

Certificate of Design Application

From Designer:	Larry A. Wichroski, P.E.
Date:	May 15, 2013
Job Name:	Congress Street Building Remodel
Address of Construction:	660 - 662 Congress Street, Portland, Maine

2009 International Building Code

Construction project was designed to the building code criteria listed below:

Building Code & Year 20	009 IBC	_Use Group Classification (s)
-	III	
Will the Structure have a Fin	re suppression	system in Accordance with Section 903.3.1 of the 2009 IRC Yes
		If yes, separated or non separated or non separated (section 302.3) Separated
Supervisory alarm System?	. <i>.</i>	_Geotechnical/Soils report required? (See Section 1802.2)N/A
Structural Design Calcula	ations	None Live load reduction

Structural Design Calculations

Yes Submitted for all structural members (106.1 – 106.11)

Design Loads on Construction Documents (1603)

Uniformly distributed floor live loads (7603.11, 1807) Floor Area Use Loads Shown

Retail	100.0 psf
Residential	40.0 psf

Wind loads (16 Method 1	03.1.4, 1609) _Design option utilized (1609.1.1, 1609.6)
100 mph	Design option utilized (1009.1.1, 1009.0)
100 mph	Basic wind speed (1809.3)
<u>Cat #1,1.00</u>	_Building category and wind importance Factor,
В	_Wind exposure category (1609.4)
0.18	_Internal pressure coefficient (ASCE 7)
18.0 psf	_Component and cladding pressures (1609.1.1, 1609.6.2.2)
25.0 psf	_Main force wind pressures (7603.1.1, 1609.6.2.1)
Earth design data (1603.1.5, 1614-1623)	
page 2	_Design option utilized (1614.1)

page 2	Seismic use group ("Category")
page 2	Spectral response coefficients, SDs & SD1 (1615.1)
page 2	Site class (1615.1.5)

None	Live load reduction	
45.0 psf		
45.0 psf	_Roof snow loads (1603.7.3, 1608)	
60.0 psf	_Ground snow load, Pg (1608.2)	
<u>45.0 psf</u>	If $P_g > 10$ psf, flat-roof snow load p_f	
0.9	_If $P_g > 10$ psf, snow exposure factor, $_G$	
1.0	_If $P_g > 10$ psf, snow load importance factor, k	
1.0	_Roof thermal factor, $_{G}$ (1608.4)	
n/a	Sloped roof snowload, _{Pt} (1608.4)	
page 2	_Seismic design category (1616.3)	
page 2	Basic seismic force resisting system (1617.6.2)	
page 2	_Response modification coefficient, $_{R'}$ and	
-	deflection amplification factor _{Cl} (1617.6.2)	
page 2	_Analysis procedure (1616.6, 1617.5)	
page 2	Design base shear (1617.4, 16175.5.1)	
Flood loads (1803.1.6, 1612)		
n/a	_Flood Hazard area (1612.3)	
40'	_Elevation of structure	
Other loads		
2000#	_Concentrated loads (1607.4)	
n/a	Partition loads (1607.5)	
n/a	_Misc. loads (Γable 1607.8, 1607.6.1, 1607.7,	
	1607.12, 1607.13, 1610, 1611, 2404	



ENGINEERING DESIGN PROFESSIONALS Consulting Engineers

P.O. Box 575, Freeport, Maine 04032 (207) 865-9505

May 15, 2013

Mr. Ken Guimond

Bayhill Building and Design 174 South Freeport Road South Freeport, Maine 04078

RE: Retail & Office Building Renovations 660 Congress Street, Portland, Maine EDP Project #02412

Dear Ken:

The buildings seismic/wind resistance system consists of plywood roof and floor diaphragms and exterior brick masonry shear walls. The work being done to this building is primarily internal and consists of replacing some of the interior masonry and wood stud bearing wall with beams supported by columns. Although we are removing a portion of the interior brick wall that exists, the wall is much smaller in size compared with the exterior walls and is not considered a contributor to resisting lateral loads due to its much lower rigidity.

In conclusion, due to the lack of modifications to the buildings current seismic/wind resisting elements, it is our professional opinion that a wind/seismic analysis is not necessary. By adding plywood sheathing to the floors which is improving the existing floor diaphragms and by removing a portion of the interior brick we are reducing overall building weight which also reduces the overall seismic shear force on the building.

If you have any questions, please do not hesitate to call.

Sincerely;

Larry A. Wichroski, P.E.

