GENERAL NOTES

THE FOLLOWING BUILDING CODES AND STANDARDS SHALL BE REFERENCED DURING CONSTRUCTION:

1999 EDITION OF THE BOCA NATIONAL BUILDING CODE

AMERICAN SOCIETY OF CIVIL ENGINEERS, MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER

AMERICAN INSTITUTE OF STEEL CONSTRUCTION, MANUAL OF STEEL CONSTRUCTION, NINTH EDITION AMERICAN CONCRETE INSTITUTE SPECIFICATION FOR STRUCTURAL CONCRETE

ACI 318-95 AMERICAN CONCRETE INSTITUTE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE

ASTM AMERICAN SOCIETY OF TESTING AND MATERIALS NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION BY NATIONAL FOREST PRODUCTS

ASSOCIATION, 2001. REFERENCE ARCHITECTURAL PLANS FOR DIMENSIONS NOT SHOWN. REFERENCE MECHANICAL ELECTRICAL. AND ARCHITECTURAL PLANS FOR SIZES AND LOCATIONS OF WALL AND SLAB OPENINGS.

DUCTS, PIPING, CURBS, AND EQUIPMENT PADS. IN THE EVENT OF A CONFLICT BETWEEN THE Drawings, specifications, or notes on the drawings, the engineer shall be notified prior

EXISTING DIMENSIONS AND CONDITIONS ARE FOR REFERNCE ONLY. CONTRACTOR SHALL VERIFY ALL EXISTING CONSTRUCTION AND DIMENSIONS IN THE FIELD PRIOR TO CONSTRUCTION OR FABRICATION. ALL DISCREPANCIES SHALL BE REPORTED TO THE ENGINEER PRIOR TO COMMENCING WORK.

THE CONTRACTOR SHALL NOTIFY THE ENGINEER IF DEVIATIONS OR CHANGES ARE REQUIRED TO THE CONTRACT DOCUMENTS OR APPROVED SHOP DRAWINGS DUE TO INTERFERENCES, FABRICATION ERRORS, OR OTHER CAUSES.

THE STRUCTURE IS SELF—SUPPORTING AND STABLE AFTER THE ENTIRE BUILDING IS COMPLETELY CONSTRUCTED. the contractor is solely responsible for erection procedures and sequencing during CONSTRUCTION AND ERECTION TO PROVIDE AND ENSURE LOCAL AND OVERALL STABILITY OF THE BUILDING AND ITS COMPONENTS DURING CONSTRUCTION AND ERECTION. THE CONTRACTOR SHALL RETAIN A LICENSED STRUCTURAL ENGINEER TO DESIGN TEMPORARY BRACING/SHORING AND DETERMINE WHERE THE TEMPORARY BRACING/SHORING IS NEEDED.

DESIGN CRITERIA

SNOW LOADS:

GROUND SNOW LOAD, Pg = 60 PSF SNOW EXPOSURE FACTOR, Ce = 1.0 SNOW LOAD IMPORTANCE FACTOR, I = 1.0 FLAT ROOF SNOW LOAD, Pf = 42 PSF

LIVE LOADS:

MULTIPLE FAMILY DWELLINGS UNITS AND CORRIDORS = 40 PSF PUBLIC ROOMS AND CORRIDORS = 100 PSF

ROOF - TOP CHORD = 15 PSF- BOTTOM CHORD = 10 PSF

MAIN WNDFORCE-RESISTING SYSTEM: BASIC WND SPEED = 90 MPH

WIND LOAD IMPORTANCE FACTOR, I = 1.1 EXPOSURE CATEGORY = B WINDLOAD (WINDWARD + LEEWARD) = 20.7 PSF

SEISMIC CRITERIA:

EFFECTIVE PEAK VELOCITY-RELATED ACCELERATION, AV = .10 EFFECTIVE PEAK ACCELERATION, Ag = 0.10 SITE COEFFICIENT, S = 1.0

FOUNDATION RELATED EARTHWORK

SUBGRADE PREPARATION AND DETERMINATION (INCLUDING ALLOWABLE BEARING PRESSURE, STRUCTURAL FILL GRADATION REQUIREMENTS, AND COMPACTION REQUIREMENTS) BENEATH FOOTINGS AND SLABS-ON-GRADE AND BEHIND FOUNDATION WALLS SHALL BE PROVIDED BY A GEOTÉCHNICAL ENGINEER. ALL FILL USED TO SUPPORT FOUNDATIONS AND SLABS-ON-GRADE SHALL CONSIST OF A WELL-GRADED, GRANULAR NATERIAL PER THE RECOMMENDATIONS OF THE GEOTECHNICAL ENGINEER. STRUCTURAL SLABS SHALL BE CONSTRUCTED ON A MININUM 12-INCH THICK LAYER OF STRUCTURAL FILL SOIL WITH PROPERTIES PER THE GEOTECHNICAL ENGINEER.

PRESUNED ALLOWABLE SOIL BEARING PRESSURE USED IN DESIGN = 3500 PSF.

BEARING CAPACITIES SHALL BE VERIFIED BY GEOTECHNICAL ENGINEER. MINIMUM FROST DEPTH COVER = 4'-0'' FOR EXTERIOR FOOTINGS BELOW FINAL EXTERIOR GRADE. CONTRACTOR SHALL NOTIFY ENGINEER OF ANY DISCREPANCIES.

FOUNDATIONS SHALL BEAR ON UNDISTURBED NATIVE SOIL, UNLESS NOTED OTHERWISE, BEARING ELEVATIONS SHALL BE LOWERED WHERE SUITABLE SOILS ARE NOT ENCOUNTERED. WHERE OVEREXCAVATION HAS OCCURRED. CONTRACTOR MAY PLACE LEAN CONCRETE ON TOP OF NATIVE SOIL. THE CONTRACTOR SHALL NOTIFY THE GEOTECHNICAL AND STRUCTURAL ENGINEER IF ANY UNSUITABLE SOILS ARE ENCOUNTERED PRIOR TO PLACING FOUNDATIONS.

FOUNDATION WALLS SHALL BE BACKFILLED SIMULTANEOUSLY ON BOTH SIDES OF THE WALL. FOUNDATION WALLS AND SLAB-ON-GRADES SHALL REACH THEIR FULL 28 DAY COMPRESSIVE STRENGTH PRIOR TO BACKFILLING. THE THE CONTRACTOR SHALL PROVIDE TEMPORARY SHORING/BRACING FOR WALLS WHEN BACKFILL IS PLACED PRIOR TO CONCRETE ACHIEVING ITS FULL 28 DAY STRENGTH. BACKFILL FOR FOUNDATION WALLS IS BASED ON DRAINED CONDITIONS. SEE ARCHITECTURAL, CIVIL, AND MECHANICAL DRAWNGS FOR FOUNDATION DRAINAGE SYSTEM.

PROTECT FOUNDATIONS FROM FROST AND KEEP BOTTON OF TRENCH DRY DURING CONSTRUCTION. IF GROUNDWATER IS ENCOUNTERED NEAR OR ABOVE THE BASE OF THE FOOTINGS, EXCAVAIONS SHALL BE DEWATERED DURING CONSTRUCTION. SURFACE WATER SHALL BE DIVERTED AWAY FROM EXCAVATIONS.

CONTRACTOR SHALL BE RESPONSIBLE FOR THE SHORING AND BRACING OF EXISTING STRUCTURES DURING EXCAVATION, BACKFILLING, AND CONSTRUCTION. CONTRACTOR SHALL SLOPE EXCAVATIONS TO ACHIEVE SOIL

CONCRETE REINFORCEMENT

USE DEFORMED BILLET-STEEL REINFORCING BARS, GRADE 60, IN CONFORMANCE WITH ASTM A 615. REINFORCEMENT SHALL BE ACCURATELY PLACED AND SUPPORTED PRIOR TO CONCRETE PLACEMENT, AND SHALL BE SECURED AGAINST DISPLACEMENT.

THE CONTRACTOR SHALL SUBMIT REINFORCING SHOP DRAWINGS TO THE ENGINEER FOR REVIEW AND ACCEPTANCE PRIOR TO COMMENCING FABRICATION. REINFORCEMENT SHALL BE DETAILED IN ACCORDANCE WITH ACI "MANUAL OF STANDARD PRACTICE FOR DETAILING OF REINFORCED CONCRETE STRUCTURES". SHOP DRAWINGS SHALL SHOW REINFORCING STEEL PLACEMENT DETAILS AND SECTIONS.

| MINIMUM CONCRETE COVER FOR REINFORCEMENT | |
|---|--------------|
| CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH | 3 INCHES |
| CONCRETE EXPOSED TO EARTH OR WEATHER | 2 INCHES |
| CONCRETE NOT EXPOSED TO EARTH OR WEATHER IN SLABS AND WALLS (FOR PRIMARY REINFORCEMENT, TIES, AND STIRRUPS) | 1-1/2 INCHES |
| CONCRETE NOT EXPOSED TO EARTH OF WEATHER IN COLUMNS AND BEAMS | 1-1/2 INCHES |

CONTINUOUS REINFORCEMENT SHALL BE TENSION LAP SPLICED PER LAP SPLICE LENGTH TABLE, UNLESS OTHERWISE NOTED.

| LAP SPLICE LENGTH TABLE | | | | | | | |
|-------------------------|------------|-------------|----------------|-------------------|----------------------|-------------------------|----------------------------|
| BAR SIZE | ‡ 3 | # 4 | ‡ 5 | # 6 | # 7 | #8 | #9 |
| MIN LAP SPLICE (INCHES) | 18 | 24 | 30 | 36 | 48 | 64 | 81 |
| | BAR SIZE | BAR SIZE #3 | BAR SIZE #3 #4 | BAR SIZE #3 #4 #5 | BAR SIZE #3 #4 #5 #6 | BAR SIZE #3 #4 #5 #6 #7 | BAR SIZE #3 #4 #5 #6 #7 #8 |

WELDING OF REINFORCEMENT IS NOT PERMITTED, UNLESS OTHERWISE NOTED.

CONCRETE NOTES

ALL CONCRETE WORK, INCLUDING MATERIAL SELECTION, ADMIXTURES, MIXING, AND PLACEMENT OF CONCRETE SHALL BE IN CONFORMANCE WITH APPLICABLE BUILDING CODES. IN ADDITION, REFERENCE THE FOLLOWING CONCRETE STANDARDS AND SPECIFICATIONS:

AMERICAN CONCRETE INSTITUTE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE AMERICAN CONCRETE INSTITUTE SPECIFICATIONS FOR STRUCTURAL CONCRETE STANDARD SPECIFICATIONS FOR HOT WEATHER CONCRETING

STANDARD SPECIFICATION FOR COLD WEATHER CONCRETING STANDARD PRACTICE FOR CURING CONCRETE

REQUIRED CONCRETE PARAMETERS ARE AS FOLLOWS:

| LOCATION | MAX W/C RATIO | ťc |
|--|---------------|-----------|
| FOUNDATIONS, FOOTINGS & FOUNDATION WALLS | .52 | 3,000 PSI |
| SLAB-ON-GRADE | .50 | 4,000 PSI |

W/C = WATER TO CEMENT RATIO AND t'c = COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS

MAXIMUM AGGREGATE SIZE SHALL BE 3/4 INCH, IN CONFORMANCE WITH ASTM C33. USE PORTLAND CEMENT TYPE II, IN CONFORMANCE WITH ASTM 150. AIR ENTRAINING ADMIXTURES SHALL CONFORM TO ASTM C 260.

ADMIXTURES SHALL CONFORM TO "SPECIFICATION FOR CHEMICAL ADMIXTURES FOR CONGRETE" ASTM C 494. FLY ASH USED AS ADMIXTURES SHALL CONFORM TO ASTM C 618. CALCIUM CHLORIDE OR ADMIXTURES CONTAINING CALCIUM CHLORIDE IS NOT PERMITTED

MAXIMUM SLUMP AFTER THE ADDITION OF A WATER-REDUCING ADMIXTURE IS 8 INCHES.

CONCRETE EXPOSED TO FREEZING AND THAWING, INCLUDING FOUNDATIONS, FOOTINGS, FOUNDATION WALLS, AND EXTERIOR WALKWAYS SHALL BE AIR ENTRAINED WITH AIR CONTENT BETWEEN 5% AND 6%. CONTRACTOR SHALL NOT PLACE CONCRETE ON FROZEN GROUND OR IN WATER. ADEQUATE EQUIPMENT SHALL BE PROVIDED FOR HEATING CONCRETE MATERIALS AND PROTECTING CONCRETE DURING NEAR-FREEZING OR FREEZING WEATHER. REFERENCE ACI 306, AS NOTED ABOVE, FOR RECOMMENDATIONS FOR COLD WEATHER CONCRETING.

CONTRACTOR SHALL SUBMIT PROPOSED CONCRETE MIX DESIGN AND LABORATORY TESTS OF FABRICATED CYLINDERS VERIFYING CONCRETE STRENGTH OR PERFORMANCE HISTORY OF MIX TO engineer for acceptance prior to placement of concrete. Concrete used on site shall BE FIELD TESTED IN ACCORDANCE WITH AND IN THE PRESENCE OF AN APPROVED TESTING AGENCY. FIELD TESTING INFORMATION SHALL INDICATE SLUMP. AIR CONTENT, AND TEMPERATURE. COMPRESSION TEST 1 CYLINDER AT 7 DAYS AND 2 AT 28 DAYS. HOLD AN ADDITIONAL CYLINDER FOR A 56 DAY BREAK, IF NECESSARY. PROVIDE A SET OF 4 CYLINDERS FOR EACH PLACEMENT AND PER 50 CUBIC YARDS OF CONCRETE PLACED. THE OWNER SHALL PAY FOR ALL CONCRETE TESTING.

CONSTRUCTION JOINTS IN WALLS SHALL BE PERMITTED AS DETAILED ON THE STRUCTURAL DRAWINGS. SURFACES OF CONCRETE CONSTRUCTION JOINTS SHALL BE CLEANED AND LAITANCE REMOVED. IMMEDIATELY BEFORE NEW CONCRETE IS PLACED, CONSTRUCTION JOINTS SHALL BE WETTED AND STANDING WATER REMOVED. VERTICAL CONSTRUCTION JOINTS IN WALLS SHALL NOT EXCEED A SPACING OF 40 FEET. WHERE ELECTRICAL CONDUIT/ RADIANT HEATING TUBES RUN IN THE SLAB, THEY SHALL BE LOCATED AT MID-DEPTH OF THE SLAB. ALUMINUM CONDUIT AND SLEEVES ARE NOT PERMITTED. anchor Bolts shall conform to astm a307. Anchor Bolts shall have heavy hex nuts and lock

STRUCTURAL STEEL

ALL STRUCTURAL STEEL WORK SHALL CONFORM TO:

AISC AMERICAN INSTITUTE OF STEEL CONSTRUCTION, MANUAL OF STEEL CONSTRUCTION, NINTH EDITION AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES

STRUCTURAL STEEL MEMBERS SHALL BE IN CONFORMANCE WITH THE FOLLOWING:

ASTN A572, GRADE 50 ANGLES. PLATES ASTM A36, Fy=36 KSI

STRUCTURAL TUBING ASTN A500. GRADE B. Fv=46 KSI ASTM A53, TYPE E OR S, GRADE B, Fy=35 KSI

SHOP DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW PRIOR TO COMMENCING FABRICATION. SHOP DRAWINGS SUBMITTALS SHALL INCLUDE: CERTIFIED MILL TEST REPORTS OF STRUCTURAL STEEL (INCLUDING NAMES AND LOCATIONS OF MILLS AND SHOPS).

CERTIFIED MILL TEST REPORTS OF BOLTS, NUTS AND WASHERS (INCLUDING NAMES AND LOCATIONS OF MILLS AND STRUCTURAL STEEL FABRICATION AND ERECTION DRAWINGS WHICH INCLUDE BOLTED CONNECTIONS (SHOP AND FIELD)

AND WELDED CONNECTIONS (SHOP AND FIELD) DEPICTING AWS WELDING SYMBOLS. METAL DECK SHOP DRAWINGS DEPICTING SHEAR STUD LAYOUT ON BEAMS AND GIRDERS.

OWNER SHALL RETAIN A QUALIFIED TESTING AGENCY TO PERFORM AND VERIFY THE FOLLOWING:

VISUAL INSPECTION OF ALL WELDS. ULTRASONIC TESTING, IN ACCORDANCE WITH ASTN E-164, ON 100% OF ALL FIELD FULL PENETRATION WELDS. PROVIDE RANDOM VERIFICATION WA ULTRASONIC TESTING OF SHOP FULL PENETRATION WELDS. FIELD BOLTED CONNECTIONS, INCLUDING VERIFICATION OF BOLT GRADES.

SHEAR STUD QUANTITY, PROPER INSTALLATION, SIZE, AND SPACING. SHEAR STUDS SHALL CONFORM TO AWS D1.1.

BOLTED CONNECTIONS

FIELD CONNECTIONS SHALL UTILIZE MINIMUM 3/4—INCH DIAMETER A325 HIGH STRENGTH BOLTS, UNO. BOLTED CONNECTION SHALL BE SUP CRITICAL (SC) AT ALL MOMENT FRAMES, BRACED FRAMES, AND AT ADDITIONAL LOCATIONS INDICATED IN THE DRAWINGS. SLIP CRITICAL CONNECTIONS SHALL UTILIZE LOAD INDICATOR WASHERS OR TENSION CONTROL BOLTS. BOLT HOLES SHALL BE STANDARD SIZE, UNO.

HIGH STRENGTH BOLTS SHALL BE INSTALLED AND TIGHTENED PER AISC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 BOLTS.

ANCHOR BOLTS SHALL CONFORM TO ASTM A307, GRADE A, STANDARD HEX HEAD FURNISHED WITH HEAVY HEX NUTS AND LOCK WASHERS.

CONTRACTOR SHALL DESIGN CONNECTIONS NOT ALREADY DETAILED ON STRUCTURAL DRAWINGS. DESIGN SHALL BE STAMPED BY A LICENSED STRUCTURAL ENGINEER AND SUBMITTED PRIOR TO COMMENCING FABRICATION.

WELDING SHALL CONFORM TO AWS D1.1. USE LOW-HYDROGEN SMAW ELECTRODES WITH MINIMUM TENSILE STRENGTH OF 70 KSI.

STRUCTURAL STEEL SHALL RECEIVE THE FOLLOWING PROTECTIVE COATINGS:

DO NOT PAINT SURFACES TO RECEIVE METAL DECK AND/ OR SHEAR CONNECTORS FASTENED BY WELDING, CONTACT SURFACES OF HIGH STRENGTH BOLTED CONNECTIONS, FINISHED BEARING SURFACES, AND SURFACES TO BE WELDED IN THE FIELD. IF REQUIRED, PROTECT THESE SURFACES BY RUST-INHIBITING COATING THAT CAN BE REMOVED EASILY PRIOR TO ERECTION

UNEXPOSED STRUCTURAL STEEL SHALL BE CLEANED IN ACCORDANCE WITH SSPC-SP3 AND PAINTED WITH PRINER PAINT, TNEMEC 10-99, OR EQUIVALENT, UNO.

EXPOSED STRUCTURAL STEEL TO RECEIVE ZINC-RICH EPOXY PAINT SHALL BE FIRST CLEANED IN ACCORDANCE WITH SSPC-SP6 ,COMMERCIAL BLAST CLEANING. USE TNEMEC ZIN-RICH EPOXY PAINT, OR EQUIVALENT. APPLY FINISH COAT PER ARCHITECT.

EXPOSED STRUCTURAL STEEL TO BE HOT-DIPPED GALVANIZED SHALL BE IN ACCORDANCE WITH ASTM A123. SHEAR CONNECTOR STUDS

SHEAR CONNECTOR STUDS SHALL BE NELSON, OR EQUIVALENT, 3/4-INCH DIAMETER, UNO. WELD STUDS PER STUD MANUFACTURER'S RECOMMENDATIONS THROUGH METAL DECKING. STUD LENGTH SHALL BE 1-INCH BELOW TOP OF CONCRETE SLAB ON DECK.

SHEAR STUDS, WHERE REQUIRED, ARE INDICATED ON THE DRAWINGS AS [XX], WHERE XX IS THE NUMBER OF STUDS EQUALLY SPACED BETWEEN SUPPORTS ON A BEAM OR GIRDER.

PREFABRICATED WOOD TRUSSES

WORK INCLUDED

FABRICATE, SUPPLY AND ERECT WOOD TRUSS AS SHOWN ON THE DRAWINGS AND AS SPECIFIED. WORK TO INCLUDE ANCHORAGE, BLOCKING, CURBING, MISCELLANEOUS FRAMING AND BRACING.

TRUSS: THE TERMS "TRUSS" AND "WOOD TRUSS COMPONENT" REFER TO OPEN WEB LOAD CARRYING ASSEMBLIES

SUITABLE FOR OF ROOF DECKS OR FLOORS IN BUILDINGS.

MANUFACTURER: A MANUFACTURER WHO IS REGULARLY ENGAGED IN DESIGN AND FABRICATION OF WOOD TRUSS COMPONENTS.

TRUSS INSTALLER: BUILDER, CONTRACTOR OR SUB-CONTRACTOR WHO IS RESPONSIBLE FOR THE FIELD STORAGE, HANDLING AND INSTALLATION OF TRUSSES.

TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THESE SPECIFICATIONS AND WHERE ANY APPLICABLE DESIGN FEATURE IS NOT SPECIFIED HEREIN, DESIGN SHALL BE IN ACCORDANCE WITH APPLICABLE PROVISIONS OF LATEST EDITION OF NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION (NDS), AMERICAN FOREST AND PAPER ASSOCIATION (AFPA), AND DESIGN SPECIFICATIONS FOR METAL PLATE CONNECTED WOOD TRUSSES (ANSI/TPI 1), TRUSS PLATE INSTITUTE (TPI), AND CODE OF JURISDICTION.

MANUFACTURER SHALL FURNISH DESIGN DRAWINGS BEARING SEAL AND REGISTRATION NUMBER OF A CIVIL OR STRUCTURAL ENGINEER LICENSED IN STATE WHERE TRUSSES ARE TO BE INSTALLED. DRAWINGS SHALL BE APPROVED BY ARCHITECT OR STRUCTURAL ENGINEER OF RECORD PRIOR TO FABRICATION.

TRUSS DESIGN DRAWINGS SHALL INCLUDE AS MINIMUM INFORMATION:

. SPAN, DEPTH OR SLOPE AND SPACING OF TRUSSES

REQUIRED BEARING WIDTH 3. DESIGN LOADS, AS APPLICABLE

A. TOP CHORD LIVE LOAD

B. TOP CHORD DEAD LOAD C. BOTTOM CHORD LIVE LOAD

D. BOTTOM CHORD DEAD LOAD E. CONCENTRATED LOADS AND THEIR POINTS OF APPLICATION

F. WND AND SEISMIC CRITERIA ADJUSTMENT TO LUMBER AND PLATE DESIGN LOADS AND CONDITION OF USE

REACTIVE FORCES, THEIR POINTS OF OCCURENCE AND DIRECTION

MANUFACTURER PLATE TYPE, GAGE, SIZE AND LOCATION OF PLATE AT EACH JOINT LUMBER SIZE, SPECIES AND GRADE FOR EACH MEMBER

LOCATION OF ANY REQUIRED CONTINUOUS LATERAL BRACING CALCULATED DEFLECTION RATIO AND/OR MAXIMUM DEFLECTION FOR LIVE AND TOTAL LOAD

10. MAXINUM AXIAL COMPRESSIVE FORCES IN TRUSS MEMBERS 11. LOCATION OF JOINTS

12. CONNECTION REQUIREMENTS FOR: A. TRUSS TO TRUSS GIRDERS B. TRUSS PLY TO PLY

C. FIELD SPLICES

MATERIALS

LUMBER USED FOR TRUSS MEMBERS SHALL BE IN ACCORDANCE WITH PUBLISHED VALUES OF LUMBER RULES WRITING AGENCIES APPROVED BY BOARD OF REVIEW OF AMERICAN LUMBER STANDARDS COMMITTEE. LUMBER SHALL BE IDENTIFIED BY GRADE MARK OF A LUNBER INSPECTION BUREAU OR AGENCY APPROVED BY THAT BOARD, AND SHALL BE AS SHOWN ON DESIGN DRAWINGS. 2. MOISTURE CONTENT OF LUMBER SHALL BE NO LESS THAN 7 PERCENT NOR GREATER THAN 19 PERCENT

AT TIME OF FABRICATION. ADJUSTMENT OF VALUES FOR DURATION OF LOAD OR CONDITIONS OF USE SHALL BE IN ACCORDANCE WITH

NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION (NDS). 4. FIRE RETARDENT TREATED LUMBER, IF APPLICABLE, SHALL MEET SPECIFICATIONS OF TRUSS DESIGN AND ANSI/TPI 1-1995, PAR 9.1.5 AND SHALL BE REDRIED AFTER TREATMENT IN ACCORDANCE WITH AWPA STANDARD C20. ALLOWABLE VALUES MUST BE ADJUSTED IN ACCORDANCE WITH NDS PAR 2.3.6 LUMBER TREATER SHALL SUPPLY CERTIFICATE OF COMPLIANCE.

METAL CONNECTOR PLATES

METAL CONNECTOR PLATES SHALL NOT BE LESS THAN 0.036 INCHES IN THICKNESS (20 GAGE) AND SHALL MEET OR EXCEED ASTM A653-94 GRADE 37, AND SHALL BE HOT DIPPED GALVANIZED ACCORDING TO ASTM A653-94, COATING DESIGNATION G60. WORKING STRESSES IN STEEL ARE TO BE APPLIED TO EFFECTIVE RATIOS FOR PLATES AS DETERMINED BY TEST IN ACCORDANCE WITH APPENDIX E AND F OF ANSI/TPI 1-1995. 2. IN HIGHLY CORROSIVE ENVIRONMENTS, SPECIAL APPLIED COATINGS OR STAINLESS STEEL MAY BE REQUIRED. AT THE REQUEST OF ARCHITECT OR STRUCTURAL ENGINEER OF RECORD, MANUFACTURER SHALL FURNISH A

TRUSSES SHALL BE FABRICATED IN A PROPERLY EQUIPPED MANUFACTURING FACILITY OF A PERMANENT NATURE. TRUSSES SHALL BE MANUFACTURED BY EXPERIENCED WORKMEN, USING PRECISION CUTTING, JIGGING AND PRESSING EQUIPMENT MEETING REQUIREMENTS OF ANSI/TPI 1-1995, SECTION 4. TRUSS MEMBERS SHALL BE ACCURATELY CUT TO LENGTH ANGLE AND TRUE TO LINE TO ASSURE PROPER FITTING JOINTS WITHIN TOLERANCES SET FORTH IN ANSI/TPI 1-1995, SECTION 4, AND PROPER FIT WITH OTHER WORK.

CERTIFIED RECORD THAT MATERIALS COMPLY WITH STEEL SPECIFICATIONS.

HANDLING, INSTALLATION AND BRACING

TRUSSES SHALL BE HANDLED DURING FABRICATION, DELIVERY AND AT JOBSITE SO AS NOT TO BE SUBJECTED TO EXCESSIVE BENDING.

Trusses shall be unloaded on smooth ground to avoid lateral strain. Trusses shall be protected FROM DAMAGE THAT MIGHT RESULT FROM ON-SITE ACTIVITIES AND ENVIRONMENTAL CONDITIONS. PREVENT TOPPLING WHEN BANDING IS REMOVED.

HANDLE DURING INSTALLATION IN ACCORDANCE WITH HANDLING, INSTALLING AND BRACING WOOD TRUSSES (BCSI 1-03), TPI. AND ANSI/TPI 1-1995. INSTALLATION SHALL BE CONSISTENT WITH GOOD WORKMANSHIP AND GOOD BUILDING PRACTICES AND SHALL BE THE RESPONSIBILITY OF THE TRUSS INSTALLER. APPARENT DAMAGE TO TRUSSES, IF ANY, SHALL BE REPORTED TO MANUFACTURER PRIOR TO INSTALLATION.

TRUSSES SHALL BE SET AND SECURED LEVEL AND PLUNB, AND IN CORRECT LOCATION. TRUSSES SHALL BE HELD IN CORRECT ALIGNMENT UNTIL SPECIFIED PERMANENT BRACING IS INSTALLED. CUTTING AND ALTERING OF TRUSSES IS NOT PERMITTED.

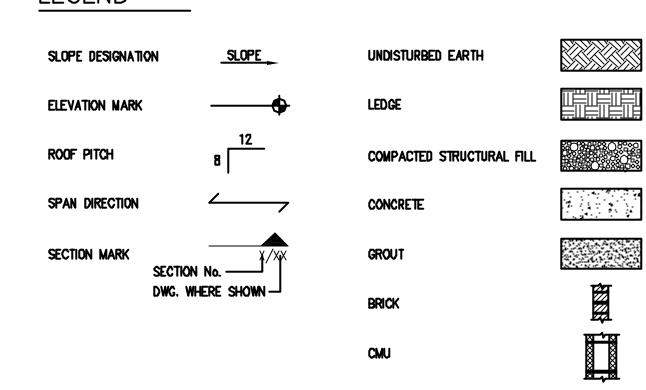
CONCENTRATED LOADS SHALL NOT BE PLACED ATOP TRUSSES UNTIL ALL SPECIFIED BRACING HAS BEEN INSTALLED AND DECKING IS PERMANENTLY NAILED IN PLACE. SPECIFICALLY AVOID STACKING FULL BUNDLES OF DECKING OR OTHER HEAVY MATERIALS ONTO UNSHEATHED TRUSSES.

ERECTION BRACING IS ALWAYS REQUIRED. PROFESSIONAL ADVICE SHOULD ALWAYS BE SOUGHT TO PREVENT TOPPLING OR DOMINOING OF TRUSSES DURING INSTALLATION.

THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING AND FURNISHING THE MATERIALS USED FOR INSTALLATION AND

LEGEND

PERMANENT BRACING.



WOOD NOTES

ALL LUMBER SHALL BE VISUALLY GRADED AND STAMPED WITH GRADE DESIGNATION, SPECIES, AND ADDITIONAL INSPECTION INFORMATION, U.N.O.

CARE SHALL BE TAKEN TO PROTECT TIMBER FROM WEATHER AND DAMPNESS. DO NOT STACK IN SUCH A WAY AS TO CAUSE WARPING OR PREVENT ADEQUATE AIR CIRCULATION.

WOOD GRADES AND SPECIES:

SPRUCE-PINE-FIR, No.1/No.2 OR BETTER FOR TYPICAL LUMBER (JOISTS, WALLS, ETC) U.N.O. USE SOUTHERN YELLOW PINE FOR EXTERIOR EXPOSURE APPLICATIONS AND WHERE SHOWN ON DRAWINGS AS

WHERE NOTED LYL ON DRAWINGS, PROVIDE VERSALAM MEMBERS BY BOISE CASCADE, OR EQUIVALENT, WHICH HAVE THE FOLLOWING MINIMUM ALLOWABLE STRESSES:

Fc = 1600 PSI (PARALLEL TO GRAIN) Fb = 2800 PSIFc = 900 PSI (PERPENDICULAR TO GRAIN) Fv = 290 PSI

E = 2,000,000 PSIFt = 2100 PSI

STRUCTURAL LUMBER SHALL HAVE A MAXIMUM MOISTURE CONTENT OF 19%.

PRESERVATIVE PRESSURE TREATED LUMBER (PT OR PPT).

PROVIDE PRESSURE TREATED LUMBER FOR ALL LUMBER IN CONTACT WITH MASONRY OR CONCRETE. NOMINAL SIZES ARE TYPICALLY REFERENCED ON THE DRAWINGS. PROVIDE ACTUAL SIZES AS SET FORTH IN U.S.

DEPARTMENT OF COMMERCE VOLUNTARY PRODUCT STANDARD PS20-99.

ALL PLYWOOD SHALL BE APA RATED CDX SHEATHING:

USE 1/2-INCH PLYWOOD WALL SHEATHING. ATTACH PLYWOOD WITH LONG SIDE PERPENDICULAR OR PARALLEL TO WALL STUDS. STAGGER PANEL ENDS AND BLOCK ALL PANEL EDGES. USE 3/4-INCH PLYWOOD FLOOR SHEATHING. ATTACH PLYWOOD WITH LONG SIDE PERPENDICULAR TO FRAMING. STAGGER PANEL ENDS.

PROVIDE FULL DEPTH BLOCKING AT ENDS AND INTERIOR SUPPORTS OF ALL JOISTS AND RAFTERS WHERE JOISTS AND RAFTERS FRAME OVER SUPPORTS. PROVIDE 1x3 DIAGONAL BRIDGING OR FULL DEPTH SOLID BLOCKING FOR EACH 8'-0" OF SPAN FOR ALL JOISTS AND RAFTERS.

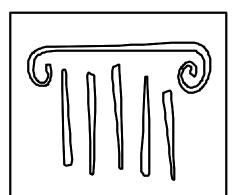
FASTENERS SHALL COMPLY WITH RECOMMENDED FASTENING SCHEDULE PER BOCA TABLE 2305.2, U.N.O. ON DRAWINGS. SPIKE TOGETHER ALL FRAMING MEMBERS WHICH ARE BUILT-UP USING A MINIMUM OF 2-ROWS OF 16d NAILS AT 12" O.C. STAGGERED, UNLESS OTHERWISE NOTED IN BOCA OR ON THE DRAWINGS. NAIL MULTIPLE LVL'S TOGETHER AS RECOMMENDED BY THE MANUFACTURER USING A MINIMUM OF 2-ROWS OF 16d NAILS AT 12"O.C. STAGGERED. ALL FASTENERS, NUTS, AND WASHERS SHALL BE HOT-DIPPED GALVANIZED.

ALIGN COLUMNS SUCH THAT COLUMNS BEAR CONTINUOUSLY TO FOUNDATION SUPPORT.

BLOCK ALL LOAD BEARING WALLS VERTICALLY AT 4'-0" O.C. MAXIMUM.

ABBREVIATIONS

| AB | ANCHOR BOLT | ı | ANGLE | |
|--------------------|---------------------------------------|---------------------------------------|--|------------------|
| ADDL | ADDITIONAL | L LL | DOUBLE ANGLE | • |
| | | ĹB | POUND | • |
| ARCH | ARCHITECT | ĹĔ | LINEAR FOOT | |
| % | AND | ĽĹΗ | ĽÖŇĠ ĹĖĠŠ HO | RIZONTAL |
| | | ĪĪΫ | LINEAR FOOT LONG LEGS HO LONG LEGS VEI | RTICAL |
| /FTG, BOF | BOTTOM OF FOOTING | | | |
| LDG | BUILDING | MAX | MAXIMUM | |
| M | BEAM | MECH MFR | MECHANICAL | _ |
| OT | BOTTOM | MFR | MANUFACTURE | ₹ |
| 3RG | BEARING | MIN | MINIMUM | _ |
| BTWN | BETWEEN | MISC | MISCELLANEOUS | 5 |
| •••• | | | | |
| ; | STRUCTURAL STEEL CHANNEL | NF | NEAR FACE | |
| ANT | CANTILEVER | NO | NUMBER | |
| JP 9K | CAST-IN-PLACE CONCRETE | NS | NEAR SIDE | |
| ม | CONTROL JOINT | NTS | NOT TO SCALE | |
| Ĺ | CENTERLINE | • | A.I. A.E.I.TED | |
| L R | CLEAR | <u>0C</u> | ON CENTER | |
| ZMÙ | CONCRETE MASONRY UNIT | OF | OUTSIDE FACE | |
| NJ | CONSTRUCTION JOINT | OPNG | | |
| OL | COLUMN | OPP | OPPOSITE | |
| ONC | CONCRETE | | | |
| CONN | CONNECTION | P | PIER DESIGNATI | ION |
| CONT | CONTINUOUS | PL | PLATE | |
| CONTR | CONTRACTOR | PP | PARTIAL PENET | RATION WELD |
| P | COMPLETE PENETRATION WELD | PREF. | | |
| CTR, CEN | CENTER | PSF | POUNDS PER S | SQUARE FOOT |
| CY CLIV | CUBIC YARD | PSI | POUNDS PER S | SQUARE INCH |
| | | | | |
| AK | DIAMETER | REINF | REINFORCING S | TFFI |
| MC | DIMENSION | REQ, | | • |
| XISCONT | DISCONTINUOUS DRAWING | · · · · · · · · · · · · · · · · · · · | THE THE STATE OF T | |
| WG | DRAWING | SC | SLIP CRITICAL | |
| E), EX, EXIST | FYISTING | SECT | | |
| EA LA LAIST | EACH | SHEA" | | |
| F | EACH FACE | SIM | SIMILAR | |
| F L, ELEV | ELEVATION | SOG SOG | | \ E |
| Q | EQUAL | SPAC | | Æ |
| QUIP | EQUIPMENT | SPAC SPECS | | • |
| Š | EACH SIDE | SFEU: SS | STAINLESS STE | |
| S W | EACH SIDE EACH WAY | STD | STANDARD | .EL |
| EXP | EXPANSION | STIFF | | |
| EXT | EXTERIOR | | | |
| | | STL STR | STEEL STRAIGHT | |
| . | FOOTING DESIGNATION | | | |
| <u>D</u> N | FOUNDATION FINISH FLOOR | STRUC | CT STRUCTURAL | |
| F | FINISH FLUUR | T | TAD | |
| T.G T.R | FLANGE | Ţ, | TOP AS | |
| · | FLOOR FOOT FOOTING | Ţ/ | TOP OF | 2014 |
| T _G | F ŏŏting | T&B | TOP AND BOTT | UM |
| ·γ | FIELD VERIFY | TOC, | T/CONC TOP OF CONCR | <u>(E</u> TE |
| • | 1.2.2.1 | | G, TOF TOP OF FOOTIN | 1G |
| GA | GAGE | TEMP | TEMPERATURE | |
| GALV | GALVANIZED | T/SHE | ELF TOP OF SHELF | |
| 100 1100:- | | Ĭ <u>/SL</u> / | AB TOP OF SLAB TOP OF STEEL | |
| HOR, HORIZ HSS | HORIZONTAL HOLLOW STRUCTURAL SHAPE | T/STL | | |
| 4 <u>5</u> 5 41 | HOLLOW STRUCTURAL SHAPE HEIGHT | ŢŜ | STRUCTURAL TO | UBING /\ |
| 11 | TEIST I | <u>T/Wai</u> | LL TOP OF WALL | <u> </u> |
| : | INSIDE FACE | TYP | TYPICAL | |
| N | INCH | | | |
| NFO | INFORMATION | UNO | UNLESS NOTED | OTHERWISE |
| - | | - | | |
| T | JOINT | VER, | VERT VERTICAL | |
| , | KID (MEIKUT) | VIF | VERIFY IN FIELD | D |
| , , | KIP (WEIGHT) KIPS PER SQUARE INCH | | | |
| (SI | NIFO PER OWNAKE INCH | W | STRUCTURAL S | TEEL WIDE FLANGE |
| | | w/ | WITH | |
| | | w /o | WITHOUT | |
| | | ₩/U | WI ITUU I | |



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