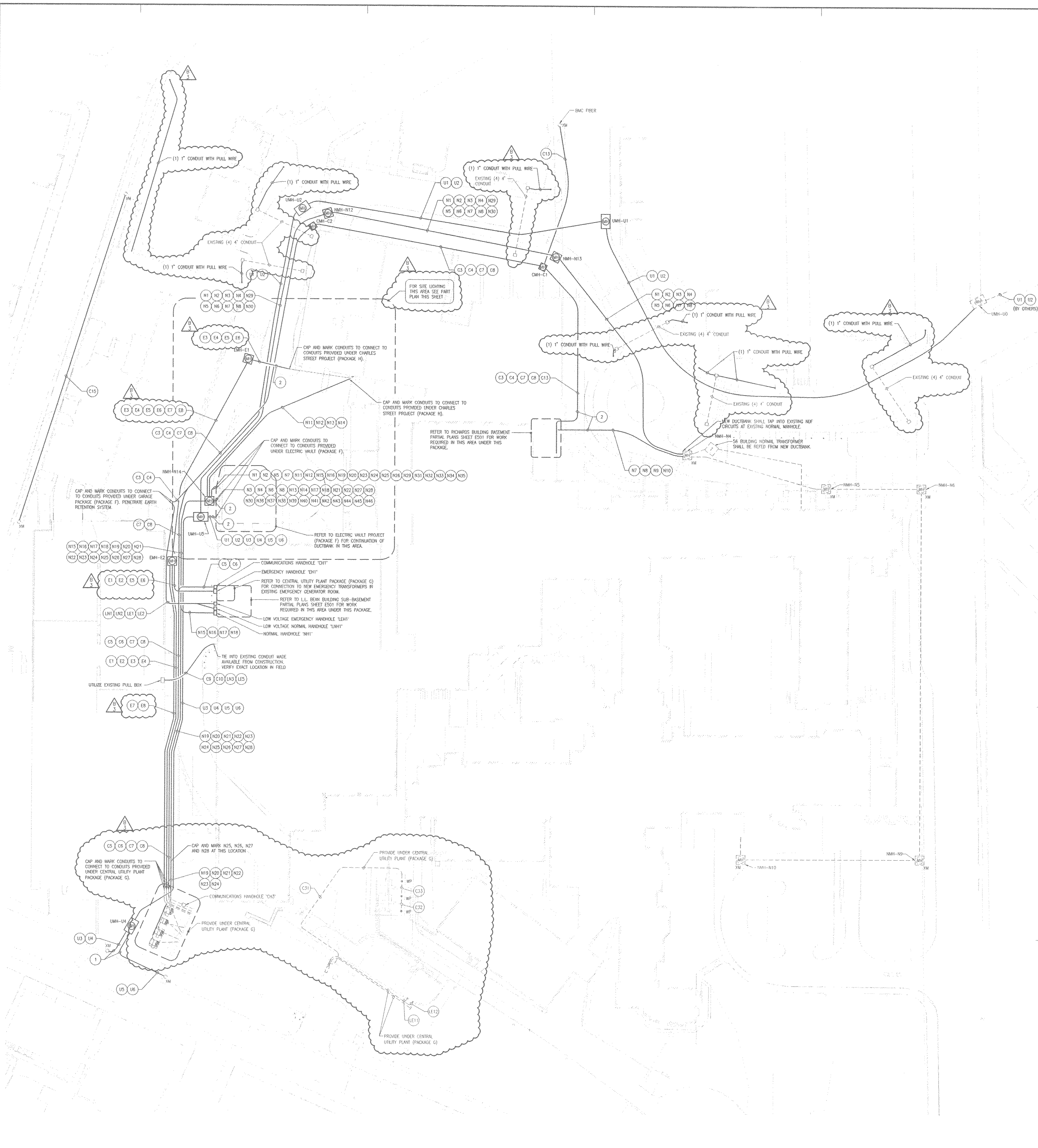


2 LIGHTING PART PLAN
1" = 20'



1 POWER PLAN
1" = 20'

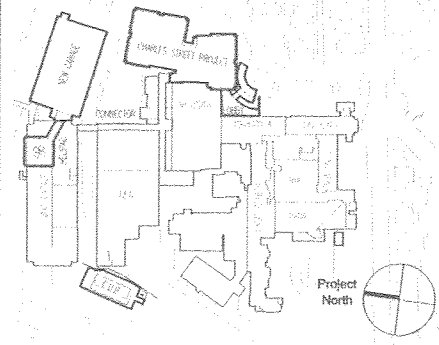
- General Notes:
- LEGEND:**
- HANDHOLE (SEE DETAIL ON SHEET E000)
 - ⊠ MANHOLE (SEE DETAILS ON SHEET E000)
 - NEW
 - - - EXISTING TO REMAIN
 - - - REFER TO ALTERNATE PACKAGE
 - T.O.D. TOP OF DUCTBANK
 - ⊠ SITE LIGHTING BASE PROVIDED BY CIVIL ENGINEERS
- GENERAL NOTES:**
1. ALL ELECTRICAL CONDUITS, DUCTBANKS, MANHOLES, AND HANDHOLES INDICATED ON THIS PLAN ARE PART OF THIS PACKAGE UNLESS SPECIFICALLY INDICATED OTHERWISE.
 2. ALL CONDUITS INDICATED ON THIS PLAN SHALL BE 1" SIZE AND ENCASED IN CONCRETE UNLESS NOTED OTHERWISE.
 3. DUCTBANK SECTIONS ARE FOR DIAGRAMMATIC PURPOSES ONLY. REFER TO CIVIL PLANS FOR CROSS SECTIONS.
 4. TOP OF ELECTRICAL DUCTBANKS SHALL BE LOCATED A MINIMUM OF 24" BELOW FINISHED GRADE.
 5. SEE SPECIFICATIONS FOR MINIMUM CLEARANCES FROM OTHER UTILITIES.
 6. UNLESS NOTED OTHERWISE, SLOPE CONDUITS AWAY FROM BUILDING ENTRANCE TO MANHOLE OR HANDHOLE.
 7. DUCTBANKS SHOWN ON THIS PLAN ARE FOR DIAGRAMMATIC PURPOSES ONLY. REFER TO CIVIL ENGINEERING PLANS FOR DUCTBANK LOCATIONS AND ELEVATIONS.
 8. MEET CMP REQUIREMENTS FOR CONDUIT INSTALLATION.

- DRAWING NOTES:**
- ① UTILITY CONDUITS WILL BE CONSTRUCTED TO POLE ENCASED IN CONCRETE. AT POLE TWO CONDUITS WILL BE CONSTRUCTED UP POLE AND TWO CONDUITS SHALL BE CAPPED FOR FUTURE USE.
 - ② PROVIDE A LOW POINT IN DUCT BANK RUN TO AVOID SLOPING INTO BUILDING.
 - ③ COORDINATE EXACT LOCATION OF LIGHT BASE WITH CIVIL DRAWINGS.

RECEIVED
OCT 2 2004
Maine Medical Center & Health
Consulting Engineers, Inc.

MARK	ISSUE DATE	DESCRIPTION

BILL OF MATERIALS	DATE	DRAWING NUMBER	DESCRIPTION
A1	09/27/04	ADDENDUM NO.1	
PERMIT	09/16/04	PERMIT SET	
BID	09/21/04	BID SET	



TRO
ARCHITECTURE
PLANNING
ENGINEERING
INTERIOR DESIGN

The Filchik Organization
80 Bridge Street
Newton, MA 02458-1134
617-559-9400

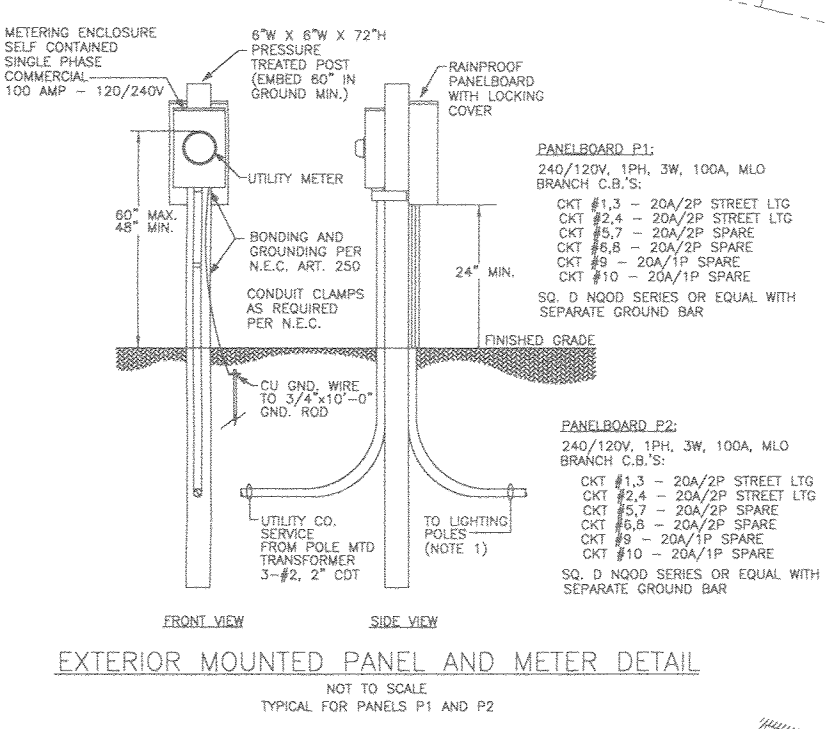
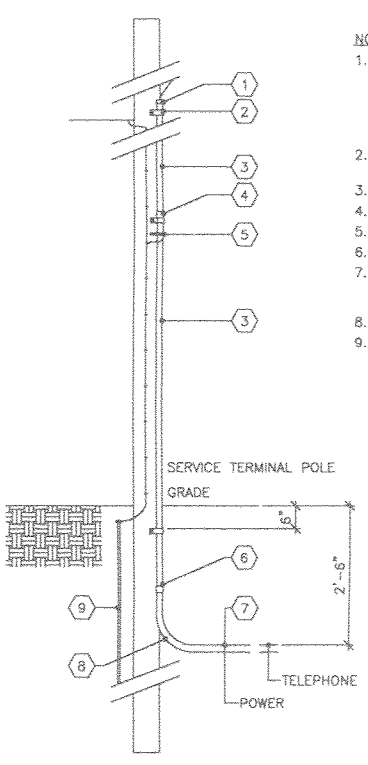
Maine Medical Center
Pkg C - Site Utilities
Portland, Maine MMC Project No. 21846

Drawing Title
ELECTRICAL SITE DISTRIBUTION PLAN

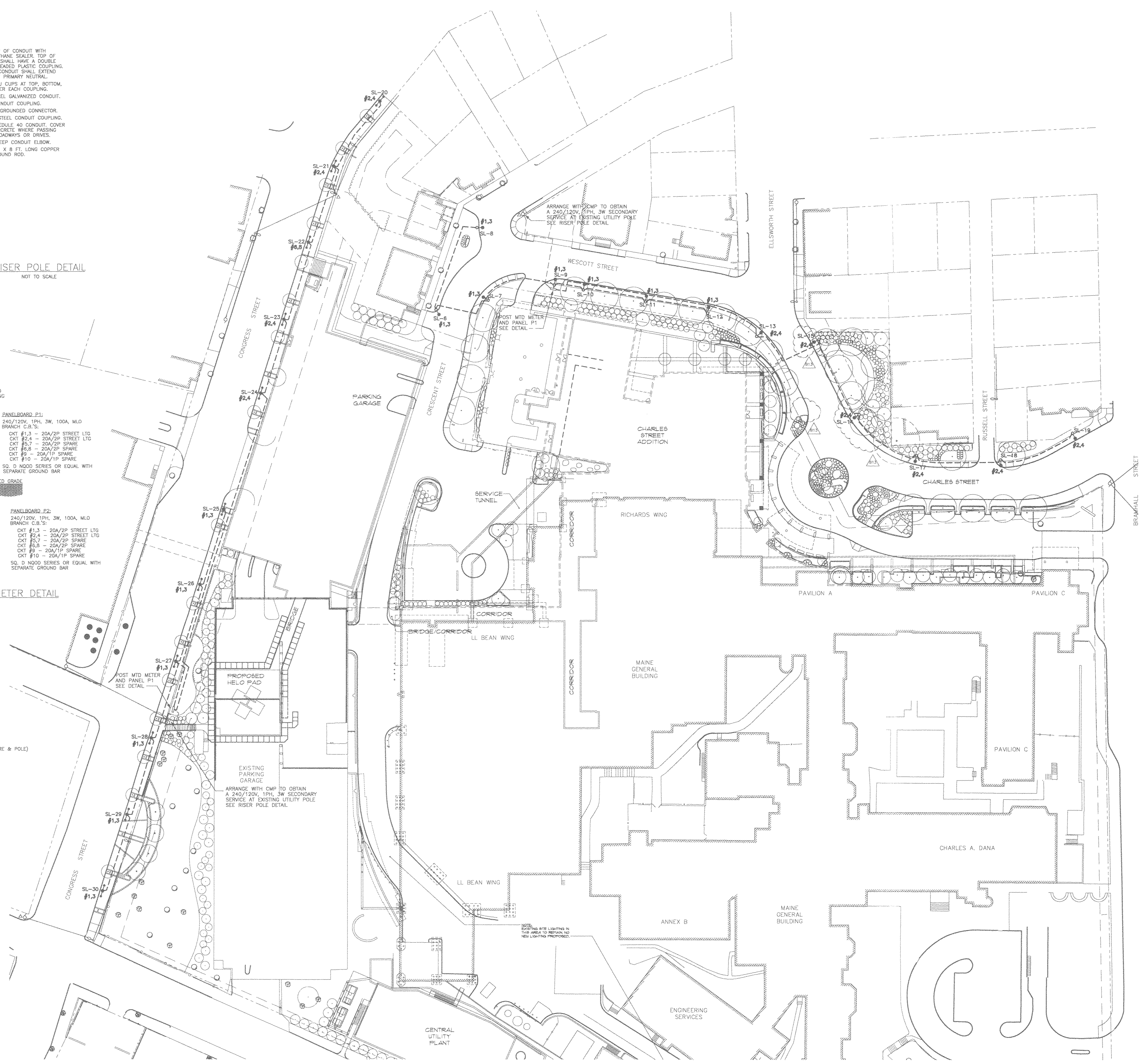
Commission No.	4677	Date Issued	09/31/04
Scale	1"=30'-0"	Sheet Number	PKG-C
Drawn By	G.S.	Approved By	T.S.
Filename	4677E040		
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- General Notes:
1. UNDERGROUND STREET LIGHTING WIRING SHALL BE #2 AWG CONDUCTORS IN 1-1/2" PVC SCHED. 40 CONDUIT.
 2. BURY ALL STREET LIGHTING CONDUITS 30" BELOW FINISHED GRADE (MINIMUM).
 3. PROVIDE A 1" CONCRETE ENVELOPE AROUND ALL STREET LIGHTING CONDUITS THAT ARE LOCATED BENEATH STREETS AND/OR VEHICLE DRIVES (CONCRETE ENVELOPE NOT REQUIRED BELOW SIDEWALKS).
 4. ALL WORK SHALL CONFORM NFPA 70.
 5. SEE CIVIL SITE DRAWINGS FOR LIGHTING POLE CONCRETE FOUNDATION BASES.

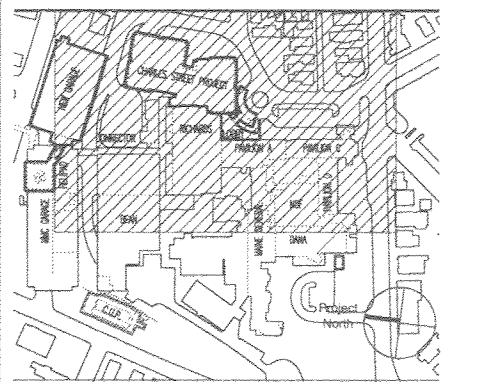
- NOTES:
1. SEAL TOP OF CONDUIT WITH POLYURETHANE SEALER. TOP OF CONDUIT SHALL HAVE A DOUBLE NON-THREADED PLASTIC COUPLING. TOP OF CONDUIT SHALL EXTEND 4" ABOVE PRIMARY NEUTRAL.
 2. 2 HOLE U CUPS AT TOP, BOTTOM, AND UNDER EACH COUPLING.
 3. RIGID STEEL GALVANIZED CONDUIT.
 4. STEEL CONDUIT COUPLING.
 5. CONDUIT GROUNDING CONNECTOR.
 6. PVC TO STEEL CONDUIT COUPLING.
 7. PVC SCHEDULE 40 CONDUIT COVER WITH CONCRETE WHERE PASSING UNDER ROADWAYS OR DRIVES.
 8. LONG SWEEP CONDUIT ELBOW.
 9. 1/2" DIA. X 8 FT. LONG COPPER CLAD GROUND ROD.



- SITE LIGHTING**
- POLES SL-6, SL-8
 - MANUFACTURER: KIM
 - MODEL: #1A/CC21A3/100MH240/BL-P/1A/VSF-1A/HS (LUMINAIRE)
 - MODEL: #PR14-4125/A/BL-P (POLE)
 - LAMP: 100 WATT METAL HALIDE
- POLES SL-15, SL-16, SL-17, SL-18
 - MANUFACTURER: KIM
 - MODEL: #1A/CC17A2/100MH240/BL/P/1A/VSF-1A/HS (LUMINAIRE)
 - MODEL: #PR14-4125/A/BL-P (POLE)
 - LAMP: 100 WATT METAL HALIDE
- POLES SL-7, SL-9, SL-10, SL-11, SL-12, SL-13
 - MANUFACTURER: KIM
 - MODEL: #1A/CC17A4/100MH240/BL-P/1A/VSF-1A (LUMINAIRE)
 - MODEL: #PR14-4125/A/BL-P (POLE)
 - LAMP: 100 WATT METAL HALIDE
- POLE SL-14
 - MANUFACTURER: ARCHITECTURAL AREA LIGHTING
 - MODEL: #SL-VTHS-70MH240/PRA-4R16-125/BLK/PHOTOCELL (LUMINAIRE & POLE)
 - LAMP: 70 WATT METAL HALIDE
- POLES SL-20 THRU SL-30
 - MANUFACTURER: HOLOPHANE
 - MODEL: #12/ESPTDM1245 (RAL-8012) (LUMINAIRE)
 - MODEL: #PM25.01R16.721162SB-CAS/HD5/CS-BC1.0X12
 - MODEL: #KWP/PROV-P069/2-CA/CS-PDCAP-CA/CA (POLE)
 - LAMP: (2) 70 WATT METAL HALIDE



MARK	ISSUE DATE	DESCRIPTION
	09/16/07	BULLETIN 13
	12/02/05	ISSUED FOR U/G ELECTRICAL
	09/28/05	ISSUED FOR R.O.W. LAYOUT



TRO
ARCHITECTURE
PLANNING
ENGINEERING
INTERIOR DESIGN

The Ritchie Organization
60 Bridge Street
Newton, MA 02458-1134
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STATE OF MAINE
LAWRENCE E. BARTLETT
#7928
REGISTERED PROFESSIONAL ENGINEER

David 9/20/08

Lawrence E. Bartlett

Maine Medical Center
Pkg C - Site Utilities
Portland, Maine MMC Project No. 21846

Drawing Title
ELECTRICAL PLAN

0 15 30 60 90 120

Commission No.	4677	Date Issued	11/22/05
Scale	1"=30'	Sheet Number	PKG-C
Drawn By	J.C.		
Approved By	LEB		
Filename			E100

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SITE DISTRIBUTION CONDUIT SCHEDULE						
CONDUIT NO.	ORIGIN	DESTINATION	VIA	SIZE	CONDUIT USE	REMARKS
UTILITY CONDUITS						
U1	UMH-U0	2' OUTSIDE PRIMARY ELECTRICAL VAULT	VIA UMH-U1, UMH-U2 AND UMH-U5	5"	UTILITY FEEDER	FOREST AVE.
U2	UMH-U0	2' OUTSIDE PRIMARY ELECTRICAL VAULT	VIA UMH-U1, UMH-U2 AND UMH-U5	5"	SPARE	FOREST AVE.
U3	CMP POLE NO. 5	2' OUTSIDE PRIMARY ELECTRICAL VAULT	VIA UMH-U4 AND UMH-U5	5"	UTILITY FEEDER	SEWALL ST.
U4	CMP POLE NO. 5	2' OUTSIDE PRIMARY ELECTRICAL VAULT	VIA UMH-U4 AND UMH-U5	5"	SPARE	SEWALL ST.
U5	CMP POLE	2' OUTSIDE PRIMARY ELECTRICAL VAULT	VIA UMH-U4 AND UMH-U5	5"	UTILITY FEEDER	FORE RIVER
U6	CMP POLE	2' OUTSIDE PRIMARY ELECTRICAL VAULT	VIA UMH-U4 AND UMH-U5	5"	SPARE	FORE RIVER
U7						
U8						
U9						
U10						
U11						
U12						
U13						
U14						
U15						
U16						
U17						
U18						
U19						
U20						
U21						
U22						
U23						
U24						
U25						
U26						
U27						
U28						
U30						
NORMAL POWER CONDUITS						
N1	2' OUTSIDE PRIMARY ELECTRICAL VAULT	NMH-N4	VIA NMH-N14, NMH-N12, AND NMH-N13	5"	NORMAL FEEDER	NDF
N2	2' OUTSIDE PRIMARY ELECTRICAL VAULT	NMH-N4	VIA NMH-N14, NMH-N12, AND NMH-N13	5"	SPARE	
N3	2' OUTSIDE PRIMARY ELECTRICAL VAULT	NMH-N4	VIA NMH-N14, NMH-N12, AND NMH-N13	5"	NORMAL FEEDER	NDF
N4	2' OUTSIDE PRIMARY ELECTRICAL VAULT	NMH-N4	VIA NMH-N14, NMH-N12, AND NMH-N13	5"	SPARE	
N5	2' OUTSIDE PRIMARY ELECTRICAL VAULT	56 BUILDING TRANSFORMER	VIA NMH-N14, NMH-N12, NMH-N13 AND NMH-N4	5"	NORMAL FEEDER	
N6	2' OUTSIDE PRIMARY ELECTRICAL VAULT	56 BUILDING TRANSFORMER	VIA NMH-N14, NMH-N12, NMH-N13 AND NMH-N4	5"	NORMAL FEEDER	
N7	2' OUTSIDE PRIMARY ELECTRICAL VAULT	RICHARDS BUILDING SWITCH 1	VIA NMH-N14, NMH-N12, NMH-N13 AND NMH-N4	5"	NORMAL FEEDER	
N8	2' OUTSIDE PRIMARY ELECTRICAL VAULT	RICHARDS BUILDING SWITCH 2	VIA NMH-N14, NMH-N12, NMH-N13 AND NMH-N4	5"	NORMAL FEEDER	
N9	NMH-N4	RICHARDS BUILDING SWITCH 1		5"	SPARE	
N10	NMH-N4	RICHARDS BUILDING SWITCH 2		5"	SPARE	
N11	2' OUTSIDE PRIMARY ELECTRICAL VAULT	4' OUTSIDE CHARLES ST. BUILDING	VIA NMH-N14	5"	NORMAL FEEDER	CHARLES ST. SWITCH 1
N12	2' OUTSIDE PRIMARY ELECTRICAL VAULT	4' OUTSIDE CHARLES ST. BUILDING	VIA NMH-N14	5"	SPARE	CHARLES ST. SWITCH 1
N13	2' OUTSIDE PRIMARY ELECTRICAL VAULT	4' OUTSIDE CHARLES ST. BUILDING	VIA NMH-N14	5"	NORMAL FEEDER	CHARLES ST. SWITCH 2
N14	2' OUTSIDE PRIMARY ELECTRICAL VAULT	4' OUTSIDE CHARLES ST. BUILDING	VIA NMH-N14	5"	SPARE	CHARLES ST. SWITCH 2
N15	2' OUTSIDE PRIMARY ELECTRICAL VAULT	HANDHOLE 'NH1'	VIA NMH-N14	5"	NORMAL FEEDER	BEAN BUILDING S&C SWITCH 1
N16	2' OUTSIDE PRIMARY ELECTRICAL VAULT	HANDHOLE 'NH1'	VIA NMH-N14	5"	SPARE	
N17	2' OUTSIDE PRIMARY ELECTRICAL VAULT	HANDHOLE 'NH1'	VIA NMH-N14	5"	NORMAL FEEDER	BEAN BUILDING S&C SWITCH 2
N18	2' OUTSIDE PRIMARY ELECTRICAL VAULT	HANDHOLE 'NH1'	VIA NMH-N14	5"	SPARE	
N19	2' OUTSIDE PRIMARY ELECTRICAL VAULT	5' FROM FIRST CUP XFMR PAD	VIA NMH-N14	5"	NORMAL FEEDER	PPNSWR1 XFMR 2A
N20	2' OUTSIDE PRIMARY ELECTRICAL VAULT	5' FROM FIRST CUP XFMR PAD	VIA NMH-N14	5"	SPARE	PPNSWR1 XFMR 2A
N21	2' OUTSIDE PRIMARY ELECTRICAL VAULT	5' FROM FIRST CUP XFMR PAD	VIA NMH-N14	5"	NORMAL FEEDER	PPNSWR2 XFMR 2B
N22	2' OUTSIDE PRIMARY ELECTRICAL VAULT	5' FROM FIRST CUP XFMR PAD	VIA NMH-N14	5"	SPARE	PPNSWR2 XFMR 2B
N23	2' OUTSIDE PRIMARY ELECTRICAL VAULT	5' FROM FIRST CUP XFMR PAD	VIA NMH-N14	5"	NORMAL FEEDER	PPNSWR3 CHILLER XFMR 1A
N24	2' OUTSIDE PRIMARY ELECTRICAL VAULT	5' FROM FIRST CUP XFMR PAD	VIA NMH-N14	5"	SPARE	PPNSWR3 CHILLER XFMR 1A
N25	2' OUTSIDE PRIMARY ELECTRICAL VAULT	CAPPED NEAR CUP	VIA NMH-N14	5"	SPARE	
N26	2' OUTSIDE PRIMARY ELECTRICAL VAULT	CAPPED NEAR CUP	VIA NMH-N14	5"	SPARE	
N27	2' OUTSIDE PRIMARY ELECTRICAL VAULT	CAPPED NEAR CUP	VIA NMH-N14	5"	SPARE	
N28	2' OUTSIDE PRIMARY ELECTRICAL VAULT	CAPPED NEAR CUP	VIA NMH-N14	5"	SPARE	
N29	2' OUTSIDE PRIMARY ELECTRICAL VAULT	NMH-N13	VIA NMH-N14 AND NMH-N12	5"	SPARE	FUTURE EXPANSION
N30	2' OUTSIDE PRIMARY ELECTRICAL VAULT	NMH-N13	VIA NMH-N14 AND NMH-N12	5"	SPARE	FUTURE EXPANSION
N31	2' OUTSIDE PRIMARY ELECTRICAL VAULT	NMH-N14		5"	SPARE	
N32	2' OUTSIDE PRIMARY ELECTRICAL VAULT	NMH-N14		5"	SPARE	
N33	2' OUTSIDE PRIMARY ELECTRICAL VAULT	NMH-N14		5"	SPARE	
N34	2' OUTSIDE PRIMARY ELECTRICAL VAULT	NMH-N14		5"	SPARE	
N35	2' OUTSIDE PRIMARY ELECTRICAL VAULT	NMH-N14		5"	SPARE	
N36	2' OUTSIDE PRIMARY ELECTRICAL VAULT	NMH-N14		5"	SPARE	
N37	2' OUTSIDE PRIMARY ELECTRICAL VAULT	NMH-N14		5"	SPARE	
N38	2' OUTSIDE PRIMARY ELECTRICAL VAULT	NMH-N14		5"	SPARE	
N39	2' OUTSIDE PRIMARY ELECTRICAL VAULT	NMH-N14		5"	SPARE	
N40	2' OUTSIDE PRIMARY ELECTRICAL VAULT	NMH-N14		5"	SPARE	
N41	2' OUTSIDE PRIMARY ELECTRICAL VAULT	NMH-N14		5"	SPARE	
N42	2' OUTSIDE PRIMARY ELECTRICAL VAULT	NMH-N14		5"	SPARE	
N43	2' OUTSIDE PRIMARY ELECTRICAL VAULT	NMH-N14		5"	SPARE	
N44	2' OUTSIDE PRIMARY ELECTRICAL VAULT	NMH-N14		5"	SPARE	
N45	2' OUTSIDE PRIMARY ELECTRICAL VAULT	NMH-N14		5"	SPARE	
N46	2' OUTSIDE PRIMARY ELECTRICAL VAULT	NMH-N14		5"	SPARE	
N47						
N48						
N49						
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N57						
N58						
N59						
N60						

SITE DISTRIBUTION CONDUIT SCHEDULE						
CONDUIT NO.	ORIGIN	DESTINATION	VIA	SIZE	CONDUIT USE	REMARKS
EMERGENCY POWER CONDUITS						
E1	25' FROM CUP BUILDING	HANDHOLE 'EH1'	VIA EMH-E2	5"	EMERGENCY FEEDER	FROM CUP SKV SWGR TO BEAN BLDG SKV SWITCH 1A
E2	25' FROM CUP BUILDING	HANDHOLE 'EH1'	VIA EMH-E2	5"	SPARE	FROM CUP SKV SWGR TO BEAN BLDG SKV SWITCH 1A
E3	25' FROM CUP BUILDING	HANDHOLE 'EH1'	VIA EMH-E2	5"	EMERGENCY FEEDER	FROM CUP SKV SWGR TO CHARLES ST. SKV SWITCH 1B
E4	25' FROM CUP BUILDING	HANDHOLE 'EH1'	VIA EMH-E2	5"	SPARE	FROM CUP SKV SWGR TO CHARLES ST. SKV SWITCH 1B
E5	HANDHOLE 'EH1'	4' OUTSIDE CHARLES ST. BUILDING	VIA EMH-E2 AND EMH-E1	5"	EMERGENCY FEEDER	FROM BEAN BLDG SKV SWITCH 1B TO CHARLES ST. SKV SWITCH 1A
E6	HANDHOLE 'EH1'	4' OUTSIDE CHARLES ST. BUILDING	VIA EMH-E2 AND EMH-E1	5"	SPARE	FROM BEAN BLDG SKV SWITCH 1B TO CHARLES ST. SKV SWITCH 1A
E7	25' FROM CUP BUILDING	EMH-E1	VIA EMH-E2	5"	FUTURE	FROM CUP SKV SWITCHGEAR
E8	25' FROM CUP BUILDING	EMH-E1	VIA EMH-E2	5"	FUTURE	FROM CUP SKV SWITCHGEAR
E9						
E10						
E11						
E12						
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E55						
E56						
E57						
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E59						
E60						
COMMUNICATIONS CONDUITS						
C1						
C2						
C3	5' FROM CUP BUILDING	RICHARDS BUILDING TEL/COM ROOM	VIA CMH-C2 AND CMH-C1	5"	COMMUNICATIONS WIRING	TO NEW GARAGE
C4	1' OUTSIDE NEW GARAGE	RICHARDS BUILDING TEL/COM ROOM	VIA CMH-C2 AND CMH-C1	5"	COMMUNICATIONS WIRING	TO NEW GARAGE
C5	25' OUTSIDE CUP BUILDING	HANDHOLE 'CH1'	VIA CH3 AND CH1	5"	COMMUNICATIONS WIRING	FROM CUP TO BEAN BUILDING
C6	25' OUTSIDE CUP BUILDING	HANDHOLE 'CH1'	VIA CH3 AND CH1	5"	COMMUNICATIONS WIRING	FROM CUP TO BEAN BUILDING
C7	25' OUTSIDE CUP BUILDING	RICHARDS BUILDING TEL/COM ROOM	VIA CH3, CMH-C2 AND CMH-C1	5"	COMMUNICATIONS WIRING	FROM CUP
C8	25' OUTSIDE CUP BUILDING	RICHARDS BUILDING TEL/COM ROOM	VIA CH3, CMH-C2 AND CMH-C1	5"	COMMUNICATIONS WIRING	FROM CUP
C9	HANDHOLE 'NH2'	BEAN BUILDING		5"	COMMUNICATIONS WIRING	FROM EXISTING GARAGE
C10	HANDHOLE 'NH2'	BEAN BUILDING		5"	COMMUNICATIONS WIRING	FROM EXISTING GARAGE
C11						
C12						
C13	BMC POLE	TEL/COM ROOM	VIA CMH-C1	5"	COMMUNICATIONS WIRING	BMC FIBER RISER
C14	5' OUTSIDE CUP BUILDING	4' FROM EXISTING FUEL TANK #1		5"	COMMUNICATIONS WIRING	
C15	CONGRESS ST. UTILITY POLE			4"	COMMUNICATIONS WIRING	MMC WIRING
C16	5' OUTSIDE CUP BUILDING	3' OUTSIDE PUMPHOUSE		5"	COMMUNICATIONS WIRING	
C17						
C18						
C19						
C20						
C21						
C22						
C23						
C24						
C25						
C26						
C27						
C28						
C29						
C30						

SITE DISTRIBUTION CONDUIT SCHEDULE						
CONDUIT NO.	ORIGIN	DESTINATION	VIA	SIZE	CONDUIT USE	REMARKS
LOW VOLTAGE NORMAL POWER CONDUITS						
LN1	HANDHOLE 'LNH1'	1' OUTSIDE GARAGE		5"	NORMAL FEEDER	NEW GARAGE SWITCHBOARD NSB1
LN2	HANDHOLE 'LNH1'	1' OUTSIDE GARAGE		5"	SPARE	NEW GARAGE SWITCHBOARD NSB1
LN3	HANDHOLE 'NH2'	BEAN BUILDING		5"	NORMAL FEEDER	FROM EXISTING GARAGE
LN4						
LN5						
LN6						
LN7						
LN8						
LN9						
LN10						
LOW VOLTAGE EMERGENCY POWER CONDUITS						
LE1	HANDHOLE 'LEH1'	1' OUTSIDE GARAGE		5"	EMERGENCY FEEDER	NEW GARAGE SWITCHBOARD NSB1
LE2	HANDHOLE 'LEH1'	1' OUTSIDE GARAGE		5"	SPARE	NEW GARAGE SWITCHBOARD NSB1
LE3	5' OUTSIDE CUP BUILDING	3' OUTSIDE PUMPHOUSE		5"	EMERGENCY FEEDER	FROM CUP TO PUMPHOUSE
LE4	5' OUTSIDE CUP BUILDING	3' OUTSIDE PUMPHOUSE		5"	SPARE	FROM CUP TO PUMPHOUSE
LE5	HANDHOLE 'NH2'	BEAN BUILDING		5"	EMERGENCY FEEDER	FROM EXISTING GARAGE
LE6						
LE7						
LE8						
LE9						
LE10						

RECEIVED

GENERAL NOTES

- SPECIFICATIONS**
SHOP DRAWINGS
SIMILAR CONDITIONS
ELEVATIONS & DIMENSIONS
BUILDING CODE
- A - DESIGN LOADS**
- A1) LIVE
A2) DEAD
A3) SEISMIC
- B - FOUNDATIONS**
- B1) GEOTECHNICAL REPORTS
B2) SOIL BEARING
B3) EXCAVATION
B4) UTILITIES AND OTHER UNDERGROUND STRUCT.
B5) Dewatering SYSTEM
B6) FOOTING SUBGRADE PREPARATION AND FILL
B7) MANHOLE SLAB SUBGRADE PREPARATION AND FILL
B8) BACKFILL UNDER SLAB-ON-GRADE
B9) BACKFILL AGAINST CANTILEVERED RETAINING WALLS
B10) BACKFILL AGAINST FOUNDATION WALLS
B11) FOUNDATION PLACEMENT & PROTECTION
B12) DE-ICING
- D - CAST-IN-PLACE CONCRETE**
- D1) CONCRETE STRENGTH
D2) PORTLAND CEMENT
D3) AGGREGATE
D4) WATER
D5) SLUMP
D6) ADMIXTURES
D7) STEEL REINFORCEMENT
D8) OPENINGS
D9) REINFORCEMENT AT OPENINGS
D10) MINIMUM CONCRETE COVER
D11) WALLS AND GRADE BEAMS
D12) SPLICING OF REINFORCEMENT
D13) MINIMUM REINFORCEMENT
D14) SHOP DRAWINGS
D15) STANDARD SPECIFICATIONS
D16) CONSTRUCTION JOINTS
D17) FOUNDATION DOWELS
D18) SURFACE TREATMENT
D19) STRUCTURAL TESTING & INSPECTION
D20) EMBEDDED ITEMS

REFER TO PROJECT SPECIFICATIONS FOR DETAILED REQUIREMENTS FOR MATERIAL AND WORKMANSHIP.

THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS AND COORDINATION DRAWINGS FOR THE ENGINEER'S APPROVAL AS STATED IN THE SPECIFICATIONS.

IN THE EVENT THAT CERTAIN DETAILS OF THE CONSTRUCTION ARE NOT FULLY SHOWN OR NOTED ON THE DRAWINGS, THEIR CONSTRUCTION SHALL BE OF THE SAME TYPE AS FOR SIMILAR CONDITIONS WHICH ARE SHOWN AND NOTED, SUBJECT TO THE STRUCTURAL ENGINEER'S APPROVAL.

SEE MECHANICAL, ELECTRICAL, PLUMBING, FIRE PROTECTION, & TELECOMMUNICATION DRAWINGS FOR THE FOLLOWING:

A. PIPE AND DUCT RUNS, SLEEVES, HANGERS, TRENCHES, WALL AND SLAB OPENINGS, ETC., EXCEPT AS SHOWN OR NOTED.

B. ELECTRICAL CONDUIT RUNS, BOXES, OUTLETS IN WALLS AND SLABS.

C. CONCRETE INSERTS FOR ELECTRICAL, MECHANICAL OR PLUMBING FIXTURES.

D. SIZE AND LOCATION OF MACHINE OR EQUIPMENT BASES, ANCHOR BOLTS FOR MOTOR MOUNTS, EXCEPT AS SHOWN OR NOTED.

ALL ELEVATIONS AND DIMENSIONS SHOWN FOR NEW CONSTRUCTION ARE BASED ON THE DESIGN DRAWINGS FOR THE EXISTING BUILDINGS. FIELD VERIFY ALL ELEVATIONS AND DIMENSIONS BEFORE PROCEEDING WITH CONSTRUCTION.

BOCA - 1999

AASHTO HS-20
ALL PERMANENT STATIONARY CONSTRUCTION.

USE GROUP 1-2 WITH EMERGENCY TREATMENT FACILITIES
SEISMIC HAZARD EXPOSURE GROUP III
SEISMIC PERFORMANCE CATEGORY C
 $\lambda_v = 4 = 0.11$
SITE COEFFICIENT, $s = 1.2$

THE CONTRACTOR SHALL BE RESPONSIBLE FOR READING, UNDERSTANDING & IMPLEMENTING THE RECOMMENDATIONS OUTLINED IN THE FOLLOWING GEOTECHNICAL REPORTS BY S.W. COLE, INC.:

I) "GEOTECHNICAL ENGINEERING SERVICES PROPOSED PARKING GARAGE, MAINE MEDICAL CENTER FACILITY, CRESCENT AND CONGRESS STREETS, PORTLAND, MAINE", DATED 31 MARCH 2004.

II) "GEOTECHNICAL ENGINEERING SERVICES PRELIMINARY INVESTIGATION PROPOSED PARKING GARAGE, MAINE MEDICAL CENTER FACILITY CRESCENT AND CONGRESS STREETS, PORTLAND, MAINE", DATED 2 APRIL 2002.

III) "GEOTECHNICAL ENGINEERING INVESTIGATION PROPOSED MEDICAL OFFICE BUILDING AND PARKING GARAGE WOMEN AND INFANTS FACILITY CHARLES STREET, PORTLAND, MAINE", DATED 29 MARCH 2002.

COPIES OF THE GEOTECHNICAL REPORT ARE AVAILABLE FROM THE PROJECT ARCHITECT. WHERE RECOMMENDATIONS IN THESE REPORTS VARY FROM INFORMATION CONTAINED IN THESE DRAWINGS & THE PROJECT SPECIFICATIONS, THE MORE STRINGENT RECOMMENDATIONS SHALL GOVERN.

CONCRETE SPREAD & STRIP FOOTINGS ARE DESIGNED FOR AN ALLOWABLE BEARING PRESSURE OF 4,000 PSF AT A DEPTH OF 4.5 FEET BELOW THE EXISTING GRADE. BEAR ALL FOOTINGS ON UNDISTURBED SOIL, U.O.N.

ALL FOUNDATION EXCAVATION TO BE INSPECTED BY THE GEOTECHNICAL ENGINEER. EXCAVATE TO LINES AND GRADES TO PROPERLY INSTALL FOUNDATIONS ON UNDISTURBED SOIL APPROVED BY THE GEOTECHNICAL ENGINEER FOR THE REQUIRED BEARING CAPACITY. THE ELEVATIONS SHOWN ON THE DRAWINGS ARE ANTICIPATED AND ACTUAL ELEVATIONS ARE TO BE ESTABLISHED IN THE FIELD BY THE GEOTECHNICAL ENGINEER, BUT IN NO CASE SHALL THE BOTTOM OF FOOTING BE LOCATED LESS THAN 4.5 FEET BELOW THE LOWEST ADJACENT SURFACE EXPOSED TO FREEZING. THE DIFFERENCE IN ELEVATION BETWEEN THE BOTTOMS OF ADJACENT FOOTINGS SHALL BE EQUAL TO OR LESS THAN THE HORIZONTAL DISTANCE BETWEEN THEM. ANY ADJUSTMENT OF FOOTING ELEVATIONS DUE TO FIELD CONDITIONS MUST HAVE THE PRIOR APPROVAL OF THE ENGINEER.

FOOTINGS TO BEAR BELOW AN IMAGINARY REFERENCE LINE DRAWN UPWARD AND OUTWARD ON A 1V:1H SLOPE FROM THE BOTTOM OF ANY ADJACENT UTILITIES OR OTHER UNDERGROUND STRUCTURES.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR TEMPORARY Dewatering OF ALL EXCAVATIONS AS NECESSARY TO ALLOW FOR CONSTRUCTION OF BELOW GRADE MANHOLES, WALLS, UTILITIES, ETC. Dewatering SHALL BE MAINTAINED UNTIL NEW CONSTRUCTION HAS BEEN WATER PROOFED (IF REQUIRED) IN ACCORDANCE WITH THE ARCH. DRAWINGS AND PROJECT SPECIFICATIONS.

FOLLOW RECOMMENDATIONS OF GEOTECHNICAL REPORT INCLUDED IN PROJECT MANUAL. PLACE ALL SPREAD AND STRIP FOOTINGS ON 2" LEAN CONCRETE "MUD" MAT OR A LAYER OF COMPACTED GRANULAR FILL. ALL FOOTINGS SHALL EXTEND AT LEAST 4'-6" BELOW GRADE FOR FROST PROTECTION.

FOLLOW RECOMMENDATIONS OF GEOTECHNICAL REPORT INCLUDED IN PROJECT MANUAL. PLACE MANHOLE FLOOR SLABS ON 12" BED OF CRUSHED STONE OVER FILTER FABRIC.

PROOF-ROLL EXISTING SOILS PER SPECIFICATION #02200 "EARTHWORK". BACKFILL WHERE REQUIRED BELOW SLAB-ON-GRADE WITH APPROVED GRANULAR SOIL PLACED IN 6 IN. LAYERS AND COMPACTED TO 95% DENSITY AT OPTIMUM MOISTURE CONTENT AS DETERMINED BY ASTM D-1557, METHOD D. USE A POLYETHYLENE VAPOR BARRIER BETWEEN THE COMPACTED GRANULAR FILL AND THE SLAB-ON-GRADE.

DO NOT BACKFILL AGAINST RETAINING WALLS UNTIL WALL CONCRETE IS AT FULL DESIGN STRENGTH. BACKFILL WITH APPROVED MATERIAL PLACED IN 6 IN. LAYERS AND COMPACTED TO 95% DENSITY AT OPTIMUM MOISTURE CONTENT AS DETERMINED BY ASTM D-1557, METHOD D.

DO NOT BACKFILL AGAINST FOUNDATION WALLS UNTIL WALL CONCRETE IS AT FULL DESIGN STRENGTH AND UNTIL SLABS AT BASE AND TOP OF WALL ARE IN PLACE, AND HAVE REACHED THEIR DESIGN STRENGTH. U.O.N. IN GEOTECHNICAL REPORT. BACKFILL WITH APPROVED MATERIAL PLACED IN 6 IN. LAYERS AND COMPACTED TO 95% DENSITY AT OPTIMUM MOISTURE CONTENT AS DETERMINED BY ASTM D-1557, METHOD D.

PROTECT ALL SOIL BEARING SURFACES FROM FREEZING BEFORE AND AFTER FOUNDATION CONSTRUCTION. IF CONSTRUCTION IS PERFORMED DURING FREEZING WEATHER, BACKFILL FOOTINGS TO A SUFFICIENT DEPTH (UP TO FOUR AND ONE HALF FEET) AS SOON AS POSSIBLE AFTER CONSTRUCTION. ALTERNATIVELY, USE APPROVED INSULATING BEANS OR OTHER PROTECTED BEANS FOR PROTECTION AGAINST FREEZING. DO NOT PLACE FOUNDATION CONCRETE IN WATER OR ON FROZEN GROUND. PROTECT IN-PLACE FOUNDATIONS AND SLABS FROM FROST PENETRATION UNTIL THE PROJECT IS COMPLETE.

DO NOT USE SALT OR CHLORIDE-COMPOUNDS TO DE-ICE SITE.

PROVIDE THE FOLLOWING 28 DAY COMPRESSIVE STRENGTH FOR FIELD CONCRETE:

I) 4,000 PSI (MIN.) NORMAL WEIGHT FOR ALL CAST-IN-PLACE CONCRETE FOUNDATIONS, COLUMNS, BASEMENT WALLS, STRUCTURAL SLABS AND SLABS-ON-GRADE.

II) 1,000 PSI (MIN.) NORMAL WEIGHT FOR ALL LEAN CONCRETE USED IN MUD MATS.

ASTM C150. TYPE II. WATER CEMENT RATIO AS REQUIRED FOR DESIGN STRENGTH.

NORMAL WEIGHT: ASTM C33, WITH MAXIMUM SIZE OF 1 IN.

POTABLE.

4 INCH MAXIMUM PRIOR TO ADDITION OF ADMIXTURES.

SEE SPECIFICATIONS. ASTM C260 AIR-ENTRAINING AGENT AS REQUIRED FOR A TOTAL ENTRAINED AIR CONTENT OF 6% (±1%) FOR ALL CONCRETE EXPOSED TO FREEZING.

ASTM C494 WATER REDUCING AGENT IN ALL CONCRETE.

DO NOT USE CALCIUM CHLORIDE.

ASTM A615 GRADE 60.

ASTM A185 FOR WELDED WIRE FABRIC (WWF). USE FLAT SHEETS ONLY (NO ROLLS).

EPOXY COATED REINFORCING AND ACCESSORIES SHALL BE USED FOR ALL CONCRETE PLACED BELOW OR ADJACENT TO ROADWAYS. PROVIDE #6 CHAIR BARS, HIGH CHAIRS, TIES, CLIPS, SLAB BOLTERS AND ALL ACCESSORIES WHERE NOT SPECIFIED ON THE DRAWINGS IN ACCORDANCE WITH MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES. ACI 315 OR CRSI-WRMS MANUAL OF STANDARD PRACTICE. USE PLASTIC TIPS ON ALL CHAIRS PLACED ON THE SIDES OF CONCRETE FORMWORK. FIELD WELDING OF CROSSING BARS ("TACK" OR "SPOT" WELDING) IS NOT PERMITTED. DO NOT CUT BARS OR OMIT BARS BECAUSE OF SLEEVES OR OPENINGS IN FLOORS, EXCEPT AS SPECIFICALLY DETAILED IN DRAWINGS.

OPENINGS, POCKETS, ETC. LARGER THAN 6" SHALL NOT BE PLACED IN CONCRETE SLABS, DECKS, OR WALLS, UNLESS SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS.

PROVIDE 2 - #6 AT EACH SIDE OF ALL OPENINGS IN WALLS AND SLABS AND EXTEND 2 FT-6 IN. BEYOND THE OPENING OR AS DETAILED, EXCEPT VERTICAL BARS AT SIDES OF OPENINGS IN WALLS ARE TO EXTEND FROM FLOOR TO FLOOR.

BARS MAY BE MOVED ASIDE AT OPENINGS OR SLEEVES, BUT DO NOT CUT OR OMIT.

CONCRETE PLACED AGAINST EARTH:

SLABS-ON-GRADE BOTTOM: 3 IN.

SLABS-ON-GRADE TOP: 1-1/2 IN.

SLABS-ON-GRADE TOP EXPOSED TO WATER OR WEATHER: 2 IN.

FORMED SLABS, TOP AND BOTTOM: 2 IN.

INTERIOR FACES OF WALLS: 1 IN.

COLUMNS OR PIERS (MAIN REINFORCEMENT): 1-1/2 IN.

PLACE WALLS AND GRADE BEAMS IN LEVEL. FULL HEIGHT LIFTS WITH CONSTRUCTION JOINTS WHERE INDICATED ON ARCHITECTURAL AND STRUCTURAL DRAWINGS. PROVIDE OPENINGS FOR WATER, ELECTRIC AND OTHER SERVICES AS REQUIRED. PROVIDE KEYS AND DOWELS AT ALL CONSTRUCTION JOINTS.

AS SHOWN IN TABLE ON THIS SHEET, BUT NOT LESS THAN 40 BAR DIAMETERS FOR SLABS AND BEAM BOTTOM BARS, AND NOT LESS THAN 48 BAR DIAMETERS FOR WALLS AND BEAM TOP STEEL. PROVIDE A LAP OF 8 IN. OR 1-1/2 SPACES, WHICHEVER IS LARGER, FOR WWF. TIE WIRES TOGETHER AT LAP.

REINFORCE ALL WALLS LESS THAN 8" THICK WITH AT LEAST #4 @ 12 IN. EACH WAY EACH FACE AND 2 - #6 EACH EDGE, U.O.N.

REINFORCE ALL WALLS THICKER THAN 12" WITH AT LEAST #6@12" O.C., E.W., E.F. W/ 2-#8 EACH EDGE, U.O.N. IN SLABS, PROVIDE AT LEAST 0.018 TIMES THE AREA OF CONCRETE IN EACH DIRECTION.

REINFORCE ALL SLAB-ON-GRADE WITH MINIMUM #5x5-6x6.

SUBMIT FOR ENGINEER'S APPROVAL COMPLETE BENDING AND PLACING DETAILS OF ALL REINFORCING STEEL, INCLUDING WELDED WIRE FABRIC, INDICATING POSITION OF SPLICES. INCLUDE ACCESSORY DRAWINGS, INCLUDING PRECISE LOCATIONS OF ALL SLEEVES CAST INTO CONCRETE TO ACCOMMODATE PLUMBING AND ELECTRICAL WORK.

COMPLY WITH THE LATEST RECOMMENDATIONS AND SPECIFICATIONS OF THE AMERICAN CONCRETE INSTITUTE:

ACI 301 STRUCTURAL CONCRETE FOR BUILDINGS

ACI 302 CONCRETE FLOOR AND SLAB CONSTRUCTION

ACI 304 MEASURING, MIXING, TRANSPORTING AND PLACING CONCRETE

ACI 305 HOT WEATHER CONCRETING

ACI 306 COLD WEATHER CONCRETING

ACI 315 DETAILING REINFORCING STEEL

ACI 318 GENERAL DESIGN OF ITEMS NOT OTHERWISE SPECIFIED

ACI 347 FORMWORK

CRSI MANUAL OF STANDARD PRACTICE

PROVIDE KEYS AND DOWELS AT ALL CONSTRUCTION JOINTS. PROVIDE DOWELS WITH AN AREA EQUAL TO THE WALL OR SLAB REINFORCING, BUT NOT LESS THAN 0.003 TIMES THE CONCRETE CROSS SECTIONAL AREA AT THE CONSTRUCTION JOINT. SUBMIT THE PROPOSED LOCATION OF CONSTRUCTION JOINTS TO THE DESIGNER FOR APPROVAL. MAXIMUM SPACING OF CONSTRUCTION JOINTS TO BE 60 FT. FOR WALLS AND STRUCTURAL FLOORS AND 80 FT. FOR SLABS ON GRADE.

PROVIDE HOOKED DOWELS TO MATCH SIZE AND SPACING OF LONGITUDINAL BARS IN WALLS U.O.N.

ROUGHEN ALL EXISTING CONCRETE SURFACES COMMON WITH NEW CONCRETE TO AN AMPLITUDE OF 1/4" (MIN.).

A MINIMUM OF THREE (3) CYLINDERS SHALL BE TAKEN NOT LESS THAN ONE DAY NOR LESS THAN ONCE FOR 100 CUBIC YARDS OF CONCRETE FOR COMPRESSIVE STRENGTH TESTING. TESTING IS TO BE PAID FOR BY THE OWNER. THE CONTRACTOR SHALL NOTIFY THE ENGINEER OF RECORD AT LEAST 24 HOURS PRIOR TO CASTING ANY CONCRETE SO THAT STRUCTURAL TESTING & INSPECTION CAN BE COORDINATED.

ABSOLUTELY NO CONCRETE IS TO BE CAST PRIOR TO REBAR BEING INSPECTED AND APPROVED.

ALL CONDUIT, PIPING, DUCTWORK, ETC. TO BE EMBEDDED WITHIN DIP SLABS, WALLS, BEAMS OR COLUMNS SHALL BE CLEARLY SHOWN ON THE SHOP DRAWINGS AND SHALL BE SUBJECT TO APPROVAL BY THE EOR. APPROVED EMBEDDED ITEMS SHALL BE LOCATED AND EVENLY DISTRIBUTED IN SUCH A MANNER TO PREVENT ADVERSELY AFFECTING THE STRENGTH OF CONCRETE MEMBERS.

D - CAST-IN-PLACE CONCRETE (CONTINUED)

- D21) FOOTING SUBGRADE
D22) BASE PLATE GROUING
D23) CURING COMPOUNDS
D24) SHEAR KEYS
- D25) HOUSEKEEPING PADS AND CURBS
- D26) EPOXY DOWELS AND ANCHORS
- D27) UNDERCUT ANCHORS
- F - STRUCTURAL STEEL**
- F1) STRUCTURAL SHAPES
F2) HOLLOW STRUCTURAL SECTIONS (HSS)
F3) PIPE
F4) ANCHOR BOLTS
F5) CORROSION PROTECTION
F6) WELDING ELECTRODES
F7) ERECTION
F8) PAINT
F9) FABRICATION
F10) STANDARD SPECIFICATIONS

NO CONCRETE FOOTING SHALL BE POURED UNTIL SUBGRADE FOR SAME HAS BEEN APPROVED BY A LICENSED PROFESSIONAL ENGINEER.

8000 PSI 28-DAY COMPRESSIVE STRENGTH.

DO NOT USE CURING COMPOUNDS WITHOUT WRITTEN APPROVAL FROM THE ENGINEER.

UNLESS OTHERWISE NOTED ON THE DRAWINGS, SHEAR KEYS BELOW WALL BOTTOMS SHALL BE 4" WIDE BY 3" DEEP. SHEAR KEYS AT WALL TOPS SHALL BE 3" WIDE BY 2" DEEP. DO NOT USE EPS MATERIAL FOR FORMING KEYS UNLESS IT IS FIRMLY ATTACHED TO RIGID BACKUP MATERIALS.

PADS AND CURBS MAY BE SHOWN ON PLAN IN CERTAIN INSTANCES FOR REFERENCE ONLY. SEE ARCHITECTURAL AND MECHANICAL DRAWINGS AND SPECIFICATIONS AND COORDINATE WITH EQUIPMENT MANUFACTURER'S REQUIREMENTS AND LOCATION.

USE SAME CONCRETE AS BASE SLAB U.O.N.

MAXIMUM PAD THICKNESS IS 6 INCHES, U.O.N.

USE HLT HSE 2421 EPOXY ADHESIVE ANCHOR SYSTEM, OR APPROVED EQUAL, FOR ALL EPOXY DOWELS AND ANCHORS.

USE HLT HSE-P UNDERCUT ANCHOR, OR APPROVED EQUAL FOR ALL UNDERCUT ANCHORS. ALL ANCHORS SHALL BE GALVANIZED, U.O.N, TYP.

WIDE FLANGE SHAPES AND CHANNELS: ASTM A992, OR ASTM A572 GRADE 50 ($F_y = 50,000$ PSI)
ANGLES, BARS AND PLATES: ASTM A36 U.O.N
ASTM A500 - GRADE B ($F_y = 46,000$ PSI).

ASTM A53 TYPE E GRADE B OR ASTM A501
ASTM A307 OR ASTM F1554 GRADE 36 BOLTS U.O.N ON THE DRAWINGS.

ALL STRUCTURAL STEEL ASSOCIATED WITH BELOW-GRADE MANHOLE SHALL BE HOT-DIP GALVANIZED, U.O.N, TYP. CONFORM TO AWS SPECIFICATIONS FOR ELECTRODES BASED ON WELDING PROCESS AND THE TYPE AND GRADE OF STEEL.

PROVIDE ANCHOR BOLTS, ERECTION BOLTS, STEEL WEDGES, THREADED SCREWS OR SHIMS AS REQUIRED TO SUPPORT AND PLUMB ALL STRUCTURAL STEEL MEMBERS. PROVIDE BEARING PLATES AND WALL ANCHORS OR ANCHOR BOLTS FOR ALL BEAMS RESTING ON CONCRETE AND ALL OTHER NECESSARY CONNECTING HARDWARE. SET ANCHOR BOLTS USING TEMPLATE. DO NOT FIELD CUT OR FIELD MODIFY ANY STRUCTURAL STEEL WITHOUT PRIOR WRITTEN APPROVAL BY ARCHITECT FOR EACH SPECIFIC CASE.

SHOP PRIME ALL STEEL, NOT ENCASED IN CONCRETE OR TO BE FIREPROOFED.

FOR ALL EXPOSED STEEL, USE A THREE COAT PAINT SYSTEM WITH A ZINC-RICH PRIMER, AN EPOXY INTERMEDIATE COAT, AND A PROTECTIVE TOP COAT, OR HOT-DIP GALVANIZE THE STEEL AFTER FABRICATION IS COMPLETE.

SHOP FABRICATE TO GREATEST EXTENT POSSIBLE BY WELDING INCLUDING BEAM STIFFENERS, COLUMN CAPS AND BASE PLATES, HOLES AND CONNECTIONS.

SUBMIT COMPLETE SHOP DRAWINGS, FROM FIELD DIMENSIONS, FOR DESIGNER'S APPROVAL.

DO NOT START FABRICATION OF STRUCTURAL STEEL MEMBERS UNTIL THE SHOP DRAWINGS HAVE BEEN REVIEWED AND APPROVED BY THE ENGINEER OF RECORD (EOR).

COMPLY WITH THE LATEST RECOMMENDATIONS AND SPECIFICATIONS OF:

AISC SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS, ALLOWABLE STRESS DESIGN AND PLASTIC DESIGN.

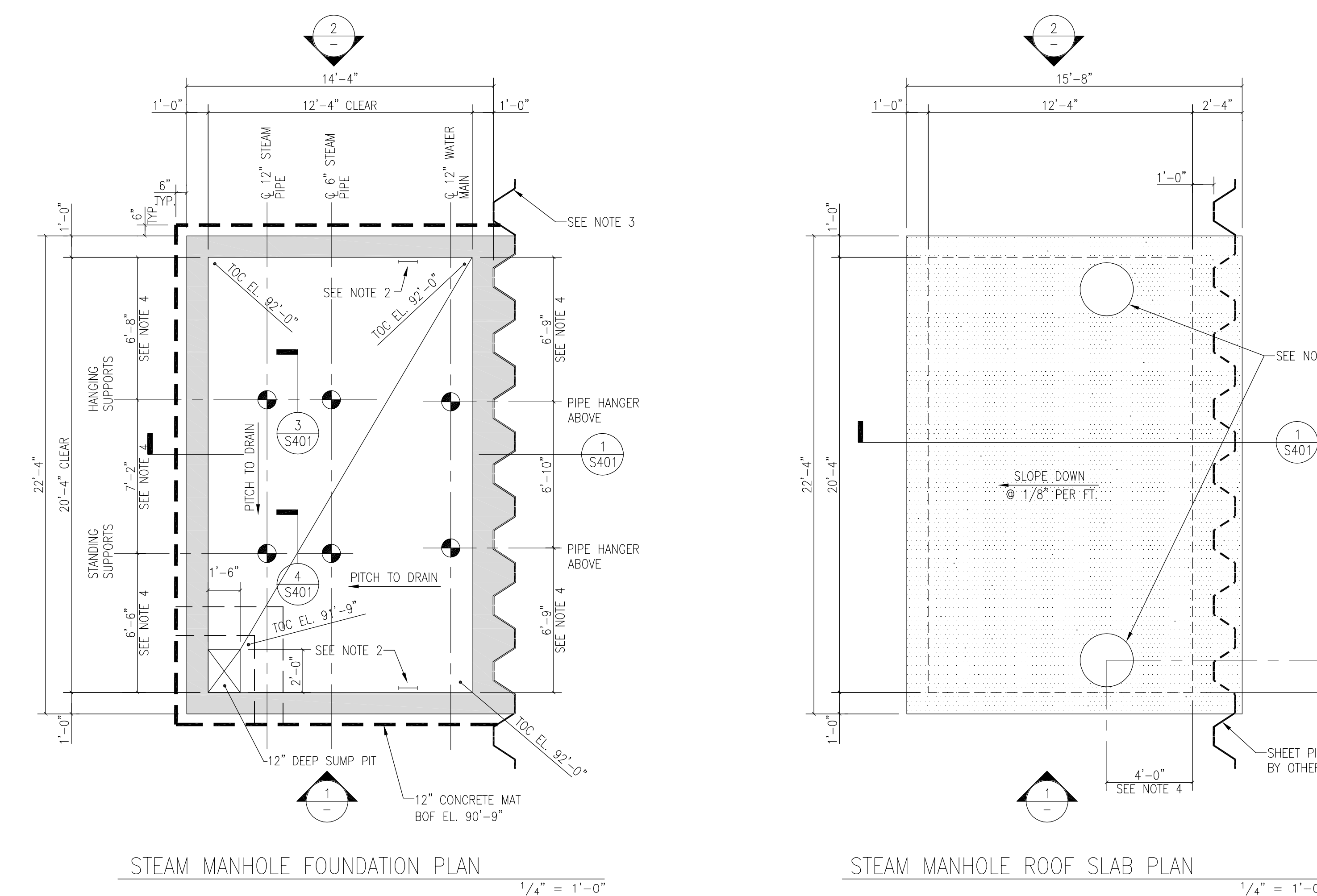
AISC LOAD AND RESISTANCE FACTOR DESIGN SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS.

LOAD AND RESISTANCE FACTOR DESIGN SPECIFICATION FOR STEEL HOLLOW STRUCTURAL SECTIONS.

SPROCKET FOR STRUCTURAL JOINTS USING ASTM A325 OR HARD BOLTS.

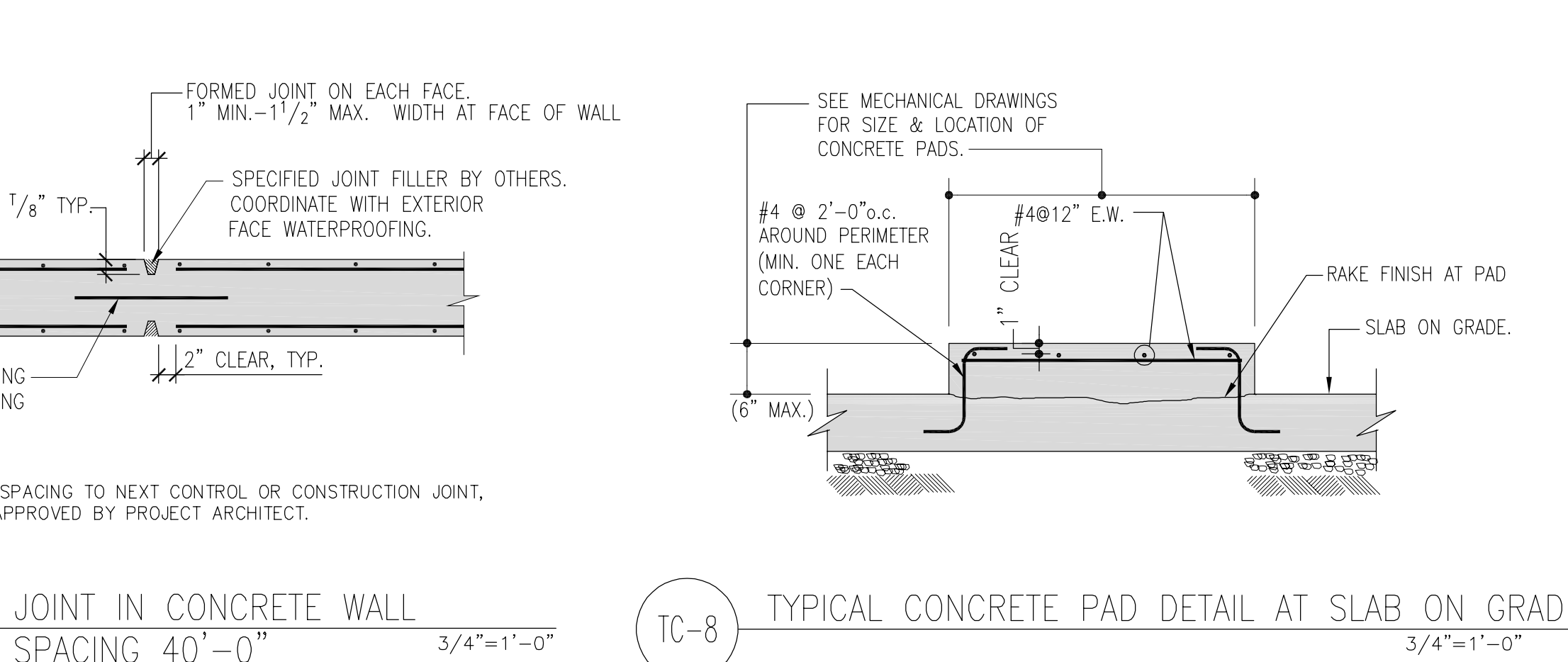
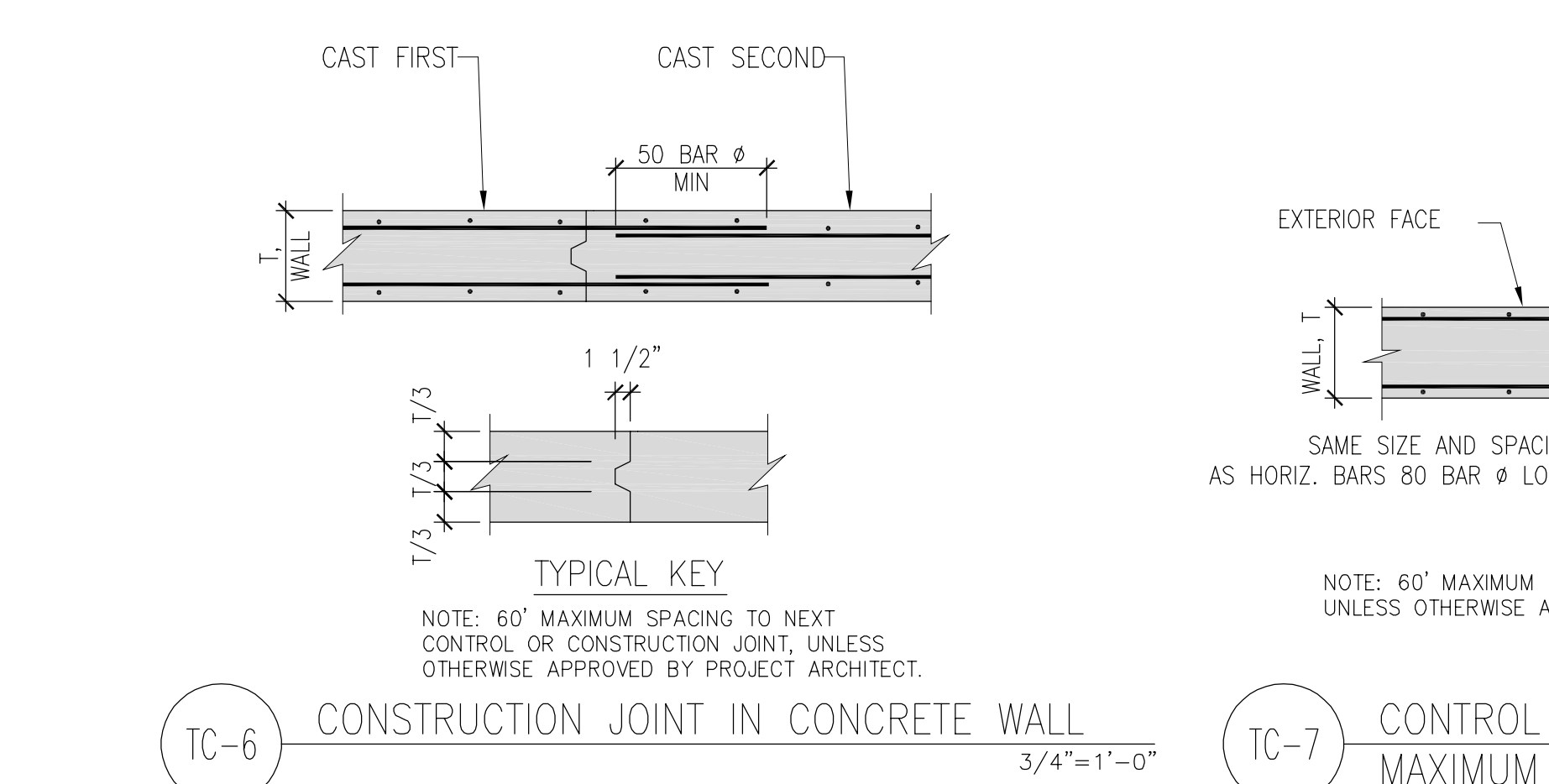
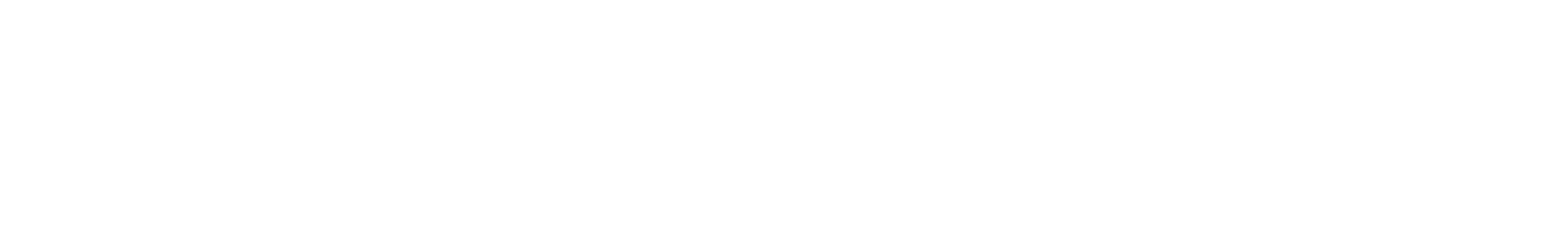
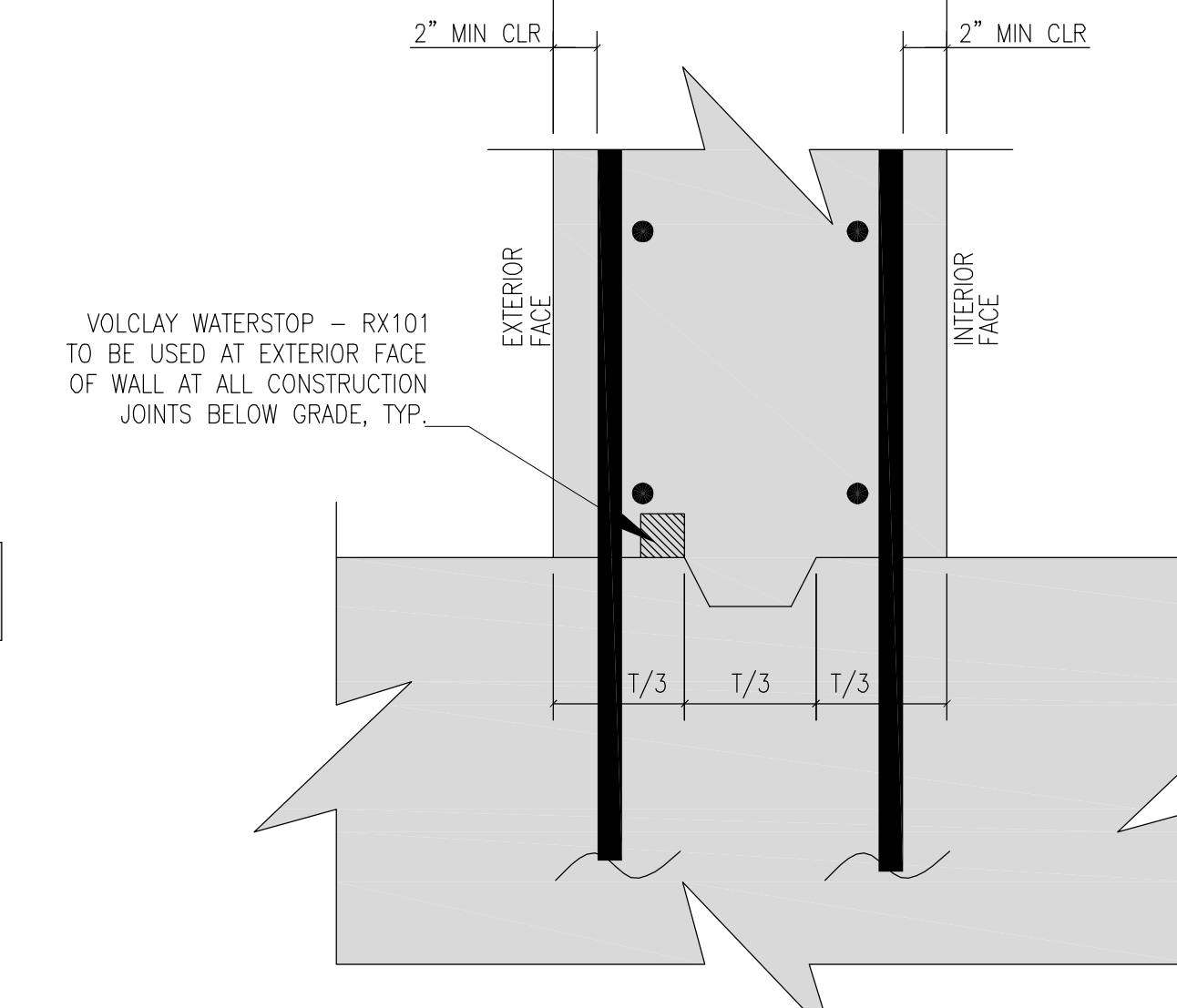
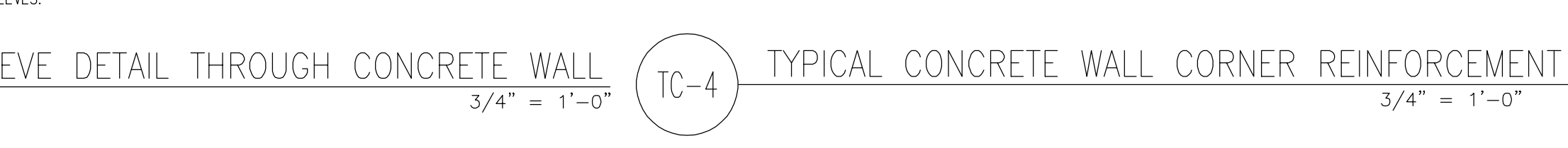
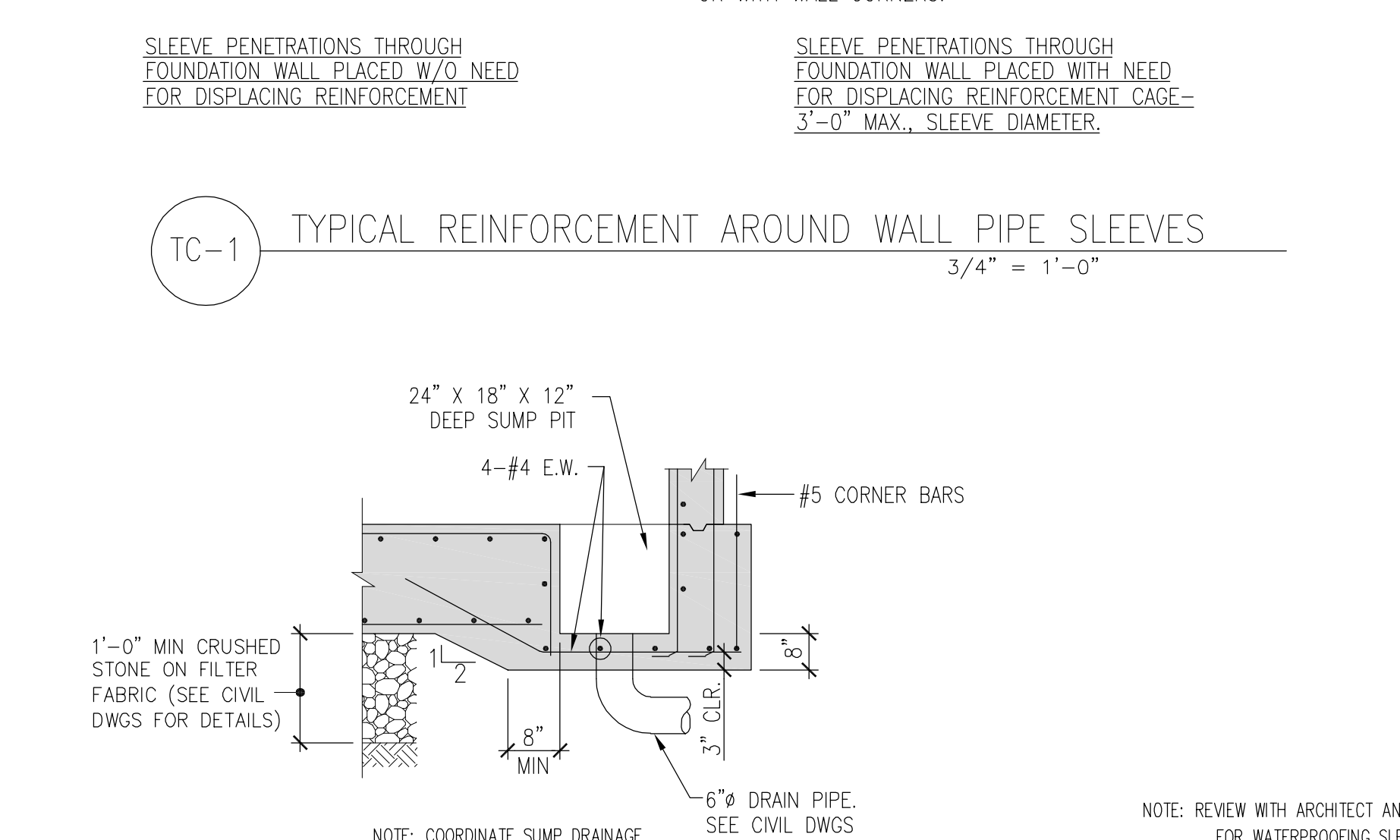
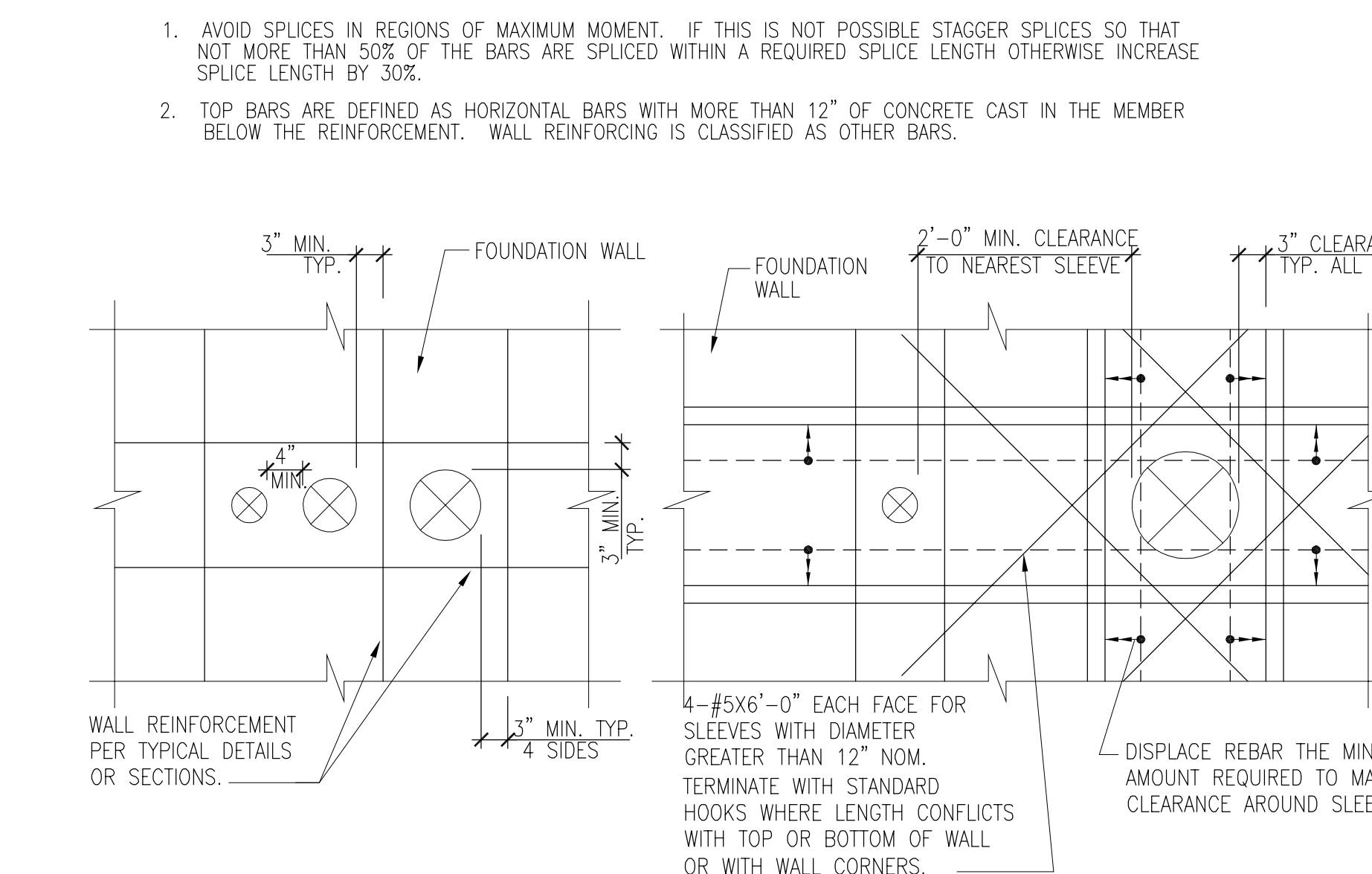
AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES.

AWS D1.1 STRUCTURAL WELDING CODE - STEEL.



MINIMUM SPLICE and DEVELOPMENT LENGTH SCHEDULE
(FY=60000 PSI)
(UNLESS OTHERWISE SHOWN ON DRAWINGS)

SPICE LENGTH	BAR SIZE										
	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#14
TOP BARS	24"	32"	40"	48"	56"	64"	72"	80"	90"	102"	114"
OTHER BARS	18"	25"	31"	37"	44"	51"	58"	66"	75"	84"	96"
DEVELOPMENT LENGTH	18"	25"	31"	37"	44"	51"	58"	66"	75"	84"	96"
OTHER BARS	14"	19"	24"	28"	34"	41"	47"	54"	60"	67"	78"



- General Notes:**
- PROVIDE 30" x 90" HEAVY-DUTY MANHOLE FRAME AND SELF SEALING SOLID LID BY NELAHA FOUNDRY COMPANY, OR APPROVED EQUAL. FRAME AND LID MUST BE CAPABLE OF SUPPORTING AASHTO HS20 ROADWAY LOADINGS.
 - CONTRACTOR SHALL PROVIDE GALVANIZED STEEL LADDER CONFORMING TO OSHA STANDARD 1926.27 FIXED LADDERS. LADDERS SHALL BE 24" WIDE WITH 12" MAXIMUM SPACING BETWEEN RUNGS. LADDER SHALL BE FASTENED TO CONCRETE WALL USING (2) - #5 HLT KWIK BOLT II EXPANSION ANCHORS WITH 4" MIN EMBEDMENT AT EACH CONNECTION POINT. CONTRACTOR SHALL SUBMIT PROPOSED LADDER AND INSTALLATION SCHEME FOR APPROVAL BY ENGINEER.
 - SHEET PILING BY OTHERS. SHEET PILING ALONG EDGE OF MANHOLE TO BE USED AS ONE-SIDED FORM AND SHALL BE LEFT IN PLACE PERMANENTLY.
 - CONTRACTOR SHALL COORDINATE ALL DIMENSIONS WITH MECHANICAL AND CIVIL DRAWINGS AND INCLUDE DIMENSIONS IN SHOP DRAWINGS FOR FINAL REVIEW AND APPROVAL PRIOR TO BEGINNING CONSTRUCTION.

SGH
Simpson Gumpertz & Heger Inc.
Consulting Engineers

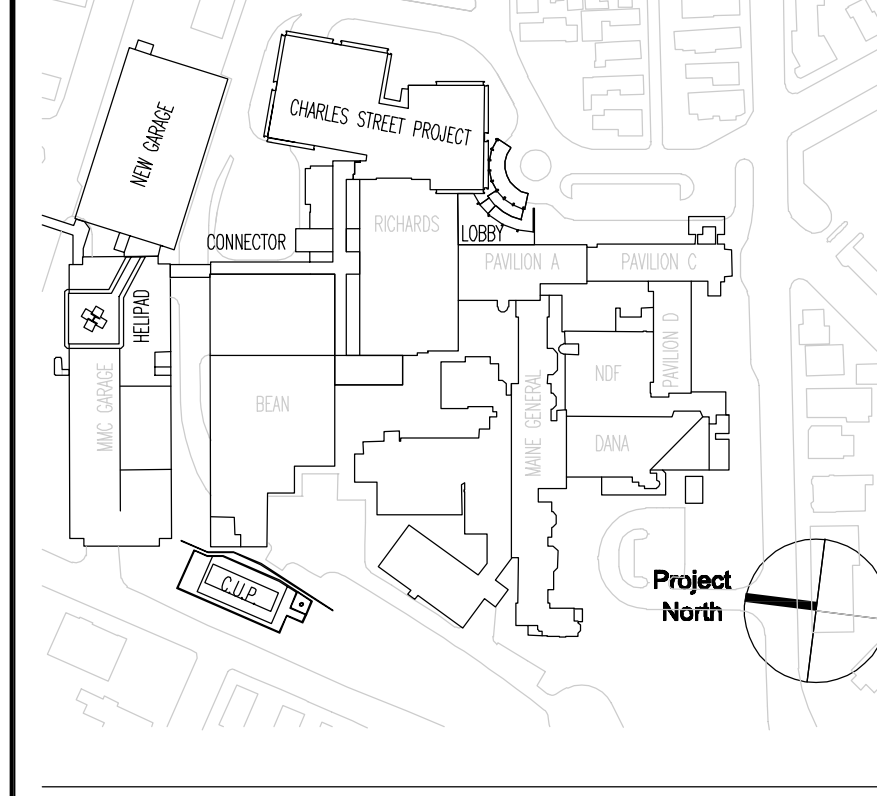
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MARK	ISSUE DATE	DESCRIPTION
PERMIT	09/16/04	PERMIT SET
BID	08/31/04	BID SET

Issue Log



TRO
ARCHITECTS
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INTERIOR DESIGN

The Ritchie Organization
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Newton, MA 02458-1154
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Maine Medical Center
Pkg C - Site Utilities
Portland, Maine MMC Project No. 21846

GENERAL NOTES
PLANS
ELEVATIONS

Commission No. 4677 Date Issued 08/31/04
Scale AS NOTED Sheet Number **PKG-C**
Drawn By SWW
Approved By JHT/JLL
Checked By S201
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