GENERAL NOTES:

1. ALL DIMENSIONS AND CONDITIONS MUST BE VERIFIED IN THE FIELD. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER OF RECORD BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK.

2. G.C SHALL COORDINATE LOCATIONS AND DIMENSIONS OF OPENINGS, CHASES, INSERTS, REGLETS, SLEEVES DEPRESSIONS, ECT. W/ MECHANICAL, ELECTRICAL & ARCHITECTURAL REQUIREMENTS.

3. THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE FRAMING IS COMPLETE. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO ENSURE THE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS, OR TIEDOWNS. SUCH MATERIAL SHALL REMAIN THE PROPERTY OF THE CONTRACTOR AFTER COMPLETION OF THE PROJECT.

4. SECTIONS AND DETAILS SHOWN ON ANY STRUCTURAL DRAWINGS SHALL BE CONSIDERED TYPICAL FOR SIMILAR CONDITIONS.

5. THE CONTRACTOR SHALL PERFORM ALL WORK IN CONFORMANCE WITH ALL APPLICABLE FEDERAL, STATE AND MUNICIPAL REGULATIONS.

DESIGN NOTES

1. Code: INTERNATIONAL BUILDING CODE, 2009
   INTERNATIONAL EXISTING BUILDING CODE, 2009
   ASCE 7-05 MINIMUM DESIGN LOADS FOR BUILDINGS

2. BUILDING OCCUPANCY CATEGORY: II

3. FLOOR DEAD LOADS = ACTUAL WEIGHTS OF COMPONENTS PLUS 4 psf ALLOWANCE FOR MECHANICAL MISCELLANEOUS DUCTWORK, SPRINKLER PIPING AND OTHER HUNG ITEMS

4. FLOOR LIVE LOADS = 80 PSF
   (60 PSF + 20 PSF PARTITION ALLOWANCE)

5. WIND LOADS:
   A. BASIC WIND SPEED: V= 95 mph
   B. WIND LOAD IMPORTANCE FACTOR I=1.0 WIND
   C. EXPOSURE= EXPOSURE C
   D. WIND INTERNAL PRESSURE COEFFICIENT GCpi= ±0.18

6. EARTHQUAKE LOADS:
   A. SEISMIC IMPORTANCE FACTOR = 1.0
   B. SPECTRAL RESPONSE ACCELERATION Ss = 0.319
   C. SPECTRAL RESPONSE ACCELERATION S1 = 0.077
   D. SITE CLASS E
   E. SPECTRAL RESPONSE COEFFICIENT SDS = 0.485
   F. SPECTRAL RESPONSE COEFFICIENT SD1 = 0.181
   G. SEISMIC DESIGN CATEGORY C
   H. BASIC SEISMIC FORCE RESISTING SYSTEM: STEEL BRACED FRAME
   I. NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE
   J. SEISMIC RESPONSE COEFFICIENT, Cs = 0.16
   K. RESPONSE MODIFICATION FACTOR, R = 3.0
   ANALYSIS PROCEDURE = EQUIVALENT LATER FORCE PROCEDURE

QUALITY ASSURANCE/ SPECIAL INSPECTIONS

1. STRUCTURAL SPECIAL INSPECTIONS SHALL BE PROVIDED BY THE OWNER DURING CONSTRUCTION AS REQUIRED BY IBC 2009, CHAPTER 17. REFER TO THE SCHEDULE OF SPECIAL INSPECTION FOR THE REQUIRED INSPECTIONS SCOPE. THE SPECIAL INSPECTOR (OR INSPECTORS) FOR THE PROJECT IS TO BE RETAINED BY THE OWNER AND APPROVED BY THE STRUCTURAL ENGINEER OF RECORD.

2. NO STATEMENTS OF COMPLETION WILL BE ISSUED BY THE SPECIAL INSPECTOR (OR INSPECTORS) WITHOUT COMPLETION OF THE SPECIAL INSPECTION REQUIREMENTS.

3. SPECIAL INSPECTIONS SHALL BE PERFORMED UNDER THE SUPERVISION OF THE SPECIAL INSPECTOR (OR INSPECTORS), AND IN ACCORDANCE WITH THE "SCHEDULE OF SPECIAL INSPECTIONS", PREPARED BY THE STRUCTURAL ENGINEER OF RECORD. ONLY PERSONNEL AUTHORIZED BY THE APPROVED "LIST OF AGENTS" ARE QUALIFIED TO PERFORM THE TASKS OUTLINED IN THE SCHEDULE.

4. IT IS THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE CONSTRUCTION ACTIVITIES AND SEQUENCES WITH THE SPECIAL INSPECTOR AND/OR HIS AGENTS TO PERFORM THE REQUIRED INSPECTION AND TESTING. ANY INSTALLED ITEMS THAT CONCEAL UNINSPECTED ITEMS SHALL BE REMOVED AND REPLACED AT THE CONTRACTOR'S EXPENSE.

CONCRETE NOTES:

1. ALL CONCRETE WORK SHALL CONFORM TO ACI 318-08 AND 301-08.

2. CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS SHALL BE 4000 PSI. PEA SIZED AGGREGATE IS REQUIRED.

3. REINFORCING BARS SHALL CONFORM TO ASTM A-615 GRADE 60 DEFORMED BARS AND SHALL BE DETAILED, FABRICATED, AND PLACED IN ACCORDANCE WITH ACI 315.

4. SPLICES OF REINFORCING BARS SHALL BE IN ACCORDANCE WITH ACI 318, UNLESS OTHERWISE NOTED ON DRAWINGS.

5. SLABS SHALL BE WET CURED FOR 7 DAYS MINIMUM

6. CONCRETE TESTING AND TEST EVALUATION SHALL BE IN ACCORDANCE WITH ACI 318.

STRUCTURAL STEEL NOTES:

1. STRUCTURAL STEEL FABRICATION, ERECTION, AND CONNECTION DESIGN SHALL CONFORM TO AISC "STEEL CONSTRUCTION MANUAL" – THIRTEENTH EDITION.

2. STRUCTURAL STEEL WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A992, GRADE 50 OR ASTM STEEL. STEEL FOR PLATES, ANGLES, AND CHANNELS SHALL CONFORM TO ASTM A36.

3. FIELD CONNECTIONS SHALL BE BOLTED USING 3/4" DIAMETER A325N HIGH STRENGTH BOLTS EXCEPT WHERE FIELD WELDING IS INDICATED ON THE DRAWINGS.

4. ALL WELDING SHALL CONFORM TO AWS D1.1–LATEST EDITION. ELECTRODES SHALL BE E70XX.

5. FEILD WELDING TO EXISTING BEAMS/COLUMNS: PREPARE AND INSPECT EXISTING SURFACE WITHIN 2" OF WELDS TO COMPLY WITH AISC SPECIFICATIONS. SURFACE SHALL BE FREE OF MATERIAL THAT WOULD PREVENT WELDING OR PRODUCE OBJECTIONABLE FUMES FURING WELDING.

6. AT LOCATIONS WHERE EXISTING FIREPROOFING IS PRESENT REINSTALL THE FIREPROOFING AFTER NEW FRAMING IS IN PLACE.

7. FABRICATOR QUALIFICATIONS: A QUALIFIED FABRICATOR WHO PARTICIPATES IN THE AISC QUALITY CERTIFICATION PROGRAM AND IS DESIGNATED AN AISC CERTIFIED PLANT, CATEGORY CBD.

8. SHOP DRAWINGS DETAILING FABRICATION AND ERECTION OF EACH METAL FABRICATION INDICATED SHALL BE SUBMITTED TO THE PROJECT ENGINEER FOR REVIEW PRIOR TO FABRICATION AND CONSTRUCTION.

COLD FORMED METAL FRAMING NOTES:

(TERM "COLD FORMED" IS USED INTERCHANGEABLY W/ "LIGHT GAUGE" IN THE NOTES BELOW.

1. THE FOLLOWING SPECIFICATIONS AND PUBLICATIONS (LATEST EDITION) SHALL BE FOLLOWED.
   A. AMERICAN IRON AND STEEL INSTITUTE COLD FORMED DESIGN MANUAL. SPECIFICATION FOR THE DESIGN OF COLD FORM STEEL STRUCTURAL MEMBERS AND FOR LATERAL DESIGN.
   B. AMERICAN SOCIETY FOR TESTING AND MATERIALS.
   C. AMERICAN INSTITUTE OF STEEL CONSTRUCTION MANUAL OF STEEL CONSTRUCTION 13TH EDITION.

2. FABRICATION OF LIGHT GAGE STEEL SHALL CONFORM WITH REQUIREMENTS OF ASTM A446 WITH THE FOLLOWING MINIMUM YIELD POINTS. (Fy)
   A. 16 GA. AND HEAVIER – Fy = 50,000 PSI (GRADE D)
   B. 18 GA. – Fy = 33,000 PSI (GRADE B)

3. ALL COLD FORMED COMPONENTS SHALL BE GALV. G60 (MIN.)

4. ALL FASTENERS CONNECTION LIGHT GAGE MEMBERS AND ACCESSORIES SHALL BE A MINIMUM OF NO. 10 SIZE SCREWS U.N.O. SPACED NO CLOSER THAN ONE-HALF INCH ON CENTER. NUMBER OF FASTENERS SHALL BE SHOWN ON DETAILS. ALL FASTENERS SHALL BE GALVANIZED OR CADMIUM PLATED

5. ALL FASTENERS CONNECTION LIGHT GAGE MEMBERS TO STRUCTURAL STEEL SHALL BE POWER DRIVEN FASTENERS (PDF) OR 0.157" DIAMETER MINIMUM.

6. CONTRACTOR SHALL CONFIRM ALL HANGER TYPES (INCLUDING HANGER DIMENSIONS) PRIOR TO THE ORDERING OF HANGERS.

FLOOR DECK NOTES:

1. STEEL FLOOR DECK SHALL BE 0.6C26 GALV. DECK AS MANUFACTURED BY VULCRAFT OR APPROVED EQUAL.

2. DECK SHALL BE FASTENED AS FOLLOWS U.N.O.

   AT EACH JOINT – #10 SCREWS @ 30/4 PATTERN

   AT SIDE LAPS – (1) #10 SCREW

   AT PERIMETER (ALL SIDES) – #10 SCREWS @ 10" O/C MAX.

CITY OF PORTLAND, MAINE
RESURGAM
Reviewed for Code Compliance
Inspections Division
Approved with Conditions
Date: 02/25/15

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ENGINEERING
PLANNING
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STATE OF MAINE
JANUSZ S. WISZOLA
NO. 6801
LICENSED
PROFESSIONAL ENGINEER

PROJECT NORTH:

CIANCHETTE SCOUT SERVICE BUILDING RENOVATION
SOUTH PORTLAND, MAINE

ISSUED FOR CONSTRUCTION
11-25-14
CURRENT ISSUE STATUS:

REV	DESCRIPTION	DATE

GRAPHIC SCALE: 0" 1"

SCALE:	AS NOTED
PROJECT MANAGER:	JSW
JC/DRAWN BY:	SJF/ SJF
A/E OF RECORD:	JSW
PROJECT NO:	14179
SMRT FILE:	SG001-14179
DATE:	11/25/14

SHEET TITLE:
STRUCTURAL GENERAL NOTES

SHEET No.
SG001

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