

### R. W. Gillespie & Associates, Inc.

Geotechnical Engineering • Geohydrology • Materials Testing Services

08 November 2005

Mr. Peter Drivas Portland Seadogs 271 Park Avenue Portland, ME 04102



Subject: Foundation Evaluation New Right Field Grandstands - Hadlock Field Portland, Maine RWG&A Project No. 235-1043

Dear Mr. Drivas:

As requested by David K. Pinkham, P.E., of Pinkham & Greer, R.W. Gillespie & Associates, Inc., (RWG&A) has estimated stresses at the storm sewer springline that might be imposed on the sewer from foundation "C-1 Relocated" under a bearing pressure of 2,000 psf. Proposed foundation location "C-1 Relocated" relative to the storm sewer and dimensions were provided by Pinkham & Greer in a drawing dated 28 October 2005. As indicated on Figure 1, the estimated increase in vertical and horizontal stresses at the storm sewer spring line are approximately 45 and 20 pounds per square foot, respectively.

We trust the foregoing meets the current needs of the project, and if you have any questions, please contact us.

Very truly yours, R. W. GILLESPIE & ASSOCIATES, INC. an Milling Erik ERIK Chief Geotechnical Engineer 3727

EJW:ci

Copy: Robert B. Metcalf, R.I..A., Mitchell & Associates, Inc. David K. Pinkham, P.E., Pinkham & Greer, Inc.

200 International Dr., Ste 170 Portsmouth, NH 03801 603-427-0244 • Fax 603-430-2041 · Corporate Office 86 Industrial Park Rd., Ste 4 Saco, ME 04072 207-286-8008 • Fax 207-286-2882

P.O. Box 289 Augusta, ME 04344 207-623-4914 • Fax 207-623-3429



# **General Building Permit Application**



If you or the property owner owes real estate or personal property taxes or user charges on any property within the City, payment arrangements must be made before permits of any kind are accepted.

TOFT	LAND SEADOGS/HADL	OCK FIGLD
Location/Address of Construction:		
Total Square Footage of Proposed Structure	Square Footage of Lot	
<b>7, 764 S.F.</b> Tax Assessor's Chart, Block & Lot	Owner:	Telephone:
Chart# Block# Lot#	City of Portlan-	
49 A 001	(ity of Vortland	233-0350
Lessee/Buyer's Name (If Applicable)	Applicant name, address & telephone:	Cost Of
		Work: \$
	Bob Leeman	Fee: \$
N/A	233-0350	R
		C of O Fee: \$
Current legal use (i.e. single family) If vacant, what was the previous use?	BASE 6ALL Stadium	
Proposed Specific use:	SAME	
Is property part of a subdivision?	O If yes, please name	
Project description:		
Contractor's name, address & telephone:	DE DETE FM. JED	
Who should we contact when the permit is rea	-	DETT. PUBLIC
Mailing address:	Phone: 233.0350	WOFKS,
		POFTLAND

Please submit all of the information outlined in the Commercial Application Checklist. Failure to do so will result in the automatic denial of your permit.

In order to be sure the City fully understands the full scope of the project, the Planning and Development Department may request additional information prior to the issuance of a permit. For further information or to download copies of this form and other applications visit the Inspections Division on-line at <u>www.portlandmaine.gov</u>, or stop by the Inspections Division office, room 315 City Hall or call 874-8703.

I hereby certify that I am the Owner of record of the named property, or that the owner of record authorizes the proposed work and that I have been authorized by the owner to make this application as his/her authorized agent. I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in this application is issued, I certify that the Code Official's authorized representative shall have the authority to enter all areas covered by this permit at any reasonable hour to enforce the provisions of the codes applicable to this permit.

Signature of applicant:	DEPT. OF I Dated INSTRUCTOR 7
This is not a permit; you may not commen	ice ANY work until the permit is issued.
	AUG 1 4 2007
Building Inspections Division • 389 Congress Street • Portland, Maine 04101	• (207) 874-870 - TTACGIM JUE/2073 74-8716 • TTY (207) 874-8936

# BUILDING PERMIT INSPECTION PROCEDURES Please call 874-8703 or 874-8693 (ONLY)

### to schedule your inspections as agreed upon Permits expire in 6 months, if the project is not started or ceases for 6 months.

The Owner or their designee is required to notify the inspections office for the following inspections and provide adequate notice. Notice must be called in 48-72 hours in advance in order to schedule an inspection:

By initializing at each inspection time, you are agreeing that you understand the inspection procedure and additional fees from a "Stop Work Order" and "Stop Work Order Release" will be incurred if the procedure is not followed as stated below.

A Pre-construction Meeting will take place upon receipt of your building permit.

Footing/Building Location Inspect	tion: Prior to pouring concrete	
Re-Bar Schedule Inspection:	Prior to pouring concrete	
<b>Foundation Inspection:</b>	Prior to placing ANY backfill	
<b>Framing/Rough Plumbing/Electri</b>	cal: Prior to any insulating or drywalling	
<b>Final/Certificate of Occupancy:</b> Prior to any occupancy of the structuuse. NOTE: There is a \$75.00 fee p inspection at this point.		

Certificate of Occupancy is not required for certain projects. Your inspector can advise you if your project requires a Certificate of Occupancy. All projects **DO** require a final inspection

If any of the inspections do not occur, the project cannot go on to the next phase, REGARDLESS OF THE NOTICE OR CIRCUMSTANCES.

CERIFICATE OF OCCUPANICES MUST BE ISSUED AND PAID FOR, BEFORE THE SPACE MAY BE OCCUPIED

ignature of Applicant/Designee Signature of Inspections Official Building Permit #: 07-098

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Location of Construction:	Owner Name:		Owner Address:		Phone:
165 PARK AVE	CITY OF PORTLAND		389 CONGRESS ST		
Business Name:	Contractor Name:		Contractor Address:		Phone
	TBD		Portland		
Lessee/Buyer's Name	Phone:		Permit Type:		
			Commercial		
Dept: DRC Status: Pending Reviewe		Reviewer	r: Approval Date:		
Note:				(	Ok to Issue:
Dept: Planning Status: A	pproved with Condition	ns Reviewer	Richard Knowland	Approval Da	te: 10/04/2007
Note:					Ok to Issue: 🗹
Comments:					
8/14/2007-mes: I can not see a site plan application for this work on file - I have e-mailed Barbara and asked her about it.					
9/11/2007-mes: there is now a site plan application #2007-0157 - I gave the plans and permit to Mike N. this morning. I will get it back for signing off on zoning.					
9/25/2007-jmb: Mike Nugent returned the permit package. Before a foundation permit can be issued, assurance must be provided for the protection and stability of the sewer line and bleacher footing. The footing design must correlate to the seismic report. Mike has relayed to Bob L.					
9/25/2007-jmb: Forwarded to Marge, she reports that no site plan has been submitted per Rick K.					
10/4/2007-jmb: Notified by Rick K. That Planning is signing off with conditons. I left a voicemsg w/Phil for DRC approval					



Certificate of Design

Bob well bring in 3<sup>rd</sup>Sheet Seald

8/6/07

DORUS AFCHITES

From:

Date:

These plans and / or specifications covering construction work on:

POFTLAND SEA DOGS CLUB HOUSE

Have been designed and drawn up by the undersigned, a Maine registered Architect / Engineer according to the 2003 International Building Code and local amendments.



(SEAL)

	1
Signature:	Cler taged
Title:	PEINCIPS-
Firm:	Dopus ARCHITESTS
Address:	P.O. Dox 301
	FREEPORT HE OHOSZ
Phone:	865-1272

For more information or to download this form and other permit applications visit the Inspections Division on our website at www.portlandmaine.gov

Building Inspections Division • 389 Congress Street • Portland, Maine 04101 • (207) 874-8703 • FACSIMILE (207) 874-8716 • TTY (207) 874-8936



# Accessibility Building Code Certificate

**Designer:** 

Address of Project:

Nature of Project:

j j
DOHUS ARCHITECTS POFTLAND SEADOGS HADLOCK FIELD
HADLOCK FIELD ATIPARK TOT, , PORTLANTS
ATTLETIC CLUB HOUSE

The technical submissions covering the proposed construction work as described above have been designed in compliance with applicable referenced standards found in the Maine Human Rights Law and Federal Americans with Disability Act. Residential Buildings with 4 units or more must conform to the Federal Fair Housing Accessibility Standards. Please provide proof of compliance if applicable.

CUYLER M. FEAGLES No.2529 (SEAL)	Signature Title: Firm:	TEINKIPSE Der DE Arcalitere
	Address: Phone:	

For more information or to download this form and other permit applications visit the Inspections Division on our website at www.portlandmaine.gov

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3

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# Certificate of Design Application

ORTLAND'	Certificate of D	esign Appl	lication
From Designer:	Dopus AZCH	TECTS	
Date:	8/6/07		
Job Name:	POFTLAND SE	A DOGS C	LUB HOUSE
Address of Construction:	1		E ST. POFTLAND
Cons	2003 Internationa truction project was designed to a	U	eria listed below:
Building Code & Year <b>FB</b>	<b>2</b> Sroup Classificati	ion (s)	
Type of Construction $\checkmark$	4		
	ppression system in Accordance wit		2003 IRC YES
	If yes, separated or non s		
		-	
Supervisory alarm System?	<b>C</b> Geotecnnical/Sous repor	rt required? (See Section	1802.2) SEE ATTACHE
Structural Design Calculation	\$	Ð	Live load reduction
EE ATTACLE IS ibmitted for al		20	Roof <i>live</i> loads (1603.1.2, 1607.11)
Contraction of the second seco		42	Roof snow loads (1603.7.3, 1608)
Design Loads on Construction			Ground snow load, Pg (1608.2)
Uniformly distributed floor live load Floor Area Use	Loads Shown		If $Pg > 10$ psf, flat-roof snow load $pr$
HEIGHT FOOD D'2	0, 7:20, 6:50, 6:250		If $P_g > 10$ psf, snow exposure factor, $C_g$
Formale & Piredula D		1.0	If $P_g > 10$ psf, snow load importance factor,
DATTING & FITCHING D	<u>, 55, C=100</u>	1.2	
······································		42	3
Wind loads (1603.1.4, 1609)			Sloped roof snowload, ps(1608.4)
	ized (1609.1.1, 1609.6)		Seismic design category (1616.3)
Basic wind speed (		DFS Z:G	Basic seismic force resisting system (1617.6.2) Response modification coefficient, R, and
II, I = 20 Building category a		<del>-</del>	
Wind exposure cat	table 1604.5, 1609.5)	e a e	deflection amplification factor <sub><math>Gl</math> (1617.6.2)</sub>
O.B. Internal pressure coe			<b>S</b> Analysis procedure (1616.6, 1617.5)
O.B. Component and clad	ding pressures (1609.1.1, 1609.6.2.2)		Design base shcar (1617.4, 16175.5.1)
LOS HAX Main force wind pres		Flood loads (	(1803.1.6, 1612)
Earth design data (1603.1.5, 16	14-1623)	N//>	Flood Hazard area (1612.3)
2 Design option utili		Other lands	Elevation of structure
L, I: 1.0 Seismic use group		Other loads	
<u> </u>	coefficients, \$25 & \$21 (1615.1)		Concentrated loads (1607.4)
Site class (1615.1.5)		- <u>10</u> N/A	Partition loads (1607.5)
			Misc. loads (Table 1607.8, 1607.6.1, 1607.7, 1607.12, 1607.13, 1610, 1611, 2404

From:	Marge Schmuckal
To:	Barbara Barhydt
Date:	8/14/2007 3:23:38 PM
Subject:	Re: Hadlock Field

Yes I can. I will make copies of what I have. Marge

>>> Barbara Barhydt 8/14/2007 3:16:28 PM >>>

They have probably talked with Rick about this, but I do not know anything about this. Could you show me the plans Wed or Thurs? Thanks.

Barbara

>>> Marge Schmuckal 08/14 2:37 PM >>>

Barbara.

I have a permit application for a new club house beneath the bleachers at Hadlock. I can not find a site plan application for this. Wouldn't they need a site plan review for this? I am putting this on hold until we figure it out.

Marge

CC: Alex Jaegerman

From:	Marge Schmuckal
To:	Barbara Barhydt
Date:	8/14/2007 2:37:19 PM
Subject:	Hadlock Field

Barbara,

I have a permit application for a new club house beneath the bleachers at Hadlock. I can not find a site plan application for this. Wouldn't they need a site plan review for this? I am putting this on hold until we figure it out. Marge

CC: ALEX JAEGERMAN



### R. W. Gillespie & Associates, Inc.

Geotechnical Engineering • Geohydrology • Materials Testing Services

12 November 2004

Mr. Peter Drivas Portland Seadogs 271 Park Avenue Portland, Maine 04102

Subject: Geotechnical Investigation New Right Field Grandstand and Concession Stand Hadlock Field Portland, Maine RWG&A Project No. 235-1043

Dear Mr. Drivas:

As requested, R. W. Gillespic & Associates, Inc., (RWG&A) has conducted a geotechnical investigation for the proposed right field grandstand and upper level concession stand at Hadlock Field in Portland, Maine. The purpose of this investigation was to obtain information regarding subsurface soil and groundwater conditions on which to base recommendations for design and construction of foundations and slabs on grade.

As completed, our scope of work included the following tasks:

- 1. Drilled and logged three test borings to depths of 37 to 42 feet below existing ground surface at the site.
- 2. Evaluated acquired subsurface data with respect to proposed construction with emphasis on foundation type, allowable contact pressure, settlement, and subgrade preparation.
- 3. Prepared this report including a project description, a description of the subsurface exploration program, a summary of the subsurface conditions, our evaluations, conclusions, and recommendations, and boring logs from subsurface exploration activities.

Corporate Office - 86 Industrial Park Rd., Ste 4 • Saco, ME 04072 • 207-286-8008 • Fax 207-286-2882 Branch Office - 200 International Dr., Ste 170 • Portsmouth, NH 03801 • 603-427-0244 • Fax 603-430-2041

### R. W. Gillespie & Associates, Inc.

#### **Project Description**

The proposed grandstand will be over the existing bullpen and will include a lower level for bathroom facilities. A new concession stand, wheelchair lift, and walkway platform will abut the picnic area but at a higher elevation. Discussions with David Pinkham, P.E., project structural engineer, indicate column loads will range from 10 to 132 kips as summarized below.

Worst Case Column	132 kips (D+L)
Grandstands	40 to 84 kips (D+L)
Walkways	10 to kips (D+L)

#### Subsurface Investigation

The subsurface exploration program for this project consisted of three soil borings drilled to depths of 37 to 42 feet. The drilling was performed by Great Works Pump & Test Boring, Inc., of Rollinsford, New Hampshire. The explorations were made with a track-mounted rotary drill rig using cased hole methodology.

Standard penetration resistance tests were taken at the ground surface and at 5-foot intervals thereafter. Recovered samples and washings were used to describe the soils and prepare the boring logs presented in the Appendix. Samples were classified in general accordance with the procedures of ASTM D2488, Standard Practice for Description and Identification of Soils (Visual-Manual Procedure). Stratification lines shown on the boring logs represent the approximate boundaries between soil types encountered; the actual transitions will be more gradual and will vary over short distances. Groundwater was measured at the completion of each bore.

A boring had been requested near the lower level concession stand, but access limitations and the number of utilities in the area precluded explorations at this time. During the early stages of construction, it may be possible to access the area for one or two borings to confirm subsurface conditions prior to foundation placement.

#### Findings

Subsurface soils include fill underlain by silty clay. In turn, the clay is followed by silty sand with fine gravel at greater depths. The fill consists of sand, silt, ash, brick, and traces of organic material and is loose to very loose. The silty clay layer is approximately 5 to more than 17 feet thick and is medium stiff to the depths explored. The silty sand layer is medium dense and consists of medium to fine-grained sand with fine gravel below a depth of about 26 feet below the local ground surface. Free water was encountered at an approximate depth of 9 feet below the local ground

RWG&A Project No. 235-1043

surface; however, these levels should not be considered representative of the stabilized groundwater level. In general, groundwater levels across the site will fluctuate due to tide, season, temperature, precipitation, and construction activity in the area.

#### **Evaluations**

Vertical loads range from 10 to 132 kips as summarized below.

Worst Case132 kipsGrandstands40 to 84 kipsWalkways10 to kips

If existing fill is removed to a depth of about 8 feet, or 1 foot above groundwater, and replaced with granular fill, loads applied to the remaining sand-ash-debris fill are reduced somewhat. Using this scenario and an allowable contact pressure of 2 kips per square foot, settlement estimates range from less than 1/2 inch to about 1  $\frac{1}{2}$  inches; differential movement is expected to be less than 1/2 inch to 1 inch.

New loads carried to existing foundations cannot be evaluated at this time since drawings are not available. Based on borings for this project, the left field fence, and the new right field sign, an assumption of similar conditions appears reasonable until actual observations can be made prior to, or shortly after, start of construction.

#### **Recommendations**

The following recommendations are presented for use in the design and construction of foundations and slabs for the project.

- 1. All topsoil, organic material, abandoned utilities, and other structures should be removed within and below the limits of the proposed grandstand and ancillary features.
- 2. Fill placed within the grandstand footprint beneath footings and floor slabs should consist of only compacted granular fill. Existing fill beneath new foundations should be removed to a depth of 8 feet below existing grade and replaced with compacted granular fill. Granular fill should extend ½ foot laterally beyond footing perimeter for each vertical foot of depth.
- 3. Granular fill should be a clean, well-graded sand meeting the following gradation requirements:

RWG&A Project No. 235-1043

### R. W. Gillespie & Associates, Inc.

Screen or Sieve Size	Bercent Passing
3 inches	100
No. 4	60-90
No. 40	20-60
No. 200	0-10

Note: Maximum particle size should be limited to 3 inches within two feet of foundation walls, footings, and floor slabs.

- 4. In open areas, granular fill should be placed in level, uniform lifts not exceeding 9 inches in uncompacted thickness and be compacted with self-propelled compaction equipment. In confined areas, structural fill should be placed in lifts not exceeding 6 inches in uncompacted thickness (note: maximum particle size 3 inches) and be compacted with hand-operated compaction equipment. Structural fill should be compacted to at least 92 percent of the maximum dry density as determined by ASTM D1557, Test Method for Laboratory Compaction Characteristics of Soil Using Modified Effort (56,000 ft-lbf/ft<sup>3</sup> (2,700 kN-m/m<sup>3</sup>)).
- 5. Site grading should provide positive drainage away from constructed facilities both during and after construction.
- 6. Dewatering requirements will vary at the site based on groundwater levels encountered during construction. In general, it should be practicable to accomplish construction dewatering from within excavations by open pumping methods. Surface runoff and infiltration of groundwater should be controlled so that excavation, filling, and foundation construction can be completed in the dry.
- 7. The proposed grandstand may be supported on spread and/or continuous footings bearing on compacted granular fill as described above. Footings should be designed for a maximum contact pressure of 2 kips per square foot. For footings with bearing areas having a least lateral dimension smaller than 3 feet, the allowable bearing pressure should be 1/3 of the above maximum times a footing's least lateral dimension in feet. Minimum footing width should be in accordance with concrete design and building codes, and not less than 2 feet.
- 8. Excavators equipped with smooth edged buckets should be used during footing excavation to minimize disturbance to naturally deposited soils. The exposed subgrade should be protected from further disturbance, moisture, and freezing until the footings are placed. Areas where fill is encountered should be over excavated to undisturbed soil and replaced

RWG&A Project No. 235-1043

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with compacted structural fill. Any soft areas or areas where moisture has accumulated should be excavated and replaced with compacted granular fill.

- 9. Exterior footings should be founded at least 4 feet below adjacent ground surface for frost protection. Footings at heated interior locations may be founded at a minimum depth of 18 inches below adjacent finished floor elevation. If the interior of the grandstand will be exposed to freezing temperatures for extended periods, either during construction or in service, the interior footings should be founded at the same depth as the exterior footings.
- 10. The integrity of natural soils and granular fill must be maintained during cold weather conditions. Footing and slab subgrades should not be allowed to freeze. The naturally deposited soils are considered frost-susceptible. Freezing of subgrade soils beneath footings and floor slabs may result in frost heaving and post-construction settlement. The Contractor should make every effort to prevent freezing of subgrade soils. In the event frost penetration occurs, granular fill or naturally deposited soils should be removed and replaced to the depth of the frozen soils.
- 11. The grandstand foundation should be designed to withstand lateral, uplift, and overturning forces due to earthquakes. In accordance with 2000 *International Building Code*<sup>®</sup>, the site is classified as Site Class D.
- 12. Lateral foundation loads from wind and earthquake may be resisted by friction between the bottom of footings and bearing subgrade, and lateral earth pressure against sides of footings. A friction coefficient of 0.35 is recommended for use in design.
- Ground floor slabs may be slab-on-grade construction bearing on a minimum 12 inch thickness of granular fill. Fill used to raise site grade beneath slabs should be granular fill. A subgrade modulus of 150 pounds per cubic inch should be used for design.
- 14. The Owner and Contractor should make themselves aware of and become familiar with applicable local, state, and federal safety regulations including the current OSHA Excavation and Trench Safety Standards. Construction site safety is considered the responsibility of the Contractor who is solely responsible for the means, methods, and sequencing of construction operations. Under no circumstances should the information provided below be interpreted to mean that RWG&A is assuming responsibility for construction site safety or the Contractor's activities; such responsibility is not being implied and should not be inferred.

RWG&A Project No. 235-1043

15. We anticipate footing excavations can generally be accomplished using sloped, open-cut techniques. Excavations in loose sands may require temporary shoring. Excavations that extend below groundwater are anticipated to require dewatering. We anticipate dewatering can be accomplished by the use of sumps and open pumping.

The Contractor should be aware that slope height, slope inclination, and excavation depths (including utility trench excavations) should in no case exceed those specified in local, state, or federal safety regulations, (e.g., OSHA Health and Safety Standards for Excavations, 29 CFR Part 1926) or successor regulations. Such regulations are strictly enforced and, if they are not followed, the Owner, Contractor, and/or earthwork and utility subcontractors could be liable for substantial penalties.

As a safety measure, it is recommended all vehicles and spoil piles be kept a minimum lateral distance from the crest of the slope equal to no less than 100 percent of the slope height. The exposed slope face should be protected against the elements.

RWG&A should be retained to monitor the soils exposed in all excavations and provide engineering services for such slopes. This will provide an opportunity to monitor for such types encountered and to modify the excavation slopes, as necessary. It also offers an opportunity to verify the stability of the excavation slopes during construction.

16. Since these geotechnical recommendations have been developed using limited numbers of observations and tests, the Owner should be sensitive to the potential need for adjustment in the field. It is in the best interest of the Owner to retain RWG&A to observe geotechnical construction aspects of the project, observe general compliance with the design concepts, specifications, and recommendations, and to assist in development of design changes should subsurface conditions differ from those anticipated prior to the start of construction. Such observation increases the likelihood of the design intent being carried out during construction and will allow RWG&A to confirm its design recommendations. Observations of and for existing foundations and foundation subgrade are of particular importance if they are subjected to new loads.

The Owner should recognize that unanticipated or changed conditions may be encountered during any earthworks construction. It is therefore recommended that the Owner retain RWG&A to observe construction. RWG&A would assign qualified personnel to observe and report on the quality of work performed by the earthwork contractor. Construction observation is a technique employed to minimize the risk of problems arising during construction. It is not insurance, nor does it constitute a warranty or guarantee of any type. In all cases, contractors retain responsibility for the quality of their work and for adhering to plans and specifications. Should RWG&A not be retained to perform observations

RWG&A Project No. 235-1043

during the full period of construction, RWG&A would not have had the opportunity to perform a complete service.

#### Closure

This report has been prepared for specific application to the proposed right field grandstand and concession stand at Hadlock Field in Portland, Maine, for the exclusive use of the Portland Seadogs and designated members of the design team. This work has been completed in accordance with generally accepted soil and foundation engineering practices. No other warranty, expressed or implied, is made. In the event any changes are made in the nature or location of the grandstand, the conclusions and recommendations of this report should be reviewed by RWG&A.

The recommendations presented are based on the results of widely spaced explorations. The nature of variations between the explorations may not become evident until construction. If variations are encountered, it will be necessary for RWG&A to re-evaluate the recommendations presented in this report. RWG&A requests an opportunity for a general review of the final design and specifications in order to determine that earthwork and foundation recommendations have been interpreted in the manner in which they were intended.

We have enjoyed serving you on this project, and if you have any questions or if we can be of further service, please contact us.

> Very truly yours, R. W. GILLESPIE & ASSOCIATES, INC.

Robert W. Gillespie, P.E.

RWG:ci In duplicate Copy:

> Brian E. Duffy, AIA, Brian E. Duffy Associates David K. Pinkham, P.E., Pinkham & Greer Consulting Engineers, Inc.

RWG&A Project No. 235-1043



### R. W. Gillespie & Associates, Inc.

#### APPENDIX

#### **TEST BORING LOGS**

Geotechnical Investigation New Right Grandstand and Concession Stand Hadlock Field Portland, Maine

RWG&A Project No. 235-1043

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Geotechnical Engineering. Geohydrology Materials Testing Services					
Project:Right Field Grandstand - Sea DogsELocation:Portland, MaineSurfaceClient:Portland Maine BaseballObserved WatProject No.235-1043Date C	ater De	epth:	_11/	B-1 9 05/	
Project No. 235-1043 DESCRIPTION OF MATERIAL	SAMPLE RECOVERY, IN.	BLOWS PER 6"	SPT-N BLOWS PER FT.	MOISTURE CONTENT %	LAB TESTS
<ul> <li>S-1 FILL; Silt, sand, clay, gravel, ash, wood.</li> </ul>	16	3 4 7 <u>6</u>	11		
S-2 	18	2 1 1 <u>1</u>	2		
S-3	10	1/12" 1 1	2		
S-4	2	6 5 3 2	8		
SILTY SANDY CLAY (CL); Mostly clay, some silt, little sand, stiff, wet, gray.	18	5 7 8 <u>12</u>	15		
S-6 SILTY SAND WITH GRAVEL (SM); Mostly coarse to fine grained sand, little silt, little fine gravel.	4	4 3 2	6		
- <sup>30</sup> - S-7	20	5 4 5 7	9		
S-8	18	7 9 11 <u>13</u>	20		

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Project: Location: Client: Project N	Rig Po Po 23	ght Field Grandstand - Sea Dogs rtland, Maine rtland Maine Baseball 5-1043	Bc Surface E Observed Wat Date Cc	er De	epth:		B-1 9 05/	
DEPTH, FT. SYMBOL	ER	DESCRIPTION OF MATERIAL		SAMPLE RECOVERY, IN.	BLOWS PER 6"	SPT-N BLOWS PER FT.	MOISTURE CONTENT %	LAB TESTS
40 -	S-9	Bottom of Exploration at 42'; Not refusal.		22	13 10 19 20	29		

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	5	R.V Geoter	V. Gillespie & Associates, Inc. chrical Engineering • Geohydrology • Materials Testing Services					
Proje Loca Clier Proje	tion	Pr	ortland Maine Basehall Observed	Boring face Eleva Water Do ate Completer	epth:		B-2 10 05/	
DEPTH, FT.		SAMPLES SAMPLE NUMBER	DESCRIPTION OF MATERIAL	SAMPLE RECOVERY, IN.	BLOWS PER 6"	SPT-N BLOWS PER FT.	MOISTURE CONTENT %	LAB TESTS
0		S-1	FILL; Silt, sand, clay, gravel, ash, wood.	20	5 7 6 5	13		
- 5 -		S-2		18	4 6 4 <u>3</u>	10		
- 10 <u>-</u>		S-3		12	WOH 1 1 1	2		
- 15 -		S-4	Some gravel, brick. SILTY CLAY (CL); Mostly clay, some silt, stiff, wet, gray.	2	5 2 1 <u>1</u>	3		
- 20 -		S-5		14	2 2 2 <u>2</u>	4		
- 25 -		S-6	Becomes sandy, fine gravel.	16	6 7 7 <u>8</u>	14		
30 -		S-7	SILTY SAND WITH GRAVEL (SM); Mostly sand, little silt, little grav medium dense, wet.	vel, 4	6 6 8 8	14		
- 35 -		S-8		10	6 8 14 <u>15</u>	22		

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Proje Loca Clier Proje	ect: ition: ht: ect N	Ri Po Po 0. 23	ght Field Grandstand - Sea Dogs ortland, Maine ortland Maine Baseball 5-1043	B Surface Observed Wa Date C	oring Eleva ter De omple	Log: tion: epth: eted:		B-2 10 05/	
DEPTH, FT.	SYMBOL	ER	DESCRIPTION OF MATERIAL		SAMPLE RECOVERY, IN.	BLOWS PER 6"	SPT-N BLOWS PER FT.	MOISTURE CONTENT %	LAB TESTS
	$\left  - \right $	{	Bottom of Exploration at 37'; Not refusal.	· · · · · · · · · · · · · · · · · · ·					
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Proje Loca Clier Proje	sti	on ·	Ri Po Po 23	ortland Maine Baseball Observed	Boring ace Eleva Water Do te Comple	epth:		B-3 05/	
DEPTH, FT.		SYMBOL	3ER	DESCRIPTION OF MATERIAL	SAMPLE RECOVERY, IN.	BLOWS PER 6"	SPT-N BLOWS PER FT.	MOISTURE CONTENT %	LAB TESTS
0			S-1	FILL; Silt, sand, clay, gravel, ash, wood.	10	3 4 6 7	10		
- 5			S-2		12	2 2 5 5	8		
- 10 -			S-3		NR				
- 15 -	11 to the second se		S-4	SILTY SANDY CLAY (CL); Mostly clay, some silt, some sand, trace shells, sand seams, stiff, wet, gray.	1	3 4 3 2	7		
- 20 -			S-5		22	2 3 4 <u>3</u>	7		
- 25 -			<b>S-</b> 6		24	1 1 2 2	3		
- 30 -			S-7		24	4 4 4 <u>10</u>	8		
- 35 -			S-8		24	5 3 18 <u>23</u>	21		

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				J. Gillespie & Associates, Inc. hnical Engineering+Geohydrology-Materials Testing Services						
Proje	ect:	~ 1	Rig Po	ght Field Grandstand - Sea Dogs rtland, Maine rtland Maine Baseball 5-1043	B Surface	oring Eleva	Log: tion:		B-3	
Clier	nt: ect	 N <u>o</u>	Po 23	rtland Maine Baseball 5-1043	Observed Wa Date C	ter De	epth:	11/	05/	
DEPTH, FT.		SAMPLES	ER	DESCRIPTION OF MATERIAL		SAMPLE RECOVERY, IN.	BLOWS PER 6"	SPT-N BLOWS PER FT.	MOISTURE CONTENT %	LAB TESTS
	+	Ħ		Bottom of Exploration at 37'; Not refusal.						
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From:	Rick Knowland
То:	Jeanie Bourke
Date:	10/4/2007 9:35:45 AM
Subject:	Fwd: Hadlock Field

Jeanie, fyi.

>>> Rick Knowland Thursday, October 04, 2007 >>> Bob, Here are the results of yesterday's staff review meeting.

1. Clearly label the water line.

2. On the site plan it states "MAX. COVERAGE (SPORTS COMPLEX) 75%". Unfortunately the plan doesn't state what the maximum impervious surface on the site is which raises a question of whether the project meets the standard. Would suggest you take the note off the plan and sent us an email indicating that the site covers an existing impervous area. Somewhere in the files we have prior documentation on this but this is a new project.

3. Need a plan of the entire site (or could be a decent aerial) showing where the locker room building footprint is on the site. GIS or maybe Leslie Kaynor could help you with this. This is important because your site plan shows only one tiny part of the site. I will not hold up your building permit on this but we need it asap for the record.

4. Can your architect stamp the site plan he prepared?

5. On the exterior building material your email response to my email didn't address the issue.

Bob, That is the latest on our review.



### R. W. Gillespie & Associates, Inc.

Geotechnical Engineering • Geohydrology • Materials Testing Services

02 October 2007

Mr. Robert Leeman City of Portland 389 Congress Street Portland, ME 04101

Subject: Response to Mike Nugent Email Sea Dogs Club House - Hadlock Field Portland, Maine RWG&A Project No. 557-10

Dear Mr. Leeman:

As requested, we have reviewed the comments of Mike Nugent, Code Enforcement Officer for Portland; a response is provided in subsequent paragraphs.

Briefly, Mr. Nugent reviewed our letter of 27 September 2007 and questioned whether or not the additional loads were "acceptable." Although this is more a structural than a geotechnical issue, the structural engineer would look to RWG&A for guidance in terms of existing earth pressures and external load distributions, as well as new loads.

The pipe is a brick arch reportedly set on a concrete slab and is on the order of 75 to 100 years old. Review of aerial photographs and tax maps show that pipe is overlain by a number of structures besides the right field grandstands at Hadlock Field. To our knowledge, none have resulted in damage to the pipe.

The loads presented in our 27 September 2007 letter are approximately the same as those calculated for the grandstands at column location C-1. The additional loads are estimated to be a 5 to 10 percent increase relative to existing stresses and, in view of pipe performance to date, are not expected to adversely impact the pipe from a geotechnical standpoint.

We trust the foregoing meets your present needs, and if you have any questions, please contact us.

EOFMAN Very truly yours ROW GILLESPIE Gillespie, PH GILLESPIE Robert W. 4975 RWG:ci G:\PROJECTS\0500\0557\557-010\Corresp\2007-10-02 Response.wpd .02-07 SONA Corporate Office 86 Industrial Park Rd., Ste 4 200 International Dr., Ste 170 P.O. Box 289 Portsmouth, NH 03801 Saco, ME 04072 Augusta, ME 04344 603-427-0244 • Fax 603-430-2041 207-286-8008 • Fax 207-286-2882 207-623-4914 • Fax 207-623-3429 www.rwgillespic.com

From:Bob LeemanTo:Jeanie Bourke; MIke NugentDate:10/1/2007 2:26:29 PMSubject:Hadlock Club House

It is my understanding that inspections is requesting that we provide documentation that the new clubhouse footings and load will not affect the underground brick sewer. The proposed footings will be a distance of eight feet from the edge of the sewer and will have minimal or no impact on the existing sewer line. According to the report generated by R. W. Gillespie the increase in vertical pressure is estimated to be 50 to 55 pounds per square foot, the horizontal pressure is estimated to be 20 to 25 pounds per square foot.

Given this information and the fact that the strength or condition of the 100 year old sewer is unknown it would be impossible to make a positive determination of the effects this structure may have on the sewer.

Given the fact that the City of Portland is the owner of both the new structure and the brick sewer we would like to move forward with this project.

Please use this for approval so the project may move forward.

Robert Leeman Public Buildings Director City of Portland

CC: Anita LaChance; Joe Gray

From:	Rick Knowland
То:	Schmuckal, Marge
Date:	9/25/2007 4:07:11 PM
Subject:	Re: Haddlock Field

Marge, Whenever I see an email with the letters H-a-d-l-o-c-k it immediately gets my reaction. The answer to your question is that they have not submitted a site plan.

>>> Marge Schmuckal Tuesday, September 25, 2007 >>> Rick,

Have we received a good site plan yet for this application? I would like to get rid of this building permit if we can.

Thanks, Marge

### CITY OF PORTLAND, MAINE DEVELOPMENT REVIEW APPLICATION PLANNING DEPARTMENT PROCESSING FORM

2007-0157

		Zoning Copy A	Application I. D. Number
City Of Portland		g	0/5/2007
Applicant	· · · · · · · · · · · · · · · · · · ·	—	Application Date
389 Congress St, Portland, ME 04101		S	Sea Dogs Clubhouse
Applicant's Mailing Address			Project Name/Description
		165 - 165 Park Ave, Portland, M	
Consultant/Agent		Address of Proposed Site	
Applicant Ph: (207) 233-0350 Agent		049 A001001	
Applicant or Agent Daytime Telephone, Fa		Assessor's Reference: Chart-Bloc	k-Lot
Proposed Development (check all that app	ly): 🗌 New Building 🔽	Building Addition Change Of Use	Residential 🗌 Office 🔲 Retail
Manufacturing Warehouse/Distri	bution 🔄 Parking Lot	Apt 0 Condo 0 Other (spe	ecify)
			R5
Proposed Building square Feet or # of Unit	s Acre	eage of Site	Zoning
Check Review Required:			
Site Plan (major/minor)	Zoning Conditional - PB	Subdivision # of lots	
Amendment to Plan - Board Review	Zoning Conditional - ZB	A Shoreland Historic Preserva	ation 🔲 DEP Local Certification
Amendment to Plan - Staff Review	-	Zoning Variance  Flood Hazard	Site Location
After the Fact - Major		Stormwater Traffic Movemer	
After the Fact - Minor			
		PAD Review 14-403 Streets F	(eview
Fees Paid:Site Plan\$400.00	Subdivision	Engineer Review	Date 9/6/2007
Zoning Approval Status:		Reviewer MAN	S-QMAP.
• • •	A	- Contract	
Approved	Approved w/Conditions See Attached	5 Denied	
	See Allached		
Approval Date	Approval Expiration	Extension to	Additional Sheets
	·		Attached
Condition Compliance	signature	date	
Performance Guarantee	Required*	Not Required	
* No building permit may be issued until a p	performance guarantee ha	s been submitted as indicated below	
Performance Guarantee Accepted		DEPT. OF LODGE SET DATE TO M	
	date	CITY COMPANY amount OF	expiration date
Inspection Fee Paid			
·	date	SEP - 6 ameunt	
Building Permit Issue			
	date		
Performance Guarantee Reduced		RECENSO	
	date	remaining balance	signature
Temporary Certificate of Occupancy		-	
,,,,,,,		Conditions (See Attached)	
	date	Conditions (See Attached)	expiration date
Final Inspection	date	Conditions (See Attached)	
Final Inspection	date		
		Conditions (See Attached)	
Final Inspection Certificate Of Occupancy	date		
Certificate Of Occupancy			
	date date	signature	
Certificate Of Occupancy Performance Guarantee Released	date		
Certificate Of Occupancy	date date date	signature	expiration date
Certificate Of Occupancy Performance Guarantee Released	date date	signature	



# City of Portland Site Plan Application

If you or the property owner owes real estate taxes, personal property taxes or user charges on any property within the City, payment arrangements must be made before permit applications can be received by the Inspections Division.

Address of Proposed Development: 165 Park Ave Zone: RS				
Existing Building Size: 1,134 sq. ft. Proposed Building Size: 9,89/ sq. ft.				
Existing Acreage of Site: sq. :	ft. P	ropsoed Acreage of Site	:	sq. ft.
Tax Assessor's Chart, Block & Lot:Chart#Block#Lot#049A001001	City of Portland		Telephone #: 233-0350	
Consultant/Agent, mailing address, phone # & contact person:	Applicant's nam telephone #/Fa Bo5 Lee 233-0	eman		Project name: Sea bog Club house
Fee For Service Deposit (all applications) (\$200.00)				
Proposed Development (check all that apply)				
Minor Site Plan Review <u>U</u> Less than 10,000 sq. ft. (\$400.00) <u>After-the-fact Review (\$1,000.00 + applicable application fee)</u>				
Plan Amendments				
Planning Staff Review (\$250.00)				
Planning Board Review (\$500.00)		~ Plea	se see ne	ext page ∼

Who billing will be sent to: (Company, Contact Person, Address, Phone #)

Bob Leemon City of Portland 388 Consress st Portland ME 04/01

Phone 874-8892 Cell 233-0350

Submittals shall include (7) separate folded packets of the following:

- a. copy of application
- b. cover letter stating the nature of the project
- c. site plan containing the information found in the attached sample plans checklist
- d. 1 set of 11 x 17 plans

Amendment to Plans: Amendment applications should include 6 separate packets of the above (a, b, & c) ALL PLANS MUST BE FOLDED NEATLY AND IN PACKET FORM

Section 14-522 of the Zoning Ordinance outlines the process which is available on our web site: portlandmaine.gov

I hereby certify that I am the Owner of record of the named property, or that the owner of record authorizes the proposed work and that I have been authorized by the owner to make this application as his/her authorized agent. I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in this application is issued, I certify that the Code Official's authorized representative shall have the authority to enter all areas covered by this permit at any reasonable hour to enforce the provisions of the codes applicable to this permit.

Signature of applicant: Date:

This application is for site review ONLY; a building Permit application and associated fees will be required prior to construction.

#### Sea Dog Clubhouse

This project is to construct a new clubhouse for the Portland Sea Dogs. The building will be located beneath the existing Pavilion seating area. The purpose of this structure is to be used only as a team clubhouse for the tenants at Hadlock Field.

The foot print of this building is 5,610 square feet and a large portion of this will be within the foot print of the Pavilion seating area.

There is an existing fire hydrant within 500 feet of the proposed structure.

There is an existing sewer main within 20 feet of the proposed building.

There is an existing water main within 30 feet of the proposed building.

There will no drainage problems with this location, the City of Portland added a number of storm drains to this location two years ago.

This project is scheduled to be completed by April 1, 2008

We have receive approval from the State Fire Marshall.

We have submitted for a building permit with the City of Portland and hope to receive it so we can start construction on September 17, 2007.

This is a City project as is being sponsored by the Portland Sea Dogs. Because it is a City project it will be tax exempt and become property of the City when complete.

TD:0740473



PO BOX 301 FREEPORT, ME 04032



Building Locat 

From:	Marge Schmuckal
То:	RICK KNOWLAND
Date:	9/25/2007 4:01:45 PM
Subject:	Haddlock Field

Rick,

Have we received a good site plan yet for this application? I would like to get rid of this building permit if we can.

Thanks, Marge

CC: Jeanie Bourke