GENERAL NOTES

- 1. THE NOTES ON THESE DRAWINGS ARE NOT INTENDED TO REPLACE SPECIFICATIONS. SEE SPECIFICATIONS FOR REQUIREMENTS IN ADDITION TO GENERAL NOTES. INCONSISTENCIES BETWEEN THESE DRAWINGS AND THE SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT PRIOR TO PROCEEDING WITH THE AFFECTED PORTION OF THE WORK.
- STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH JOB SPECIFICATIONS AND ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND SITE DRAWINGS. CONSULT THESE DRAWINGS FOR LOCATIONS AND DIMENSIONS OF OPENINGS, CHASES, INSERTS, REGLETS, SLEEVES, DEPRESSIONS, AND OTHER DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS.
- 3. ALL DIMENSIONS. EXISTING CONDITIONS. AND AS-BUILT CONDITIONS MUST BE VERIFIED IN THE FIELD. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK.
- 4. THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE ONLY AFTER THE STRUCTURAL WORK CONTAINED IN THE S— DRAWINGS IS COMPLETED. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO ENSURE THE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS OR TIEDOWNS. SUCH MATERIAL SHALL REMAIN THE PROPERTY OF THE CONTRACTOR AFTER COMPLETION OF THE PROJECT.
- SECTIONS AND DETAILS SHOWN ON ANY STRUCTURAL DRAWINGS SHALL BE CONSIDERED TYPICAL FOR SIMILAR CONDITIONS AS DETERMINED BY THE STRUCTURAL ENGINEER. THE STRUCTURAL ENGINEER RESERVES THE RIGHT TO INTERPRET DETAILS TO ADDRESS OTHER PROJECT CONDITIONS.
- 6. PROVIDE AND INSTALL NECESSARY MATERIAL TO CONNECT ELEVATOR SUPPORT BEAMS AND GUIDE RAILS. LOCATION AND SIZE OF MEMBERS AND ANY INSERTS REQUIRED SHALL BE DETERMINED BY THE ELEVATOR
- 7. THE CONTRACTOR SHALL SUBMIT COMPLETE SHOP DRAWINGS FOR ALL PARTS OF THE WORK, INCLUDING DESCRIPTION OF SHORING, AND CONSTRUCTION METHODS AND SEQUENCING WHERE APPLICABLE. NO PERFORMANCE OF THE WORK INCLUDING. BUT NOT LIMITED TO. DEMOLITION OF EXISTING STRUCTURE, OR FABRICATION OR ERECTION OF NEW STRUCTURAL ELEMENTS, SHALL COMMENCE WITHOUT REVIEW OF THE SHOP DRAWINGS BY THE ARCHITECT AND ENGINEER. SUBMIT ONE COPY AND ONE SEPIA. COPY WILL BE REVIEWED AND SEPIA WILL BE RETURNED. FOR SHOP DRAWINGS AND SUBMITTALS REQUIRED, REFERENCE THE PROJECT SPECIFICATION.
- ALL APPLICABLE FEDERAL, STATE, AND MUNICIPAL REGULATIONS SHALL B FOLLOWED, INCLUDING THE FEDERAL DEPARTMENT OF LABOR OCCUPATIONAL SAFETY AND HEALTH ACT.
- 9. IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE (2006 EDITION, SECTION 1704.1). A STATEMENT OF SPECIAL INSPECTIONS IS REQUIRED AS A CONDITION FOR PERMIT ISSUANCE BY THE LOCAL CODE OFFICIAL. THIS STATEMENT SHALL INCLUDE A COMPLETE LIST OF MATERIALS AND WORK REQUIRING SPECIAL INSPECTIONS, THE INSPECTIONS TO BE PERFORMED AND A LIST OF THE INDIVIDUALS, APPROVED AGENCIES AND FIRMS INTENDED TO BE RETAINED FOR CONDUCTING SUCH INSPECTIONS.
- 10. REFERENCE THE PROJECT SPECIFICATIONS FOR ALL TESTING REQUIREMENTS.

<u>DESIGN LOADS</u>

- 1. BUILDING CODE: INTERNATIONAL BUILDING CODE, 2006 EDITION ASCE 7-05 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES.
- DESIGN FLOOR LIVE LOADS: OFFICES: 50 PSF + 15 PSF PARTITION ALLOWANCE CORRIDORS ABOVE THE FIRST FLOOR: 80 PSF LOBBIES AND CORRIDORS AT FIRST FLOOR: 100 PSF
- DESIGN ROOF SNOW LOAD: GROUND SNOW LOAD (Pg): SNOW EXPOSURE FACTOR (Ce): SNOW LOAD IMPORTANCE FACTOR (Is): 1.0 SNOW LOAD THERMAL FACTOR (Ct): 1.1 FLAT ROOF SNOW LOAD (Pf): 42 PSF + DRIFT
- 4. DESIGN WIND LOAD: BASIC WIND SPEED: 100 MPH WIND LOAD IMPORTANCE FACTOR (IW): 1.0 WIND EXPOSURE: INTERNAL PRESSURE COEFFICIENT: COMPONENTS & CLADDING PER ASCE 7-05
- DESIGN SEISMIC LOADS: LATERAL FORCE RESISTING SYSTEM IS EXISTING PLAIN MASONRY SHEARWALLS. SEISMIC UPGRADE OF THE STRUCTURE IS NOT REQUIRED. PER CITY OF PORTLAND (AHJ) AUTHORIZATION. SOUTHEAST WALL PERMITTED TO BE ALTERED AND REINFORCED TO MEET 2006 IBC WITHOUT UPGRADING REMAINDER OF STRUCTURE. NEW MASONRY SHAFTS ARE VOLUNTARY IMPROVEMENTS TO THE LATERAL FORCE RESISTING SYSTEM. THE FOLLOWING CRITERIA APPLIES TO THE SOUTHEAST WALL AND MASONRY SHAFTS:

EQUIVALENT LATERAL FORCE PROCEDURE SEISMIC USE GROUP: SEISMIC IMPORTANCE FACTOR (Ie): 1.0 MAPPED SPECTRAL RESPONSE ACCELERATIONS:

Ss: 0.314 S1: 0.077 SEISMIC SITE CLASS: E SPECTRAL RESPONSE COEFFICIENTS: Sds: 0.481 Sd1: 0.179

SEISMIC DESIGN CATEGORY: C BASIC STRUCTURAL SYSTEM: BEARING WALL SYSTEM BASIC SEISMIC FORCE RESISTING SYSTEM: ORDINARY REINFORCED CONCRETE SHEARWALLS SPECIALLY REINFORCED MASONRY SHEARWALLS

RESPONSE MODIFICATION FACTOR (R): X: 4.0 SEISMIC RESPONSE COEFFICIENT (Cs): X: 0.120

CONCRETE NOTES

- CONCRETE WORK SHALL CONFORM TO "ACI MANUAL OF CONCRETE PRACTICE". LATEST EDITION. THIS PUBLICATION IS AVAILABLE THROUGH THE AMERICAN CONCRETE INSTITUTE
- 2. ALL CONCRETE SHALL HAVE A 28-DAY COMPRESSIVE STRENGTH OF 3,000 PSI, U.N.O. EXTERIOR SLAB-ON-GRADE SHALL HAVE A 28-DAY COMPRESSIVE STRENGTH OF 4.500 PSI ADDITIONAL CONCRETE MIX PERFORMANCE DATA INCLUDING AIR CONTENT, WATER—CEMENT RATIO. AGGREGATE SIZE. SLUMP. ETC. HAS BEEN INCLUDED IN THE PROJECT SPECIFICATIONS. SEE THE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS
- CONCRETE SHALL NOT BE PLACED IN WATER OR ON FROZEN GROUND.
- 4. PROVIDE PVC SLEEVES WHERE PIPES PASS THROUGH EXTERIOR CONCRETE, OR SLABS.
- 5. REINFORCING BARS SHALL CONFORM TO ASTM A615 GRADE 60 DEFORMED BARS AND SHALL BE DETAILED, FABRICATED AND PLACED IN ACCORDANCE WITH ACI 315, LATEST
- 6. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185 AND BE PROVIDED IN FLAT
- 7. FIBER REINFORCEMENT SHALL BE TYPE III SYNTHETIC VIRGIN HOMOPOLYMER POLYPROPYLENE FIBERS CONFORMING TO ASTM C1116.
- 8. MINIMUM CONCRETE PROTECTIVE COVERING FOR REINFORCEMENT, UNLESS NOTED OTHERWISE, SHALL BE AS FOLLOWS:
- A.SURFACES CAST AGAINST AND PERMANENTLY IN CONTACT WITH EARTH, 3.0" B.FORMED SURFACES IN CONTACT WITH EARTH OR EXPOSED TO WEATHER #5 BARS, 5/8" DIAMETER WIRE AND SMALLER, 1.5" #6 THROUGH #11 BARS. 2.0" C.SURFACES NOT IN CONTACT WITH EARTH OR EXPOSED TO WEATHER
- WALLS, SLABS, JOISTS #11 BARS AND SMALLER, 1.0" BEAMS. GIRDERS. AND COLUMNS: ALL REINFORCEMENT. 1.5" 9. REINFORCEMENT SHALL BE CONTINUOUS AROUND CORNERS AND AT INTERSECTIONS. PROVIDE LAPPED BARS AT NECESSARY SPLICES OR HOOKED BARS AT DISCONTINUOUS
- 10. WELDING OF REINFORCEMENT IS NOT PERMITTED

OTHERWISE SHOWN ON PLAN.

11. FOR ALL OPENINGS IN CONCRETE WALLS AND SLABS, PROVIDE SUPPLEMENTAL REINFORCING AROUND OPENING AS SHOWN ON THE CONTRACT DOCUMENTS TYPICAL DETAILS. NO PENETRATIONS SHALL BE MADE THROUGH FOOTINGS WITHOUT WRITTEN PERMISSION FROM

ENDS. PROVIDE TENSION LAP SPLICES PER THE SCHEDULE, FOR ALL REINFORCING UNLESS

- 12. CONSTRUCTION JOINTS ARE SHOWN ON DRAWINGS ARE MANDATORY. OMISSIONS, ADDITIONS, OR CHANGES SHALL NOT BE MADE EXCEPT WITH THE SUBMITTAL OF A WRITTEN REQUEST TOGETHER WITH DRAWINGS OF THE PROPOSED JOINT LOCATIONS FOR APPROVAL OF THE STRUCTURAL ENGINEER. WHERE CONSTRUCTION JOINTS ARE NOT SHOWN, OR WHEN ALTERNATE LOCATIONS ARE PROPOSED, DRAWINGS SHOWING LOCATION OF CONSTRUCTION AND CONTROL JOINTS AND CONCRETE PLACING SEQUENCE SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW PRIOR TO PREPARATION OF THE REINFORCEMENT SHOP PRAWINGS. CONCRETE SHALL BE PLACED WITHOUT HORIZONTAL CONSTRUCTION JOINTS EXCEPT WHERE SHOWN OR NOTED. VERTICAL CONSTRUCTION JOINTS AND STOPS IN CONCRETE BEAMS / GRADE BEAMS SHALL BE MADE AT MIDSPAN OR AT POINTS OF MINIMUM SHEAR, UNLESS NOTED OTHERWISE.
- 13. SPACING OF CONSTRUCTION JOINTS. UNLESS NOTED OTHERWISE SHALL BE AS FOLLOWS: A.FOOTINGS AND WALLS MAX LENGTH 40'-0" NOR 15'-0" FROM ANY CORNER**
- B.SLABS CON GRADE SEE FOUNDATION PLAN ** EXCEED ONLY WHERE INTERMEDIATE CONTRACTION JOINTS ARE PROVIDED. MINIMUM OF
- 72 HOURS SHALL ELAPSE BETWEEN ADJACENT CONCRETE PLACEMENTS. 14. ANCHOR RODS SHALL BE HEADED RODS CONFORMING TO ASTM F1554, GRADE 36 KSI
- BE IN CONTACT WITH PRESSURE TREATED LUMBER SHALL BE HOT-DIPPED GALVANIZED. 15. ALL GROUT BENEATH BASE PLATES & BEARING PLATES SHALL BE "5-STAR" 5000-PSI

NELDABLE STEEL, UNLESS NOTED OTHERWISE ON DRAWINGS. ANCHOR RODS THAT ARE TO

NON-SHRINK GROUT BY U.S. GROUT CORP. 16. SLAB THICKNESSES INDICATED ON THE DRAWINGS ARE MINIMUMS. PROVIDE SUFFICIENT CONCRETE TO ACCOUNT FOR STRUCTURE DEFLECTION, SUBGRADE FLUCTUATIONS, AND TO

OBTAIN THE SPECIFIED SLAB ELEVATION AT THE FLATNESS AND LEVELNESS INDICATED.

- 17. INSTALLATION OF REINFORCEMENT SHALL BE COMPLETED AT LEAST 24 HOURS PRIOR TO THE SCHEDULED CONCRETE PLACEMENT. NOTIFY SPECIAL INSPECTION COORDINATOR OF COMPLETION AT LEAST 24 HOURS PRIOR TO THE SCHEDULED COMPLETION OF THE INSTALLATION OF REINFORCEMENT.
- 18. ALL ITEMS TO BE EMBEDDED INTO CONCRETE SHALL BE INSTALLED PRIOR TO PLACEMENT OF CONCRETE. PROVIDE ADDITIONAL REINFORCEMENT AND/OR TEMPLATES AS REQUIRED TO ENSURE THE CORRECT POSITIONS OF EMBEDMENTS. "<u>WET SETTING" OF EMBEDMENTS INTO</u> CONCRETE IS PROHIBITED.

- THE METAL FLOOR DECK SHALL BE FORMED OF STEEL SHEETS CONFORMING TO ASTM STANDARD A611.
- 2. ROOF DECK SHALL BE AS NOTED ON THE DRAWINGS (OR EQUIVALENT).
- 3. FOR DECK ATTACHMENTS, PENETRATIONS AND ACCESSORIES REFER TO SPECIFICATIONS.

FOUNDATION NOTES (SOIL SUPPORTED)

- 1. FOUNDATIONS HAVE BEEN DESIGNED IN ACCORDANCE WITH A REPORT ENTITLED "GEOTECHNICAL ENGINEERING SERVICES, PROPOSED BUILDING RENOVATIONS, 258 COMMERCIAL STREET, PORTLAND, MAINE", DATED 08/06/2010, AND SUPPLEMENTAL LETTER DATED 09/15/2010 PREPARED BY S.W. COLE ENGINEERING, INC. THE RECOMMENDATIONS OF THE REPORT & SUPPLEMENTS ARE PART OF THIS WORK. REFER TO THIS REPORT FOR SPECIFIC RECOMMENDATIONS.
- . FOUNDATION DESIGN IS BASED ON DEEP FOUNDATIONS BEARING ON BEDROCK PER THE REQUIREMENTS OF THE GEOTECHNICAL REPORT. REFER TO THIS REPORT FOR SPECIFIC
- 3. EXTEND BOTTOM OF EXTERIOR FOOTINGS AT LEAST 4.5 FEET BELOW THE FINAL EXTERIOR GRADE FOR PROTECTION AGAINST FROST.
- 4. REFERENCE THE GEOTECHNICAL REPORT FOR ALL EXCAVATION, BACKFILL, COMPACTION, CONSTRUCTION DEWATERING AND PERMANENT DRAINAGE REQUIREMENTS.
- 5. SOILS EXPOSED AT THE BASE OF ALL SATISFACTORY FOUNDATION EXCAVATIONS SHOULD BE PROTECTED AGAINST ANY DETRIMENTAL CHANGE IN CONDITION. SUCH AS DISTURBANCE FROM RAIN OR FROST. SURFACE RUNOFF SHOULD BE DRAINED AWAY FROM THE EXCAVATIONS AND NOT BE ALLOWED TO POND. FOUNDATION EXCAVATIONS SHOULD BE ADEQUATELY PROTECTED FROM RAINFALL OR FREEZING CONDITIONS. GROUNDWATER SHOULD BE ANTICIPATED FOR EXCAVATIONS AND APPROPRIATE DEWATERING MEASURES SHALL BE
- 6. EXCAVATIONS FOR BUILDING CONSTRUCTION SHALL BE IN ACCORDANCE WITH OSHA REQUIREMENTS. BRACED EXCAVATIONS SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF MAINE. DO NOT UNDERMINE EXISTING FOUNDATIONS OF ANY ADJACENT STRUCTURES. REFER TO THE GEOTECHNICAL REPORT FOR ADDITIONAL AND/OR MORE SPECIFIC REQUIREMENTS.

STRUCTURAL STEEL NOTES

- . STRUCTURAL STEEL FABRICATION, ERECTION, AND CONNECTION DESIGN SHALL CONFORM TO AISC "SPECIFICATION FOR THE DESIGN FABRICATIONS, AND ERECTION OF STRUCTURAL STEEL" LATEST EDITION, AND THE "CODE OF STANDARD PRACTICE",
- 2. STRUCTURAL STEEL: STEEL PLATES, SHAPES, AND BARS, CONFORM TO ASTM A36 UNLESS NOTED OTHER WISE (U.N.O.). STRUCTURAL STEEL SHAPES DESIGNATED ON THE DRAWINGS FOR WIDE-FLANGE SECTIONS: ASTM A992 (ASTM A572 GRADE 50 WITH SPECIAL REQUIREMENTS PER AISC TECHNICAL BULLETIN #3 DATED MARCH,
- 3. STRUCTURAL TUBING: CONFORM TO ASTM A500 GRADE B46 KSI.
- 4. FIELD CONNECTIONS SHALL BE BOLTED USING ASTM A325N HIGH STRENGTH BOLTS (U.N.O.) EXCEPT WHERE SLIP CRITICAL CONNECTIONS ARE REQUIRED.
- 5. WHERE WELDING IS INDICATED, ALL WELDING SHALL CONFORM TO AWS D1.1-LATEST EDITION. ELECTRODES SHALL CONFORM TO AWS A5.1 E70XX SERIES WITH PROPER ROD TO PRODUCE OPTIMUM WELD (LOW HYDROGEN)
- 6. SEE CONCRETE NOTES AND DRAWINGS FOR ANCHOR BOLT INFORMATION, TYP.
- 7. PROVIDE 3/8" MINIMUM STIFFENER PLATES EACH SIDE OF BEAM WEB AT BEAMS FRAMING ÓVER COLUMNS AND AT BEAMS SUPPORTING COLUMNS ABOVE.
- 8. PROVIDE 1/4" THICK LEVELING PLATE UNDER ALL COLUMN BASE PLATES UNLESS OTHERWISE NOTED. LEVELING PLATES SHALL BE SET AND GROUTED PRIOR TO ERECTING COLUMNS.
- 9. PROVIDE ALL MISCELLANEOUS ANGLES, PLATES, ANCHOR BLOTS ETC., SHOWN ON ARCHITECTURAL DRAWINGS FOR SUPPORT OF BLOCKING, PARAPETS, FINISHES, ETC COORDINATE WITH MISCELLANEOUS METAL FABRICATOR TO ENSURE COMPLETE COVERAGE OF ALL ITEMS.
- 10. PROVIDE L 4 x 4 x 1/4 SLAB SUPPORT ANGLE AS REQUIRED AT COLUMNS WHERE STRUCTURAL MEMBERS DO NOT FRAME IN AT ALL FOUR SIDES.

LOOSE LINTELS

1. THE FOLLOWING LOOSE LINTELS SHALL BE USED FOR BRICK MASONRY OPENINGS, U.N.O. ON DRAWINGS:

MASONRY OPENING	<u>LINTEL SIZE</u>
UP TO 3'-0"	L 3 1/2 x3 1/2 x 5/16
3'-1" TO 4'-6"	L 4 x 3 1/2 x 5/16 (LLV)
4'-7" TO 6'-0"	L 5 x 3 1/2 x 5/16 (LLV)
6'-1" TO 8'-0"	L 6 x 3 1/2 x 5/16 (LLV)
8'-1" TO 9'-6"	L6 x 4 x 3/8 (LLV)

- PROVIDE ONE ANGLE FOR EACH 4" WALL THICKNESS. FOR 6" WALL THICKNESS, PROVIDE WT OR BUILT-UP SECTION WITH PROPERTIES EQUAL TO OR GREATER THAN 1 ½ TIMES THE ANGLES PROPERTIES FOR A 4" WALL THICKNESS.
- 3. PROVIDE 8" OF BEARING AT EACH END OF ALL LINTELS.
- 4. ALL EXTERIOR LINTELS SHALL BE HOT—DIPPED GALVANIZED.

<u>TIMBER NOTES</u>

- 1. ALL TIMBER FRAMING SHALL BE IN ACCORDANCE WITH THE AITC TIMBER CONSTRUCTION MANUAL— LATEST EDITION, AND THE AF & PA NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION (NDS) LATEST EDITION.
- 2. INDIVIDUAL TIMBER FRAMING MEMBERS SHALL BE VISUALLY GRADED. MINIMUM GRADE NO1/NO2 SPRUCE-PINE-FIR KILN DRIED TO 19% MAXIMUM MOISTURE CONTENT UNLESS OTHERWISE INDICATED ON THE DRAWINGS.
- 3. ENGINEERED WOOD PRODUCTS SHALL BE AS SPECIFIED ON THE DRAWINGS. REFER TO MANUFACTURER'S LITERATURE FOR PROPER HANDLING AND INSTALLATION GUIDELINES. MANUFACTURER AND PRODUCT SHALL BE:
- I—JOIST (TJI), PARALLAM (PSL), MICROLAM (LVL), TIMBERSTRAND (LSL) I-JOIST (BCI), VERSALAM (LVL)
- 4. Pressure treated lumber shall be used for sill members, exterior exposure, OR WHERE SHOWN ON THE DRAWINGS. TIMBER SHALL BE SOUTHERN YELLOW PINE TREATED WITH CCA OR ACQ TO 0.4 #/CF IN ACCORDANCE WITH AWPA C-18. ACZA IS STRICTLY PROHIBITED.
- 5. ALL ROOF AND WALL SHEATHING SHALL BE APA PERFORMANCE—RATED. SHEATHING SHALL BE NAILED TO THE FRAMING AS FOLLOWS, U.N.O.:
- A.ROOFS: 8d NAILS AT 6" O.C. AT SUPPORTED PANEL EDGES AND 12"O.C. AT INTERMEDIATE SUPPORTS B.WALLS: 8d NAILS AT 6" O.C. AT SUPPORTED PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS.
- 6. FLOOR SHEATHING SHALL BE 3/4". APA RATED TONGUE AND GROOVE PANELS. GLUE AND NAIL TO FLOOR FRAMING WITH 8d RING SHANK NAILS AT 6" O.C. AT SUPPORTED PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS. HUBER ADVANTECH TONGUE AND GROOVE PANELS MAY BE SUBSTITUTED ONLY WITH WRITTEN PERMISSION FROM THE
- 7. ALL BUILT-UP BEAMS AND COLUMNS SHALL BE NAILED AS FOLLOWS (FASTENING IN EACH

<u>UNIFORMLY LOADED BEAMS:</u> BEAM DEPTH <16" - 2 ROWS OF 16d NAILS AT 12" O.C., STAGGERED BEAM DEPTH >=16" - 3 ROWS OF 16d NAILS AT 12" O.C. STAGGERED NOTE: SIDE LOADED BEAMS REQUIRE ADDITIONAL FASTENING. SEE DETAILS

2-10d NAILS AT 6" O.C.

- 8. FASTENING NOT SPECIFIED SHALL CONFORM WITH IBC TABLE 2304.9.1. NAIL FASTENERS SHALL MEET THE REQUIREMENTS OF ASTM F1667. UNLESS NOTED OTHERWISE, NAILS REFERENCED ON DRAWINGS ARE TO BE COMMON NAILS WITH DIMENSIONS AS FOLLOWS.
- 6d: 2" LONG BY 0.113" DIAMETER SHANK WITH 0.266" DIAMETER HEAD 8d: 2 ½" LONG BY 0.131" DIAMETER SHANK WITH 0.281" DIAMETER HEAD 10d: 3" LONG BY 0.148" DIAMETER SHANK WITH 0.312" DIAMETER HEAD 12d: 3 ¼" LONG BY 0.148" DIAMETER SHANK WITH 0.312" DIAMETER HEAD 16d: 3 ½" LONG BY 0.162" DIAMETER SHANK WITH 0.344" DIAMETER HEAD 20d: 4" LONG BY 0.192" DIAMETER SHANK WITH 0.406" DIAMETER HEAD 30d: 4 ½" LONG BY 0.207" DIAMETER SHANK WITH 0.438" DIAMETER HEAD
- 9. ALL TIMBER CONNECTION HARDWARE (JOIST HANGERS, POST BASES, SHEARWALL HOLDOWNS, ETC) SHALL BE AS INDICATED ON THE DRAWINGS AND MANUFACTURED BY SIMPSON STRONG—TIE. ALL CONNECTION HARDWARE SHALL BE HOT—DIPPED GALVANIZED G-90 (U.N.O.). CONNECTION HARDWARE USED IN CONJUNCTION WITH PRESERVATIVE TREATMENT SHALL BE GALVANIZED G185 (ZMAX) USE FASTENERS AND HANGERS OF SAME MATERIAL & COATING. REFER TO MANUFACTURER'S LITERATURE FOR PROPER HANDLING AND INSTALLATION GUIDELINES.
- 10. FASTENERS USED IN CONJUNCTION WITH PT LUMBER. BUT NOT AT TIMBER CONNECTION HARDWARE REFERENCED IN NOTE ABOVE, SHALL BE POST HOT-DIPPED GALVANIZED (ASTM

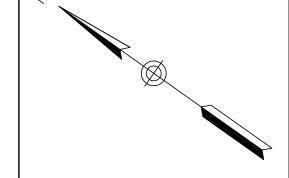
MASONRY NOTES

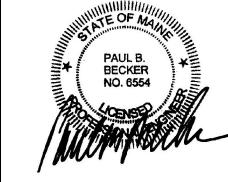
- 1. ALL MASONRY CONSTRUCTION SHALL CONFORM TO ACI 530.1, LATEST EDITION.
- 2. ALL CONCRETE MASONRY UNITS SHALL BE ASTM C90 GRADE N, TYPE I STANDARD WEIGHT BLOCKS INCLUDING STRETCHERS AND CORNER BLOCKS. MINIMUM PRISM STRENGTH OF BLOCK SHALL BE F'M = 1500 PSI IN 28 DAYS.
- 3. MORTAR SHALL CONFORM TO ASTM SPECIFICATION C270, TYPE M OR S
- 4. GROUT SHALL CONFORM TO ASTM-C476
- 5. REINFORCING FOR BOND BEAMS, LINTEL BLOCKS AND VERTICAL WALL REINFORCING SHALL BE BILLET STEEL CONFORMING TO ASTM A615, GRADE 60
- 6. HORIZONTAL JOINT REINFORCING SHALL BE DUR—O—WAL TRUSS DESIGN, STANDARD CLASS MILL GALVANIZED WITH 3/16" DIAMETER SIDE RODS AND 9 GAUGE CROSS TIES. U.N.O. REINFORCING SHALL BE PLACED IN MASONRY WALLS AT EVERY SECOND BLOCK
- 7. CONCRETE MASONRY UNITS SHALL BE LAID IN RUNNING BOND UNLESS OTHERWISE NOTED. PROVIDE FULL MORTAR COVERAGE ON ALL WEBS AND FACE SHELLS. PROVIDE CORNER BLOCKS AND END BLOCKS TO FINISH ALL 90 DEGREE CORNERS AND WALL
- 8. PROVIDE LINTELS AT WALL PENETRATIONS AS SHOWN IN THE LINTEL SCHEDULE.
- 9. STANDARD LAP LENGTH OF GRADE 60 MASONRY REINFORCING BARS SHALL BE 48 BAR DIAMETERS. PROVIDE CORNER BARS TO MATCH HORIZONTAL REINFORCEMENT.
- 10. CELLS TO BE GROUTED SHALL BE 2—CELL BLOCK. ALIGN CELLS TO MAINTAIN A CLEAR UNOBSTRUCTED, CONTINUOUS VERTICAL CHASE. CELLS MUST BE KEPT CLEAN OF PROTRUSIONS OR FINS OF MORTAR. FILL CELLS OF MASONRY UNITS AND WALI CAVITIES WHERE INDICATED WITH 2500 PSI GROUT. MAXIMUM GROUT LIFT WITHOUT CLEAN-OUTS SHALL BE 4'-0". HIGH LIFT GROUTING SHALL CONFORM TO CODE REQUIREMENTS WITH A MINIMUM CEMENT CONTENT OF 8 SACKS PER CUBIC YARD. SUPPORT ALL VERTICAL BARS IN CENTER OF GROUTED CELLS WITH VERTICAL BAR
- 11. FIELD PENETRATIONS THROUGH BLOCK WALLS SHALL NOT BE MADE THROUGH BOND BEAMS, LINTELS OR GROUTED CELLS.

ACOUSTICAL CEILING TILE INSIDE FACE A.F.F. ABOVE FINISH FLOOR ISOLATION JOINT INSULATION AGG AGGREGATE **ALTERNATE** APPROX **APPROXIMATE** KIPS (1K=1000LBS) ANCHOR ROD ARCH ARCHITECTURAL LIVE LOAD BOTTOM CHORD EXTENSION LONG LEGS BACK TO BACK LLBB LONG LEG HORIZ BITUMINOUS LONG LEG VERT BLDG BUILDING BOTTOM OF/ BY OTHERS MAXIMUM MECHANICAL MANUFACTURER MINIMIJM BOTH SIDES **MISCELLANEOUS** BTWN MASONRY OPENING BETWEEN NOT IN CONTRACT CEMENT BOARD CEM BD NEAR SIDE CAST IN PLACE NOT TO SCALE CONTRACTION/CONST. JOINT ON CENTER CENTER LINE OUTSIDE DIAMETER OUTSIDE FACE CONCRETE CONT CONTINUOUS POWDER ACTUATED FASTENER POUNDS PER LINEAR FOOT POUNDS PER SQUARE FOOT DEAL LOAD POUNDS PER SQUARE INCH PRESSURE TREATED POLYVINYL CHLORIDE DOWEL(S) PAVEMENT EACH FACE ROOF DRAIN **EXPANSION JOINT** RFINFORCEMENT ELEVATION REQUIRED LECTRICAL ROUGH OPENING MBEDMEN ROOF TOP UNIT EACH SIDE *SQUARE FOOT* EACH WAY BOTTOM SHFAR KFY EXTERIOR STAINLESS STEE SHORT SLOT FLOOR DRAIN STANDARD FINISH FLOOR FINISH FLOOR / FAR FACE STRUCTURAL FURNISHED BY OTHERS STIFFENER SHEARWALL FINISH FLOOR ELEVATION FACE OF BRICK TOP AND BOTTOM FACE OF CONCRETE TOP CHORD EXTENSION TIE JOIST FOOTING TOP OF STEEL etc. GAGE/GAUGE TYPICAL *GALV GALVANIZED* GENERAL CONTRACTOR UNLESS NOTED OTHERWISE GYPSUM WALL BOARD VERIFY IN FIELD H.D.GALV HOT DIPPED GALVANIZED HEATING VENTILATION & COOLING WIDE FLANGE HOLLOW STRUCTURAL SHAPE WORK POINT WELDED WIRE FABRIC

<u>ABBREVIATIONS</u>

NOT FOR CONSTRUCTION PERMIT SET 09/23/10





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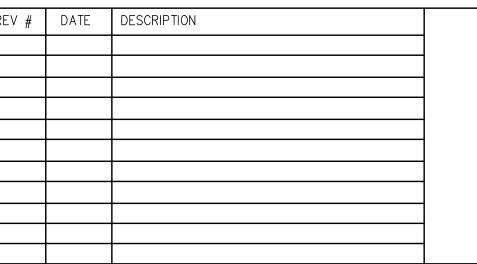
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Renovations To:

CUMBERLAND COLD STORAGE BUILDING Merrill's Wharf, Portland, Maine

GENERAL NOTES

9. 23. 2010