



CITY OF PORTLAND
BUILDING CODE CERTIFICATE
389 Congress St., Room 315
Portland, Maine 04101

TO: Inspector of Buildings City of Portland, Maine
Department of Planning & Urban Development
Division of Housing & Community Service

FROM: NEILL AND GUNTER INCORPORATED

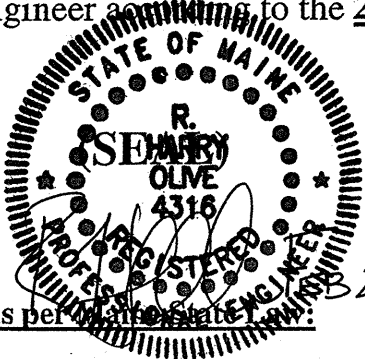
RE: Certificate of Design

DATE: FEBRUARY 24, 2005

These plans and / or specifications covering construction work on:

FOUNDATIONS FOR RUBB BUILDING SYSTEMS PRE-ENGINEERED
BUILDING - PORTLAND INTERNATIONAL MARINE TERMINAL

Have been designed and drawn up by the undersigned, a Maine registered Architect / Engineer according to the 2003 International Building Code and local amendments.



As per Maine State Engineer:

Signature: [Handwritten Signature]

Title: STRUCTURAL ENGINEER

Firm: NEILL AND GUNTER INCORPORATED

Address: 482 PAYNE ROAD
SCARBOROUGH, ME 04074

\$50,000.00 or more in new construction, repair expansion, addition, or modification for Building or Structures, shall be prepared by a registered design Professional.

FROM DESIGNER: R. HARRY OLIVE, P.E.
 DATE: FEBRUARY 24, 2005
 Job Name: FOUNDATIONS FOR RUBB BUILDING SYSTEMS PRE-ENGINEERED BLDG.
 Address of Construction: PORTLAND INTERNATIONAL MARINE TERMINAL

2003 International Building Code

Construction project was designed according to the building code criteria listed below:

Building Code and Year BC 2003 Use Group Classification(s) _____
 Type of Construction FOUNDATIONS - DESIGNED TO RESIST LOADS PROVIDED BY RUBB BUILDING SYSTEMS (SEE ATTACHED SHEET)
 Will the Structure have a Fire suppression system in Accordance with Section 903.3.1 of the 2003 IRC _____
 Is the Structure mixed use? _____ if yes, separated or non separated (see Section 302.3) _____
 Supervisory alarm system? _____ Geotechnical/Soils report required?(See Section 1802.2) _____

STRUCTURAL DESIGN CALCULATIONS

_____ Submitted for all structural members
 (106.1, 106.1.1)

DESIGN LOADS ON CONSTRUCTION DOCUMENTS
 (1603)

Uniformly distributed floor live loads (1603.1.1, 1607)

Floor Area Use	Loads Shown
_____	_____
_____	_____
_____	_____
_____	_____
_____	_____

Wind loads (1603.1.4, 1609)

_____ Design option utilized (1609.1.1, 1609.6)
 _____ Basic wind speed (1609.3)
 _____ Building category and wind importance factor, I_w (Table 1604.5, 1609.5)
 _____ Wind exposure category (1609.4)
 _____ Internal pressure coefficient (ASCE 7)
 _____ Component and cladding pressures (1609.1.1, 1609.6.2.2)
 _____ Main force wind pressures (1609.1.1, 1609.6.2.1)

Earthquake design data (1603.1.5, 1614 - 1623)

_____ Design option utilized (1614.1)
 _____ Seismic use group ("Category") (Table 1604.5, 1616.2)
 _____ Spectral response coefficients, S_{DS} & S_{D1} (1615.1)
 _____ Site class (1615.1.5)

_____ Live load reduction
 (1603.1.1, 1607.9, 1607.10)

_____ Roof live loads (1603.1.2, 1607.11)

_____ Roof snow loads (1603.1.3, 1608)

_____ Ground snow load, P_g (1608.2)

_____ If $P_g > 10$ psf, flat-roof snow load, P_f
 (1608.3)

_____ If $P_g > 10$ psf, snow exposure factor, C_e
 (Table 1608.3.1)

_____ If $P_g > 10$ psf, snow load importance factor, I_s (Table 1604.5)

_____ Roof thermal factor, C_t (Table 1608.3.2)

_____ Sloped roof snowload, P_s (1608.4)

_____ Seismic design category (1616.3)

_____ Basic seismic-force-resisting system
 (Table 1617.6.2)

_____ Response modification coefficient, R ,
 and deflection amplification factor, C_d
 (Table 1617.6.2)

_____ Analysis procedure (1616.6, 1617.5)

_____ Design base shear (1617.4, 1617.5.1)

_____ Flood loads (1603.1.6, 1612)

_____ Flood hazard area (1612.3)

_____ Elevation of structure

Other loads

_____ Concentrated loads (1607.4)

_____ Partition loads (1607.5)

_____ Impact loads (1607.8)

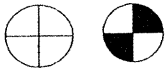
_____ Misc. loads (Table 1607.6, 1607.6.1,
 1607.7, 1607.12, 1607.13, 1610,
 1611, 2404)

60' BVE w/ 19.7 Leg with metal clad sidewalls.

The following estimated foundation forces are the maximum values of all applicable load case combinations and are based upon the following assumptions:

- Bay Spacing:** 12'-6"
- Wind Bracing:** One (1) X -Braced Bay Each End
- Basic Wind Speed:** 100 mph 3 sec gust, Exp. C, IBC 2003
- Live Roof Load:** 30 psf
- Ground Snow Load:** 50 psf
- Mechanical Load:**
- Dead Load:** 5 psf

I) Unbraced Sidewall Leg Location



- A. D + Live Load: $F_x = 4.7k$ Lateral
 $F_y = 15.4k$ Downward
- B. D + Wind Load: $F_x = 3.1k$ Lateral
 $F_y = 4.7k$ Net Uplift

II) Corners and Second Span From Ends



- A. D + Gable Wind: $F_x = 1.0k$ Lateral
 $F_y = 9.0k$ Net Uplift
 $F_z = 6.6k$ Lateral
- or
- $F_x = 4.1k$ Lateral
 $F_y = 21.4$ Downward
 $F_z = 0.0k$ Lateral

III) Gable End Wall Columns:

(Assuming 3 Columns per endwall)



- A. Gable Wind: $F_z = 4.0k$ Lateral

Notes:

- 1) The forces in I are for one 12'-6" bay length. The loads are point loads at each column.
- 2) Forces in II and III are all point loads at span or column locations.