#### TRAFFIC ANALYSIS MEMORANDUM

TO: Ms. Jean Fraser

Planner

City of Portland

FROM: John Q. Adams, P.E., PTOE

Senior Transportation Engineer Milone & MacBroom, Inc.

DATE: November 30, 2012

RE: Response to Traffic Movement Permit

(TMP) Scoping Meeting

JB Brown & Sons

Proposed Hotel, Restaurant, and

Residences

321 Commercial Street, Portland, Maine

MMI #5002-01-3



#### **Introduction**

The purpose of this memo is to respond to the requests for additional information and traffic analysis that were determined at the Traffic Movement Permit Scoping Meeting held at the City of Portland's Department of Planning on November 7, 2012. The following items were requested:

- 1. Traffic operations and queuing analysis should be performed during both the weekday AM and PM peak hours for the intersections of:
  - Maple Street at Commercial Street
  - Proposed site entrance on Commercial Street
  - Proposed site entrance on Maple Street
- 2. Other development trips to be included in the traffic analysis should include the proposed school located at the intersection of Maple Street and York Street and the Canal Plaza Hotel development located at the intersection of Union Street and York Street.
- 3. The intersection of High Street at Commercial Street was identified as a high crash location with eight crashes and a critical rate factor of 1.50. Of the eight crashes, four were rear-ends on the High Street approach. An accident diagram of the intersection and an analysis of contributing factors to the rear-end accident pattern were requested.



#### **Proposed Development**

The requested response to the scoping meeting was for the proposed mixed-use development, including a 131-room hotel, 7,000-square-foot (sf) restaurant, and 14 residences. The development is located at the corner of Commercial Street and Maple Street in Portland. Figure 1 at the end of this memorandum shows the project site and area extents. The site will be served by a full-access site entrance on Commercial Street located approximately 300 feet north of Maple Street and a full-access site entrance located 250 feet west of Commercial Street.

During the weekday AM and PM peak hours, the proposed development is expected to generate the following trip ends:

- Weekday AM Peak Hour (of Site) 109 trip ends
- Weekday AM Peak Hour (of Roadway 7-9 AM) 108 trip ends
- Weekday PM Peak Hour (of Site) 137 trip ends
- Weekday PM Peak Hour (of Roadway 4-6 PM) 111 trip ends

#### **Traffic Operations and Queuing Analysis**

Traffic operations analysis was performed for the requested study intersections, including Maple Street at Commercial Street, site entrance at Commercial Street, and the site entrance at Maple Street.

#### Assignment of Site-Generated Trips

In performing the analysis, we first distributed the site-generated trip ends to the area roadway networks as follows:

- Generally 75% of trip ends entering and exiting the site utilized Commercial Street, and 25% utilized York Street.
- For trips utilizing Commercial Street, 2/3 generally entered/exited from the south and 1/3 to/from the north.
- We also assumed that the hotel trips that utilize the York Street valet parking areas would utilize the Maple Street site entrance via a right turn out of the site and a left turn into the site. These movements have been adjusted (increased) to reflect these activities.

The site-generated trip ends are shown in Figure 2 located at the end of this memorandum. The "Other Development Trips" for developments in the permitting process but not yet approved or built are shown in Figure 3. The other development trips include the proposed school at the intersection of Maple Street and Commercial Street and the One Canal Plaza Hotel development at the corner of Union Street and Fore Street.



#### **Traffic Counts and Adjustments**

We performed traffic counts at the intersection of Maple Street and Commercial Street on Wednesday, November 14, 2012. Traffic count data sheets are included in the appendix. These traffic volumes are shown in Figure 4. These traffic counts were adjusted both seasonally and annually to account for the 2014 build year. The seasonal adjustment was based on Commercial Street being classified as a Type 1 urban commuter/roadway. The seasonal adjustment to the sixth highest week resulted in a 10% increase. Based on a flattening and decreasing trend in Maine Department of Transportation (DOT) annual average daily traffic counts in the area, the annual adjustment applied was 0.5% per year for a 1% increase total. The adjusted traffic volumes are shown in Figure 5. The other development trips shown in Figure 3 were combined with adjusted traffic volumes shown in Figure 5 to compile the 2014 background traffic volumes shown in Figure 6.

The background traffic volumes in Figure 6 were combined with the site-generated trips shown in Figure 2 to arrive at the 2014 Post Development Traffic Volumes shown in Figure 7.

#### Weekday AM and PM Peak-Hour Traffic Operations Analysis

We have performed traffic operations analysis at the study intersections, including Maple Street at Commercial Street, site entrance at Commercial Street, and the site entrance at Maple Street. The analysis was completed for unsignalized intersections utilizing *SimTraffic* traffic modeling software with data inputs utilizing *Synchro* 7.

Commercial Street was modeled as having enough width for through vehicles to bypass left-turning vehicles at Maple Street and at the site entrance on Commercial Street. This was based on both field observations of traffic operations and on roadway widths in each direction on Commercial Street at Maple Street (approximate widths of 30 feet northbound half section, 24 feet southbound half section) and at the site entrance on Commercial Street (approximate widths of 24 to 30 feet northbound half section).

Tables 1, 2, and 3 summarize the results of the traffic operations analysis under the 2014 weekday AM and PM peak hours for the background and postdevelopment conditions.

Synchro traffic data input sheets along with SimTraffic analysis outputs are enclosed in the appendix.



# TABLE 1 Maple Street at Commercial Street Traffic Operations Analysis SimTraffic

	2014 AM	Peak Hour	2014 PM 1	Peak Hour
	Background	Postdevelopment	Background	Postdevelopment
	LOS/Delay/95 <sup>th</sup> Q	LOS/Delay/95 <sup>th</sup> Q	LOS/Delay/95 <sup>th</sup> Q	LOS/Delay/95 <sup>th</sup> Q
Overall	A / 3.6 s	A / 3.9 s	A / 4.1 s	A / 3.6 s
Commercial NB	A / 4.4 s / 49 ft	A / 4.8 s / 50 ft	A / 3.2 s / 62 ft	A / 3.0 s / 59 ft
Commercial SB	A / 0.9 s / 6 ft	A / 0.6 s / 6 ft	A / 2.7 s / 8 ft	A / 1.2 s / 8 ft
Maple Street EB	A / 6.1 s / 48 ft	A / 7.2 s / 52 ft	C / 24.6 s / 92 ft	C / 23.0 s / 108 ft
Browns Wharf WB	B / 14.6 s / 32 ft	D / 25.2 s / 36 ft	D / 32.8 s / 37 ft	E / 37.7 s / 48 ft

Notes: LOS = Level of Service

Q = queue s = secondsft = feet

NB = northbound, SB = southbound, EB = eastbound, WB = westbound

TABLE 2 Site Entrance at Maple Street Traffic Operations Analysis SimTraffic

	2014 AM	Peak Hour	2014 PM I	Peak Hour
	Background	Postdevelopment	Background	Postdevelopment
	LOS/Delay/95 <sup>th</sup> Q	LOS/Delay/95 <sup>th</sup> Q	LOS/Delay/95 <sup>th</sup> Q	LOS/Delay/95 <sup>th</sup> Q
Overall	-	A / 1.0 s	-	A / 1.0 s
Maple St EB	-	A / 0.5 s / 23 ft	-	A / 0.4 s / 21 ft
Maple St WB	-	A / 0.4 s / -	-	A / 0.6 s / -
Site SB	-	A / 2.7 s / 51 ft	-	A / 2.8 s / 5 ft

Notes: LOS = Level of Service

 $\begin{aligned} Q &= queue \\ s &= seconds \\ ft &= feet \end{aligned}$ 

NB = northbound, SB = southbound, EB = eastbound, WB = westbound



# TABLE 3 Site Entrance at Commercial Street Traffic Operations Analysis SimTraffic

	2014 AM	Peak Hour	2014 PM 1	Peak Hour
	Background	Postdevelopment	Background	Postdevelopment
	LOS/Delay/95 <sup>th</sup> Q	LOS/Delay/95 <sup>th</sup> Q	LOS/Delay/95 <sup>th</sup> Q	LOS/Delay/95 <sup>th</sup> Q
Overall	-	A / 0.9 s	-	A / 1.6 s
Commercial NB	-	A / 0.9 s / 26 ft	-	A / 0.5 s / 25 ft
Commercial SB	-	A / 0.6 s / -	-	A / 1.5 s / 3 ft
Site EB	-	A / 5.6 s / 33 ft	-	C / 18.7 s / 65 ft

Notes: LOS = Level of Service

Q = queue s = seconds ft = feet

NB = northbound, SB = southbound, EB = eastbound, WB = westbound

The results of the *SimTraffic* analysis indicate that all of the intersections will function satisfactorily under the weekday AM and PM peak-hour postdevelopment conditions. At the intersection of Maple Street at Commercial Street, the Browns Wharf approach will experience some delay but will operate at Levels of Service (LOS) D and E in the AM and PM peak hours, respectively.

#### Accident Analysis - High Street at Commercial Street

Accident data from the Maine DOT was reviewed for the latest three-year period (2009 – 2011). We have prepared an accident diagram for the eight accidents recorded at the intersection. The diagram is located in the appendix to this memorandum. A review of the accident reports indicates that there is an accident pattern on the High Street approach to the intersection. We have reviewed the accident reports for the rear-end accident pattern on the High Street approach to the intersection and concluded the following:

#### Rear-end Accidents on High Street

- Two of the accidents occurred at night and two in daylight.
- Weather did not appear to be a factor as the accident reports indicated that they all occurred in clear and dry conditions.
- All were due to "driver following too closely."

The one factor common to all of the accidents was the steep grade of the High Street approach. The grade based on initial measurements from Google Earth maps is approximately -7%. This is likely a contributing factor in this rear-end accident pattern.



#### **Conclusions**

We have performed the requested analysis and offer the following conclusions:

- ➤ The proposed development will include a 131-room hotel, 7,000-sf restaurant, and 14 residences.
- Traffic operations have been analyzed for the weekday AM and PM peak hours at the following requested study intersections:
  - Maple Street at Commercial Street
  - Proposed site entrance on Commercial Street
  - Proposed site entrance on Maple Street
- > Traffic operations from a LOS and delay standpoint will function satisfactory at the study intersections.
- ➤ There was no excessive queuing noted in the analysis.
- ➤ There is an accident pattern on the High Street approach to Commercial Street, including four rear-end type accidents over a three-year period (2009 2011). The excessive grade on the High Street approach (-7%) appears to be a contributing factor.

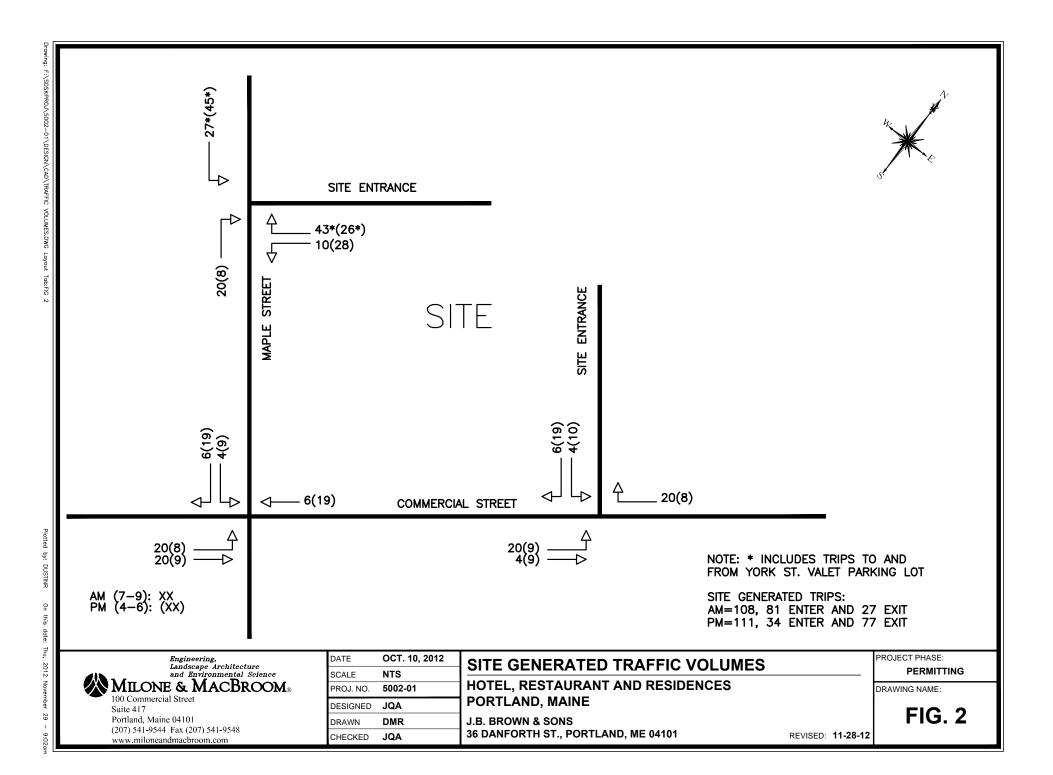
Overall, it is our opinion that we do not expect the proposed development to have a significant impact on existing traffic operations at the study intersections. We trust that the above analysis has addressed the concerns of the City of Portland. Please let us know if you need any further information.

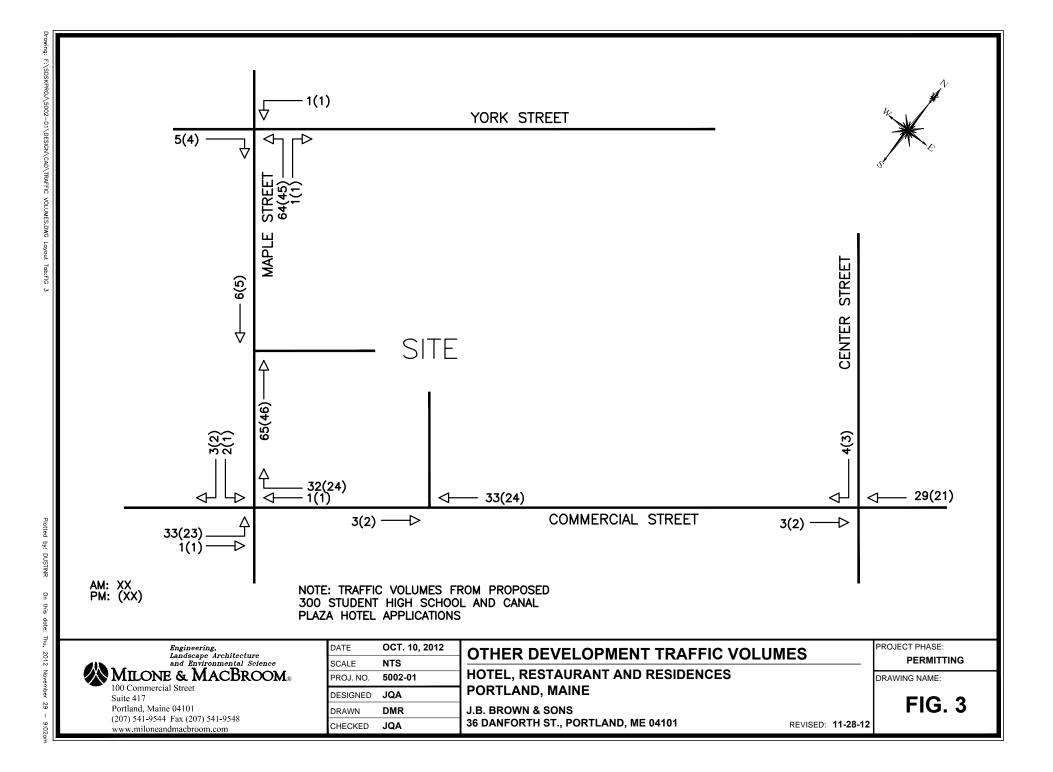
cc: Tom Errico, City Traffic Engineer
Opechee Construction Corp., LLC
Derek Olson, Region Traffic Engineer, MDOT Scarborough

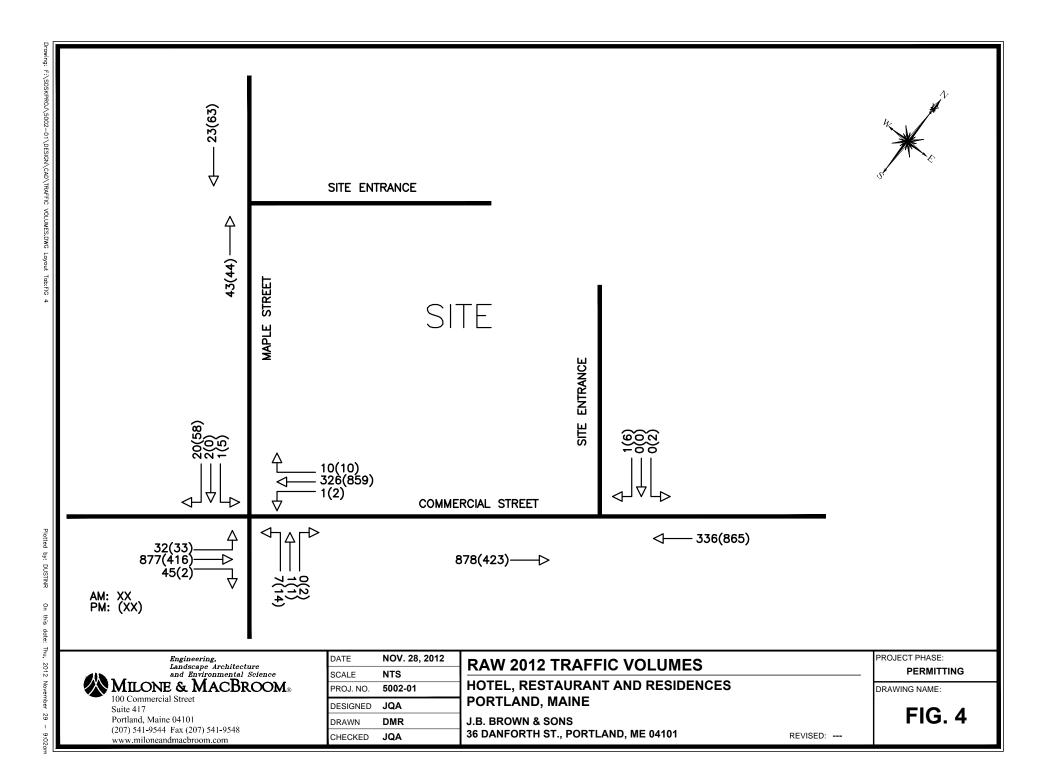
5002-01-3-n2912-memo.doc

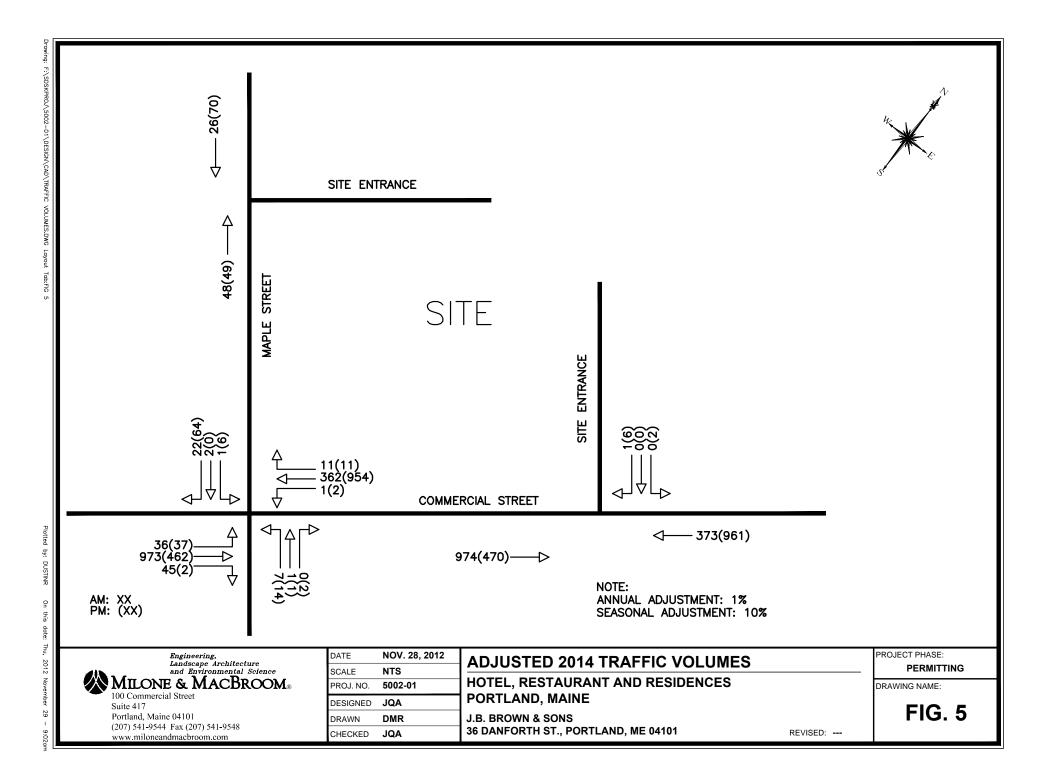


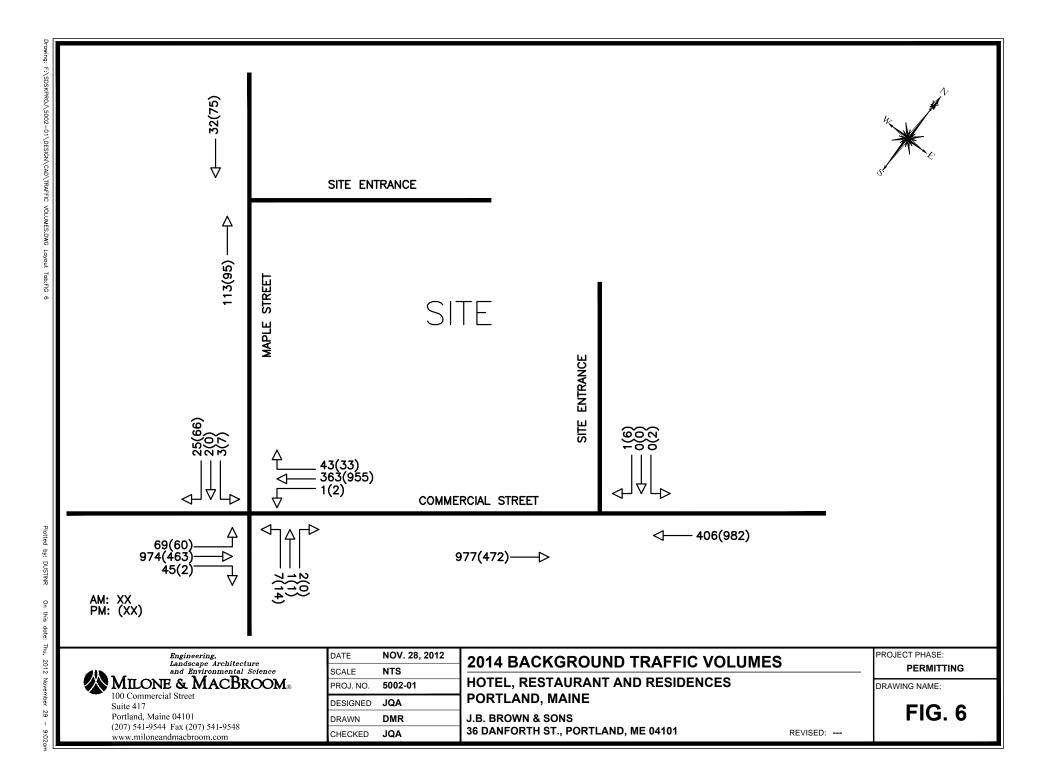
Map Name: PORTLAND WEST Scale: 1 inch = 2,000 ft. Horizontal Datum: NAD83 Print Date: 09/12/12 Map Center: 043°39'20.98" N 0 High Seh Tidal Fats -Chenery-St-Jack Lr High Sch Godman-St BACK COVEe Stipillion Culvert Tidel Flats Fish: \*Point ogPiles PORTLAN o Dolphin Deering Oaks
Deering Oaks PORTLAND High Sch Deering Oaks State Little Diamond Island Gravel PORTLAND HARBOR Portland Bridge RIVER Piling 31 Filing Guadine Tanii Farm Island Graye Forest City lark Rometery

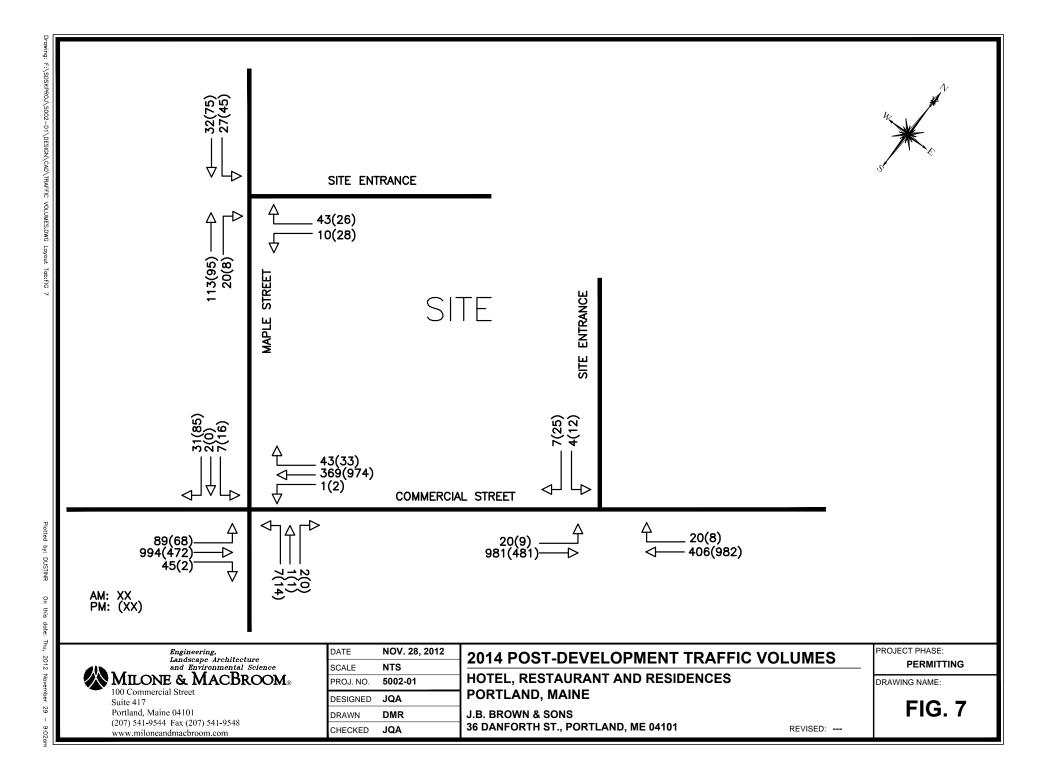












# **Appendix**

- > Traffic Count Data Sheets Maple Street at Commercial Street
- > Synchro SimTraffic Outputs
- > Accident Diagram High Street at Commercial Street



<u>DATE LOCATION:</u> 11/14/2012 COMMERCIAL STREET AT MAPLE STREET, PORTLAND, ME ~WEEKDAY AM PEAK HOURS~

			,,, <u>,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,</u>							MOVE	MENTS										
		MAPL	E ST EA	STBOUN	D	C	OMMERO	CIAL ST	NORTHB	OUND		BROWNS	WARF	WESTBO	UND	С	OMMER	CIAL ST	SOUTHE	OUND	TOTALS
TIME	1	2	3	Peds	15 Minute Totals	4	5	6	Peds	15 Minute Totals	7	8	9	Peds	15 Minute Totals	10	11	12	Peds	15 Minute Totals	15 Minute Totals
700 - 715	0	1	4	7	5	1	92	4	0	97	1	0	0	5	1	1	57	0	1	58	161
715 - 730	1	1	1	4	3	5	146	6	0	157	2	2	0	5	4	0	81	3	0	84	248
730 - 745	1	0	6	6	7	7	195	7	0	209	1	0	0	4	1	0	96	1	0	97	314
<mark>745 - 800</mark>	0	0	4	7	4	5	243	10	3	258	1	0	0	4	1	0	79	3	0	82	345
800 - 815	0	0	3	2	3	13	222	11	1	246	1	1	0	5	2	1	76	3	1	80	331
815 - 830	0	1	8	3	9	6	216	12	0	234	0	0	0	5	0	0	85	0	0	85	328
830 - 845	1	1	5	1	7	8	196	12	0	216	5	0	0	4	5	0	86	4	1	90	318
845 - 900	3	0	9	6	12	1	187	7	0	195	1	0	1	2	2	2	85	3	0	90	299
PHF		0.64					0.92					0.40					0.94				
PEAK HOUR % Heavy	1.0%	1.0%	1.0%			1.0%	2.0%	1.0%			1.0%	1.0%	1.0%			1.0%	5.0%	1.0%			
PEAK HOUR TOTALS	1	2	20	13	23	32	877	45	4	954	7	1	0	18	8	1	326	10	2	337	1322

# <u>DATE LOCATION:</u> 11/14/2012 COMMERCIAL STREET AT MAPLE STREET, PORTLAND, ME ~WEEKDAY PM PEAK HOURS~

	MOVEMENTS																				
		MAPL	E ST EA	STBOUN	D	C	OMMERC	CIAL ST I	NORTHB	OUND	ı	BROWNS	WARF	WESTBO	UND	COMMERCIAL ST SOUTHBOUND				TOTALS	
TIME	1	2	3	Peds	15 Minute Totals	4	5	6	Peds	15 Minute Totals	7	8	9	Peds	15 Minute Totals	10	11	12	Peds	15 Minute Totals	15 Minute Totals
					0				0	0					0					0	0
																				0	0
430 - 445 PM	0	0	17	1	17	2	102	1	2	105	5	0	0	2	5	0	193	1	1	194	321
445 - 500 PM	2	0	17	3	19	8	97	1	3	106	7	0	0	3	7	0	186	2	1	188	320
500 - 515 PM	3	0	16	1	19	9	118	0	1	127	3	0	0	2	3	0	236	2	3	238	387
515 - 530 PM	0	0	12	2	12	7	103	1	1	111	1	0	2	2	3	2	233	2	2	237	363
530 - 545 PM	0	0	13	4	13	9	98	0	1	107	3	1	0	1	4	0	204	4	0	208	332
545 - 600 PM	3	0	20	2	23	3	97	1	0	101	3	0	0	1	3	2	161	4	1	167	294
PHF		0.83					0.89					0.61					0.91				
PEAK HOUR % Heavy	1.0%	0.0%	1.0%			1.0%	4.0%	1.0%			2.0%	1.0%	1.0%			1.0%	2.0%	1.0%			
PEAK HOUR TOTALS	5	0	58	10	63	33	416	2	6	451	14	1	2	8	17	2	859	10	6	871	1402

#### **PEAK HOUR FACTOR:**

PHF = (Addition of the 4 - 15 minute Intervals)
(Highest 15 minute Interval) X (4)

#### Summary of All Intervals

Run Number	1	2	3	4	5	Avg	
Start Time	7:40	7:40	7:40	7:40	7:40	7:40	
End Time	8:45	8:45	8:45	8:45	8:45	8:45	
Total Time (min)	65	65	65	65	65	65	
Time Recorded (min)	60	60	60	60	60	60	
# of Intervals	2	2	2	2	2	2	
# of Recorded Intvls	1	1	1	1	1	1	
Vehs Entered	1517	1614	1501	1476	1494	1522	
Vehs Exited	1523	1612	1506	1483	1485	1522	
Starting Vehs	15	14	17	14	9	13	
Ending Vehs	9	16	12	7	18	12	
Denied Entry Before	0	1	1	1	0	1	
Denied Entry After	0	0	0	0	0	0	
Travel Distance (mi)	339	363	335	331	335	341	
Travel Time (hr)	13.8	14.8	13.3	13.3	13.2	13.7	
Total Delay (hr)	2.1	2.3	1.7	1.9	1.6	1.9	
Total Stops	67	60	79	59	63	66	
Fuel Used (gal)	11.5	12.2	11.2	11.1	11.0	11.4	

#### Interval #0 Information Seeding

Start Time 7:40
End Time 7:45
Total Time (min) 5
Volumes adjusted by Growth Factors.
No data recorded this interval.

#### Interval #1 Information Recording

Start Time 7:45
End Time 8:45
Total Time (min) 60
Volumes adjusted by Growth Factors.

Run Number	1	2	3	4	5	Avg	
Vehs Entered	1517	1614	1501	1476	1494	1522	
Vehs Exited	1523	1612	1506	1483	1485	1522	
Starting Vehs	15	14	17	14	9	13	
Ending Vehs	9	16	12	7	18	12	
Denied Entry Before	0	1	1	1	0	1	
Denied Entry After	0	0	0	0	0	0	
Travel Distance (mi)	339	363	335	331	335	341	
Travel Time (hr)	13.8	14.8	13.3	13.3	13.2	13.7	
Total Delay (hr)	2.1	2.3	1.7	1.9	1.6	1.9	
Total Stops	67	60	79	59	63	66	
Fuel Used (gal)	11.5	12.2	11.2	11.1	11.0	11.4	

# 3: Maple & Commercial Performance by movement

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Delay / Veh (s)	16.8	16.2	4.3	20.1	12.2	7.1	9.8	4.1	3.4		0.9	0.7

# 3: Maple & Commercial Performance by movement

Movement	All	
Delay / Veh (s)	3.6	

# **Total Network Performance**

Delay / Veh (s) 4.5

# Intersection: 3: Maple & Commercial

Movement	EB	WB	NB	SB	SB	
Directions Served	LTR	LTR	L	L	TR	
Maximum Queue (ft)	54	35	56	12	8	
Average Queue (ft)	19	9	20	0	0	
95th Queue (ft)	48	32	49	6	4	
Link Distance (ft)	177	152			580	
Upstream Blk Time (%)						
Queuing Penalty (veh)						
Storage Bay Dist (ft)			100	100		
Storage Blk Time (%)						
Queuing Penalty (veh)						

#### **Network Summary**

Network wide Queuing Penalty: 0

#### Summary of All Intervals

Run Number	1	2	3	4	5	Avg	
Start Time	4:40	4:40	4:40	4:40	4:40	4:40	
End Time	5:45	5:45	5:45	5:45	5:45	5:45	
Total Time (min)	65	65	65	65	65	65	
Time Recorded (min)	60	60	60	60	60	60	
# of Intervals	2	2	2	2	2	2	
# of Recorded Intvls	1	1	1	1	1	1	
Vehs Entered	1576	1690	1628	1597	1566	1611	
Vehs Exited	1573	1687	1637	1601	1563	1612	
Starting Vehs	11	14	18	12	14	13	
Ending Vehs	14	17	9	8	17	12	
Denied Entry Before	0	1	0	0	0	0	
Denied Entry After	0	0	0	0	0	0	
Travel Distance (mi)	354	379	367	358	352	362	
Travel Time (hr)	14.1	16.1	15.1	14.3	14.1	14.8	
Total Delay (hr)	1.9	3.0	2.5	2.0	2.0	2.3	
Total Stops	142	154	125	152	122	139	
Fuel Used (gal)	11.7	12.9	12.5	11.9	11.7	12.1	

#### Interval #0 Information Seeding

Start Time 4:40
End Time 4:45
Total Time (min) 5
Volumes adjusted by Growth Factors.
No data recorded this interval.

# Interval #1 Information Recording

Start Time 4:45
End Time 5:45
Total Time (min) 60
Volumes adjusted by Growth Factors.

Run Number	1	2	3	4	5	Avg	
Vehs Entered	1576	1690	1628	1597	1566	1611	
Vehs Exited	1573	1687	1637	1601	1563	1612	
Starting Vehs	11	14	18	12	14	13	
Ending Vehs	14	17	9	8	17	12	
Denied Entry Before	0	1	0	0	0	0	
Denied Entry After	0	0	0	0	0	0	
Travel Distance (mi)	354	379	367	358	352	362	
Travel Time (hr)	14.1	16.1	15.1	14.3	14.1	14.8	
Total Delay (hr)	1.9	3.0	2.5	2.0	2.0	2.3	
Total Stops	142	154	125	152	122	139	
Fuel Used (gal)	11.7	12.9	12.5	11.9	11.7	12.1	

# 3: Maple & Commercial Performance by movement

Movement	EBL	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR	All	
Delay / Veh (s)	41.5	22.4	33.5	29.1	16.4	1.4	1.8	7.8	2.7	2.1	4.1	

# **Total Network Performance**

Delay / Veh (s) 5.0

# Intersection: 3: Maple & Commercial

Movement	EB	WB	NB	SB	SB
Directions Served	LR	LTR	L	L	TR
Maximum Queue (ft)	132	39	82	12	15
Average Queue (ft)	44	12	32	1	1
95th Queue (ft)	92	37	62	8	9
Link Distance (ft)	177	152			580
Upstream Blk Time (%)	0				
Queuing Penalty (veh)	0				
Storage Bay Dist (ft)			100	100	
Storage Blk Time (%)			0		
Queuing Penalty (veh)			0		

#### **Network Summary**

Network wide Queuing Penalty: 0

#### Summary of All Intervals

Run Number	1	2	3	4	5	Avg	
Start Time	7:40	7:40	7:40	7:40	7:40	7:40	
End Time	8:45	8:45	8:45	8:45	8:45	8:45	
Total Time (min)	65	65	65	65	65	65	
Time Recorded (min)	60	60	60	60	60	60	
# of Intervals	2	2	2	2	2	2	
# of Recorded Intvls	1	1	1	1	1	1	
Vehs Entered	1714	1804	1684	1717	1649	1712	
Vehs Exited	1715	1802	1690	1716	1653	1714	
Starting Vehs	12	14	16	16	18	16	
Ending Vehs	11	16	10	17	14	12	
Denied Entry Before	2	2	1	2	0	1	
Denied Entry After	0	1	2	0	1	1	
Travel Distance (mi)	371	388	359	367	354	368	
Travel Time (hr)	15.5	16.4	15.2	15.2	14.3	15.3	
Total Delay (hr)	2.5	2.7	2.5	2.3	1.8	2.4	
Total Stops	178	163	179	154	166	166	
Fuel Used (gal)	12.8	13.6	12.6	12.7	12.0	12.7	

#### Interval #0 Information Seeding

Start Time 7:40
End Time 7:45
Total Time (min) 5
Volumes adjusted by Growth Factors.
No data recorded this interval.

#### Interval #1 Information Recording

Start Time 7:45
End Time 8:45
Total Time (min) 60
Volumes adjusted by Growth Factors.

Run Number	1	2	3	4	5	Avg	
Vehs Entered	1714	1804	1684	1717	1649	1712	
Vehs Exited	1715	1802	1690	1716	1653	1714	
Starting Vehs	12	14	16	16	18	16	
Ending Vehs	11	16	10	17	14	12	
Denied Entry Before	2	2	1	2	0	1	
Denied Entry After	0	1	2	0	1	1	
Travel Distance (mi)	371	388	359	367	354	368	
Travel Time (hr)	15.5	16.4	15.2	15.2	14.3	15.3	
Total Delay (hr)	2.5	2.7	2.5	2.3	1.8	2.4	
Total Stops	178	163	179	154	166	166	
Fuel Used (gal)	12.8	13.6	12.6	12.7	12.0	12.7	

#### 3: Maple & Commercial Performance by movement

Movement	EBL	EBT	EBR	WBL	WBR	NBL	NBT	NBR	SBL	SBT	SBR	All
Delay / Veh (s)	18.6	10.4	5.5	28.5	15.3	9.7	4.4	4.1	3.6	0.7	0.3	3.9

#### 6: Maple & Site Performance by movement

Movement	EBL	EBT	WBT	WBR	SBL	SBR	All
Delay / Veh (s)	0.8	0.2	0.4	0.4	3.6	2.5	1.0

#### 8: Commercial & Performance by movement

Movement	EBL	EBR	NBL	NBT	SBT	SBR	All
Delay / Veh (s)	10.3	4.2	3.6	0.9	0.6	0.5	0.9

#### **Total Network Performance**

Delay / Veh (s) 5.0

# Intersection: 3: Maple & Commercial

Movement	EB	WB	NB	SB	SB
Directions Served	LTR	LTR	L	L	TR
Maximum Queue (ft)	58	43	61	12	4
Average Queue (ft)	24	11	22	0	0
95th Queue (ft)	52	36	50	6	5
Link Distance (ft)	206	152			258
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)			100	100	
Storage Blk Time (%)					
Queuing Penalty (veh)					

#### Intersection: 6: Maple & Site

Movement	EB	SB
Directions Served	LT	LR
Maximum Queue (ft)	40	54
Average Queue (ft)	4	28
95th Queue (ft)	23	51
Link Distance (ft)	91	92
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

#### Intersection: 8: Commercial &

Movement	EB	NB
Directions Served	LR	L
Maximum Queue (ft)	34	36
Average Queue (ft)	10	6
95th Queue (ft)	33	26
Link Distance (ft)	68	
Upstream Blk Time (%)	0	
Queuing Penalty (veh)	0	
Storage Bay Dist (ft)		50
Storage Blk Time (%)		0
Queuing Penalty (veh)		1

#### **Network Summary**

Network wide Queuing Penalty: 1

#### Summary of All Intervals

Run Number	1	2	3	4	5	Avg	
Start Time	4:40	4:40	4:40	4:40	4:40	4:40	
End Time	5:45	5:45	5:45	5:45	5:45	5:45	
Total Time (min)	65	65	65	65	65	65	
Time Recorded (min)	60	60	60	60	60	60	
# of Intervals	2	2	2	2	2	2	
# of Recorded Intvls	1	1	1	1	1	1	
Vehs Entered	1794	1763	1727	1773	1672	1745	
Vehs Exited	1790	1772	1723	1775	1672	1747	
Starting Vehs	13	20	13	17	10	13	
Ending Vehs	17	11	17	15	10	14	
Denied Entry Before	1	1	0	0	0	0	
Denied Entry After	0	0	1	0	2	1	
Travel Distance (mi)	389	382	374	382	361	378	
Travel Time (hr)	16.7	16.7	15.6	16.4	15.3	16.1	
Total Delay (hr)	3.0	3.2	2.4	2.9	2.5	2.8	
Total Stops	269	288	285	290	267	279	
Fuel Used (gal)	13.7	13.4	13.0	13.4	12.6	13.2	

#### Interval #0 Information Seeding

Start Time 4:40 **End Time** 4:45 Total Time (min) 5 Volumes adjusted by Growth Factors. No data recorded this interval.

# Interval #1 Information Recording

Start Time 4:45 **End Time** 5:45 Total Time (min) 60 Volumes adjusted by Growth Factors.

Run Number	1	2	3	4	5	Avg	
Vehs Entered	1794	1763	1727	1773	1672	1745	
Vehs Exited	1790	1772	1723	1775	1672	1747	
Starting Vehs	13	20	13	17	10	13	
Ending Vehs	17	11	17	15	10	14	
Denied Entry Before	1	1	0	0	0	0	
Denied Entry After	0	0	1	0	2	1	
Travel Distance (mi)	389	382	374	382	361	378	
Travel Time (hr)	16.7	16.7	15.6	16.4	15.3	16.1	
Total Delay (hr)	3.0	3.2	2.4	2.9	2.5	2.8	
Total Stops	269	288	285	290	267	279	
Fuel Used (gal)	13.7	13.4	13.0	13.4	12.6	13.2	

11/28/2012

# 3: Maple & Commercial Performance by movement

Movement	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR	All
Delay / Veh (s)	34.4	1.0	22.1	36.4	58.5	15.2	1.4	1.5	5.8	1.2	0.6	3.6

#### 6: Maple & Site Performance by movement

Movement	EBL	EBT	WBT	WBR	SBL	SBR	All
Delay / Veh (s)	0.7	0.2	0.6	0.4	3.4	2.2	1.0

# 8: Site & Commercial Performance by movement

Movement	EBL	EBR	NBL	NBT	SBT	SBR	All
Delay / Veh (s)	23.1	14.3	7.0	0.3	1.5	1.2	1.6

#### **Total Network Performance**

Delay / Veh (s) 5.8

# Intersection: 3: Maple & Commercial

Movement	EB	WB	NB	SB	SB
Directions Served	LR	LTR	L	L	TR
Maximum Queue (ft)	132	53	78	18	9
Average Queue (ft)	57	18	29	1	0
95th Queue (ft)	108	48	59	8	4
Link Distance (ft)	206	152			258
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)			100	100	
Storage Blk Time (%)			0		
Queuing Penalty (veh)			0		

#### Intersection: 6: Maple & Site

Movement	EB	SB
Directions Served	LT	LR
Maximum Queue (ft)	35	64
Average Queue (ft)	4	29
95th Queue (ft)	21	52
Link Distance (ft)	91	92
Upstream Blk Time (%)		0
Queuing Penalty (veh)		0
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

#### Intersection: 8: Site & Commercial

Movement	EB	NB	SB
Directions Served	LR	L	TR
Maximum Queue (ft)	81	31	4
Average Queue (ft)	29	6	0
95th Queue (ft)	65	25	3
Link Distance (ft)	68		266
Upstream Blk Time (%)	2		
Queuing Penalty (veh)	0		
Storage Bay Dist (ft)		50	
Storage Blk Time (%)		0	
Queuing Penalty (veh)		0	

#### **Network Summary**

Network wide Queuing Penalty: 0

	۶	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	<b>/</b>	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ሻ	₽		7	₽	
Volume (vph)	3	2	25	7	1	2	69	974	45	1	363	43
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	12	12	10	12	12
Grade (%)		-5%			0%			0%			0%	
Storage Length (ft)	0		0	0		0	100		0	100		0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (ft)	25		25	25		25	25		25	25		25
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.888			0.973			0.993			0.984	
Flt Protected		0.995			0.965		0.950			0.950		
Satd. Flow (prot)	0	1687	0	0	1749	0	1652	1850	0	1652	1786	0
Flt Permitted		0.995			0.965		0.950			0.950		
Satd. Flow (perm)	0	1687	0	0	1749	0	1652	1850	0	1652	1786	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		210			185			627			608	
Travel Time (s)		4.8			4.2			14.3			13.8	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.64	0.64	0.64	0.40	0.40	0.40	0.92	0.92	0.92	0.94	0.94	0.94
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	5%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	5	3	39	18	3	5	75	1059	49	1	386	46
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	47	0	0	25	0	75	1108	0	1	432	0
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Area Type:	Other											
Control Type: Unsignalized												
Intersection Capacity Utiliz	ation 67.3%			IC	CU Level	of Service	e C					
Analysis Dariad (min) 15												

Analysis Period (min) 15

Synchro 7 - Report Baseline Page 1

	•	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	<b>/</b>	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ሻ	£		7	4	
Volume (vph)	7	0	66	14	1	0	60	463	2	2	955	33
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	12	12	10	12	12
Grade (%)		-5%			0%			0%			0%	
Storage Length (ft)	0		0	0		0	100		0	100		0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (ft)	25		25	25		25	25		25	25		25
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.877						0.999			0.995	
Flt Protected		0.995			0.956		0.950			0.950		
Satd. Flow (prot)	0	1666	0	0	1781	0	1652	1825	0	1652	1853	0
Flt Permitted		0.995			0.956		0.950			0.950		
Satd. Flow (perm)	0	1666	0	0	1781	0	1652	1825	0	1652	1853	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		210			185			627			608	
Travel Time (s)		4.8			4.2			14.3			13.8	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.83	0.83	0.83	0.61	0.61	0.61	0.84	0.84	0.84	0.90	0.90	0.90
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	4%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	8	0	80	23	2	0	71	551	2	2	1061	37
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	88	0	0	25	0	71	553	0	2	1098	0
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
<i>3</i> i	Other											
Control Type: Unsignalized												
Intersection Capacity Utiliza	ition 63.7%			IC	CU Level	of Service	B					

Intersection Capacity Utilization 63.7% Analysis Period (min) 15

	٠	<b>→</b>	•	•	<b>←</b>	•	•	<b>†</b>	~	<b>/</b>	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ň	f)		7	f)	
Volume (vph)	7	2	31	7	1	2	89	994	45	1	369	43
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	12	12	10	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	0		0	0		0	100		0	100		0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (ft)	25		25	25		25	25		25	25		25
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.895			0.973			0.993			0.984	
Flt Protected		0.991			0.965		0.950			0.950		
Satd. Flow (prot)	0	1652	0	0	1749	0	1652	1850	0	1652	1786	0
Flt Permitted		0.991			0.965		0.950			0.950		
Satd. Flow (perm)	0	1652	0	0	1749	0	1652	1850	0	1652	1786	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		273			185			627			308	
Travel Time (s)		6.2			4.2			14.3			7.0	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.64	0.64	0.64	0.40	0.40	0.40	0.92	0.92	0.92	0.94	0.94	0.94
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	5%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	11	3	48	18	3	5	97	1080	49	1	393	46
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	62	0	0	25	0	97	1129	0	1	439	0
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Area Type:	Other											
Control Type: Unsignalized												
Intersection Capacity Utiliza	ntion 71.7%			IC	CU Level	of Service	e C					
Analysis Davis d (min) 15												

Intersection Capacity Utilization 71.7% Analysis Period (min) 15

	۶	<b>→</b>	<b>←</b>	•	<b>/</b>	4
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		ર્ન	f)		W	
Volume (vph)	27	32	113	20	10	43
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)		-5%	5%		0%	
Storage Length (ft)	0			0	0	0
Storage Lanes	0			0	1	0
Taper Length (ft)	25			25	25	25
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.980		0.891	
Flt Protected		0.978			0.991	
Satd. Flow (prot)	0	1867	1780	0	1645	0
Flt Permitted		0.978			0.991	
Satd. Flow (perm)	0	1867	1780	0	1645	0
Link Speed (mph)		30	30		30	
Link Distance (ft)		113	273		120	
Travel Time (s)		2.6	6.2		2.7	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Adj. Flow (vph)	29	35	123	22	11	47
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	64	145	0	58	0
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized	d					
Intersection Capacity Utiliz	de (%)					of Service
Analysis Daried (min) 1E						

Analysis Period (min) 15

Synchro 7 - Report Baseline Page 2

	•	•	4	<b>†</b>	<b>↓</b>	4		
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations	¥		¥	<b>†</b>	f)			
Volume (vph)	4	7	20	981	406	20		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Lane Width (ft)	12	12	10	12	12	12		
Grade (%)	0%			0%	0%			
Storage Length (ft)	0	0	50			0		
Storage Lanes	1	0	1			0		
Taper Length (ft)	25	25	25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00		
Ped Bike Factor								
Frt	0.910				0.994			
Flt Protected	0.984		0.950					
Satd. Flow (prot)	1668	0	1652	1863	1801	0		
Flt Permitted	0.984		0.950					
Satd. Flow (perm)	1668	0	1652	1863	1801	0		
Link Speed (mph)	30			30	30			
Link Distance (ft)	101			308	300			
Travel Time (s)	2.3			7.0	6.8			
Confl. Peds. (#/hr)								
Confl. Bikes (#/hr)								
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92		
Growth Factor	100%	100%	100%	100%	100%	100%		
Heavy Vehicles (%)	2%	2%	2%	2%	5%	2%		
Bus Blockages (#/hr)	0	0	0	0	0	0		
Parking (#/hr)								
Mid-Block Traffic (%)	0%			0%	0%			
Adj. Flow (vph)	4	8	22	1066	441	22		
Shared Lane Traffic (%)								
Lane Group Flow (vph)	12	0	22	1066	463	0		
Sign Control	Stop			Free	Free			
Intersection Summary								
Area Type:	Other							
Control Type: Unsignalized								
Intersection Capacity Utiliz	ation 61.6%			IC	CU Level	of Service E		
Analysis Period (min) 15								

	٠	<b>→</b>	•	•	<b>←</b>	•	•	<b>†</b>	<b>/</b>	<b>/</b>	<b>↓</b>	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ň	f)		Ţ	f)	
Volume (vph)	16	0	85	14	1	0	68	472	2	2	974	33
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	12	12	10	12	12
Grade (%)		-5%			0%			0%			0%	
Storage Length (ft)	0		0	0		0	100		0	100		0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (ft)	25		25	25		25	25		25	25		25
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.886						0.999			0.995	
Flt Protected		0.992			0.956		0.950			0.950		
Satd. Flow (prot)	0	1678	0	0	1781	0	1652	1825	0	1652	1853	0
Flt Permitted		0.992			0.956		0.950			0.950		
Satd. Flow (perm)	0	1678	0	0	1781	0	1652	1825	0	1652	1853	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		273			185			627			308	
Travel Time (s)		6.2			4.2			14.3			7.0	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.83	0.83	0.83	0.61	0.61	0.61	0.84	0.84	0.84	0.90	0.90	0.90
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	4%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	19	0	102	23	2	0	81	562	2	2	1082	37
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	121	0	0	25	0	81	564	0	2	1119	0
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
<i>J</i> I	Other											
Control Type: Unsignalized												
Intersection Capacity Utiliza	ition 69.0%			IC	CU Level	of Service	e C					

Intersection Capacity Utilization 69.0% Analysis Period (min) 15

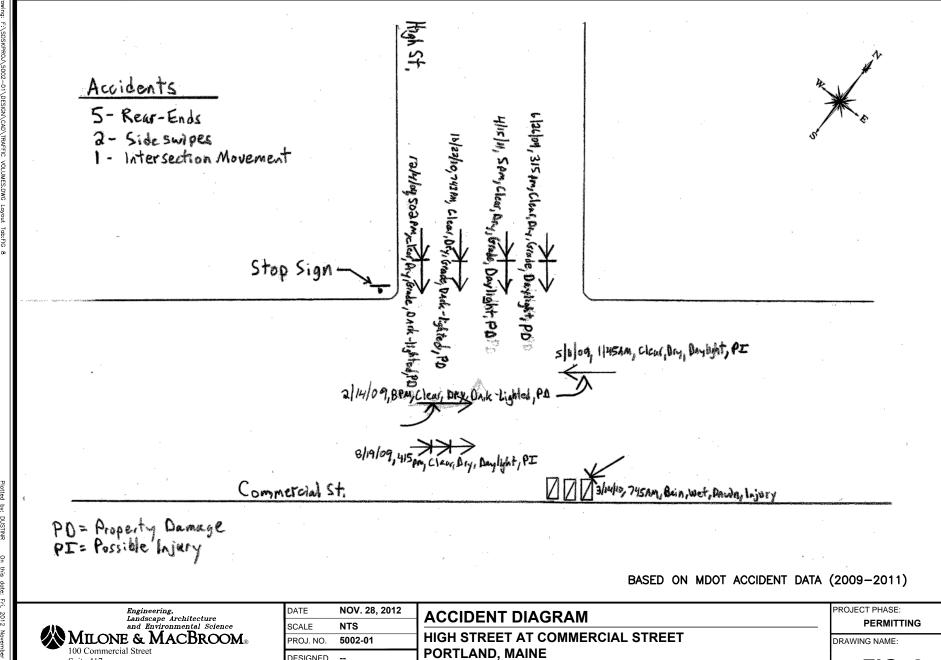
	•	<b>→</b>	<b>←</b>	•	<b>&gt;</b>	4
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		ર્ન	<b>^}</b>		W	
Volume (vph)	45	75	95	8	28	26
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)		-5%	5%		0%	
Storage Length (ft)	0			0	0	0
Storage Lanes	0			0	1	0
Taper Length (ft)	25			25	25	25
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.989		0.935	
Flt Protected		0.982			0.975	
Satd. Flow (prot)	0	1875	1796	0	1698	0
Flt Permitted		0.982			0.975	
Satd. Flow (perm)	0	1875	1796	0	1698	0
Link Speed (mph)		30	30		30	
Link Distance (ft)		113	273		120	
Travel Time (s)		2.6	6.2		2.7	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Adj. Flow (vph)	49	82	103	9	30	28
Shared Lane Traffic (%)	.,			•		
Lane Group Flow (vph)	0	131	112	0	58	0
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized	1					
Intersection Capacity Utilization				IC	U Level	of Service

Analysis Period (min) 15

	۶	•	4	<b>†</b>	ļ	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W		*	<b></b>	f)	
Volume (vph)	21	25	9	481	982	8
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	10	12	12	12
Grade (%)	0%			0%	0%	
Storage Length (ft)	0	0	50			0
Storage Lanes	1	0	1			0
Taper Length (ft)	25	25	25			25
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.927				0.999	
Flt Protected	0.978		0.950			
Satd. Flow (prot)	1689	0	1652	1827	1861	0
Flt Permitted	0.978		0.950			
Satd. Flow (perm)	1689	0	1652	1827	1861	0
Link Speed (mph)	30			30	30	
Link Distance (ft)	101			308	300	
Travel Time (s)	2.3			7.0	6.8	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	4%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Adj. Flow (vph)	23	27	10	523	1067	9
Shared Lane Traffic (%)						
Lane Group Flow (vph)	50	0	10	523	1076	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized	d					
Intersection Capacity Utiliz				IC	CU Level	of Service

Analysis Period (min) 15

Synchro 7 - Report Page 3 Baseline



J.B. BROWN & SONS

36 DANFORTH ST., PORTLAND, ME 04101

FIG. 8

REVISED: ---

DESIGNED

DRAWN

CHECKED

JQA

Suite 417

Portland, Maine 04101

(207) 541-9544 Fax (207) 541-9548

www.miloneandmacbroom.com