



February 28, 2018 15466

Jean Fraser, City Planner City of Portland 389 Congress Street Portland, ME 04101

Response to Peer Review Comments dated February 7, 2018 Maine Medical Center, East Tower and Visitor Parking

Dear Jean:

We are writing to provide supplemental information and respond to review comments presented in a memorandum From Wright Pierce Engineers dated February 7, 2018 regarding the above referenced applications.

The attached information documents existing subsurface conditions on the Maine Medical Center campus, specifically related to below grade construction along the access road at the southwest corner of the visitor parking garage, between the visitors garage and the emergency department drop-off area, with its below grade electrical room.

The text of the comments from the City's February 7, 2018 memorandum is presented below for reference followed by our response. Additional information is attached as noted.

- Level III Site Plan applications with the City of Portland must submit a stormwater plan pursuant to the regulations of MaineDEP Chapter 500 Stormwater Management Rules. This includes conformance with the Basic, General, and Flooding Standards (Ref: Technical Manual, Section 5. II. Applicability in Portland. C. a.; and Ref: City of Portland Code of Ordinances Sec. 14-526. Site Plan Standards, (b). 3. b.)
 - a) Basic Standard: The applicant has confirmed that no additional impervious surfaces are being added to the facility, the proposed work is not causing a change of use, and no site disturbance is proposed. As such, the project is not subject to the requirements of the Basic Standard. Information provided in the updated Construction Management Plan indicates that downstream catch basin inlets will be protected.
 - b) General Standard: The applicant has provided information that no additional impervious surfaces are being added to the facility and that the proposed work is not causing a change of use. A s such, the project is not subject to the requirements of the General Standard.
 - c) Flooding Standard: The applicant has provided information that no additional impervious surfaces or changes of use are occurring as part of the proposed

development. As such, the project is not subject to the requirements of the Flooding Standard.

Noted. There are no proposed changes to the site that would increase impervious area, alter drainage patters or increase the rate or volume of Stormwater runoff.

- 2) East Tower Expansion Connections:
 - a) The applicant has provided information confirming that the East Tower storm runoff currently discharges to a separated storm sewer system, and that the proposed construction will also discharge to the separated storm sewer system. The roof connections are internal to the facility and no additional information regarding the East Tower Expansion is requested at this time.

Noted.

b) It is understood that the sewers adjacent to the facility are near capacity to serve new development. The applicant has indicated that increases to dry-weather sanitary flows are minimal, and has submitted a Wastewater Capacity Application for review by the Department of Public Works.

Noted.

- *3)* Visitor Garage Expansion
 - c) The existing facility currently discharges to a combined sewer system within Congress Avenue. The applicant has provided a Stormwater Management Report from 2004, indicating stormwater rates to the separate storm sewer in the A-Street corridor.
 - *i)* The information provided indicates that the separate storm drain within the A-street corridor is at or near capacity during the modeled 10-year, 24-hour rain event, but that capacity may be available for lower interval rain events.

The storm drainage system approved by the Planning Board in 2004 and constructed in 2004-2007 for the east Tower is connected to a separated storm drain in A Street constructed by the City of Portland in the late 1990's.

As part of the 2004 approvals, which included the construction of the East Tower and the Visitors Garage, Maine Medical Center constructed on site storm drains to fully separate sewers and storm drains at the perimeter of the campus. The project extended the A Street Storm to Gilman Street and constructed new public storm drains to fully separate runoff

The redevelopment separated and redirected Stormwater runoff from approximately 6.3 acres of existing development from the combined sewers in Crescent St, Ellsworth St, and Congress St to the separated storm drain in A St. A 6' treatment unit diameter unit was installed in the Visitors Garage and a 10' diameter unit was installed in Gilman St.

There are no changes proposed which would increase the rate or volume of runoff from the site when compared to the approved plan. The storm drainage system approved in 2004 tributary to A Street was designed to the capacity of the existing A Street storm drain such that the pipe

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would flow crown full in a 10 year storm event without surcharging inlets in a 25 year event. Additional tributary area is likely to exceed this criteria

- *ii)* Plan 2D Boundary Survey indicates a connection or stub connection leading from the catch basin on the south side of the Visitor Garage that may allow for connection from the Visitor Garage roof drain into the separate storm drain system. The applicant shall provide the following:
 - (1) An updated boundary survey in the vicinity of "Plan 2D Boundary Survey" with utility elevations, pipe sizes, and pipe materials in accordance with the City's Chapter 14 and Technical Manual requirements for a Boundary Survey. Anecdotal information regarding the difficulty for construction in this corridor was discussed in the February 1 meeting. However, a sewer pipe crossing currently exists in this corridor, as does a storm drain or storm drain stub. City staff need additional information regarding existing utilities to confirm if a connection to the separate storm drain will be feasible or not.

A revised boundary survey plan with structures labels as well as rim and invert elevations has been submitted separately.

We have attached additional documentation further illustrating the existing subsurface conditions adjacent to the Visitors Garage and request a meeting with City Staff to more fully explain our concerns related to the feasibility of making a connection from the visitor garage to the separated drain system.

Attachment 1 to this letter included plans presented to city staff on February 1,2018 identifying the existing sewer and storm drain connections from the East Tower and Visitors Garage constructed under the 2004 site plan approval. There are no proposed changes that would increase or alter stormwater runoff when compared to the approved plans.

Attachment 2 includes additional record plan information related to the subsurface utilities at the southwest corner of the visitor garage. These include the Utility Plans and Profile drawings for work associated with the Bramhall Campus Expansion project. This project was reviewed by the City staff and approved by the Planning Board in 2004 and constructed from 2004 -2007. The Bramhall Expansion project included the construction of the East Tower, Visitor Garage, Emergency Department renovations, the reconstruction of Westcott Street and construction of the Central Utility Plant.

The utility work included the complete reconstruction the Access Road between the Visitor Garage. Utilities constructed within the access road include:

- Separated storm drains diverting 7 acres of the campus to the separated sewer in A Street
- A 12" water main

- Concrete encased conduit and manhole systems entering the electrical vault below the Emergency Department parking area that includes:
 - The Campus' three (3) primary electrical feeds (owned by CMP)
 - o The campus' emergency power feeds from the Central Utility plans
 - Public and private telecommunications cable and fiber optic system
- Mechanical piping serving the main campus from the Central Utility plant including
 - o Steam feed and Condensate return
 - Chilled water and return water

Each of these systems converge in the access road, in the vicinity of a drain manhole (identified as DMH# 22098 on the survey plan and is highlighted as DMH-112 on the design plans sheet C300) at the southwest corner of the parking garage. It is our understanding that it it's this structure which the review comments refer to as a potential location for connection.

Attachment 3 also includes design plans for the Visitor Garage's earth retention system. The earth retention wall was constructed in 2004 to support a 40' to 60' vertical excavation for the construction of the garage. The retaining wall is a soldier pile and lagging wall with reinforced concrete facing backed with insulation and drainage board. The soldier piles are supported by post tensioned grouted tiebacks which extend 44 to 76 feet horizontally behind the wall. With the top row of tiebacks between 8' and 9' below grade. The area the Staff has suggested for a storm drain connection is highlighted on Sheets SCF-2, SCF-3 and detail 1A on Sheet SFC-4.

It is our opinion that making a drainage connection to DMH #22098 (DMH-112) or the catch basin CB# 22087 (CB-204) is not feasible due to interferences with other existing utilities and due to the potential risk associated with excavating behind the Visitor Garage earth retention system.

The staff comments correctly note that a sewer penetration of this wall does exist, as does a penetration for electrical conduit shown on the earth retention plans. These penetrations through the wall were carefully designed and coordinated with design-build contractor responsible for the design and installation of the retaining wall and tie backs and with the floor and spandrel locations of the garage.

The existing penetrations are reinforced to maintain the integrity of the wall and are spaced to avoid tie backs. We do not recommend cutting new penetrations through this wall. The wall was not designed with future penetrations in mind and have concerns regarding potential risk/liability associated modifying this wall or excavating in close proximity to the tie back tendons behind it.

It is also noted that all of the utility structures, duct banks and electrical manholes in the access road at this location were constructed prior to the installation of retaining wall tie backs and tendons. This was an intentional sequence of construction employed to mitigate risks associated with excavating in close proximity to the highly tensioned tie back tendons.

The staff comments note a drainage stub as a potential connection. We understand this to be a reference to the line shown projecting to the northeast from structure CB#22087 (CB-112). We

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have investigated this opening in the manhole. This is not a drain stub that extends as far as the survey plan implies. This line represents the location of a temporary sewer pipe stub that was cored into the structure during the 2004-2007 construction as temporary means to convey a former sewer in this location to Gilman Street, while the Visitor Garage and the permanent sewer was under construction.

It appears that this opening is generally oriented toward the adjacent sewer manhole. The stub was cut and capped and sealed close to the structure after construction and prior to the installation of the adjacent sewer manhole, electrical manholes and duct banks. We believe that access to this entrance into the manhole is now obstructed by these other structures. Due to the tight clearances of the subsurface utilities in this area, excavations were backfilled with flow able concrete fill in this area.

- (2) If a connection from the Visitor Garage roof to the separate storm drain that conveys flows to the A Street corridor is feasible, then the applicant shall provide:
 - (a) A connection that allows for a 1" rain event to be conveyed to the separate storm drain on A Street, consistent with the City's Long-Term CSO Control Plan.
 - (b) An overflow connection that conveys larger interval storm events towards the combined sewer on Congress Avenue.

As noted above we do not consider a connection of the Visitor Garage roof to the separated sewer to be feasible given the utility conflicts and potential impacts to the earth retention system. The building design team is currently reviewing additional implications related to the Visitor Garage structure associated with this proposal. These include:

- Such a connection would necessitate exposed horizontal piping in an un heated structure. This is not a recommended practice. All of the existing roof deck piping is plumbed vertically down the center of the structure to avoid this condition.
- Potential penetration's of the existing garage structure deck. The existing structure includes pre-tensioned T-beam construction with the parking surface integral the beams top flange. These decks were not designed or intended for coring or penetrations.

The performance standards of Section 14-526(b)3.a.(i)-(iv) specifically relate to mitigating proposed *increases* in stormwater runoff. As a vertical expansion of the buildings there is no increase or change in the rate or volume of runoff generated by the site and is therefore not expected to have any new or increased impacts during wet weather or CSO events.

The Performance Standards in Section 14-526(c)3.a. relates to sewer capacity. As discussed in response to previous comments the project proposes a minimal change in the dry weather sewer flows. The project does not increase the number of patient beds in the building. The increase in flow is associated with additional staff and a net addition of six (6) staff toilets and

two (2) sinks. This is not expected to have any measureable impact on the combined sewer system.

iii) For floor drains not exposed to roof runoff, flows shall be conveyed to an oil-water separator connected to the combined sewer. Locations of oil-water separator shall be confirmed on the engineering permitting plans, and detailed.

The proposed projects are vertical expansions of the upper stories of existing structures, utilizing existing internal plumbing connections. No Floor drains are proposed.

4) If new connections to the municipal system are constructed, then confirmation of adherence to the Technical Manual and Site Plan Standards regarding storm drain, sewer, and connections to the existing system shall be provided.

The proposed projects are vertical expansions of the upper stories of existing structures, utilizing existing internal plumbing connections. No site work or new connections to the existing utility systems are proposed.

- 5) General Comments:
 - d) Ability to Serve letters from affected utilities are required as part of the Level III Application process. Ability to Serve Letter from Central Maine Power has been provided.

We have requested an ability to serve confirmation from the Portland Water District and have not received it at this time

e) The plans should note a location for snow storage or provide a written snow storage plan. The proposed conditions are very similar to the existing conditions. If all snow is removed from the site in the existing conditions, then a statement from the applicant indicating method of removal and that snow removal protocol will not be changed will suffice for this item. If snow is stored on site, snow storage locations shall be indicated.

The proposed projects are vertical expansions of the upper stories of existing structures, there no changes to the snow removal operations proposed.

Sincerely,

SEBAGO TECHNICS, INC

Daniel L. Riley , PE Vice President, Engineering

DLR:jg Enc.

ATTACHMENT 1

Bramhall Campus Expansion Utility Plans





		1	
STRUCTURE	RIM	INV. IN	INV. OUT
SMH-105	135.66	127.20	MATCH EXIST.
		MATCH EXIST.	
SMH-107	128.75	110.20	110.10
SMH-107A	129.60	MATCH EXISTING	MATCH EXISTING
SMH-108	129.60	TBD (SS-9) 109.6 (SS-8)	109.50
SMH-109	115.27	108.77	108.67
SMH-1*	107.63	9 9 9 9	7.92 92. 7.79 2.41 2.32
SMH-200	106.60	91.00	86.0
SMH-201	86.50	83.50	81.50
SMH-202	78.0	73.0	65.0
SMH-203	55.75	50.0	45.10
SMH-203A	62.50	55.0	54.90
SMH-204	116.67	101.80	101.70
SMH-205	119.25	TBD	TBD
SMH-206	111.50	101.20	101.10
<u>SMH-206A</u>	87.50	82.50	73.00
SMH-207	81.50	72.75	72.65
SMH-208	67.0	57.50	57.40
SMH-208A	83.44	TBD	59.00
SMH-209	67.0	57.0	56.90
SMH-210	62.50	56.40 55.90	MATCH EXISTING
SMH-211	85.00	80.50 80.50	70.00
SMH-212	78.0	69.50	69.40
SMH-213	75.90	65.20 (SS-65) 66.90 (SD-86)	65.10
SMH-215	62.0	52.0 53.40	51.90

SS NO.	DIA. (IN.)	LENGTH (FT.)	SLOPE	MATERIAL
SS-7	12	82	0.0097	PVC
SS-8	12	44	0.011	DI
SS-9	15	29	0.01	PVC
SS-10	18	80	0.01	DI
SS-11	16	37	0.027	DI
SS-50	12	130	0.0089	PVC
SS-51	12	43	0.058	PVC
SS-52	12	54	0.157	PVC
SS-53	12	51	0.196	PVC
SS-54	12	214	0.09	PVC
SS-55	12	61	0.306	PVC
SS-55A	12	33	0.0075	PVC
SS-56	8	TBD	TBD	PVC
SS-57	8	47	0.011	PVC
SS-58	12	35	0.0065	PVC
SS-58A	12	13	0.376	PVC
SS-59	8	43	0.302	PVC
SS-60	15	46	0.021	DI
SS-61	15	34	0.0117	PVC
SS-62	15	26	0.019	PVC
SS-63	12	103	0.068	PVC
SS-64	8	44	0.011	PVC
SS-65	8	43	0.097	PVC
SS-65A	8	66	0.0106	PVC
SS-66	8	57	0.50	PVC
SS-66A	8	22	0.01 (MIN.)	PVC

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07.0	6. 90	7.9	06.7	7.8	<u>9.23</u>	<u>0.2</u> 0.38	11.52 11.52 2.5 2.67	3.9	3.76	4.7 4.30	5.7 14.81	17.1 6.03	8.5 8.03	20.0 6.7	21.6	23.01	24.8	4.96	4.56
10	10	, 		1	10 10	====		Ę		<u>, </u>		+ - +	7		12		1 2 1 2 1 2		<u> </u> 2 1
8+	00	8+50	9+	·00	9+50	1	0+00	10+	-50		11+00	•	11+50		12-	+00	12-	-50	-
					PROFILE: S	TORM DRAIN, I	ELECTRICAL	<u>AND GA</u>	<u>s pipi</u>	<u>NG</u>									
						STA. 7+00 TO) STA. 12+7	<u>5)</u>											
	I	I		I	I	1"=4' VERT.	1	I			I		I			l	I		

									HIGH POINT ELEV =	124.96 1 2+51.72
									PVI STA = 12+2 $PVI ELEV = 126.$	7.65
									A.D. = -14.58 K = 6.86	8
		SEE GRADING PLANS FOR GRADES LEFT OF STA 9+	PROPOSED -20						100.0000' VC	
			BEGIN TYPICAL CRC	WNED ROADWAY					PROPOSED GRADE AT <	
			SECTION STA. 9+20	D < ELEV. = 107.50 (c600)						
							74.4000' VC	<u> </u>		į
					PVI S PVI E	TA = 10+53.81 LEV = 113.99	PVI = 115.02 PVI = 9.30	.86 .11+77 120.9		
					A.C	$M_{\rm H} = -3.08$ $M_{\rm H} = 8.12$	K = 8.00	1+59 119.0. VCS: VCE:		
					25	.0000' VC	15.46 0'RT			-
					12	5.31 17 5.46 .5.46				
					13.41 13.41	10+66 10+8 10+8 114.		10.80%		
					CE: 10	B VCE EVCE		<u> </u>		
					→ 5.5' MIN. COVER → →			/		
					OVER WATER, TYP.	A1.50%	A	TER		
		5	MATCH EXISTING — GRADE STA. 9+50 ELEV. = 109.16					12 WAY	8,	" WATER SERVICE
	8			4.585	%					
	H N N S S S S S S S S S S S S S S S S S									
	Sili				0					
					TER					
				TRUCTURE 12" W 05.0			CONCRETE THRUS	Г Н		
							MECH. PLANS (SE	E		
		12" WATER					MECHANICAL PIPIN	IG TURNS 90' TO ENI	IER_LL	
						8" FIRE WAT	BEAN BUILDING: 6 18" SCHWS/R < 5 ER SERVICE MINIMUM TOP OF	P.D. < STA. 10+84 STA. 10+92.20 PIPE FLEV = 106.00		
				RETE THRUST 6" 1	DOMESTIC WATER SERVICE					
				SUPPORT, (SEE		REDUCE COVER OVER W	ATER LINE TO 4' MINIMUM			
				TO ENTER AND EXIT MECHANICA	λL	FROM STATION 10+65; INSULATE PIPE WHERE D	TO STA 11+20; DEPTH OF COVER IS LESS			
			PROVIDE W	ALL SLEEVE AND LINK SEAL. WATER LINE PIPE (WRAP PIPE) 6	Г.	IHAN 5.5				,
			HERE BOTH SIDE	S OF MECH. MANHOLE F STRUCTURE ELEV. = 90.8						
			REINFORCE	D CONCRETE MECHANICAL VAULT	. INSTALL LINK SEAL AND					/
			STEEL WAL	L SLEEVES AT ALL PIPE PENETRA AND MECHANICAL PLANS FOR E	ATIONS INTO VAULT. SEE DETAILS OF STRUCTURE					
			EE SCHEDULE SHEET C605 FOR FAL AT WALL PENETRATIONS	S). SLEEVE AND						
			ONCRETE THRUST							
	FOR ADDITIONAL INFORMATION		IPE SUPPORT, (SEE OTE 5)							DATUM FI.F.V
Ċ	, o ,			22 23		² 76	⁷ 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2		60 V 44	82.00 80 00 m
	1106.	106.	107.	109.2 110.2	111.	113. 114. 114.	116. 116. 118. 118. 118. 118. 118. 118.	120.6 120.6	121. 123. 124.4	124. 124.5
8-	↓ +00 8-	+50 9-	+00	9+50 10)+00 10)+50 11	I I I I I I I I I I	 50	12+00	12+50
			PROFIL	E: SAN. SEWER. WA	ATER AND MECHAN	ICAL PIPING				
				(STA. 7+00 TO	<u>STA. 12+75</u>	<u> </u>				
		I		1"=4' VERT.			I			I

General Notes:

- 1. SEE SHEET C302 FOR DRAINAGE AND SANITARY SEWER SCHEDULES.
- 2. FOR CLARITY ONLY PROPOSED UTILITIES ARE SHOWN UNLESS OTHERWISE INDICATED. REFER TO EXISTING CONDITIONS AND SITE DEMOLITION PLANS.
- 3. ELECTRICAL/TELECOMMUNICATIONS CONDUIT DUCTBANK DIMENSIONS ARE SHOWN FOR UTILITY COORDINATION. THE CONTRACTOR MAY ALTER THE PROPOSED DUCT BANK ARRANGEMENT PROVIDED THAT REQUIRED CLEARANCES FROM ADJACENT UTILITIES ARE MAINTAINED. REFER TO ELECTRICAL PLANS AND SPECIFICATIONS FOR: DUCT BANK SCHEDULE, DETAILS OF DUCT BANK CONSTRUCTION, ELECTRICAL MANHOLE DETAILS AND SHOP DRAWING REQUIREMENTS FOR SITE ELECTRICAL WORK.
- 4. SEE STRUCTUAL PLANS FOR WALL PENETRATION REINFORCEMENT DETAILS SEE SHEET C605 FOR SHCEDULE OF WALL SLEEVE AND SEALS 5. MECHANICAL PIPING IS SHOWN TO ESTABLISH REQUIRED GRADES FOR UTILITY COORDINATION. THE CONTRACTOR SHALL PROVIDE SHOP DRAWINGS OF THE MECHANICAL PIPING SYSTEM AS REQUIRED IN THE PROJECT SPECIFICATIONS. LOCATION OF THRUST BLOCK AND MOMENT GUIDES ARE SHOW HERE FOR INFORMATION ONLY AND ARE SUBJECT TO CHANGE

MARK ISSUE DATE DESCRIPTION

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BUL 03 10/21/05 BULLETIN NUMBER 03 – PACKAGE C 08/09/05 BULLETIN NUMBER 02 – PACKAGE C
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ATTACHMENT 2

Garage Earth retention Plans

LEGEND EXISTING GRADE CONTOUR [79'-11 1/4"] BOTTOM OF EXISTING FOOTING BOTTOM OF PROPOSED FOOTING (76.50)SOLDIER BEAM NUMBER (\mathbf{I}) - TIEBACK NUMBER OF ARROWS DESIGNATE THE NUMBER OF TIERS UNDERPINNING PIT B-203 BORING NUMBER APPROXIMATE BORING LOCATION DEFLECTION MONITORING 2 70 AL POINT (BY OTHERS) DMP-10 (BY OTHERS) (2 TOTAL SHOWN) (BY OTHERS) (4 TUTAL SHOWN) <u>____</u> − _INCLINOMETER (BY OTHERS) (10 TUTAL SHOW) VIBRATION MONITORING (5 TOTAL SHOW) _TEMPORARY WALE/BRACE TO BE REMOVED AFTER SLAB INSTALLATION HARRY * W. SCHNABEL EP: No. 8646 \Harry W. Schnabel, P.E. Maine P.E. No. 8646 Job Number: 06-3634

SECTION 1A

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EXISTING 1111111111111 BUILDING PERMANENT TIEBACK (TYP) HARRY W. . SCHNABEL Job Number: 06-3634 |Scale: 3/32"=1'-0"| Drawing No: SFC-4