Part II Division 0

Supplemental Information

SECTION 00 30 00

INFORMATION AVAILABLE TO BIDDERS

- 1.1 SW COLE GEO TECHNICAL REPORT
- 1.2 SW COLE BORING LOG
- 1.3 **IBC AND NFPA CODE ANLYSIS**
- ... END OF SECTION

PROPOSED BUILDING RENOVATIONS 645 CONGRESS STREET PORTLAND, MAINE

09-0077 March 30, 2009

Prepared for:

Bayside Maine, LLC c/o Greg Shinberg Consulting, LLC 477 Congress Street, Suite 1012 Portland, ME 04101

Prepared by:



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• Geotechnical Engineering • Field & Lab Testing • Scientific & Environmental Consulting

09-0077

March 30, 2009

Bayside Maine, LLC c/o Shinberg Consulting, LLC Attn: Greg Shinberg 477 Congress Street, Suite 1012 Portland, ME 04101

Subject:

Geotechnical Engineering Services

Proposed Building Renovations

645 Congress Street Portland, Maine

Dear Greg:

In accordance with our Proposal and Addendum No. 1, dated February 18, 2009, we have made a subsurface investigation for the proposed renovations for the 5 and 6 story building located at 645 Congress Street in Portland, Maine. This report presents our findings and recommendations relative to seismic soil site class related to the proposed building renovations. The contents of this report are subject to the limitations set forth in Attachment A.

1.0 INTRODUCTION

1.1 Scope and Purpose

The purpose of our geotechnical work was to obtain subsurface soils information in order to assess the seismic soil Site Class according to 2006 IBC (N-value method) relative to the planned building renovations at 645 Congress Street. Our scope of work included three test boring explorations, soils laboratory testing, a geotechnical analysis of the subsurface findings relative to seismic soil site class, and preparation of this report.

1.1.1 Additional Scope and Purpose

The purpose of our additional geotechnical work was to obtain preliminary subsurface soils information in areas of proposed future buildings on the site. As requested, our additional scope included four test borings at locations selected by the Client, soils



laboratory testing, preparation of logs and a plan showing the approximate locations of the test borings, which are attached as Sheet 1 and Appendix A.

1.2 Proposed Construction

Based on information provided by Shinberg Consulting, we understand renovation plans call for demolition of three, 3 story buildings on the site and renovation of the existing 5 and 6 story building along Congress Street. We understand structural framing of the existing building will be altered and modified as part of the renovation work. Proposed and existing site features are shown on the "Exploration Location Plan" attached as Sheet 1.

2.0 EXPLORATION AND TESTING

2.1 Exploration

Three test boring explorations (B-1, B-2 and B-3) were made at the site on March 24, 2009. The test borings were drilled by Northern Test Boring of Gorham, Maine working under subcontract to S.W.COLE ENGINEERING, INC. The exploration locations were selected and established in the field by S.W.COLE ENGINEERING, INC using taped measurements from existing site features. The approximate exploration locations are shown on the "Exploration Location Plan" attached as Sheet 1. Logs of the test borings are attached as Sheets 2 through 4. Logs of rock core obtained at B-1 through B-3 are attached as Sheets 5 through 7. A key to the notes and symbols used on the logs is attached as Sheet 8.

2.2 Testing

Soil samples obtained from the explorations were returned to our laboratory for further classification and testing. The results of moisture content tests are shown on the logs. The results of one gradation test are attached as Sheet 9.

2.3 Additional Exploration and Testing

According to our Proposal Addendum No. 1, dated February18, 2009, we also completed four additional test borings (B-101 through B-104) in areas of proposed future buildings on the site. The locations of these borings were selected by the Client and are shown on Sheet 1. Logs and laboratory test results for these additional borings are attached in Appendix A.



3.0 SITE AND SUBSURFACE CONDITIONS

3.1 Surficial Conditions

The site is located at 645 Congress Street in Portland, Maine. The site is relatively flat and slopes gently downward from Congress Street at about elevation 116 feet (project datum) to elevation 107 along Deering Street with about 5 feet of topographic relief from the front of the building area to the rear. The existing buildings to be renovated are attached 5 and 6 story structures with a basement level extending about 10 feet below the elevation of Congress Street.

3.2 Subsurface Conditions

Below a surficial layer of asphalt, test borings B-1 through B-3 encountered uncontrolled fill and backfill to depths of 6 to 12 feet overlying relic foundations and slabs overlying native glacial till that was dense where undisturbed. These test boring encountered bedrock at depths varying from about 11 to 17 feet below the ground surface (approximately 6 to 7 feet below basement slab level of the existing buildings to be renovated). Please refer to the attached logs for more detailed descriptions of the subsurface findings.

3.3 Groundwater Conditions

Groundwater was observed at depths varying from about 5 to 12 feet below the ground surface. Actual groundwater levels were not determined due to the short time-frame which the explorations remained open. In general, it should be anticipated that seasonal groundwater levels will fluctuate, especially during times of snowmelt and heavy precipitation.

4.0 EVALUATION AND RECOMMENDATIONS

4.1 Seismic Soil Site Class

Based on the subsurface findings, the elevation of the basement level and foundations, and our understanding of the proposed renovations, we interpret the existing building is likely founded on bedrock or less than 10 feet of stable, dense soils overlying sound bedrock. Therefore, we recommend that structural analysis for renovation of the existing 5 and 6 story building consider a seismic soil Site Class B according to IBC 2006.



4.2 Design Review and Construction Testing

S. W. COLE ENGINEERING, INC. should be retained to review the foundation design and specifications to determine that our earthwork recommendations have been properly interpreted and implemented.

A soils and concrete testing program should also be implemented during construction to observe compliance with the design concepts, plans, and specifications. S. W. COLE ENGINEERING, INC. is available to provide field and laboratory testing services for soil, concrete, steel, masonry, spray-applied fire-proofing, and asphalt construction materials.

5.0 CLOSURE

It has been a pleasure to be of assistance to you with this phase of your project. We look forward to working with you as the design progresses and during the construction phase of this project.

Sincerely,

S. W. COLE ENGINEERING, INC.

Timothy J. Boyce, P. E. Senior Geotechnical Engineer

•

TJB:pfk

P:\2009\09-0077 S_Bayside Maine, LLC_Portland, ME_645 Congress Street Renovations_TJB\Reports and Letters\09-0077 Report.doc

Attachment A - Limitations

This report has been prepared for the exclusive use of Bayside Maine, LLC for specific application to the proposed renovation of the existing 5 and 6 story buildings at 645 Congress Street in Portland, Maine. S.W.COLE ENGINEERING, INC. has endeavored to conduct the work in accordance with generally accepted soil and foundation engineering practices. No warranty, expressed or implied, is made.

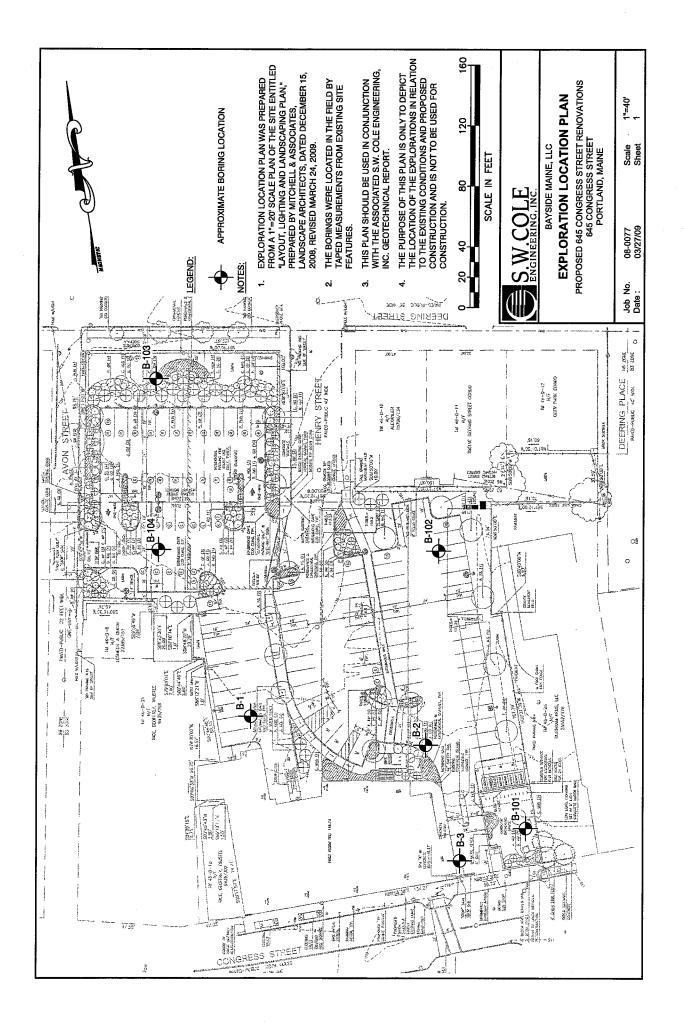
The soil profiles described in the report are intended to convey general trends in subsurface conditions. The boundaries between strata are approximate and are based upon interpretation of exploration data and samples.

The analyses performed during this investigation and recommendations presented in this report are based in part upon the data obtained from subsurface explorations made at the site. Variations in subsurface conditions may occur between explorations and may not become evident until construction. If variations in subsurface conditions become evident after submission of this report, it will be necessary to evaluate their nature and to review the recommendations of this report.

Observations have been made during exploration work to assess site groundwater levels. Fluctuations in water levels will occur due to variations in rainfall, temperature, and other factors.

S. W. COLE ENGINEERING, INC.'s scope of work has not included the investigation, detection, or prevention of any Biological Pollutants at the project site or in any existing or proposed structure at the site. The term "Biological Pollutants" includes, but is not limited to, molds, fungi, spores, bacteria, and viruses, and the byproducts of any such biological organisms.

Recommendations contained in this report are based substantially upon information provided by others regarding the proposed project. In the event that any changes are made in the design, nature, or location of the proposed project, S. W. COLE ENGINEERING, INC. should review such changes as they relate to analyses associated with this report. Recommendations contained in this report shall not be considered valid unless the changes are reviewed by S. W. COLE ENGINEERING, INC.





MIKE NADEAU

BORING NO.:

B-1

SHEET:

1 OF 1 09-0077

PROJECT NO .: DATE START:

3/24/2009 3/24/2009

DATE FINISH: **ELEVATION:**

111' +/-

TYPE

LABORATORY TEST

PROJECT / CLIENT: CONGRESS STREET BUILDING RENOVATIONS / BAYSIDE MAINE, LLC

645 CONGRESS STREET PORTLAND, MAINE

4"

2 3/8"

2"

NORTHERN TEST BORING, INC.

SWC REP.:

K. GIMPEL

CASING:

LOCATION:

DRILLING CO.:

SIZE I.D. HAMMER WT. HAMMER FALL

WATER LEVEL INFORMATION

SAMPLER:

U = 3.5" SHELBY TUBE

CORE BARREL:

HW SS

NQ

140-lb 30"

DRILLER:

WATER AT 8.8' +/-

BORING NO.:

CASING SAMPLER BLOWS PER 6" SAMPLE BLOWS DEPTH STRATA & TEST DATA PER DEPTH PEN. REC. 12-18 18-24 FOOT @ BOT 2 1/4" 0.1 1 1/2-INCHES ASPHALT PAVEMENT **HSA** 1D 24" 14" 2.5' 7 9 8 7 BROWN GRAVELLY SAND SOME SILT (FILL) TO 10' ~MEDIUM DENSE~ 3.5 **BROWN SILTY SAND TRACE GRAVEL** 24" SET 2D 15" 7.0' 4 3 4 3 WITH TRACE ASPHALT DEBRIS (FILL) CASING ~LOOSE~ TO 10' 3D 2" 2" 10.2' 50/2" 10.2 (DETECTABLE PETROLEUM ODOR IN 3D) CORE 11.0 PROBABLE CONCRETE OVERLYING BEDROCK (ADVANCED BY ROLLER CONE) (SEE ROCK CORE LOG) 54" 40" 1R 15.5' 15.5 **RQD = 30 % BOTTOM OF EXPLORATION AT 15.5'** SAMPLES: SOIL CLASSIFIED BY: REMARKS: D = SPLIT SPOON DRILLER - VISUALLY STRATIFICATION LINES REPRESENT THE C = 3" SHELBY TUBE SOIL TECH. - VISUALLY APPROXIMATE BOUNDARY BETWEEN SOIL TYPES

AND THE TRANSITION MAY BE GRADUAL.



MIKE NADEAU

DRILLER:

30"

B-2 BORING NO.: SHEET: 1 OF 1

PROJECT NO.: 09-0077

DATE START: 3/24/2009 DATE FINISH: 3/24/2009

ELEVATION:

111' +/-

TYPE SIZE I.D. HAMMER WT. HAMMER FALL

140-lb

PROJECT / CLIENT: CONGRESS STREET BUILDING RENOVATIONS / BAYSIDE MAINE, LLC

645 CONGRESS STREET PORTLAND, MAINE

2 3/8"

2"

SWC REP.: K. GIMPEL

CASING:

LOCATION:

DRILLING CO.:

SS

NQ

WATER LEVEL INFORMATION

SAMPLER: CORE BARREL: HW 4"

NORTHERN TEST BORING, INC.

NO FREE WATER OBSERVED WHILE DRILLING

WATER AT 4.5' +/- AFTER CASING WAS PULLED

CASING BLOWS PER		in one was come	иPLE I	DEPTH	100	PLER BI	i de la companion de la compan		DEPTH	STRATA & TEST DATA
FOOT	NO.	PEŅ.	REC.	@ BOT	0-6	6-12	12-18	18-24		
SSA									0.1'	1 -INCH ASPHALT PAVEMENT
TO 10'	1D	24"	14"	2.5'	6	6	7	4		BROWN SILTY SAND SOME GRAVEL TRACE BRICK (FILL) ~LOOSE TO MEDIUM DENSE~
									6.0'	· · · · · · · · · · · · · · · · · · ·
	2D	24"	17"	7.0'	14	7	7	6	7.5	BROWN SILT AND SAND
SET		<u> </u>							7.5'	TRACE GRAVEL TRACE BRICK (FILL / REWORKED NATIVE SOILS)
CASING										BROWN SILT AND SAND TRACE GRAVEL BECOMING
TO 12.6				-						Brown old rate of the Brown of the Brown old rate of the Brown old
₩	3D	24"	24"	12.0'	17	22	31	41		w = 14.6 %BROWN SAND AND SILT SOME GRAVEL (TILL)
CORE									12.6'	~DENSE~
										(SEE ROCK CORE LOG)
+	1R	56"	44"	17.2'					17.2'	RQD - 46 %
				17.2					17.2	BOTTOM OF EXPLORATION AT 17.2'
SAMPLE	S:	I		SOIL C	LASSIF	IED BY	·:		REMARK	SS:
D = SPL C = 3" SI U = 3.5"	HELBY	TUBE	■	X	SOIL	LER - \ TECH. ORATO	VISU	ALLY	A	STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY BETWEEN SOIL TYPES AND THE TRANSITION MAY BE GRADUAL. BORING NO.: B-2



MIKE NADEAU

BORING NO.:

B-3

SHEET:

1 OF 1

PROJECT NO.:

09-0077 3/24/2009

DATE START: DATE FINISH:

3/24/2009

ELEVATION:

116' +/-

TYPE SIZE I.D. HAMMER WT. HAMMER FALL

140-lb

PROJECT / CLIENT: CONGRESS STREET BUILDING RENOVATIONS / BAYSIDE MAINE, LLC

645 CONGRESS STREET PORTLAND, MAINE

2"

CASING:

LOCATION:

DRILLING CO.:

NQ

K. GIMPEL SWC REP.: WATER LEVEL INFORMATION

WATER AT 12' +/-

SAMPLER: CORE BARREL: HW 4" SS 2 3/8"

NORTHERN TEST BORING, INC.

30"

DRILLER:

CASING BLOWS		SAM	MPLE :		SAMPLER BLOWS PER 6"			PER 6"	DEPTH	STRATA & TEST DATA
PER FOOT	NO.	PEN.	REC.	DEPTH @ BOT	0-6	6-12	12-18	18-24		
SSA									0.2'	3-INCHES ASPHALT PAVEMENT
TO 10'	1D	24"	16"	2.5'	11	11	10	9	0.5'	BROWN GRAVELLY SAND TRACE SILT (FILL)
										BROWN SILTY SAND (FILL)
	2D	24"	19"	7.0'	2	2	2	2		~LOOSE~
									9.8'	
CASING										BLACK SILTY SAND
TO 15'	3D	24"	22"	12.0'	2	2	7	5	12.1'	WITH WOOD AND BRICK (FILL)
									12.6'	RELIC CONCRETE SLAB
								-	16.0'	PROBABLE LOOSE FILL OR SATURATED SANDY TILL
-	4D	19"	17"	16.7'	WOH	/ 12"	6	50/2"		GRAY GRAVELLY SILT AND SAND (TILL)
CORE									17.2'	~LOOSE~
										(SEE ROCK CORE LOG)
▼	1R	50"	42"	21.4'			-		21.4'	RQD = 30 %
										BOTTOM OF EXPLORATION AT 21.4'
		,								
										·
SAMPLE		ON	ı	SOIL CI				.,	REMAR	
D = SPL C = 3" S U = 3.5"	HELBY	TUBE	_	X	SOIL	. TECH	VISUAL VISU	ALLY		STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY BETWEEN SOIL TYPES AND THE TRANSITION MAY BE CRADUAL
0 - 3.5	SHELE	100	- L		LAB	JRAIO	RY TES	, I		AND THE TRANSITION MAY BE GRADUAL. BORING NO.: B-3



PROJECT: 64	45 CONGRESS STREET BUILDING RENOV	/ATIONS	BORING NO.: _		
CLIENT: B	AYSIDE MAINE, LLC		PROJECT NO.:	09-0077	
LOGGED BY: K.	GIMPEL	DATE: 3/25/2009	SHEET NO.:	1 OF 1	
CHECKED BY: G		DATE: 3/26/2009	CORE SIZE:	NQ2	
• <u> </u>	. BOOKEN	DATE. 5/20/2009	CORE SIZE.	INQZ	

	KED BY:	G. BUC	KLIN				DATE: 3/26/2009 CORE SIZE: NQ	12
DEPTH BELOW SURFACE (FT)	CORE RUN	CORE INTERVAL (FT)	CORE RECOVERY (FT)	RQD (%)	ROCK QUALITY	GRAPHIC LOG	ROCK DESCRIPTION AND IDENTIFICATION	
							(SEE SHEET 2 FOR SOILS DESCRIPTION) BEGINNING OF CORE RUN AT 11.0'	
15	1R	11.0' 54"	40"	16/54	POOR		GRAY PELITE / PHYLLITE MODERATELY WEATHERED FINE GRAINED TEXTURE MODERATELY HARD THIN CALCITE BANDS PARALLEL TO FOLIATION (85°) 30° SLIGHT IRON OXIDE STAINING ON FRACTURE SURFACES 85° ZONE OF NO RECOVERY BOTTOM OF EXPLORATION AT 15.5'	
						րակավարկակակակակակակակակակակակակակա		5



PROJECT: 645 CONGRESS STREET BUILDING REN	OVATIONS	BORING NO.:_	B-2
CLIENT: BAYSIDE MAINE, LLC		PROJECT NO.:	09-0077
LOGGED BY: K. GIMPEL	DATE: 3/25/2009	SHEET NO.:	1 OF 1
CHECKED BY: G. BUCKLIN	DATE: 3/26/2009	CORE SIZE:	NQ2

CHEC	KED BY:	G. BUC	KLIN				DATE: 3/26/2009	CORE SIZE:	NQ2
DEPTH BELOW SURFACE (FT)	CORE RUN	CORE INTERVAL (FT)	CORE RECOVERY (FT)	RQD (%)	ROCK QUALITY	GRAPHIC LOG	ROCK DESCRIPTION AND IDE	NTIFICATION	
11 11							(SEE SHEET 3 FOR SOILS DESC		
15	1R	12.6' 56"	44"	26/56 46%	POOR		GRAY PELITE MODERATELY WEATHERED 40° FINE GRAINED TEXTURE CALCITE BANDS ABUNDANT MODERATELY HARD SOME PITTING ON FRACTURE SURFACES 40° ZONE OF NO RECOVERY	T 12.6'	
						ուսիավագետիակահայտուհակավավարևորուի	BOTTOM OF EXPLORATION A	T 17.2'	6



PROJECT: 645 CONGRESS STREET BUILDIN	G RENOVATIONS	BORING NO.:	B-3	
CLIENT: BAYSIDE MAINE, LLC		PROJECT NO.:	09-0077	
LOGGED BY: K, GIMPEL	DATE: 3/25/2009	SHEET NO.:	1 OF 1	
CHECKED BY: G. BUCKLIN	DATE: 3/26/2009	CORE SIZE:	NQ2	-

CHEC	KED BY:	G. BUC	KLIN				DATE: 3/26/2009 CORE SIZE: NQ2
DEPTH BELOW SURFACE (FT)	CORE RUN	CORE INTERVAL (FT)	CORE RECOVERY (FT)	RQD (%)	ROCK QUALITY	GRAPHIC LOG	ROCK DESCRIPTION AND IDENTIFICATION
15							(SEE SHEET 4 FOR SOILS DESCRIPTION)
20 =	1R	17.2' 50"	42"	15/50 30%	POOR		GRAY PELITE MODERATELY TO SEVERELY WEATHERED 50° FINE GRAINED TEXTURE CALCITE BANDS ABUNDANT 65° MODERATELY HARD SOME PITTING ON ROCK SURFACES HIGHLY FRACTURED ZONE 20° ZONE OF NO RECOVERY
						<u>ավուդուդուդուդուդուդուդուդուդուդուդուդուդո</u>	BOTTOM OF EXPLORATION AT 21.4'



KEY TO THE NOTES & SYMBOLS Test Boring and Test Pit Explorations

All stratification lines represent the approximate boundary between soil types and the transition may be gradual.

Key to Symbols Used:

w - water content, percent (dry weight basis)

qu - unconfined compressive strength, kips/sq. ft. - based on laboratory unconfined

compressive test

S_v - field vane shear strength, kips/sq. ft. L_v - lab vane shear strength, kips/sq. ft.

q_o - unconfined compressive strength, kips/sq. ft. based on pocket

penetrometer test

O - organic content, percent (dry weight basis)

W_L - liquid limit - Atterberg test
 W_P - plastic limit - Atterberg test
 WOH - advance by weight of hammer
 WOM - advance by weight of man
 WOR - advance by weight of rods

HYD - advance by force of hydraulic piston on drill

RQD - Rock Quality Designator - an index of the quality of a rock mass. RQD is computed

from recovered core samples.

 $\begin{array}{cccc} \gamma_T & \text{-} & \text{total soil weight} \\ \gamma_B & \text{-} & \text{buoyant soil weight} \end{array}$

Description of Proportions:

0 to 5% TRACE 5 to 12% SOME 12 to 35% "Y" 35+% AND

REFUSAL: Test Boring Explorations - Refusal depth indicates that depth at which, in the drill foreman's opinion, sufficient resistance to the advance of the casing, auger, probe rod or sampler was encountered to render further advance impossible or impracticable by the procedures and equipment being used.

REFUSAL: Test Pit Explorations - Refusal depth indicates that depth at which sufficient resistance to the advance of the backhoe bucket was encountered to render further advance impossible or impracticable by the procedures and equipment being used.

Although refusal may indicate the encountering of the bedrock surface, it may indicate the striking of large cobbles, boulders, very dense or cemented soil, or other buried natural or man-made objects or it may indicate the encountering of a harder zone after penetrating a considerable depth through a weathered or disintegrated zone of the bedrock.



Report of Gradation

ASTM C-117 & C-136

Project Number 09-0077

Project Name

Client

PORTLAND ME - 645 CONGRESS STREET RENOVATIONS -

GEOTECHNICAL ENGINEERING SERVICES

Lab ID

10431G

Date Received

3/26/2009

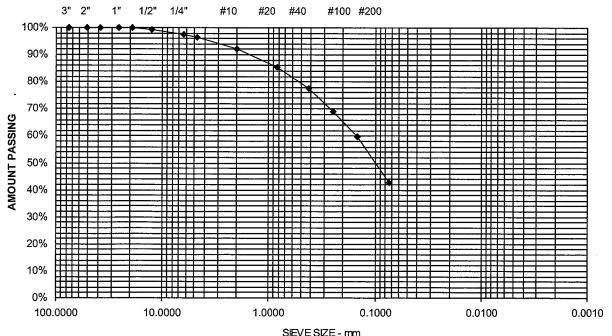
Date Complete 3/27/2009

Tested By

JUSTIN BISSON

Material Source B-2, 3D (10' - 12')

STANDARD DESIGNATION (mm/µm)	SIEVE SIZE	AMOUNT PASSING (%	1
150 mm	6"	100	
125 mm	5"	100	
100 mm	4"	100	
75 mm	3"	100	
50 mm	3 2"	100	
38.1 mm	1-1/2"	100	
25.0 mm	1"	100	
19.0 mm	3/4"	100	
12.5 mm	1/2"	99	
6.3 mm	1/4"	98	
4.75 mm	No. 4	97	3.4% Gravel
2.00 mm	No. 10	92	
850 um	No. 20	86	
425 um	No. 40	77	54% Sand
250 um	No. 60	69	
150 um	No. 100	60	
75 um	No. 200	42.6	42.6% Fines



APPENDIX A



BORING NO .:

B-101

SHEET:

1 OF 1

ENC	GINEERI	NG,INC	•			PROJECT NO.:	09-0077
PROJECT / CLIENT:	PROPOSED	NEW BUILDIN	IGS / BAYSIDE I	MAINE, LLC		DATE START:	3/25/2009
LOCATION:	645 CONGRI	ESS STREET	PORTLAND, MA	INE		DATE FINISH:	3/25/2009
DRILLING CO.:	NORTHERN	TEST BORING	9, INC.	DRILLER:	MIKE NADEAU	ELEVATION:	116' +/-
	TYPE	SIZE I.D.	HAMMER WT.	HAMMER FALL		SWC REP.:	K. GIMPEL
CASING:	N/A					WATER LEVEL INFORM	MATION
SAMPLER:	SS	2 3/8"	140-lb	30"		WATER AT 12' +/	<i>_</i>
CORE BARREL:	N/A		****				

CASING BLOWS		SAI	MPLE .		SAM	PLER BI	LOWS F	′⊏K 6" '	DEPTH	STRATA & TEST DATA
PER FOOT	NO.	PEN.	REC.	DEPTH @ BOT	0-6	6-12	12-18	18-24		
SSA		1							0.1'	3/4 -INCH ASPHALT PAVEMENT
0 15.6	1D	24"	17"	2.5'	2	3	4	7	0.3'	
t										BROWN SAND TRACE SILT TRANSITIONING TO
	2D	24"	18"	7.0'	2	3	3	3	- - -	BROWN SILTY SAND (FILL) ~LOOSE~
										L005E
	3D	24"	22"	12.0'	3	3	3	1	12.1'	
							<u> </u>	<u> </u>	12.6	RELIC CONCRETE SLAB
1									13.0	PROBABLE GRANULAR FILL
-				<u> </u>					1 10.01	GRAY SAND AND SILT SOME GRAVEL (TILL)
\downarrow	4D	6"	6"	15.5'	18	25/0"			15.6'	~DENSE~
										REFUSAL AT 15.6' (PROBABLE BEDROCK)
									·	
AMPLE	MPLES: SOIL CLASSIFIED BY:				' :		REMAR			
	HELBY	ON TUBE		X	SOIL	LER - \ TECH.	VISU	ALLY		STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY BETWEEN SOIL TYPES AND THE TRANSITION MAY BE GRADUAN

U = 3.5" SHELBY TUBE

LABORATORY TEST

AND THE TRANSITION MAY BE GRADUAL.

BORING NO.:

B-101



BORING NO.:

B-102

SHEET:

1 OF 1

ENC	GINEERI	NG,INC	•			PROJECT NO.:	09-0077
PROJECT / CLIENT:	PROPOSED	NEW BUILDIN	IGS / BAYSIDE	MAINE, LLC		DATE START:	3/24/2009
LOCATION:	645 CONGRI	ESS STREET	PORTLAND, MA	AINE		DATE FINISH:	3/25/2009
DRILLING CO.:	NORTHERN	TEST BORING	3, INC.	DRILLER:	MIKE NADEAU	ELEVATION:	110' +/-
	TYPE	SIZE I.D.	HAMMER WT	. HAMMER FALL		SWC REP.:	K. GIMPEL
CASING:	HW	4"				WATER LEVEL INFORM	IATION
SAMPLER:	SS	2 3/8"	140-lb	30"		WATER AT 9' +/-	
CORE BARREL:	NQ	2"			_		

CASING BLOWS	4	SAI	MPLE		SAM	PLER B	LOWS F	PER 6" .	DEPTH	STRATA & TEST DATA
PER FOOT	NO.	PEN.	REC.	DEPTH @ BOT	0-6	6-12	12-18	18-24		
SSA									0.1'	1 -INCH ASPHALT PAVEMENT
ГО 13'	1D	24"	18"	2.5'	3	4	4	7	1.5'	BROWN SILTY SAND SOME GRAVEL (FILL)
										BLACK SILTY SAND SOME GRAVEL
										WITH OCCASIONAL COBBLES (FILL)
										~MEDIUM DENSE~
									5.2'	
	2D	24"	24"	7.0'	4	6	9	10		
•										BROWN SILT AND SAND
SET]	~MEDIUM DENSE~
ASING]	w = 19.1 %
ΓΟ 10'									10.3'	
	3D	24"	23"	12.0'	7	9	8	12		GRAY SAND AND SILT SOME GRAVEL (TILL)
									13.0'	~MEDIUM DENSE~
	-									PROBABLE WEATHERED ROCK
CORE									15.0'	(ADVANCED BORING BY ROLLER CONE)
1										
									1]	(SEE ROCK CORE LOG)
$\neg \neg$	1R	36"	32"	18.0'					18.0'	l '
V	2R	24"	17"	20.0'				- "	20.0'	RQD = 46 %
										BOTTOM OF EXPLORATION AT 20.0'
	318-13				·				'	
							• • • • • • • • • • • • • • • • • • • •			,
i										
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	-					-				
		ļ					ا			
AMPLE	ES:			SOIL C	LASSIF	ED BY	′ :		REMARI	KS:
				_						

D = SPLIT SPOON C = 3" SHELBY TUBE U = 3.5" SHELBY TUBE

X X

DRILLER - VISUALLY SOIL TECH. - VISUALLY LABORATORY TEST STRATIFICATION LINES REPRESENT THE
APPROXIMATE BOUNDARY BETWEEN SOIL TYPES
AND THE TRANSITION MAY BE GRADUAL.

BORING NO.:

B-102



MIKE NADEAU

BORING NO.:

B-103 1 OF 1

SHEET:

09-0077

PROJECT NO.: DATE START:

3/25/2009

DATE FINISH:

3/25/2009

ELEVATION:

108' +/-

TYPE SIZE I.D. HAMMER WT. HAMMER FALL

PROPOSED NEW BUILDINGS / BAYSIDE MAINE, LLC

645 CONGRESS STREET PORTLAND, MAINE

SWC REP.:

K. GIMPEL

CASING:

SAMPLER:

PROJECT / CLIENT:

LOCATION:

DRILLING CO.:

HW 4"

NORTHERN TEST BORING, INC.

WATER LEVEL INFORMATION

SS 2 3/8"

140-lb 30"

DRILLER:

WATER AT 8.5' +/-

CORE BARREL: NQ 2"

SSA	ASING LOWS PER	14.	en e	VPLE	DEPTH	对而是1953年	PLER B		PER 6"	DEPTH	STRATA & TEST DATA
TO 15' 1D 12' 7" 1.5' 6 32 BROWN GRAVELLY SILTY SAND WITH OCCASIONAL COBBLES (FILL) -MEDIUM DENSE- ### 20 24' 16' 7.0' 5 4 3 5 SET		NO.	PEN.	,REC.		0-6	6-12	12-18	18-24	15.5	
WITH OCCASIONAL COBBLES (FILL) -MEDIUM DENSE- 4.5' BROWN SILT AND SAND TRACE GRAVEL -MEDIUM DENSE BECOMING SET	SSA									0.2'	2 1/2 -INCHES ASPHALT PAVEMENT
## 2D 24* 16* 7.0' 5 4 3 5 BROWN SILT AND SAND TRACE GRAVELMEDIUM DENSE BECOMING ## 20.3 %	O 15'	1D	12"	7"	1.5'	6	32				BROWN GRAVELLY SILTY SAND
2D 24* 16* 7.0' 5 4 3 5 SET											WITH OCCASIONAL COBBLES (FILL)
DESCRIPTION OF EXPLORATION AT 19.5' BROWN SILT AND SAND TRACE GRAVEL -MEDIUM DENSE BECOMING W = 20.3 % BROWN SILT AND SAND TRACE GRAVEL -MEDIUM DENSE BECOMING W = 20.3 % BROWN SILT AND SAND TRACE GRAVEL -MEDIUM DENSE BECOMING W = 20.3 %											~MEDIUM DENSE~
-MEDIUM DENSE BECOMING SET										4.5'	
-MEDIUM DENSE BECOMING SET										1	
SET		2D	24"	16"	7.0'	5	4	3	5		
ASSING TO 10' 3D 24" 24" 12.0' 7 15 22 21 12.0' PROBABLE GRAY SAND AND SILT SOME GRAVEL (TILL) (SEE ROCK CORE LOG) RQD = 20 % 19.5' BOTTOM OF EXPLORATION AT 19.5'	<u> </u>]	
TO 10' 3D 24" 24" 12.0' 7 15 22 21 12.0'DENSE- PROBABLE GRAY SAND AND SILT SOME GRAVEL (TILL) CORE											w = 20.3 %
3D 24" 24" 12.0" 7 15 22 21 12.0"DENSE~ PROBABLE GRAY SAND AND SILT SOME GRAVEL (TILL) CORE 14.5" (SEE ROCK CORE LOG) RQD = 20 % BOTTOM OF EXPLORATION AT 19.5" BOTTOM OF EXPLORATION AT 19.5"											
PROBABLE GRAY SAND AND SILT SOME GRAVEL (TILL) (SEE ROCK CORE LOG) RQD = 20 % BOTTOM OF EXPLORATION AT 19.5' BOTTOM OF EXPLORATION AT 19.5'				ļ							
GRAY SAND AND SILT SOME GRAVEL (TILL) (SEE ROCK CORE LOG) RQD = 20 % 19.5' BOTTOM OF EXPLORATION AT 19.5'		3D	_24"	24"	12.0'	7	15	22	21	12.0'	
CORE (SEE ROCK CORE LOG) RQD = 20 % 18. 60" 56" 19.5" BOTTOM OF EXPLORATION AT 19.5"											
(SEE ROCK CORE LOG) RQD = 20 % 19.5' BOTTOM OF EXPLORATION AT 19.5'											GRAY SAND AND SILT SOME GRAVEL (TILL)
RQD = 20 % 19.5' BOTTOM OF EXPLORATION AT 19.5'	ORE									14.5	
RQD = 20 % 19.5' BOTTOM OF EXPLORATION AT 19.5'											(055 Dook oops Loo)
19.5' 19.5'	1 +									1	(SEE ROCK CORE LOG)
▼ 1R 60° 56° 19.5' 19.5' BOTTOM OF EXPLORATION AT 19.5'	+									- 1	DOD - 00 %
BOTTOM OF EXPLORATION AT 19.5'	 	10	60"	EG!!	10 E'				ļ	40.51	RQD = 20 %
	* 	ıĸ	60	30	19.5					19.5	
										-	POTTOM OF EVDI ODATION AT 10 F
							-			-	BOTTOM OF EXPLORATION AT 19.5
										1 1	
										1	
										1	
	<u> </u>									1	
						-				1	
										1	
										-	
]	
	<u> </u>									1	
										1 1	
										1	
]	
AMPLES: SOIL CLASSIFIED BY: REMARKS:		\.			00" 5					<u> </u>	1/0

D = SPLIT SPOON C = 3" SHELBY TUBE U = 3.5" SHELBY TUBE

DRILLER - VISUALLY SOIL TECH. - VISUALLY LABORATORY TEST

STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY BETWEEN SOIL TYPES

AND THE TRANSITION MAY BE GRADUAL.

BORING NO.: B-103



MIKE NADEAU

BORING NO.:

B-104

SHEET:

1 OF 1 09-0077

PROJECT NO .: DATE START:

3/25/2009

DATE FINISH:

3/25/2009

ELEVATION:

110' +/-

TYPE

NORTHERN TEST BORING, INC.

PROJECT / CLIENT: PROPOSED NEW BUILDINGS / BAYSIDE MAINE, LLC

645 CONGRESS STREET PORTLAND, MAINE

2 3/8"

2"

DRILLER:

K. GIMPEL

CASING:

LOCATION:

DRILLING CO.:

 HW

SS

SIZE I.D. HAMMER WT. HAMMER FALL

SWC REP.:

SAMPLER:

4"

140-lb 30" WATER LEVEL INFORMATION NO FREE WATER OBSERVED

CORE BARREL: NQ

SSA TO 10' SET CASING TO 6' CORE	NO. 1D 2D 1R	24" 4"	13" 4*	2.5' 5.3'	0-6 7 50/4"	6-12	3	3	0.1' 3.0'	2 -INCHES ASPHALT PAVEMENT BROWN GRAVELLY SILTY SAND (FILL) ~LOOSE TO MEDIUM DENSE~
TO 10' SET CASING TO 6' CORE	2D			2.5'		6	3	3		2 -INCHES ASPHALT PAVEMENT BROWN GRAVELLY SILTY SAND (FILL)
SET CASING TO 6'	2D					6	3	3	3.0'	BROWN GRAVELLY SILTY SAND (FILL)
SET CASING TO 6'		4"	4"	5.3'	50/4"				3.0'	
SET CASING TO 6'		4"	4"	5.3'	50/4"					
SET CASING TO 6'		4"	4"	5.3'	50/4"]	
SET CASING TO 6'		4"	4"	5.3'	50/4"					PROBABLE BROWN SILT AND SAND SOME GRAVEL (TILL)
CASING TO 6'	1R									~VERY DENSE~
TO 6'	1R]	
CORE	1R								i i	[BELOW 4.5-FEET WAS EITHER VERY DENSE TILL OR WEATHERED ROCK]
	1R									
	1R		1						9.5'	ATTENDA
	1R								10.0	WEATHERED ROCK
1	1R									(SEE ROCK CORE LOG)
*	1R	455		110						RQD = 31 %
		48"	48"	14.0'					14.0'	
										POTTOM OF EVEL ODATION AT 44 CL
										BOTTOM OF EXPLORATION AT 14.0'
							-		1 1	
-+										
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AMPLES:				SOIL CI						

D = SPLIT SPOON C = 3" SHELBY TUBE U = 3.5" SHELBY TUBE DRILLER - VISUALLY SOIL TECH. - VISUALLY LABORATORY TEST

STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY BETWEEN SOIL TYPES

AND THE TRANSITION MAY BE GRADUAL.

BORING NO.:

B-104



PROJECT: PROPOSED NEW BUILDINGS AT 645 CONGRESS STREET

BORING NO.: B-102

CLIENT: BAYSIDE MAINE, LLC

PROJECT NO.: 09-0077

LOGGED BY: K. GIMPEL

DATE: 3/25/2009

SHEET NO.: 1 OF 1

CHECKED BY: G. BUCKLIN

DATE: 3/26/2009

CORE SIZE: NO2

CHECK	(ED BA:	G. BUC	KLIN				DATE: <u>3/26/2009</u> CORE S	SIZE:	NQ2
DEPTH BELOW SURFACE (FT)	CORE RUN	CORE INTERVAL (FT)	CORE RECOVERY (FT)	RQD (%)	ROCK QUALITY	GRAPHIC LOG	ROCK DESCRIPTION AND IDENTIFICA	TION	
							(SEE SHEET A-2 FOR SOILS DESCRIPTION)	
15 =	1R	15.0° 36"	32"	0/36	VERY POOR		BEGINNING OF CORE RUN AT 15.0' GRAY PELITE 85° FINE GRAINED TEXTURE HIGHLY FRACTURED 50° MODERATELY WEATHERED MODERATELY HARD 50° 55° ZONE OF NO RECOVERY		
20	2R	24" 20.0'	17"	11/24 46%	POOR		IRON OXIDE STAINING ON SOME FRACTURE SURFACE CALCITE AND MUSCOVITE MICA EVIDENT ZONE OF NO RECOVERY	ES .	
20						ահակավայիակարհակակարևակակարևակարահակա	BOTTOM OF EXPLORATION AT 20.0'		A-5



PROJECT: PROPOSED NEW BUILDINGS AT	645 CONGRESS STREET	BORING NO.:	B-103	
CLIENT: BAYSIDE MAINE, LLC		PROJECT NO.:	09-0077	
	,			
LOGGED BY: K. GIMPEL	DATE: 3/25/2009	SHEET NO.:	1 OF 1	_
CHECKED BY: G. BUCKLIN ,	DATE: 3/26/2009	CORE SIZE:	NQ2	

	KED BY:	G. BUC	KLIN	•			DATE: 3/26/2009 CORE SIZE:	NQ2
DEPTH BELOW SURFACE (FT)	CORE RUN	CORE INTERVAL (FT)	CORE RECOVERY (FT)	RQD (%)	ROCK QUALITY	GRAPHIC LOG	ROCK DESCRIPTION AND IDENTIFICATION	
14							(SEE SHEET A-3 FOR SOILS DESCRIPTION) BEGINNING OF CORE RUN AT 14.5'	
20	1R	14.5' 60"	56"	12/60 20%	VERY POOR		20° GRAY PELITE FINE TO MEDIUM GRAINED TEXTURE SEVERELY TO MODERATELY WEATHERED SOME PITTING ON ROCK SURFACES SLIGHT IRON OXIDE STAINING ON FRACTURE SURFACES CALCITE BANDS UP TO 1/2-INCH DIAMETER 10° 25° HIGHLY FRACTURED ZONE ZONE OF NO RECOVERY	
20 =						ավավակակակակակակակակակակակակա	BOTTOM OF EXPLORATION AT 19.5'	A-6



PROJECT: PROPOSED NEW BUILDINGS AT	645 CONGRESS STREET	BORING NO.:	B-104	
CLIENT: BAYSIDE MAINE, LLC	·	PROJECT NO.:	09-0077	
LOGGED BY: K. GIMPEL	DATE: 3/25/2009	SHEET NO.:	1 OF 1	
CHECKED BY: G. BUCKLIN	DATE: 3/26/2009	CORE SIZE:	NQ2	

CHEC	KED BY:	G. BUC	KLIN				DATE: <u>3/26/2009</u>	CORE SIZE:	NQ2
DEPTH BELOW SURFACE (FT)	CORE RUN	CORE INTERVAL (FT)	CORE RECOVERY (FT)	RQD (%)	ROCK QUALITY	GRAPHIC LOG	ROCK DESCRIPTION AND	DIDENTIFICATION	
10						mılımlımlım	(SEE SHEET A-4 FOR SOIL BEGINNING OF CORE		
	1R	48"	48'	15/48	POOR		GRAY PELITE / META-SANDSTONE FINE TO MEDIUM GRAINED TEXTURE MODERATELY WEATHERED MODERATELY HARD IRON OXIDE STAINING ON FRACTURE CALCITE BANDS HIGHLY FRACTURED AND SEVERELY WEATHER AND SEVERE WEATHER WEAT		
						ահայտահայտանանարակարարարակարարարարարարար	BOTTOM OF EXPLORATE	FION AT 14.0'	A-7



Report of Gradation

ASTM C-117 & C-136

Project Name

Client

PORTLAND ME - 645 CONGRESS STREET RENOVATIONS -

GEOTECHNICAL ENGINEERING SERVICES

75 um

Material Source B-102, 2D (5' - 7')

Project Number 09-0077

Lab ID 10432G

Date Received 3/26/2009

Date Complete 3/27/2009

52.8% Fines

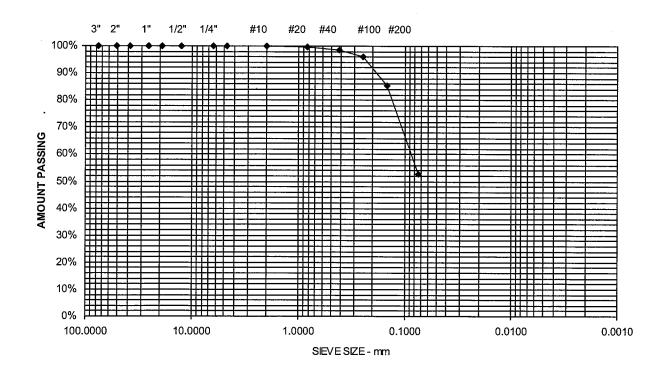
Tested By

JUSTIN BISSON

<u>STANDARD</u> DESIGNATION (mm/µm)	SIEVE SIZE	AMOUNT PASSING (%)	1
150 mm	6"	100	
125 mm	5"	100	
100 mm	4"	100	
75 mm	3"	100	
50 mm	2"	100	
38.1 mm	1-1/2"	100	
25.0 mm	1"	100	
19.0 mm	3/4"	100	
12.5 mm	1/2"	100	
6.3 mm	1/4"	100	
4.75 mm	No. 4	100	0% Gravel
2.00 mm	No. 10	100	
850 um	No. 20	99	
425 um	No. 40	99	47.2% Sand
250 um	No. 60	96	
150 um	No. 100	85	

No. 200

52.8





Report of Gradation

ASTM C-117 & C-136

Project Name

PORTLAND ME - 645 CONGRESS STREET RENOVATIONS -

GEOTECHNICAL ENGINEERING SERVICES

Lab ID

Project Number 09-0077

Client

10433G

Date Received

Tested By

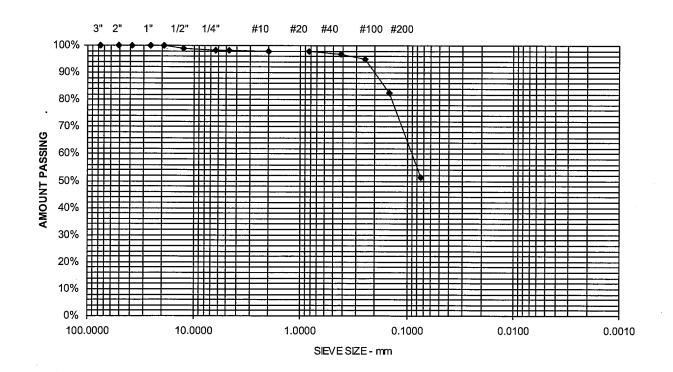
3/26/2009

Date Complete

3/27/2009 JUSTIN BISSON

Material Source B-103, 2D (5' - 7')

150 mm 6" 100 125 mm 5" 100 100 mm 4" 100 75 mm 3" 100 50 mm 2" 100 38.1 mm 1-1/2" 100 25.0 mm 1" 100 19.0 mm 3/4" 100 12.5 mm 1/2" 99 6.3 mm 1/4" 98 4.75 mm No. 4 98 1.8% Gravel 2.00 mm No. 10 98 850 um No. 20 98 425 um No. 40 97 47.1% Sand 250 um No. 60 95 150 um No. 100 83 75 um No. 200 51.1 51.1% Fines	STANDARD DESIGNATION (mm/µm)	SIEVE SIZE	AMOUNT PASSING (%)	
125 mm 5" 100 100 mm 4" 100 75 mm 3" 100 50 mm 2" 100 38.1 mm 1-1/2" 100 25.0 mm 1" 100 19.0 mm 3/4" 100 12.5 mm 1/2" 99 6.3 mm 1/4" 98 4.75 mm No. 4 98 1.8% Gravel 2.00 mm No. 10 98 850 um No. 20 98 425 um No. 40 97 47.1% Sand 250 um No. 60 95 150 um No. 100 83				
100 mm	150 mm	6"	100	
75 mm 3" 100 50 mm 2" 100 38.1 mm 1-1/2" 100 25.0 mm 1" 100 19.0 mm 3/4" 100 12.5 mm 1/2" 99 6.3 mm 1/4" 98 4.75 mm No. 4 98 1.8% Gravel 2.00 mm No. 10 98 850 um No. 20 98 425 um No. 40 97 47.1% Sand 250 um No. 60 95 150 um No. 100 83	125 mm	5"	100	
50 mm 2" 100 38.1 mm 1-1/2" 100 25.0 mm 1" 100 19.0 mm 3/4" 100 12.5 mm 1/2" 99 6.3 mm 1/4" 98 4.75 mm No. 4 98 1.8% Gravel 2.00 mm No. 10 98 850 um No. 20 98 425 um No. 40 97 47.1% Sand 250 um No. 60 95 150 um No. 100 83	100 mm	4"	100	
38.1 mm 1-1/2" 100 25.0 mm 1" 100 19.0 mm 3/4" 100 12.5 mm 1/2" 99 6.3 mm 1/4" 98 4.75 mm No. 4 98 1.8% Gravel 2.00 mm No. 10 98 850 um No. 20 98 425 um No. 40 97 47.1% Sand 250 um No. 60 95 150 um No. 100 83	75 mm	3"	100	
25.0 mm 1" 100 19.0 mm 3/4" 100 12.5 mm 1/2" 99 6.3 mm 1/4" 98 4.75 mm No. 4 98 1.8% Gravel 2.00 mm No. 10 98 850 um No. 20 98 425 um No. 40 97 47.1% Sand 250 um No. 60 95 150 um No. 100 83	50 mm	2"	100	
19.0 mm 3/4" 100 12.5 mm 1/2" 99 6.3 mm 1/4" 98 4.75 mm No. 4 98 1.8% Gravel 2.00 mm No. 10 98 850 um No. 20 98 425 um No. 40 97 47.1% Sand 250 um No. 60 95 150 um No. 100 83	38.1 mm	1-1/2"	100	
12.5 mm 1/2" 99 6.3 mm 1/4" 98 4.75 mm No. 4 98 1.8% Gravel 2.00 mm No. 10 98 850 um No. 20 98 425 um No. 40 97 47.1% Sand 250 um No. 60 95 150 um No. 100 83	25.0 mm	1"	100	
6.3 mm 1/4" 98 4.75 mm No. 4 98 1.8% Gravel 2.00 mm No. 10 98 850 um No. 20 98 425 um No. 40 97 47.1% Sand 250 um No. 60 95 150 um No. 100 83	19.0 mm	3/4"	100	
4.75 mm No. 4 98 1.8% Gravel 2.00 mm No. 10 98 850 um No. 20 98 425 um No. 40 97 47.1% Sand 250 um No. 60 95 150 um No. 100 83	12.5 mm	1/2"	99	
2.00 mm No. 10 98 850 um No. 20 98 425 um No. 40 97 47.1% Sand 250 um No. 60 95 150 um No. 100 83	6.3 mm	1/4"	98	
850 um No. 20 98 425 um No. 40 97 47.1% Sand 250 um No. 60 95 150 um No. 100 83	4.75 mm	No. 4	98	1.8% Gravel
425 um No. 40 97 47.1% Sand 250 um No. 60 95 150 um No. 100 83	2.00 mm	No. 10	98	•
250 um No. 60 95 150 um No. 100 83	850 um	No. 20	98	
150 um No. 100 83	425 um	No. 40	97	17.1% Sand
	250 um	No. 60	95	
75 um No. 200 51.1 51.1% Fines	150 um	No. 100	83	
	75 um	No. 200	51.1	51.1% Fines



Notes:

- 1. This renovation includes the demolition three (3) existing building wings consisting of 112 dormatory unitis.
 2. For the most part, the proposed renovations do not represent a change of use. Where changes of use do occure, they are generally a reduction in the required level of protection (i.e. Mercantile is generally a less restrictive use than Assembly).

Code Analysis

Category	Reference	IBC 2003 Code Requirements	Reference	NFPA 101 - 2006 Code Requirements	Proposed
Existing Use Group	310.0	R-2 Dormatory	6.1.8.1.4	Residential - Existing Dormatory	Existing: 43 Units, 1
		A-2 Restaurants / A-3 Community Halls 6.1.2.1	6.1.2.1	bullding, Re: 29.1.1.2 Assembly	Office, 1 Common Room
Proposed Use Group	310.0	R-2 Apartments	6.1.8.1.5	Residential - New Apartment	Proposed: 56 Units
		M Mercantile		Bullding, Re: 30.1.1.2 Mercantile Class C, Re: 36.1.1.2	
Building Construction	Table 602		Table 8.2.1.2		
	Ground Level	Type IIIB - Non-Combustible Protected	36.1.6	New Mercantile Class C, Type III (200) Provided	Ordinary Concstruction
	Upper Four Levels	Type IIIB - Combustible Protected	30.1.6	New Apartment Building Type III (200) Provided	Ordinary Construction
Height & Area Limitations	Table 503	See IBC 2003 - Code Calculations			
Height Modifications 504.2 Maximum allowable height Area Modifications 506.2	504.2 506.2	See IBC 2003 - Code Calculations 4 stories* / 55' See IBC 2003 - Code Calculations			Sprinklered - NFPA 13 4 stories* / 63.42'
506.3 Maximum allowable area 506.1	506.3 506.1	See IBC 2003 - Code Calculations See IBC 2003 - Code Calculations			43,127 SF 1st floor 24,984 SF 2nd-5th floors
	*	* From level above S-2 per 508.2			

Fire ratings	Table 602	Loadbearing Ext walls - 2 hr	Table 8.2.1.2	Table 8.2.1.2 Loadbearing Ext walls - 2 hr	2 hour
		Nonloadbearing Ext walls - 0 hr		Nonloadbearing Ext walls - 0 hr	0 hour
	Table 1016.1	Table 1016.1 Exit access corridors - 1/2 hour (with sprinkler system)	30.3.6.1.2	Exit access corridors - 1/2 hour	1/2 hour (1 hour protected wood frame provided)
	·		30.3.6.2	Exit access corridor doors - 20-	20 minute doors, spring
	1019.1	Fire Enclosure of 6 story Vertical Exit	30.2.2.1.2	nimute sen-closing & latening Exits: 2 Hour (Sprinklered)	ninges 2 hour
	707.4	vator Hoistways - 2	30.3.1.1.4	Shafts: 2 Hour (Sprinklered) and 1-	2 hour
	708.3 711.3	relling Unit Separation - 1/2 hour relling Unit Separation floor and roof -	30.3.7.2	nour doors Dwelling Unit Separation - 1 hour	1 hour 1 hour
	Table 601 Table 601 Table 601	Interior loadbearing walls - 0 hour lnt. nonloadbearing walls - 0 hour Floor Construction - 0 hour			1 hour 0 Hour 1 hour protected wood
	Table 601	Roof construction - 0 hour			frame provided 1 hour protected wood
	Table	Boiler Rooms over 10 HP - 1 hour or	T 30.3.2.1.1	Boiler Rooms - 1 Hour and sprinkler	rrame provided 1 Hour & Sprinklered
	Table 302.1.1	Laundry Rooms over 100 s.f 1 hour or T 30.3.2.1.1 automatic fire extinguishing	Т 30.3.2.1.1	Laundries over 100 s.f 1 hour and sprinklers with smoke partitions	Sprinklered
	Table 302.1.1	Storage Rooms over 100 s.f 1 hour or automatic fire extinguishing	Т 30.3.2.1.1	Storage rooms outside of dwelling units - 1 hour OR sprinklers with smoke partitions	Sprinklered
Opening Protectives	Table 715.3 Table 715.3 Table 715.3	1/2 Hour Rated Exit access corridors - 20 min. Doors 1 Hour Barriers - 45 min. Doors 1 Hour Shafts & Exits - 60 min. Doors	30.3.6	Exit access corridor doors - 20- minute self-closing & latching	20 min. 45 min. 60 min.
Wired Glass Panels	Table	20 min. opening = Not limited			As required
	?	3/4 hour opening = 1296 sq in 1-1/2 hour opening = 100 sq in			As required As required
Interior Flame Spread Index	Table 803.5	Required vertical exits and	30.3.3.2	In exits - Class A	As required
	Table 803.5 Table 803.5	g exit access - C ed spaces - C	30.3.3.2 30.3.3.2 30.3.3.3	Lobbies and corridors - Class A or B Other Spaces - Class A, B or C Exit enclosures and corridor floors - Not less than Class II	As required As required

Occupant Load	T 1004.1.2	Residential Occupancy = 1/200 GSF	Table 7.3.1.2	Table 7.3.1.2 Residential Apartment occupant load Refer to IBC 2003 Code = 1/200 SF Calculations, NFPA the	Refer to IBC 2003 Code Calculations, NFPA the
	T 1004.1.2	Storage areas, mechanical equipment rooms = 1/300 GSF		Storage areas, mechanical equipment rooms = 1/300 GSF	same.
Means of Egress					Refer to IBC 2003 Code Calculations, NFPA the same.
Emergency Lighting	1011.1 (exc. Exit Signs	are required except at main	7.10.1.2	Exit Signs are required except at	Provided
	1006.1	Emergency lighting is required except in 30.2.9 dwelling units	30.2.9	Emergency lighting is required	Provided
Fire Suppression Systems	903.2.7	13 Required	30.3.5	13 Required	13 Provided
Standpipes	905.3	Required		Required	Provided
Fire Alarm Systems	907.2.9	Required	30.3.4.1.3	Required	Provided
Smoke Detetectors	907.2.10.1.2 Required		30.3.4.5.1	Required outside every sleeping area	Provided
Extinguishment requirements	906	Portable fire extinguishers shall be installed as per NFPA 10	30.3.5.11	Not Required	Provided

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Floor Plate - Floors 1	10,725			
Second - Fifth Floors	8,406			
Sixth Floor	4,982			
OKAT FIOOI	7,502			<u> </u>
Section 302.3.2 Separated Uses				
M (Mercantile) to R2 (Residential Apartments)	1 Hour Requi	red (Sprinkled p	per 903.3.1.1)	1 Hour Provid
M (Mercantile) to S-1 (Residential Apartments)		red, 1 Hour Pro		
			<u>, , , , , , , , , , , , , , , , , , , </u>	
	Floor 0	Floor 1		Floors 2-6
Buildng Use Summary	Storage	Mercantile		Residential
	S-1	М		R-2
Number of Floors (Proposed)	1	1		5
Level 0 Basement	4,262	2,130		
Level 1		10,725		
Level 2				8,406
Level 3				8,406
Level 4				8,406
Level 5				8,406
Level 6				4,982
Total floor area per use (SF)	4,262	12,855		38,606
Total Building Area		51,4	461	
Allowable Height and Building Areas		Mercantile		Residential
Table 503		M		R-2
Type of Construction		IIIB		IIIB
Number of Floors		4		4
Building Height (Feet)		55		55
Area (Square Feet)		12,500		16,000
Frontage Calculations 506.2	North	East	South	West
Frontage	96.00	62.00	135.00	0.00
Width	24	24	30	0
Total Frontage	293.00	293.00	293.00	293.00
Perimeter	332	332	332	332
Frontage increase I(f) = 100[F/P-0.25]W/30	63.25%	63.25%	63.25%	63.25%
Area Modifications 506				
Allowable tabular area		100%		100%
Increase for frontage		63.25%		63.25%
Increase for include Increase for sprinklers (NFPA 13 system)		200%		200%
Total percentage factor		363%	·,	363%
Conversion factor	-	3.63		3.63
Adjusted Allowable Building Area		45,407		58,120
Actual building area		12,855		24,984
If Actual building area < Adjusted, then OK		12,000 OK		24,904 OK
The state of the s				
Requide Fire Wall Separation per 506.4 (c)		0		0

645 Congress Street Apartments - Renovations			CV	VS Architects
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	· · ·			
Section 504 Height Modifications		Group	R-2 Use	
A - 4 1 1 1 1 - 1	Feet	Stories	Feet	Stories
Actual building height *	11	1	71.33	6 *
Tabular building height - Type IIIB Increase for sprinklers (NFPA 13 system)	55 20	4	55 20	4
Allowable building height (b)	75	5	75	<u> </u>
Allowable building neight (b)	+ 73	3	75	3
	* Existing Cor	ndition / No Cha	inge of Use	
Section 506.4 Area determinations				
Total floor area (all stories)	4,262	12,855	0	38,606
A. Allowable area per floor (SF)	0		0	58,120
B. Number of Applicable Floors (Exception 2)	1	+	0	5
C. Tabular area A x B	0	45,407	0	290,602
If C > Total Building Floor Area, then OK	OK		OK	Ok
Till ood 5: Did Did W			5.011	
Table 601 Fire Resistance Ratings (hours)	M Use	Provided	R-2 Use	Provided
Structural Frame including Columns	IIIA	IIIA	IIIA	IIIA
Structural Frame including Columns Bearing Walls - Exterior	0 2	0 2	2	0 2
Bearing Walls - Interior	0	0	0	0
Nonbearing Walls - Interior	1 0	0	0	0
Floor Construction including Beams	0	0	0	0
Roof Construction including Beams	 0	0	0	0
704.8 Allowable area of openings				
Table 704.8				
Between 5-10' - Unprotected 10% Max, Protected	25% Max			
Between 10-15' - Unprotected 15% Max, Protected				
Between 15-20' - Unprotected 25% Max, Protected				
Between 20-25' - Unprotected 45% Max, Protected				
Between 25-30' - Unprotected 25% Max, Protected	d No Limit			
>30' - Unprotected No Limit, Protected No Limit				
Provided: "Unprotected"				
Section 707 Shaft Enclosures				
Required fire rating	2 hours			
Provided	2 hours	Evicting		
1.1041000	2 110UI 5	Existing		
Section 708 Fire Partitions	+			
Required fire rating - dwelling unit separation - NFPA				
13 sprinkler system	1/2 hour			
Provided	1/2 hour	Existing		
Sectin 903.2.10.3 Buildings over 55 feet in height				
Automatic Sprinkler System Required, Provided				



645 Congress Street Apartments - Renovations		CWS Architects
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Section 1004 Occupant Load	Mercantile M	Residential R-2
Floor area allowance - persons/SF	60	200
Largest Floor area (SF)	10,725	8,406
Occupancy load per floor	179	42
Section 1005 Required Egress Width		
Total Stairway Width - 0.3"/person	53.6	12.6
Provided (See Plans)	N/A	92
Egress Doors - 0.2"/person	35.8	8.4
Provided - minimum @ doorways	68	68
Provided 34" Clear Door Leafs	2	2
Section 1018 Number of Exits		
Required	2	2
Provided	2	2