



Strengthening a Remarkable City, Building a Community for Life - www.portlandmaine.gov

Jeff Levine, AICP, Director Director of Planning and Urban Development Tammy Munson Director, Inspections Division

Electronic Signature and Fee Payment Confirmation

Notice: Your electronic signature is considered a legal signature per state law.

By digitally signing the attached document(s), you are signifying your understanding this is a legal document and your electronic signature is considered a *legal signature* per Maine state law. You are also signifying your intent on paying your fees by the opportunities below.

I, the undersigned, intend and acknowledge that no permit application can be reviewed until payment of appropriate permit fees are *paid in full* to the Inspections Office, City of Portland Maine by method noted below:

Within 24-48 hours, upon receipt of an e-mailed invoice from Building Inspections, which signifies that my electronic permit application and corresponding paperwork have been received, determined complete, entered by an administrative representative, and assigned a permit number, I then have the following four (4) payment options:

- to provide an on-line electronic check or credit/debit card (we now accept American Express, Discover, VISA, and MasterCard) payment (along with applicable fees beginning July 1, 2014),
- call the Inspections Office at (207) 874-8703 and speak to an administrative representative to provide a credit/debit card payment over the phone,

whand-deliver a payment method to the Inspections Office, Room 315, Portland City Hall,

or deliver a payment method through the U.S. Postal Service, at the following address:

City of Portland Inspections Division 389 Congress Street, Room 315 Portland, Maine 04101

Once my payment has been received, this then starts the review process of my permit. After all approvals have been met and completed, I will then be issued my permit via e-mail. No work shall be started until I have received my permit.

Applicant Signature:	Culmy.	Date: 23 pry 20/4
I have provided digital copies and sent them or	BAAL	Date: 23 Jon Loty

NOTE: All electronic paperwork must be delivered to <u>buildinginspections@portlandmaine.gov</u> or by physical means ie; a thumb drive or CD to the office.

Room 315 - 389 Congress Street- Portland, Maine 04101 (207) 874-8703 - Fax: 874-8716 - TTY: 874-8936



Commercial Interior & Change of Use Permit Application Checklist



All of the following information is required and must be submitted. Checking off each item as you prepare your application package will ensure your package is complete and will help to expedite the permitting process.

One (1) complete set of construction drawings must include:

Note: Construction documents for costs in excess of \$50,000.00 must be prepared by a Design Professional and bear their seal.

- Cross sections w/framing details
- Detail of any new walls or permanent partitions
- Floor plans and elevations
- Window and door schedules
- Complete electrical and plumbing layout.
- Mechanical drawings for any specialized equipment such as furnaces, chimneys, gas equipment, HVAC equipment or other types of work that may require special review
- Insulation R-factors of walls, ceilings, floors & U-factors of windows as per the IEEC 2009
- Proof of ownership is required if it is inconsistent with the assessors records.
- Reduced plans or electronic files in PDF format are required if originals are larger than 11" x 17".
- Per State Fire Marshall, all new bathrooms must be ADA compliant.

Separate permits are required for internal and external plumbing, HVAC & electrical installations.

For additions less than 500 sq. ft. or that does not affect parking or traffic, a site plan exemption should be filed including:

- The shape and dimension of the lot, footprint of the existing and proposed structure and the distance from the actual property lines.
- Location and dimensions of parking areas and driveways, street spaces and building frontage.
- Dimensional floor plan of existing space and dimensional floor plan of proposed space.

A Minor Site Plan Review is required for any change of use between 5,000 and 10,000 sq. ft. (cumulatively within a 3-year period)



Fire Department requirements.

The following shall be submitted on a separate sheet:

- □ Name, address and phone number of applicant and the project architect.
- Proposed use of structure (NFPA and IBC classification)
- Square footage of proposed structure (total and per story)
- Existing and proposed fire protection of structure.
- Separate plans shall be submitted for
 - a) Suppression system
 - b) Detection System (separate permit is required)
- A separate Life Safety Plan must include:
 - a) Fire resistance ratings of all means of egress
 - b) Travel distance from most remote point to exit discharge
 - c) Location of any required fire extinguishers
 - d) Location of emergency lighting
 - c) Location of exit signs
 - f) NFPA 101 code summary
- □ Elevators shall be sized to fit an 80" x 24" stretcher.

For questions on Fire Department requirements call the Fire Prevention Officer at (207) 874-8405.

Please submit all of the information outlined in this application checklist. If the application is incomplete, the application may be refused.

In order to be sure the City fully understands the full scope of the project, the Planning and Development Department may request additional information prior to the issuance of a permit. For further information or to download copies of this form and other applications visit the Inspections Division on-line at <u>www.portlandmaine.gov</u>, or stop by the Inspections Division office, room 315 City Hall or call 874-8703.

Permit Fee: \$25.00 for the first \$1000.00 construction cost, \$11.00 per additional \$1000.00 cost

This is not a Permit; you may not commence any work until the Permit is issued.





General Building Permit Application

you or the property owner owes real estate or personal property taxes or user charges on any property within the City, payment arrangements must be made before permits of any kind are accepted.

Total Square Footage of Proposed Struct 12, 0 D/7	P Square Footage of Lot	>
Tax Assessor's Charl, Block & Lot Chart# Block# Lot# 39 A]3	Applicant * must be swner, Lessee of Bu Name GEOFFET NGE Address 658 CON6NESS 57- City, State & Zip D + 10)	yer* Telephone:
Lessee/DBA (If Applicable)	Owner (if different from Applicant) Name	Cost Of 36,000
	Address	C of O Fee: \$
-	City, State & Zip	Total Fee: \$
Current legal use (i.e. single family)	APPER MENTS/COMMERCE	176
Proposed Specific use: Is property part of a subdivision? Project description:	If yes, please name	
Contractor's name: 7 (KTY	ONS AUGIEMS SEE DRA	wings
Proposed Specific use: s property part of a subdivision? Project description: INSPALL STOREBR Contractor's name: 7 (KTY	ONS AUGIEMS SEE DRA	MINES
Proposed Specific use: s property part of a subdivision? Project description: INSPALL STAREBR Contractor's name:P(RTY Address:P(RTY	ONS AUGIEMS SEE DRA	WINTE 2
Proposed Specific use: Is property part of a subdivision? Project description: INSTALL STOREBR Contractor's name: Address: City, State & Zip	ONS AUGIEMS SEE DRA	Telepho

Please submit all of the information outlined on the applicable Checklist. Failure to do so will result in the automatic denial of your permit.

In order to be sure the City fully understands the full scope of the project, the Planning and Development Department may request additional information prior to the issuance of a permit. For further information or to download copies of this form and other applications visit the Inspections Division on-line at <u>www.portlandmaine.gov</u>, or stop by the Inspections Division office, room 315 City Hall or call 874-8703.

I bereby certify that I am the Owner of record of the named property, or that the owner of record authorizes the proposed work and that I have been authorized by the owner to make this application as his/her authorized agent. I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in this application is issued, I certify that the Gode Official's authorized representative shall have the authority to enter all areas covered by this permit at any reasonable hour to enforce the provisions of the codes applicable to this permit.

Signature:	Millio	Date: 29 1 2014	
Т	'his is not a permit; you may	v not commence ANY work until the permit is issue	

A CONTRACTOR OF THE OWNER OWNER OF THE OWNER OWNE		esign Application			
From Designer:	LEE VIOVAL	Approved with Condition			
Date:					
Job Name:					
Address of Construction:					
Address of Construction:					
Co	2009 International nstruction project was designed to th				
Building Code & Year	Use Group Classificatio	n (s)			
Type of Construction					
Will the Structure have a Fire s	uppression system in Accordance with ?	Section 903.3.1 of the 2009 IRC			
Is the Structure mixed use?	If yes, separated or non sep	parated or non separated (section 302.3)			
Supervisory alarm System?	Geotechnical/Soils report r	required? (See Section 1802.2)			
Structural Design Calculatio		Live load reduction			
Submitted for a	all structural members (106.1 - 106.11)	Roof dire loads (1603.1.2, 1607.11)			
Design Loads on Constructi	on Documents (1603)	Roof snow loads (1603.7.3, 1608)			
Uniformly distributed floor live lo	ads (7603.11, 1807)	Ground snow load, Pg (1608.2)			
Floor Area Use	Loads Shown				
		If Pg > 10 psf, snow exposure factor, G			
		If Pg > 10 psf, snow load importance factor, h			
		Roof thermal factor, G(1608.4)			
		Sloped roof snowload, p.(1608.4)			
Wind loads (1603.1.4, 1609)	4	Seismic design category (1616.3)			
Design option ut	tilized (1609.1.1, 1609.6)	Basic seismic force resisting system (1617.6.2)			
Basic wind speed	I (1809.3)	Response modification coefficient, go and			
Building categor	y and wind importance Factor, <u>1</u> , table 1604.5, 1609.5)	deflection amplification factor _{Cd} (1617.6.2)			
Wind exposure e		Analysis procedure (1616.6, 1617.5)			
Internal pressure of	nefficient (ASCIS 7)	Design base shear (1617.4, 16175.5.1)			
	ladding pressures (1609.1.1, 1609.6.2.2)	Flood loads (1803.1.6, 1612)			
and the second	ressures (7603.1.1, 1609.6.2.1)	Flood Hazard area (1612.3)			
Earth design data (1603.1.5,	1614-1623)	Elevation of structure			
Design option at	ulized (1614.1)	Other loads			
Seismic use grou	p ("Category")				
Spectral response	e coefficients, SDA SDI (1615.1)	Concentrated loads (1607.4) Partitions loads (1607.5)			
Site class (1615.1.	5)	Partition loads (1607.5) Miss. loads (Table 1607.8, 1607.6,1, 1607.7			
		Misc. loads (Table 1607.8, 1607.6.1, 1607.7, 1607.12, 1607.13, 1610, 1611, 2404			

Building Inspections Division • 389 Congress Street • Portland, Maine 04101 • (207) 874-8703 • FACSIMILE (207) 874-8716 • TTY (207) 874-8936



(SEAL)

Accessibility Building Code Certificate



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Designer:	PREVINGLY WRMMD	62/20/2010
Address of Project:		
Nature of Project:		

The technical submissions covering the proposed construction work as described above have been designed in compliance with applicable referenced standards found in the Maine Human Rights Law and Federal Americans with Disability Act. Residential Buildings with 4 units or more must conform to the Federal Fair Housing Accessibility Standards. Please provide proof of compliance if applicable.

Signature:	
Title:	
Firm:	
Address:	
Phone:	

For more information or to download this form and other permit applications visit the Inspections Division on our website at www.portlandmaine.gov

Building Inspections Division + 389 Congress Street + Portland, Maine 04101 + (207) 874-8703 + FACSIMILE (207) 874-8716 + TTY (207) 874-8936



Certificate of Design



Date:

PREVIDOUS SUBMITTED 2/10/2010

From:

These plans and / or specifications covering construction work on:

Have been designed and drawn up by the undersigned, a Maine registered Architect / Engineer according to the **2009 International Building Code** and local amendments.

	Signature:	
	Title:	
(SEAL)	Firm:	
	Address:	
	Phone:	

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Project: 602 Congress Street Renovations Date Prepared: 4/4/11 ST TWC TOB # 10-002_3 Structural Statement of Special Inspections



Project: 602 Congress Street Renovation

Location: Portland, ME

Owner: *R-T Realty Trust*

This Statement of Special Inspections encompass the following discipline: Structural

This Statement of Special Inspections is submitted as a condition for permit issuance in accordance with the Special Inspection and Structural Testing requirements of the Building Code. It includes a schedule of Special Inspection services applicable to this project as well as the name of the Structural Special Inspection Coordinator (SSIC) and the identity of other approved agencies to be retained for conducting these inspections and tests.

The Structural Special Inspection Coordinator shall keep records of all Structural inspections and shall furnish inspection reports to the Building Code Official (BCO) and the Structural Registered Design Professional in Responsible Charge (SRDP). Discovered discrepancies shall be brought to the immediate attention of the Contractor for correction. If such discrepancies are not corrected, the discrepancies shall be brought to the attention of the Building Official and the Structural Registered Design Professional in Responsible Charge. The Special Inspection program does not relieve the Contractor of his or her responsibilities.

Interim reports shall be submitted to the Building Official and the Structural Registered Design Professional in Responsible Charge at an interval determined by the SSIC and the BCO.

A *Final Report of Special Inspections* documenting completion of all required Special Inspections, testing and correction of any discrepancies noted in the inspections shall be submitted to the BCO prior to issuance of a Certificate of Use and Occupancy.

Job site safety and means and methods of construction are solely the responsibility of the Contractor.

Interim Report Frequency:

 \boxtimes Upon request of Building Official _____

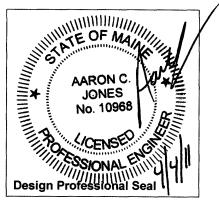
or i per attached schedule.

Prepared by:

Aaron C. Jones, P.E.

(type or print name of the Structural Registered Design Professional in Responsible Charge)

Signature



Owner's Authorization:

Building Code Official's Acceptance:

Signature

Date

Signature

<u>4/4/11</u> Date

Date

Structural Statement of Special Inspections (Continued)

List of Agents

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Project: 602 Congress Street Renovation

Location: Portland, ME

Owner: **R-T Realty Trust**

This Statement of Special Inspections encompass the following discipline: Structural

(Note: Statement of Special Inspections for other disciplines may be included under a separate cover)

This Statement of Special Inspections / Quality Assurance Plan includes the following building systems:

- Soils and Foundations \boxtimes
- \boxtimes Cast-in-Place Concrete
- Precast Concrete System
- \boxtimes Masonry Systems
- \square Structural Steel
 - Mmor Tw Natore. D Special Cases Wood Construction-

03/05/15

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Date

Special Inspection Agencies	Firm	Address, Telephone, e-mail
1. STRUCTURAL Special Inspections Coordinator (SSIC)	Structural Integrity, Inc	77 Oak St. Portland, ME 04101 aaron@structuralinteg.com
2. Special Inspector (SI 1)	Structural Integrity, Inc	77 Oak St. Portland, ME 04101 aaron@structuralinteg.com
3. Special Inspector (SI 2)	TBD OR SUMMIT GEOTECH LEWISJON, ME	
4. Testing Agency (TA 1)	TBD	
5. Testing Agency (TA 2)	TBD	
6. Other (O1)	N/A	

Note: The inspectors and testing agencies shall be engaged by the Owner or the Owner's Agent, and not by the Contractor or Subcontractor whose work is to be inspected or tested. Any conflict of interest must be disclosed to the Building Official, prior to commencing work.

Structural Statement of Special Inspections (Continued)



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Final Report of Special Inspections (SSIC/SI 1)

[To be completed by the Structural Special Inspections Coordinator (SSIC/SI 1). Note that all Agent's Final Reports must be received prior to issuance.]

 Project:
 602 Congress Street Renovation

 Location:
 Portland, ME

 Owner:
 R-T Realty Trust

Owner's Address:

Architect of Record:	James Sterling, AIA	Sterling Architects		
	(name)		(firm)	
Structural Registered Des	ign			
Professional in Responsib	le Charge:	Aaron C. Jones, PE		Structural Integrity, Inc.
		(name)		(firm)

To the best of my information, knowledge and belief, the Special Inspections required for this project, and itemized in the *Statement of Special Inspections* submitted for permit, have been performed and all discovered discrepancies have been reported and resolved.

Interim reports submitted prior to this final report form a basis for and are to be considered an integral part of this final report.

Respectfully submitted, Structural Special Inspection Coordinator <u>Aaron</u> <u>Soves</u> (Type or print name) <u>Structural</u> <u>Lutegrity</u>, <u>Luc</u>. (Firm Name)

Signature

Date



Structural Statement of Special Inspections (Continued) Special Inspector's/Agent's Final Report

Project:

602 Congress Street Renovation

(name)

Special Inspector or Agent:

Designation:

To the best of my information, knowledge and belief, the Special Inspections or testing required for this project, and designated for this Inspector/Agent in the *Statement of Special Inspections* submitted for permit, have been performed and all discovered discrepancies have been reported and resolved.

(firm)

Interim reports submitted prior to this final report form a basis for and are to be considered an integral part of this final report.

Respectfully submitted, Special Inspector or Agent:

(Type or print name)

Signature

Date

Licensed Professional Seal or Certification Number



Structural Schedule of Special Inspections

Qualifications of Inspectors and Testing Technicians

The qualifications of all personnel performing Special Inspection and testing activities are subject to the approval of the Building Official. The credentials of all Inspectors and testing technicians shall be provided to the Special Inspector for their records. NOTE VERIFICATION THAT QUALIFIED INDIVIDUALS ARE AVAILABLE TO PERFORM STIPULATED TESTING AND/OR INSPECTION SHOULD BE PROVIDED PRIOR TO SUBMITTING STATEMENT. AGENT QUALIFICATIONS IN SCHEDULE ARE SUGGESTIONS ONLY; FINAL QUALIFICATIONS ARE SUBJECT TO THE DISCRETION OF THE REGISTERED DESIGN PROFESSIONAL PREPARING THE SCHEDULE.

Key for Minimum Qualifications of Inspection Agents:

When the Registered Design Professional in Responsible Charge or Special Inspector of Record deems it appropriate that the individual performing a stipulated test or inspection have a specific certification, license or experience as indicated below, such requirement shall be listed below and shall be clearly identified within the schedule under the Agent Qualification Designation.

 PE/SE
 Structural Engineer – a licensed SE or PE specializing in the design of building structures

 PE/GE
 Geotechnical Engineer – a licensed PE specializing in soil mechanics and foundations

 EIT
 Engineer-In-Training – a graduate engineer who has passed the Fundamentals of Engineering examination

Experienced Testing Technician

ETT Experienced Testing Technician – An Experienced Testing Technician with a minimum 5 years experience with the stipulated test or inspection

American Concrete Institute (ACI) Certification

ACI-CFTT	Concrete Field Testing Technician – Grade 1
ACI-CCI	Concrete Construction Inspector
ACI-LTT	Laboratory Testing Technician – Grade 1&2
ACI-STT	Strength Testing Technician

American Welding Society (AWS) Certification

AWS-CWI Certified Welding Inspector AWS/AISC-SSI Certified Structural Steel Inspector

American Society of Non-Destructive Testing (ASNT) Certification

ASNT Non-Destructive Testing Technician – Level II or III.

International Code Council (ICC) Certification

- ICC-SMSIStructural Masonry Special InspectorICC-SWSIStructural Steel and Welding Special InspectorICC-SFSISpray-Applied Fireproofing Special Inspector
- ICC-PCSI Prestressed Concrete Special Inspector
- ICC-RCSI Reinforced Concrete Special Inspector

National Institute for Certification in Engineering Technologies (NICET)

NICET-CT	Concrete Technician – Levels I, II, III & IV
NICET-ST	Soils Technician - Levels I, II, III & IV
NICET-GET	Geotechnical Engineering Technician - Levels I, II, III & IV

Other



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Structural Schedule of Special Inspections SOILS & FOUNDATION CONSTRUCTION



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VERIFICATION AND INSPECTION IBC Section 1704.7, 1704.8, 1704.9	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS	AGENT	AGENT QUALIFICATION	TASK COMPLETED
1. Verify existing soil conditions, fill placement and load bearing requirements						
 a. Prior to placement of prepared fill, determine that the site has been prepared in accordance with the approved soils report. 	Y	Р	IBC 1704.7.1	SI-2	PE/GE, EIT or ETT	
 b. During placement and compaction of fill material, verify material being used and maximum lift thickness comply with the approved soils report. 	N/A		IBC 1704.7.2		PE/GE, EIT or ETT	
c. Test in-place dry density of compacted fill complies with the approved soils report.	N/A		IBC 1704.7.2		PE/GE, EIT or ETT	
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Structural Schedule of Special Inspections CONCRETE CONSTRUCTION



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VERIFICATION AND INSPECTION IBC Section 1704.4	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS	AGEN T	AGENT QUALIFICATION	TASK COMPLETED
1. Inspection of reinforcing steel, including prestressing tendons, and placement	Y	Р	ACI 318: 3.5, 7.1-7.7	SI 1	PE/SE or EIT	
2. Inspection of reinforcing steel welding in accordance with Table 1704.3, Item 5B	N/A		Welding of Reinf Not Allowed		AWS-CWI	
 Inspect bolts to be installed in concrete prior to and during placement of concrete where allowable loads have been increased 	N/A		IBC 1912.5		PE/SE or EIT	
4. Verifying use of required design mix	Y	Р	ACI 318: Ch 4, 5.2-5.4	SI 1	PE/SE or EIT	
5. At time fresh concrete is sampled to fabricate specimens for strength test, perform slump and air content test and temperature	N		ASTM C 172 ASTM C 31 ACI 318: 5.6, 5.8		ACI-CFTT or ACI-STT	Verify w/ submittals
6. Inspection of concrete and shotcrete placement for proper application techniques	Y	р	ACI 318: 5.9, 5.10	SI 1	PE/SE or EIT	
7. Inspection for maintenance of specified curing temperature and techniques	Y	Р	ACI 318: 5.11- 5.13	SI I	PE/SE or EIT	

Project: 602 Congress Street Renovations Date Prepared: 4/4/11

Structural Schedule of Special Inspections MASONRY CONSTRUCTION – LEVEL 1 (NON-ESSENTIAL FACILITY)

VERIFICATION AND INSPECTION IBC Section 1704.5	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS	AGENT	AGENT QUALIFICATION	TASK COMPLETED
1. As masonry construction begins, the following shall be verified to ensure compliance:						
a. Proportions of site-prepared mortar.	Y	Р	ACI530.1, 2.6A	SI 1	PE/SE or EIT	
b. Construction of mortar joints.	Y	Р	ACI530.1, 3.3B	SI 1	PE/SE or EIT	
c. Location of reinforcement and connectors.	Y	Р	ACI530.1, 3.4, 3.6A	SI 1	PE/SE or EIT	
d. Prestressing technique.	N		ACI530.1, 3.6B		PE/SE or EIT	
e. Grade and size of prestressing tendons and anchorages.	N		ACI530.1, 2.4B, 2.4H		PE/SE or EIT	
2. The inspection program shall verify:						
a. Size and location of structural elements.	Y	Р	ACI530.1, 3.3G	SI 1	PE/SE or EIT	
 b. Type, size and location of anchors, including other details of anchorage of masonry to structural members, frames or other construction. 	Y	Р	ACI530, 1.2.2(e), 2.1.4, 3.1.6	SI 1	PE/SE or EIT	
c. Specified size, grade and type of reinforcement.	Y	Р	ACI530, 1.12, ACI530.1, 2.4, 3.4	SI 1	PE/SE or EIT	
d. Welding of reinforcing bars.	N		AC530, 2.1.10.6.2, 3.24 (b)		AWS-CWI	
e. Protection of masonry during cold weather (temperature below 40°F) or hot weather (temperature above 90°F).	Y	Р	IBC 2104.3, 2104.4; ACI530.1, 1.8C, 1.8D	SI 1	PE/SE or EIT	
f. Application and measurement of prestressing force.	N		ACI530.1, 3.6B		PE/SE or EIT	
3. Prior to grouting, the following shall be verified to ensure compliance:						
a. Grout space is clean.	Y	Р	ACI530.1, 3.2D	SI 1	PE/SE or EIT	
 b. Placement of reinforcement and connectors and prestressing tendons and anchorages. 	Y	Р	ACI530, 1.12, ACI530.1, 3.4	SI 1	PE/SE or EIT	
 c. Proportions of site-prepared grout and prestressing grout for bonded tendons. 	N		ACI530.1, 2.6B		PE/SE or EIT	
d. Construction of mortar joints.	Y	Р	AC1530.1, 3.3B	SI 1	PE/SE or EIT	
 Grout placement shall be verified to ensure compliance with code and construction document provisions. 	Y	Р	ACI530.1, 3.5	SI 1	PE/SE or EIT	
a. Grouting of prestressing bonded tendons.	N		ACI530.1, 3.6C		PE/SE or EIT	
 Preparation of any required grout specimens, mortar specimens and/or prisms shall be observed. 	N		IBC 2105.2.2, 2105.3; ACI530.1, 1.4		PE/SE or EIT	
 Compliance with required inspection provisions of the construction documents and the approved submittals shall be verified. 	Y	S	ACI530.1, 1.5	SSIC	PE/SE or EIT	



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Project: 602 Congress Street Renovations Date Prepared: 4/4/11

Structural Schedule of Special Inspections - STEEL CONSTRUCTION

VERIFICATION AND INSPECTION	Y/N	<u>EXTENT:</u> CONTINUOUS,	COMMENTS	AGENT	AGENT QUALIFICATION	C Date:_	Approved with Cond 03/05/1
IBC Section 1704.3		PERIODIC, SUBMITTAL, OR NONE			QUALIFICATION		
 Material verification of high-strength bolts, nuts and washers: 							
a. Identification markings to conform to ASTM standards specified in the approved construction documents.	Y	S	Applicable ASTM material specifications; AISC 335, Section A3.4; AISC LRFD, Section A3.3	SSIC	PE/SE or EIT		
b. Manufacturer's certificate of compliance required.	Y	S		SSIC	PE/SE or EIT		
2. Inspection of high-strength bolting							2
a. Bearing-type connections.	Y	Р	AISC LRFD Section M2.5	SI 1	AWS/AISC-SSI		
b. Slip-critical connections.	N		IBC Sect 1704.3.3		AWS/AISC-SSI		
3. Material verification of structural steel (IBC Sect 1708.4):							
a. Identification markings to conform to ASTM standards specified in the approved construction documents.	Y	S	ASTM A 6 or ASTM A 568 IBC Sect 1708.4	SI 1	PE/SE or EIT		
b. Manufacturers' certified mill test reports.	Y	S	ASTM A 6 or ASTM A 568 IBC Sect 1708.4	SI 1	PE/SE or EIT		
4. Material verification of weld filler materials:							
a. Identification markings to conform to AWS specification in the approved construction documents.	Y	S	AISC, ASD, Section A3.6; AISC LRFD, Section A3.5	SI 1	PE/SE or EIT		
b. Manufacturer's certificate of compliance required.	N		50000115.5		PE/SE or EIT		
5. Submit current AWS D1.1 welder certificate for all field welders who will be welding on this project.	Y	S	AWS D1.1	SI 1	PE/SE or EIT		
6. Inspection of welding (IBC 1704.3.1): a. Structural steel:							
1) Complete and partial penetration groove welds.	N				AWS-CWI		
2) Multipass fillet welds.	N				AWS-CWI		
3) Single-pass fillet welds> 5/16"	Y	Р	AWS D1.1	SI 1	AWS-CWI		
4) Single-pass fillet welds< 5/16"	Y	Р		SI 1	AWS-CWI		
5) Floor and deck welds.			AWS D1.3		AWS-CWI		
 Inspection of steel frame joint details for compliance (IBC Sect 1704.3.2) with approved construction documents: 							- <u></u>
a. Details such as bracing and stiffening.	Y	Р		SI 1	PE/SE or EIT		
b. Member locations.	Y	Р		SI 1	PE/SE or EIT		
c. Application of joint details at each connection.	Y	Р		SI 1	PE/SE or EIT		

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Structural Schedule of Special Inspection Services FABRICATION AND IMPLEMENTATION PROCEDURES – STRUCTURAL STEEL

VERIFICATION AND INSPECTION IBC Section 1704.2	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS	AGENT	AGENT QUALIFICATION	TASK COMPLETED
 Fabrications Procedures: Review of fabricator's written procedural and quality control manuals and periodic auditing of fabrication practices by an approved special inspection agency. At the completion of fabrication, the approved fabricator shall submit a certificate of compliance to the building code official stating that the work was performed in accordance with the approved construction documents. -OR- 2. AISC Certification 	Y	S	Fabricator shall submit one of the two qualifications	SSIC	PE/SE or EIT	
3. At completion of fabrication, the approved fabricator shall submit a certificate of compliance to the building code official stating that the work was performed in accordance with the approved construction documents.	N		IBC 1704.2.2		PE/SE or EIT	

Prepared by: Aaron C. Jones, PE

ignature Date

Building Code Official's Acceptance:

Signature

Date



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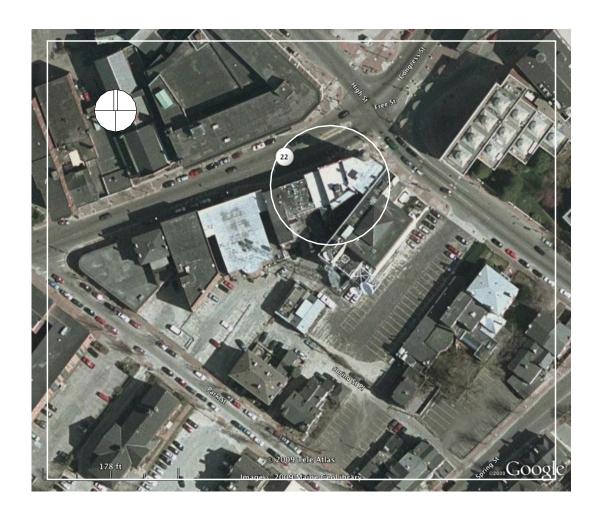
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ARCHITECTURAL DRAWINGS:

A-1.0 BASEMENT A-1.1 FIRST LEVEL A-1.2 SECOND LEVEL A-1.3 THIRD LEVEL A-1.4 FOURTH LEVEL A-1.5 ROOF PLAN A-2.0 CONGRESS STREET HIGH STREET ELEVATIONS A-2.1 REAR ELEVATION A-5.0 STOREFRONT DETAILS A-5.1 STOREFRONT DETAILS A-5.2 DELETED A-5.3 STOREFRONT DETAILS A-6.0 WALL TYPE SCHEDULE A-6.1 DOOR SCHEDULE

STRUCTURAL DRAWINGS:

S-1.0 GENERAL NOTES S-1.1 ROOF PLAN S-1.2 FIRST LEVEL S-1.3 SECOND LEVEL S-1.4 THIRD LEVEL S-1.5 FOURTH LEVEL S-2.1 SECTIONS S-2.2 SECTIONS S-1 L+L TOWER S-2 L+L TOWER

ELECTRICAL DRAWINGS:

E-1 BASEMENT E-2 FIRST LEVEL E-3 SECOND LEVEL E-4 THIRD LEVEL E-5 FOURTH LEVEL E-6 DETAILS + NOTES E-7 RISER DIAGRAM + DETAILS



Reviewed for Code Compliance Inspections Division Approved with Conditions

Date: 03/05/15

SHWARTZ BUILDING 600-604 CONGRESS STREET PORTLAND, MAINE

OWNER: RICE MANAGEMENT COMPANY 658 CONGRESS STREET PORTLAND, MAINE

CODE COMPLIANCE RENOVATIONS

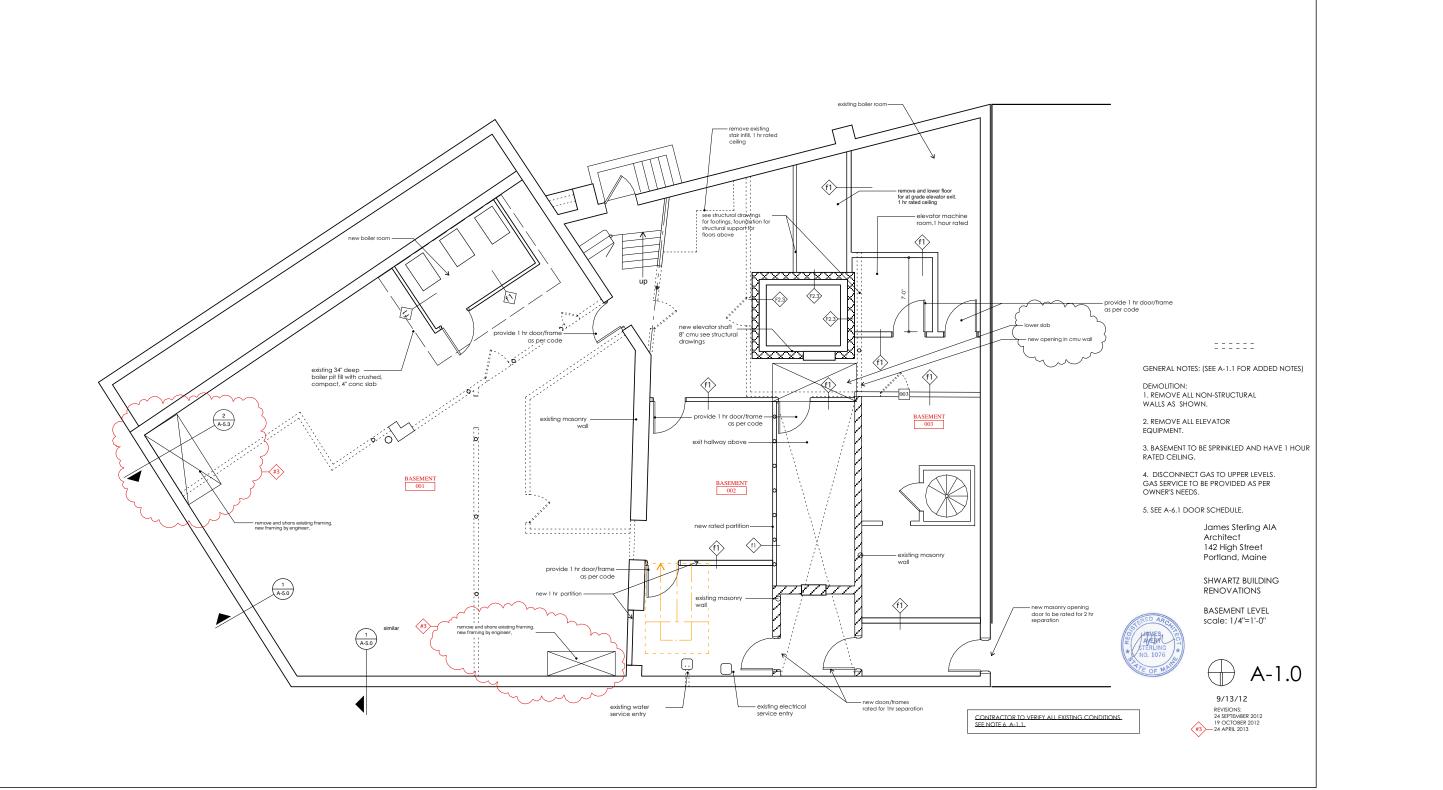
HISTORIC PRESERVATION AND REHABILITATION APPROVED PLANS

James Sterling AIA Architect 142 High Street Portland, Maine

Jeremy Moser Historic Preservation 142 High Street Portland, Maine ALED AACH

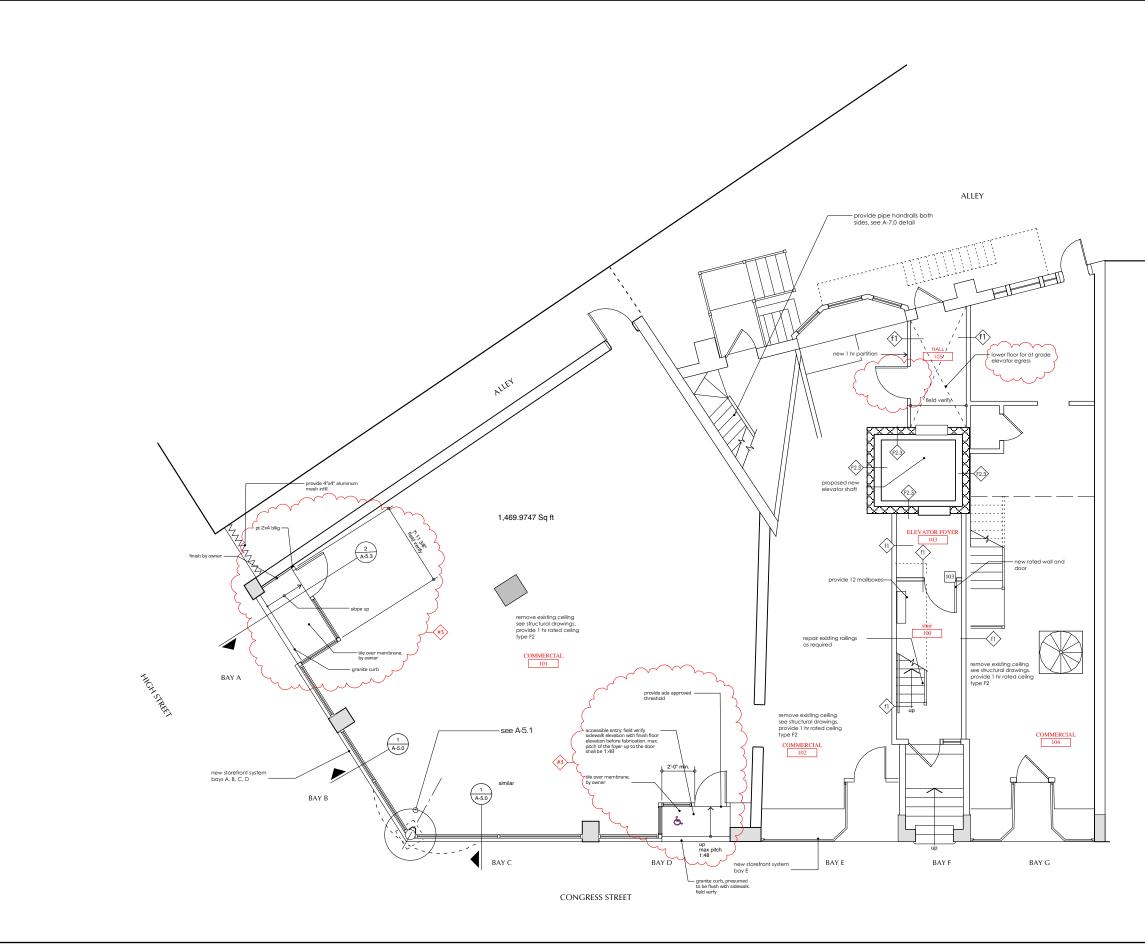
DATE: 01 AUGUST 2012

Revisions: 09 October 2012 19 October 2012 24 April 2013 06 February 2014 12 February 2014





Date: ______03/05/15





Date: 03/05/15

GENERAL NOTES

Demolition: 1. remove all non-structural walls as shown.

2. REMOVE ALL ELEVATOR EQUIPMENT.

3. FLOOR AND CEILING FINISHES IN NEW APARTMENTS TO BE DETERMINED BY OWNER. 1 HOUR RATED CEILING BETWEEN FIRST LEVEL AND SECOND (APARTMENTS) LEVEL.

4. 1 HOUR CEILING BETWEEN BASEMENT AND FIRST LEVEL.

5. ALL EXISTING PANELING ON BOTH SIDES OF STAIR 200 ENCLOSURE TO BE REMOVED EXPOSING EXISTING PLASTER, APPLY F1 FINISH OVER PLASTER.

6. ALL EXISTING OPENINGS TO STAIR 200 TO BE INFILLED, F1.

7. DIMENSIONS ARE FOR NEW PARTITIONS AND OPENINGS ONLY, OVERALL APARTMENT DIMENSIONS ARE TO BE FIELD VERIFIED, CONTRACTOR TO NOTIFY THE ARCHITECT OF ANY DISCREPANCIES BEFORE STARTING WORK.

8. SEE A-6.1 FOR DOOR SCHEDULE AND HISTORIC DOOR PANELS.

9. ALL LIGHTING FIXTURES BY OWNER.

10. CONTRACTOR TO COORDINATE ELEVATOR INSTALLATION WITH STRUCTURAL ENGINEER, ELECTRICAL ENGINEER AND ELEVATOR PROVIDER.

11. HVAC, PLUMBING, FIRE PROTECTION PLANS BY OWNER.

12. APPARTMENT APPLIANCE SCHEDULE TO BE COORDINATED BY OWNER WITH ELECTRICAL ENGINEER.

James Sterling AIA Architect 142 High Street Portland, Maine

SHWARTZ BUILDING RENOVATIONS

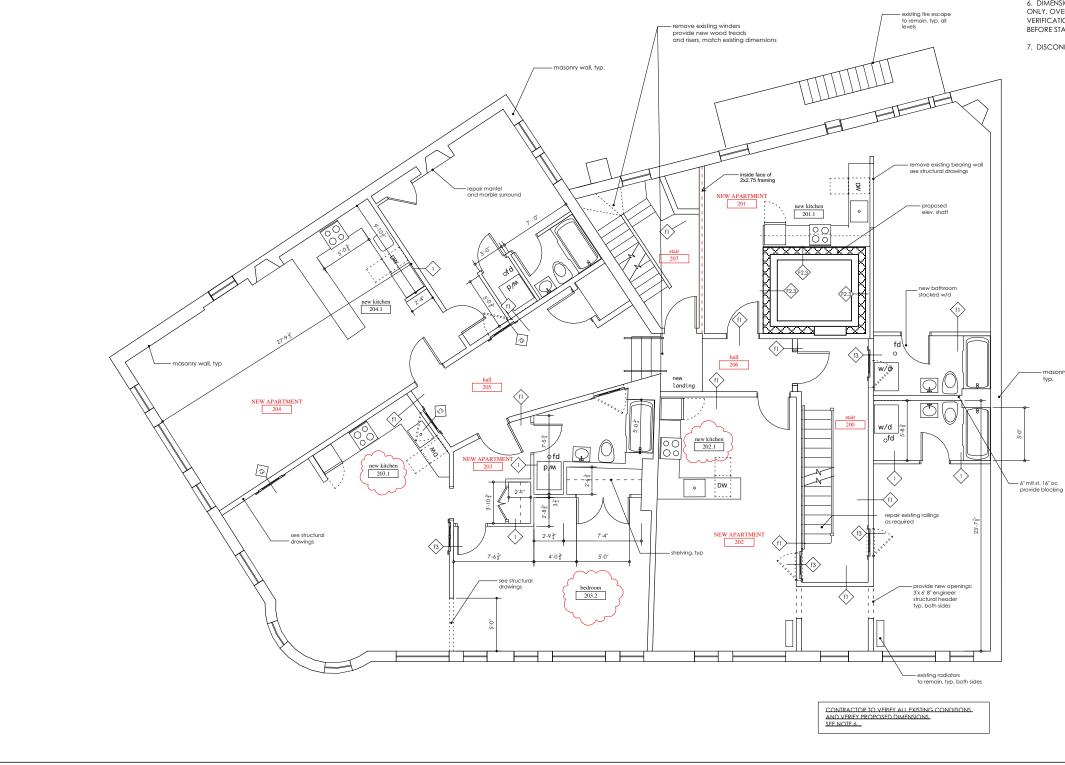
FIRST LEVEL





REVISIONS: 24 SEPTEMBER 2012 19 OCTOBER 2012 24 APRIL 2013





DEMOLITION: 1. REMOVE ALL NON-STRUCTURAL WALLS AS SHOWN AND REMOVE EXISTING DOOR OPENINGS.



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Reviewed for Code Compliance Inspections Division Approved with Conditions

Date: ____03/05/15

2. REMOVE ALL ELEVATOR EQUIPMENT.

3. FLOOR AND CEILNG FINISHES IN NEW APARTMENTS TO BE DETERMINED BY OWNER.

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7. DISCONNECT AND REMOVE ALL GAS PIPING.

REVISIONS: 24 SEPTEMBER 2012 19 OCTOBER 2012

9/13/12 A-1.2

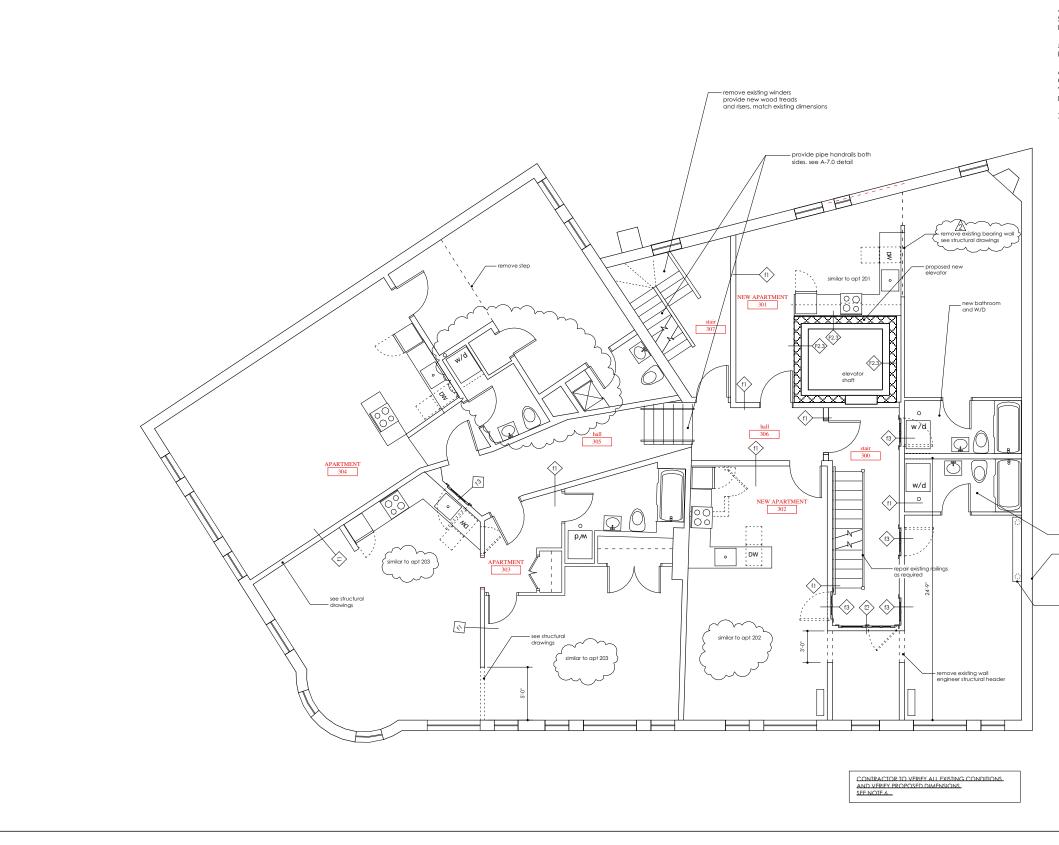
James Sterling AIA Architect 142 High Street Portland, Maine

SHWARTZ BUILDING RENOVATIONS

SECOND LEVEL scale: 1/4" = 1'-0"

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DEMOLITION: 1. REMOVE ALL NON-STRUCTURAL WALLS AS SHOWN AND REMOVE EXISTING DOOR OPENINGS.





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Date: ____03/05/15

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new bathroom and W/D

typ

xisting mantel to remair

James Sterling AIA Architect 142 High Street Portland, Maine

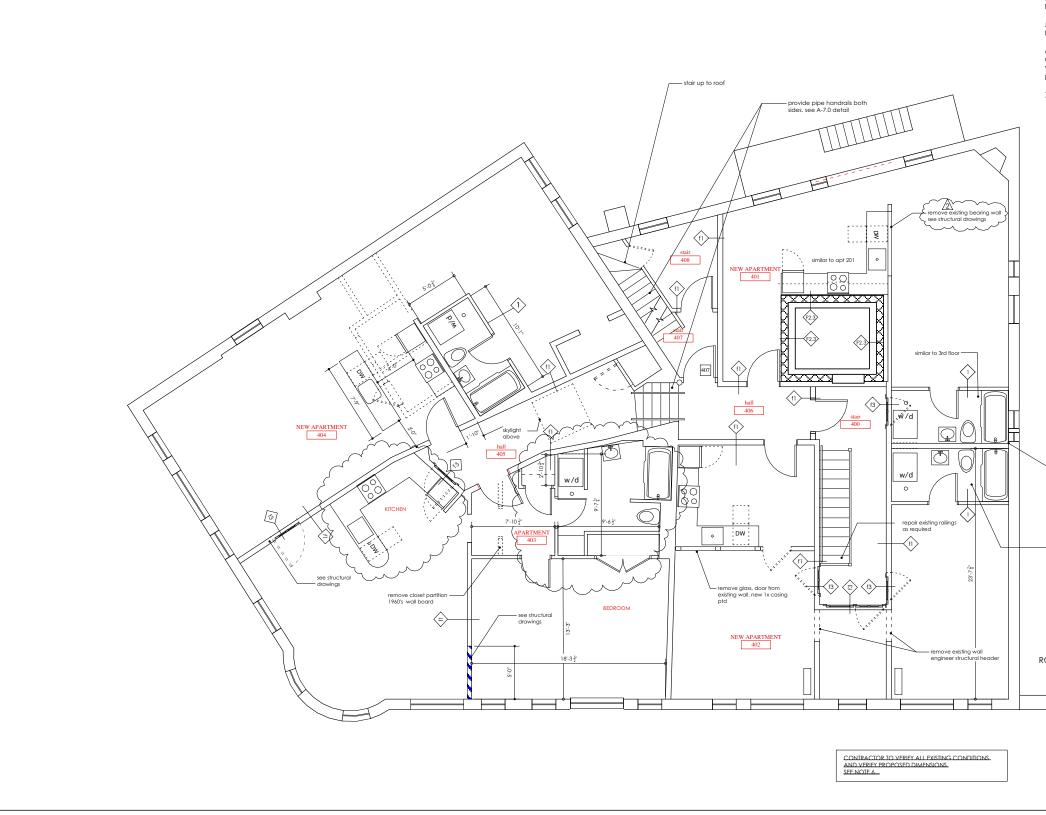
SHWARTZ BUILDING RENOVATIONS

THIRD LEVEL

scale: 1/4" = 1'-0"

REVISIONS: 24 SEPTEMBER 2012 19 OCTOBER 2012

_{9/13/12} A-1.3



DEMOLITION: 1. REMOVE ALL NON-STRUCTURAL WALLS AS SHOWN AND REMOVE EXISTING DOOR OPENINGS.





Reviewed for Code Compliance Inspections Division Approved with Conditions

Date: _____03/05/15

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7. DISCONNECT AND REMOVE ALL GAS PIPING.

similar to 3rd floor

ROOF



Architect 142 High Street Portland, Maine

James Sterling AIA

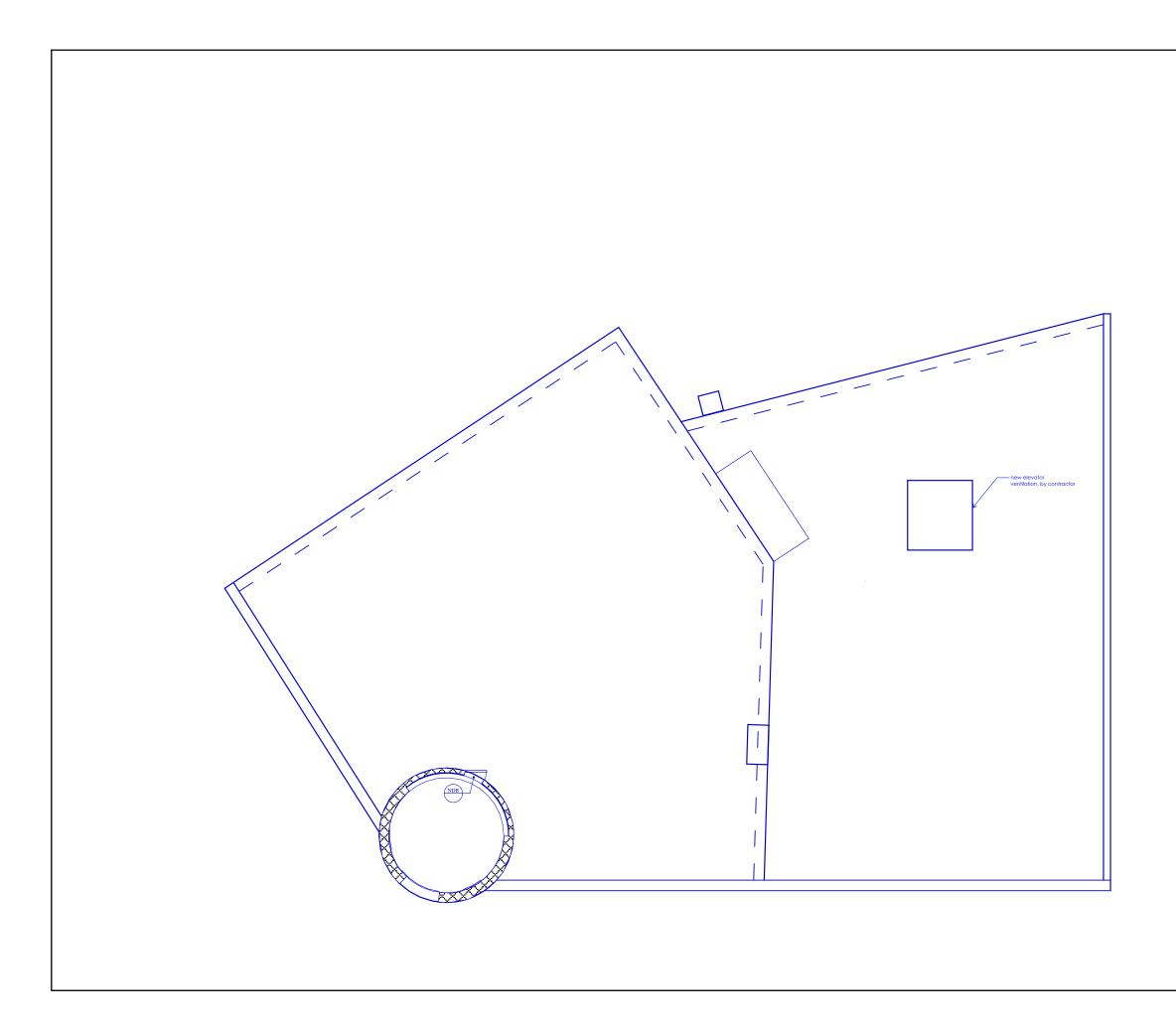
SHWARTZ BUILDING RENOVATIONS

FOURTH LEVEL

scale: 1/4" = 1'-0"



9/13/12





Date: _____03/05/15



James Sterling AIA Architect 142 High Street Portland, Maine

Shwartz building Renovations

ROOF PLAN

scale: 1/4" = 1'-0"

REVISIONS: 24 SEPTEMBER 2012 19 OCTOBER 2012

A-1.5





Date: _____

James Sterling AIA Architect 142 High Street Portland, Maine

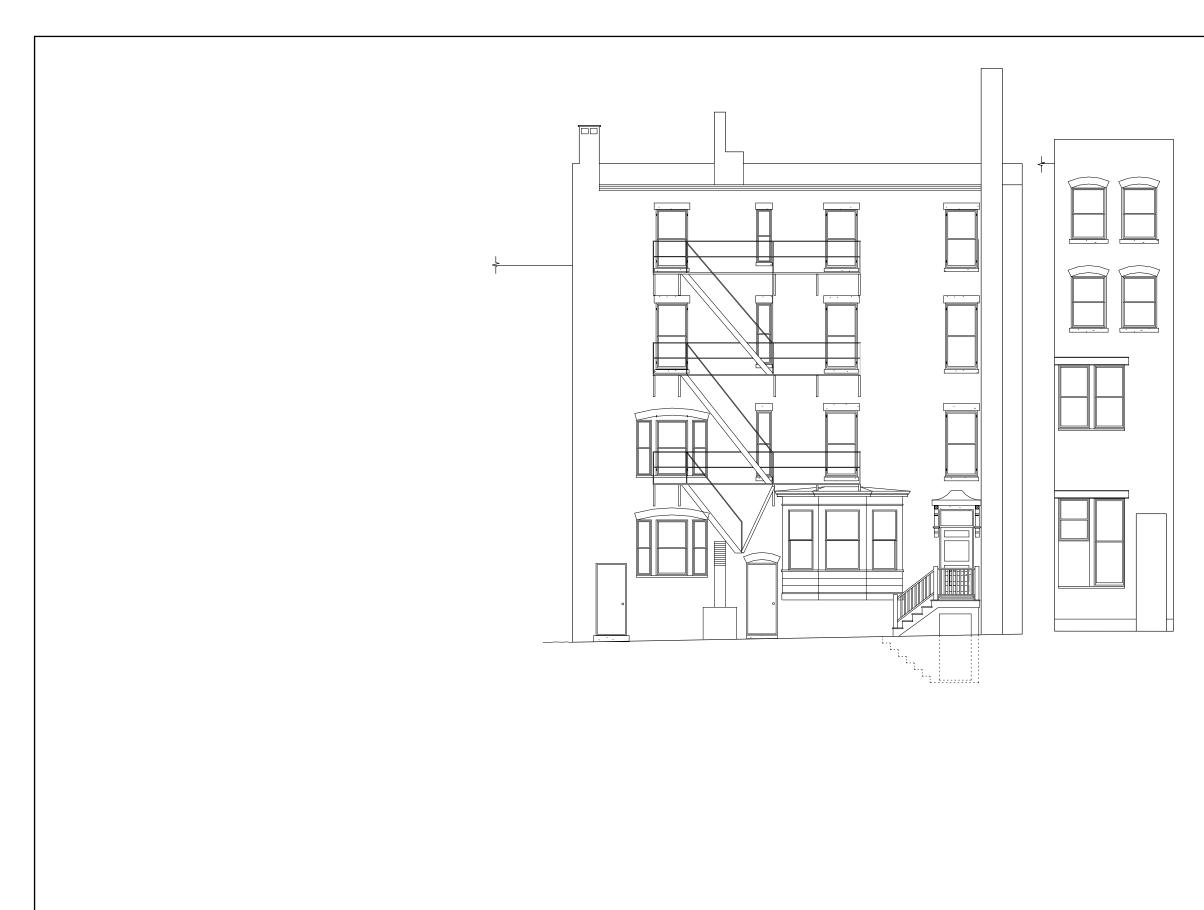
SHWARTZ BUILDING RENOVATIONS

ELEVATIONS

scale: 1/4" = 1'-0"

date: 25 july 2013

A-2.0





Demolition: 1. Remove all non-structural walls as shown.

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7. DISCONNECT AND REMOVE ALL GAS PIPING.

Reviewed for Code Compliance Inspections Division Approved with Conditions

James Sterling AIA Architect 142 High Street Portland, Maine

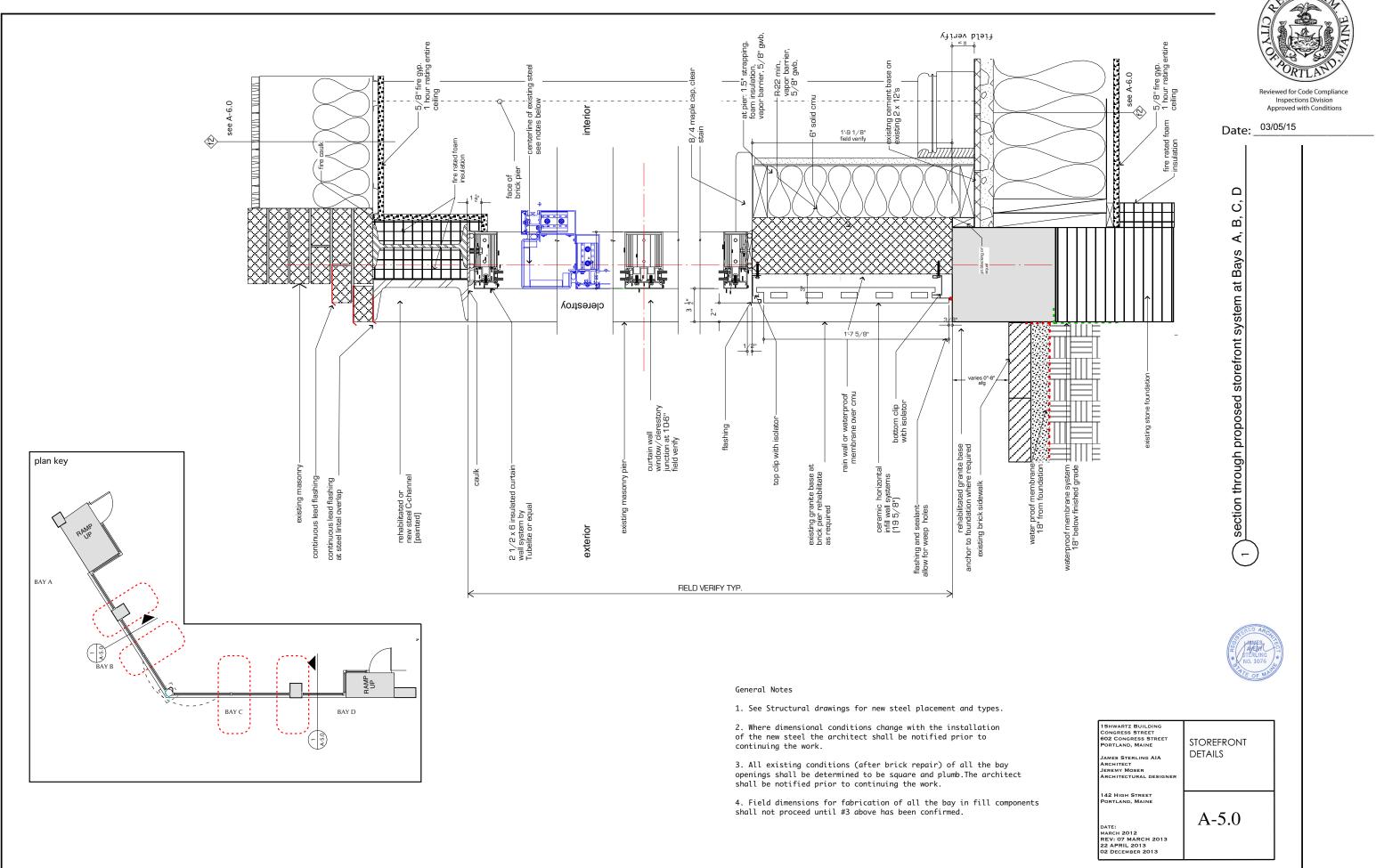
Shwartz Building Renovations

ELEVATIONS

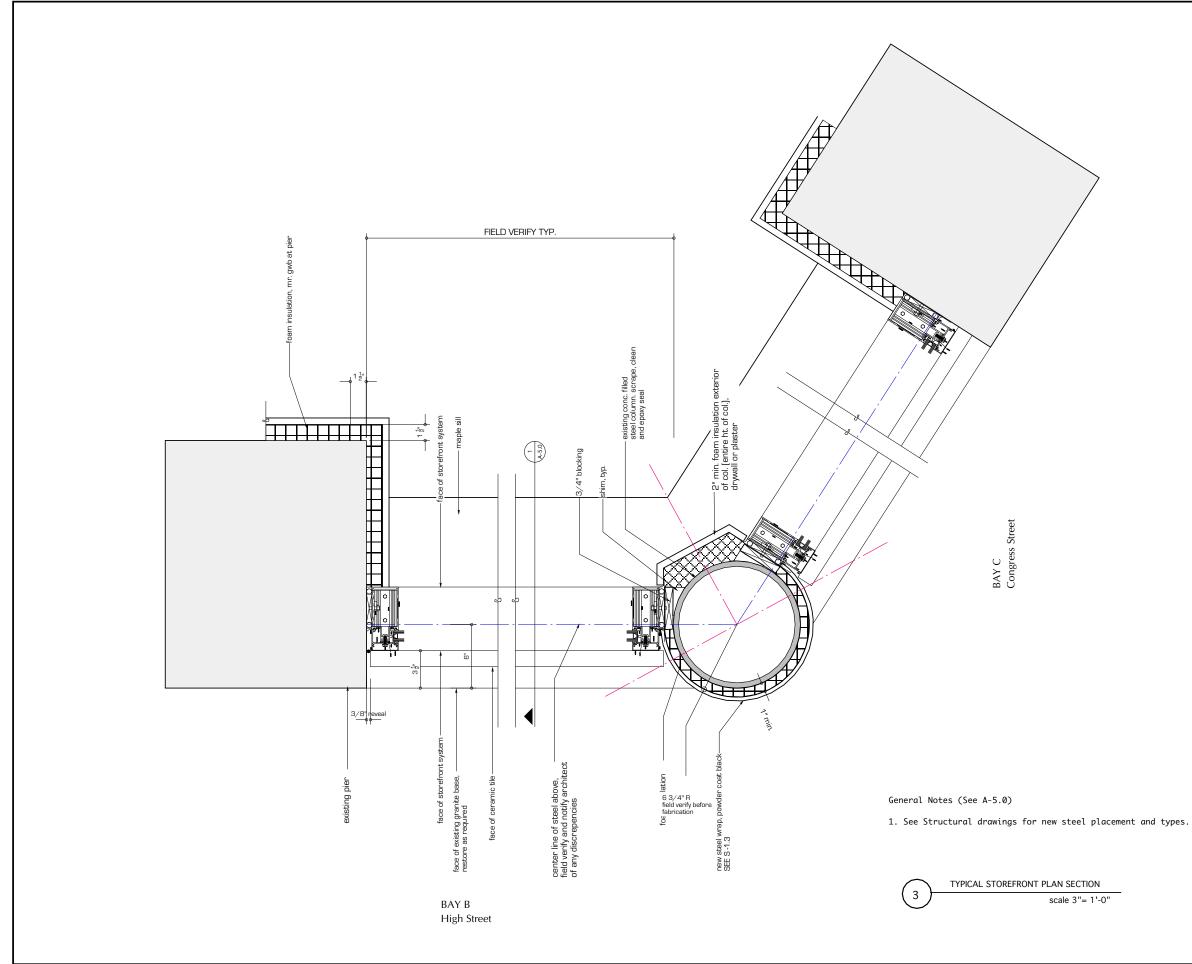
scale: 1/4" = 1'-0"







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SHWARTZ BUILDING Congress Street 602 Congress Street Portland, Maine

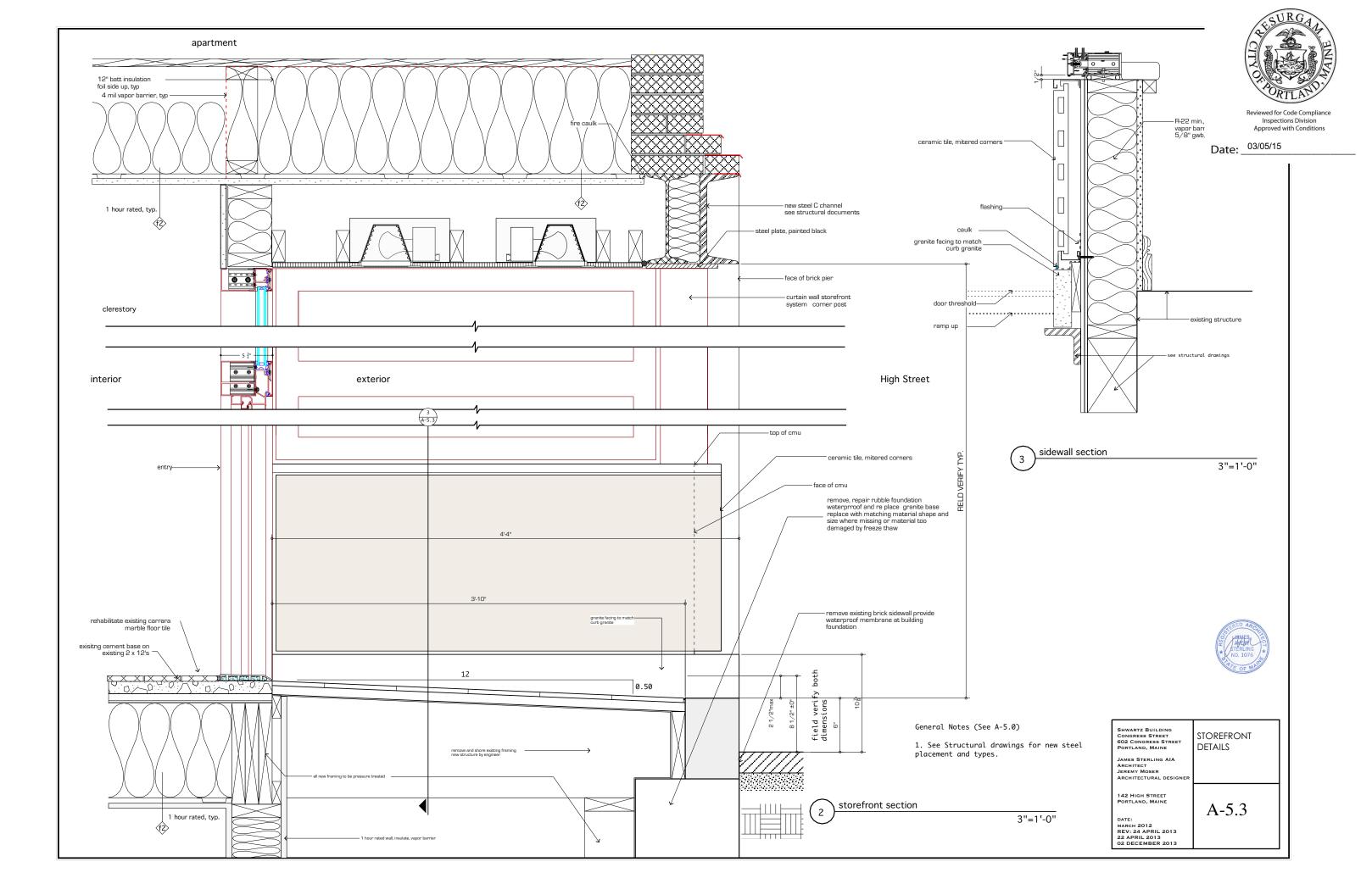
James Sterling AIA Architect Jeremy Moser Architectural design

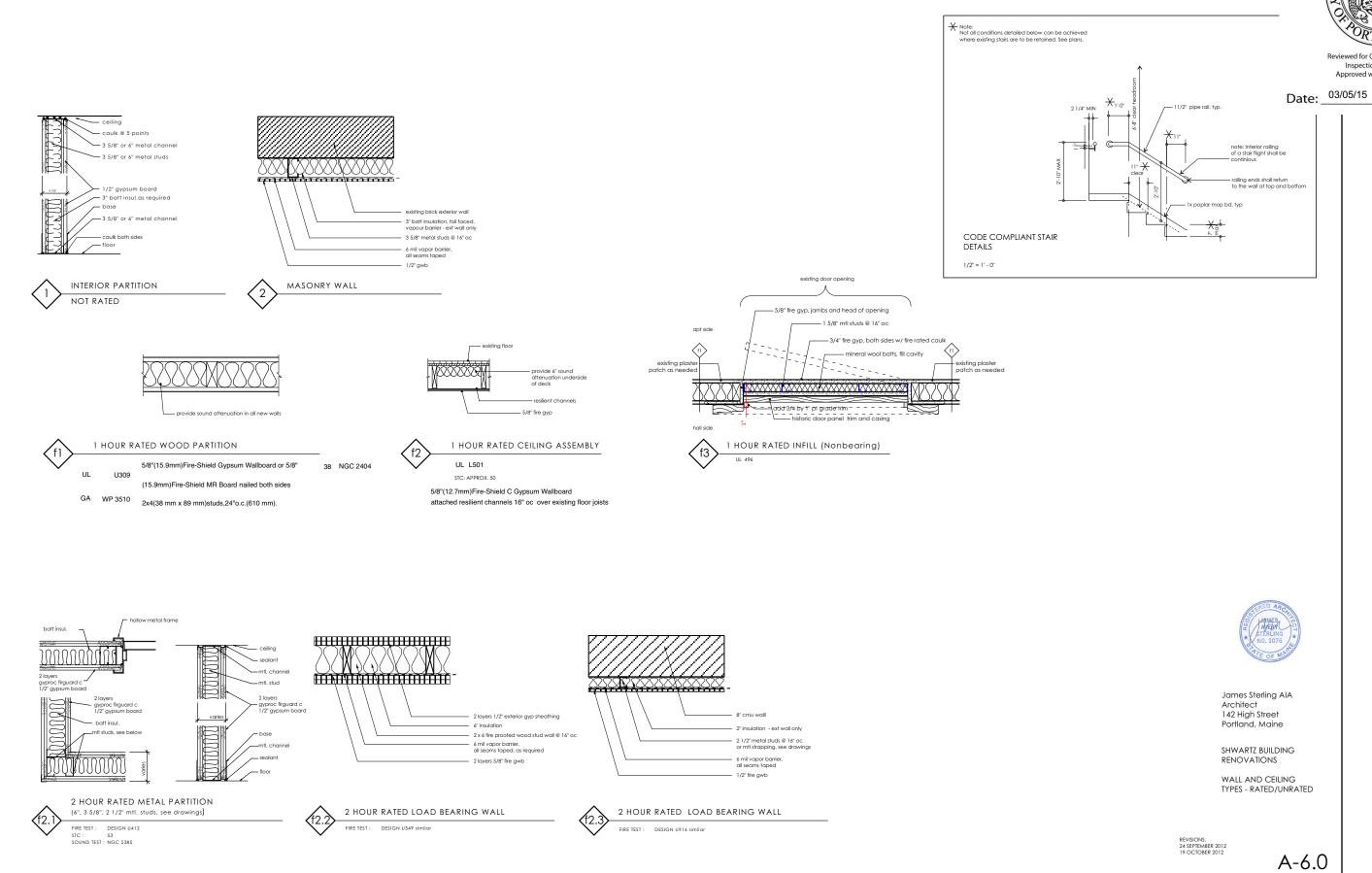
142 High Street Portland, Maine

DATE: MARCH 2012 REV: 07 MARCH 2013 22 APRIL 2013 02 DECEMBER 2013

STOREFRONT DETAILS PLAN

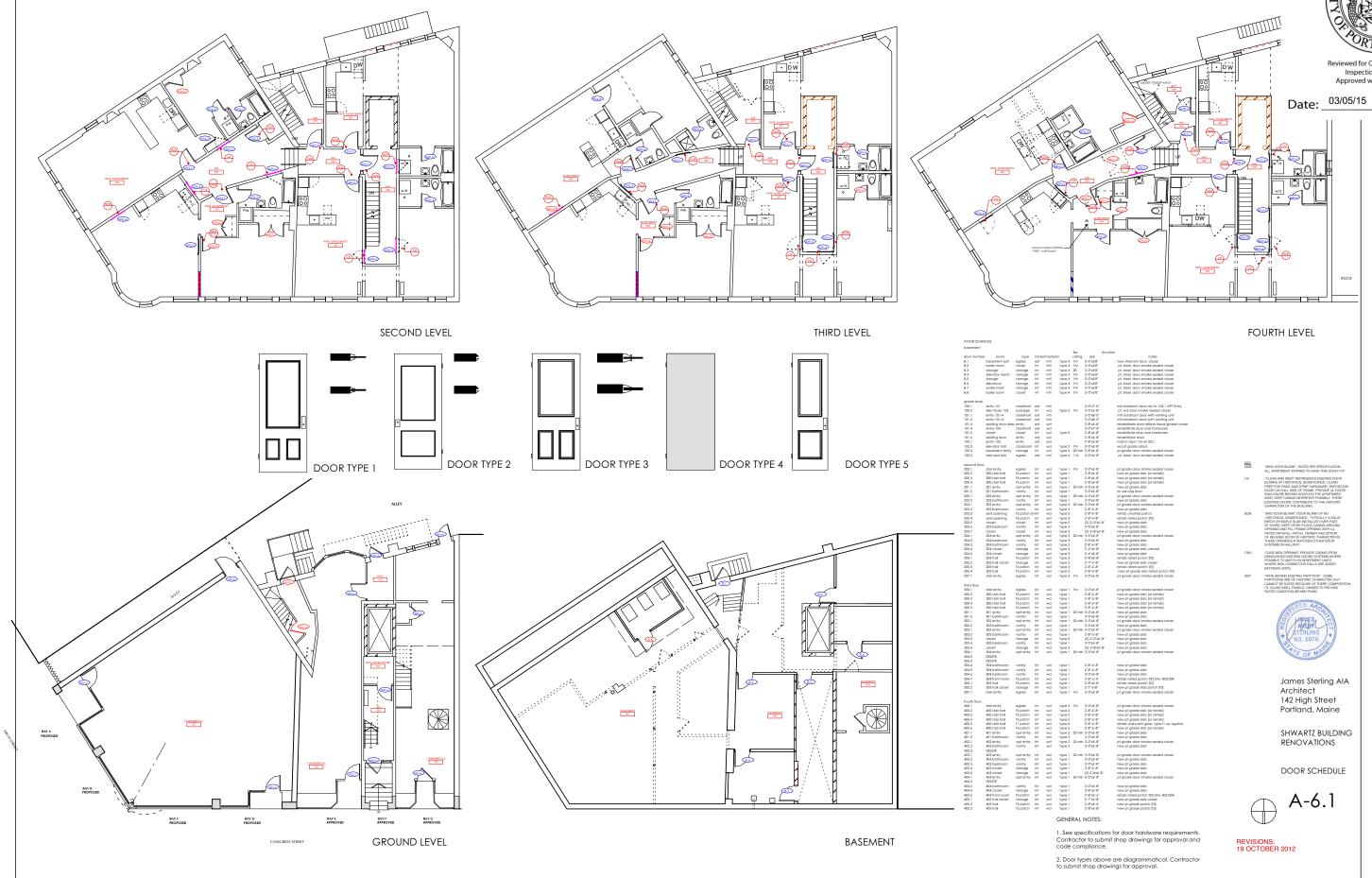
A-5.1







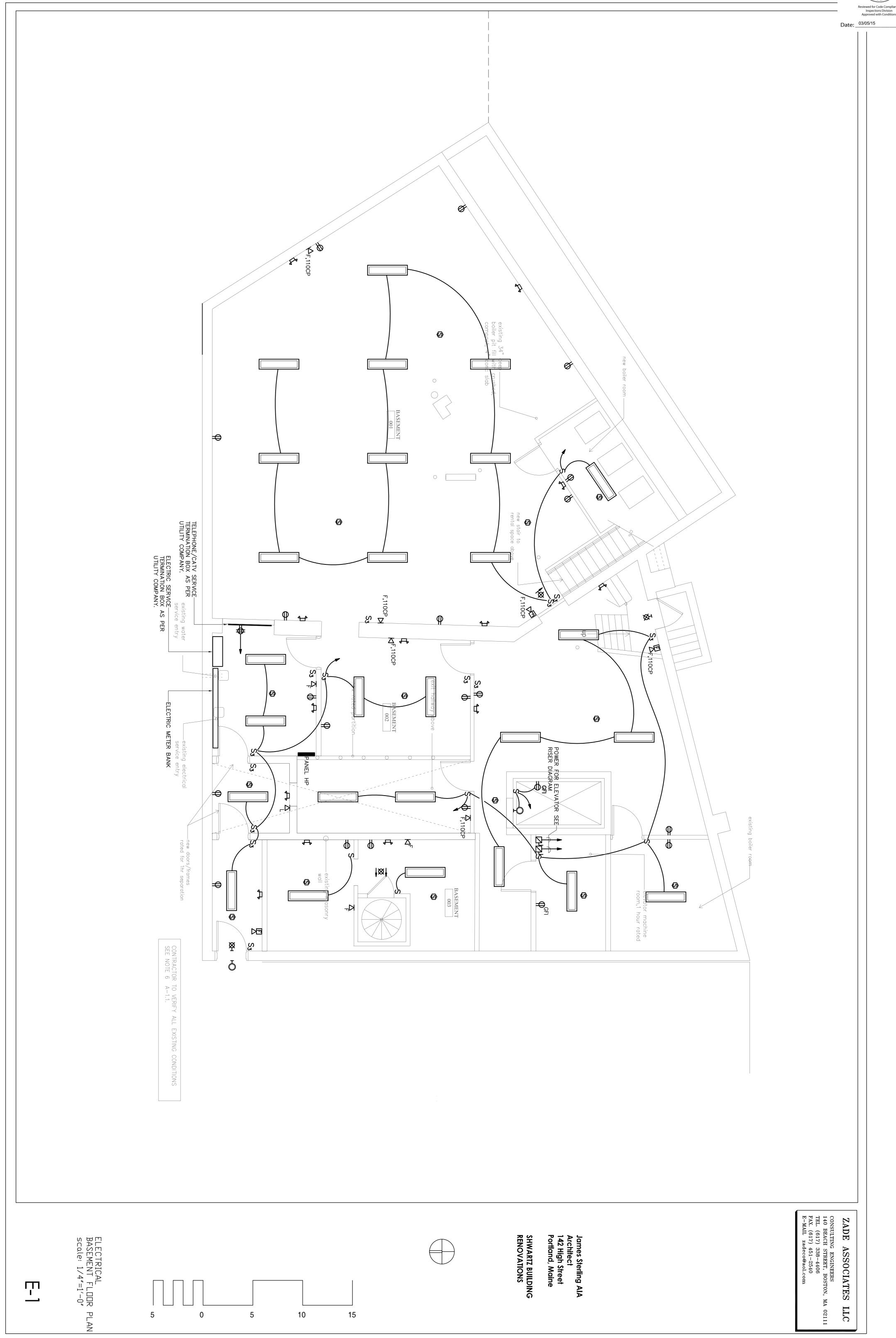
A-6.0



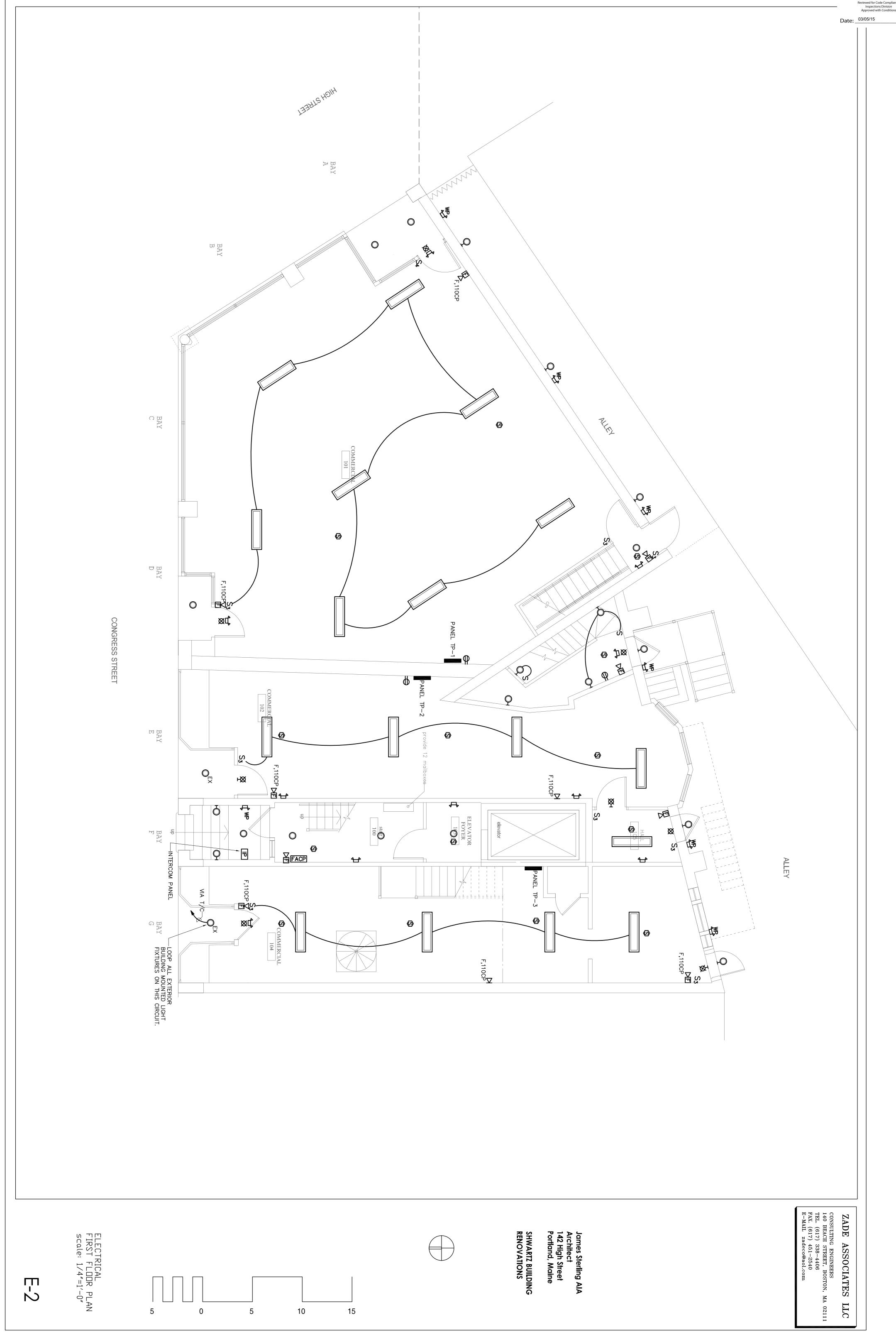


				гU	UKIH LEVEL
	fire	functi			
tvpe 4	rating 1hr	size 3'-0"x6'8"	notes new steel ext door closer		
type 4 type 4	1hr	3-0'x6'8"	new steel ext door closer pt. steel door smoke sealed closer		
type 4	20	3'-0"x6'8"	pt. steel door smoke sealed closer		
type 4 type 4	1hr 1hr	3'-0"x6'8" 3'-0"x6'8"	pt. steel door smoke sealed closer pt. steel door smoke sealed closer		
type 4	1hr	3'-0"x6'8"	pt. steel door smoke sealed closer		
type 4	1hr	3'-0"x6'8"	pt. steel door smoke sealed closer		
type 4	1hr	3'-0"x6'8"	pt. steel doorsmoke sealed closer		
type 3		3'-0'x7'-0' 3'-0'x6'-8''	mtl storefront door sim to 102.1 APT Entry		
type 3	Inr	3-0'x8-0'	pt. wd door smoke sealed closer mti storefront door with venting unit		
		3'-0"x8'-0"	mtl storefront door with venting unit		
		2'-8'x6'-8' 3'-0'x7'-0'	rehabilitate door reflash head/gasket closer rehabilitate door and hardware		
type 3		2'-8'x6'-8"	rehabilitate door and hardware		
		2'-8'x6'-8"	rehabilitate door		
type 3	1hr	2'-8'x6'-8" 3'-0'x6'-8"	match door 101.4/102.1 wd pt grade rated		
type 3	20 min	2'-8'x6'-8"	pt grade door smoke sealed closer		
type 4	1 hr	3'-0"x6'-8"	pt. steel door smoke sealed closer		
type 1	1hr	3'-0"x6'-8"	pt grade door smoke sealed claser	KEY NDB	"NEW DOOR BLANK"; RATED SEE SPECIFICATION.
type 1		2'-8"x6'-8"	new pt grade slab [or rehab]		ALL APARTMENT ENTRIES TO HAVE THIS DOOR T
type 1 type 1		2'-8'x6'-8" 2'-8'x6'-8"	new pt grade slab [or rehab] new pt grade slab [or rehab]	CK	"CLEAN AND KEEP" REPRESENTS EXISTING DOOP BLANKS OF HISTORICAL SIGNIFICANCE. CLEAN
type 1	20 min	2-0 X0-0 3'-0'X6'-8"	new prigrade slab		PREP FOR PAINT AND STRIP HARDWARE, REPOSI
type 1		3'-0"x6'-8"	re use orig door		PREP FOR PAINT AND STRIP HARDWARE, REPOR DOOR ON HALL SIDE OF FRAME. PROVIDE UL RAT ENCLOSURE BEHIND DOOR (ON THE APARTMENT
type 1 type 1	20 min	3'-0"x6'-8" 3'-0"x6'-8"	pt grade door smoke sealed closer new pt grade slab		
	20 min	3'-0"x6'-8"	pt grade door smoke sealed closer		EXISTING DOORS CONTRIBUTE TO THE HISTORIC CHARACTER OF THE BUILDING.
type 3		2'-8"-6'-8"	new pt grade slab		
type 3 type 3		2'-8'-6'-8'' 2'-8'-6'-8''	rehab unrated patch rehab rated patch [13]	BDB	"BAD DOOR BLANK" DOOR BLANK OF NO HISTORICAL SIGNIFICANCE: TYPICALLY A SOLID BIRCH OR MAPLE SLAB INSTALLED OVER PAST 30 YEARS, KEEP OR RE-PLACE CASING APOLIND OPENING AND FILL FRAME OPENING WITH UL
type 5		(2)-2'-0'x6'-8"	new pt grade slab		BIRCH OR MAPLE SLAB INSTALLED OVER PAST
type 3		3'-0"x6'-8"	new pt grade slab		30 YEARS, KEEP OR RE-PLACE CASING AROUND OPENING AND FILL FRAME OPENING WITH UI
type 3 type 3	20 min	(2)-2"-8"x6"-8" 3"-0"x6"-8"	new pt grade slab pt grade door smoke sealed closer		
type 3	20110	3'-0"x6'-8"	new pf grade slab		OF REUSING DOOR OF HISTORIC CHARACTER IN THESE OPENINGS IF MATCHES OTHER DOOR
type 3 type 3		2'-8"-6'-8" 2'-6"x6'-8"	new pt grade slab new pt grade slab vented		SYSTEMS IN HALLWAY.
type 3		2-5'X5-8" 7-5'X5-8"	new pt grade slab vented new pt grade slab	CND	*CASE NEW OPENING* PROVIDE CASING FROM
type 3		2-'8''x6'-8"	rehab rated patch [13]		DEMOLISHED EXISTING DOORS SYSTEMS WHERE FEASIBLE TO MATCH IN APARTMENT LINITS
type 3 type 3		2'-7"-6'-8" 2'-8"-6'-8"	new pt grade slab closer rehab rated patch (13)		WHERE NEW CONNECTOR HALLS ARE ADDED
type 3		2'-8"-6'-8"	new pt grade slab rated patch [13]		BETWEEN UNITS.
type 3	1hr	3'-0'x6'-8"	pt grade door smoke sealed closer	REP	"RATE BEHIND EXISTING PARTITION": SOME PARTITIONS ARE OF HISTORIC CHARACTER, BUT CANNOT BE RATED BECAUSE OF THERE COMPOSE
	1hr	3'-0"x6'-8"	pt grade door smoke sealed closer		LE. GLASS INFILL PANELS. OWNER TO PROVIDE RATED CONDITION BEHIND PANEL
type 1 type 1		2'-8''-6'-8'' 2'-8''-6'-8''	new pt grade slab [or rehab] new pt grade slab [or rehab]		THE CONTRACTOR ALTING FAILL
type 1		2-8-6-8	new pt grade slab (or rehab)		
type 1		2'-8"-6'-8"	new pt grade slab [or rehab]		TERED ARCA
type 1 type 1	20 min	3'-0"x6'-8" 3'-0"x6'-8"	new pt grade slab new pt grade slab		19 7
type 1	20 min	3'-0"x6'-8"	pt grade door smoke sealed closer		O LARMES M
type 1		3'-0"x6'-8"	new pt grade slab		HE AVEBY 12
type 1 type 1	20 min	3'-0"x6'-8" 2'-8"-6'-8"	pt grade door smoke sealed closer		TERLING
type 5		(2)-2'-0'x6'-8"	new pt grade slab new pt grade slab		NO. 1076
type 1 type 3		3'-0"x6'-8" (21-2'-8'x6'-8"	new pt grade slab		1021 4
type 3 type 1	20 min	(2)-2-8'85-8" 3'-0"x6'-8"	new pt grade slab pt grade door smoke sealed closer		TE OF MA
type 1		2'-8"-6'-8"	new pt grade slab		
type 1		2'-8'-6'-8'' 3'-0'x6'-8''	new pt grade slab new pt grade slab		
type 1 type 1		3-0'x6-8' 2'-8''-6'-4''	rehab rated patch [13] btw 303/304		
type 1		2-'8''x6'-8''	rehab rated patch [13] new pt grade slab patch [13]		James Sterling A
type 1 type 1	1hr	2-'7"-6-8" 3'-0"x6'-8"	new pt grade slab patch [13] pt grade door smoke sealed closer		Architect
type i	1110	3-0 x6-6	pi globe door smoke sedied closer		
type 2	1br	3'-0"x6'-8"	pt grade door smoke sealed claser		142 High Street
type 2	1110	2'-8"-6'-8"	new pt grade slab [or rehab]		Portland, Maine
type 2		2'-8"-6'-8"	new pt grade slab [or rehab]		
type 2 type 4		2'-8"-6'-8" 2'-8"-6'-8"	new pt grade slab (or rehab) rehab wall point alarr, tune (1 up gradiert		
type 2		2'-8"-6'-8"	rehab wall paint glass type f1 up against new pt grade slab (or rehab)		
type 2	20 min	3'-0"x6'-8" 3'-0"x6'-8"	new pt grade slab		SHWARTZ BUILDII
type 2 type 2	20 min	3-0'x6'-8"	new pt grade slab pt grade door smoke sealed claser		RENOVATIONS
type 2		3'-0"x6'-8"	new pt grade slab		RENOVATIONS
type 1	20 min	3'-0"x6'-8" 3'-0"x6'-8"	pt grade door smoke sealed closer new pt grade slab		
type 1		3'-0"x6'-8"	new pt grade slab		
type 1 type 1		2'-8"-6'-8"	new pt grade slab new pt grade slab		DOOR SCHEDUL
type 1	20 min	(2) 2'-8x6'-8" 3'-0"x6'-8"	pt grade door smoke sealed closer		
type 1		3'-0"x6'-8" 2'-8"x6'-8"	new pt grade slab new pt grade slab		
type 1 type 1		2'-8"x6'-6"	new pt grade slab rehab rated patch [13] btw 303/304		
type 1		2-'7''x6'-8"	new pt grade slab closer	/	∧ A-6.1
type 1 type 1		2'-8''x6'-6" 2'-8''x6'-8"	new pt grade patch [13] new pt grade patch [13]	(
iype I		A ~~ XO -O	new bir Brong barati [19]		// →
GENE	RAL	NOTES:			u
1.See	e spe	cifications	for door hardware requirements.		



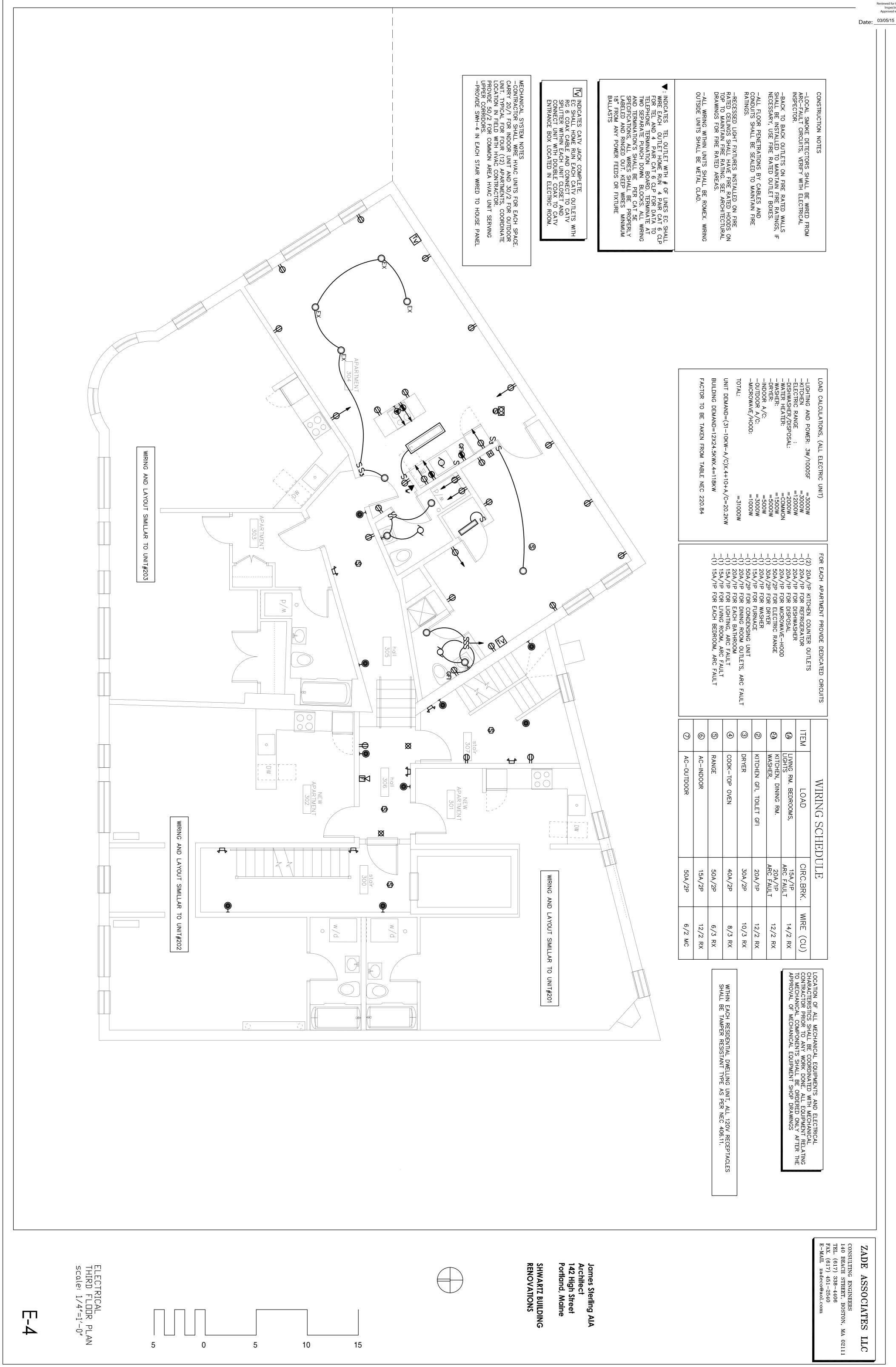




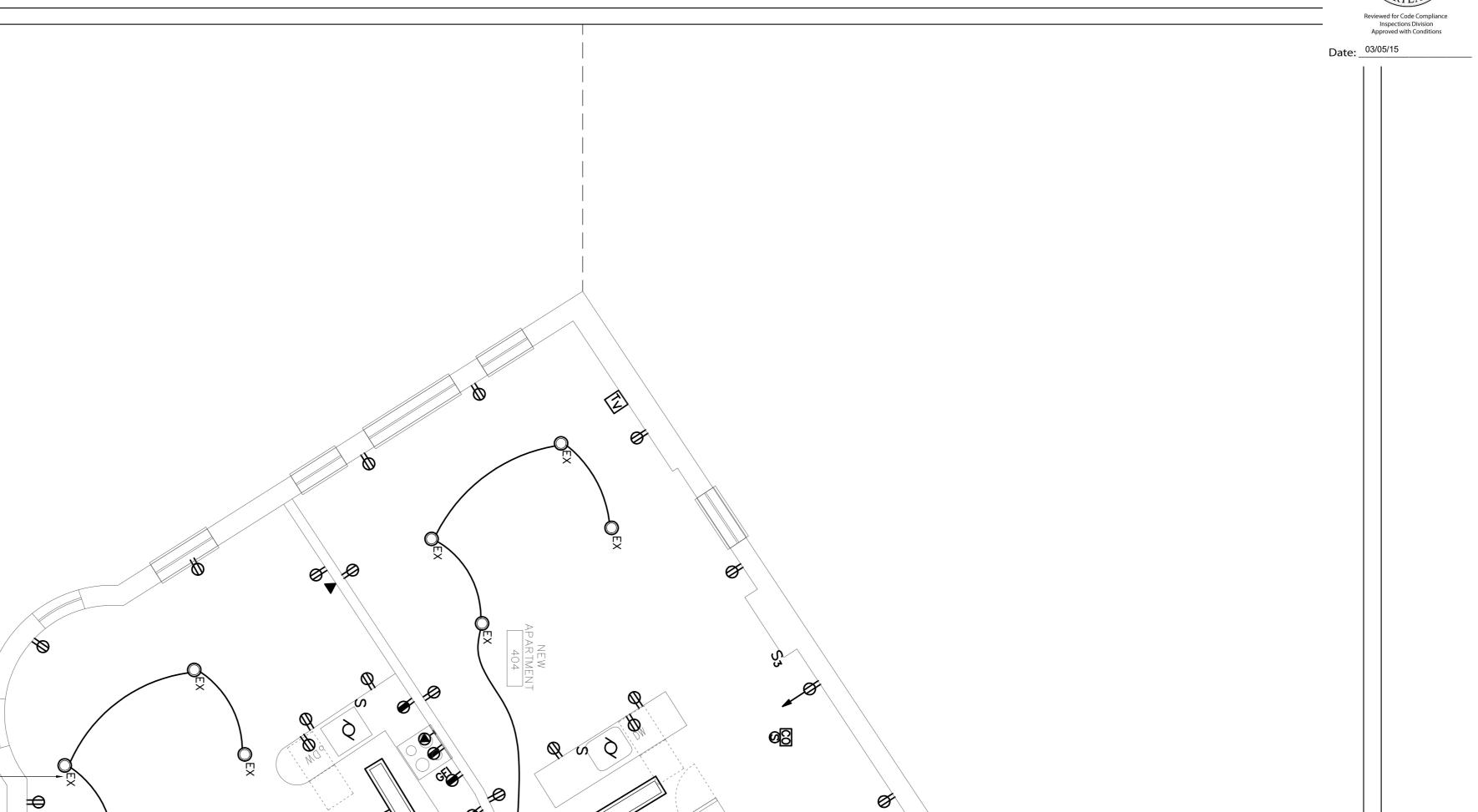


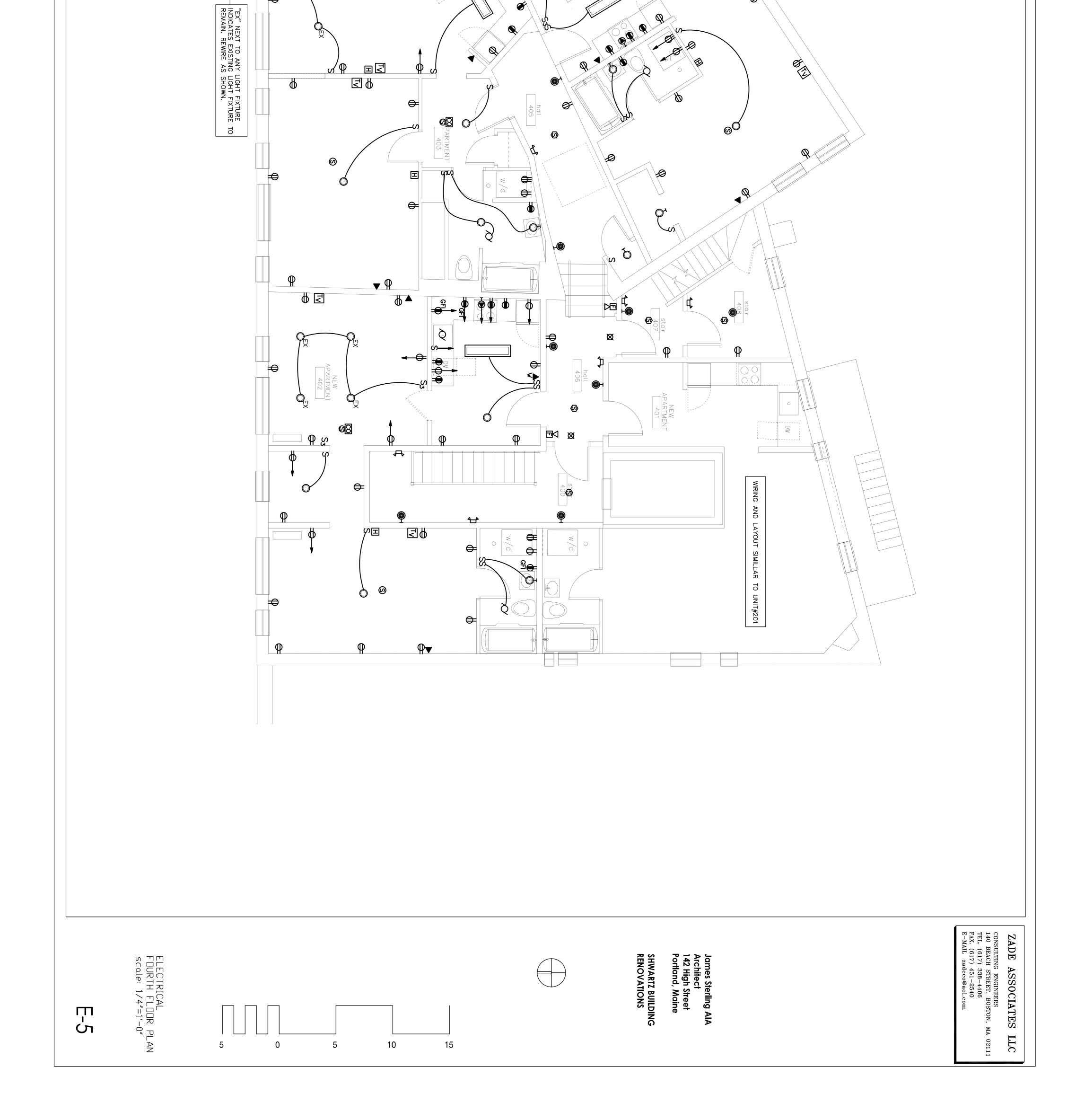




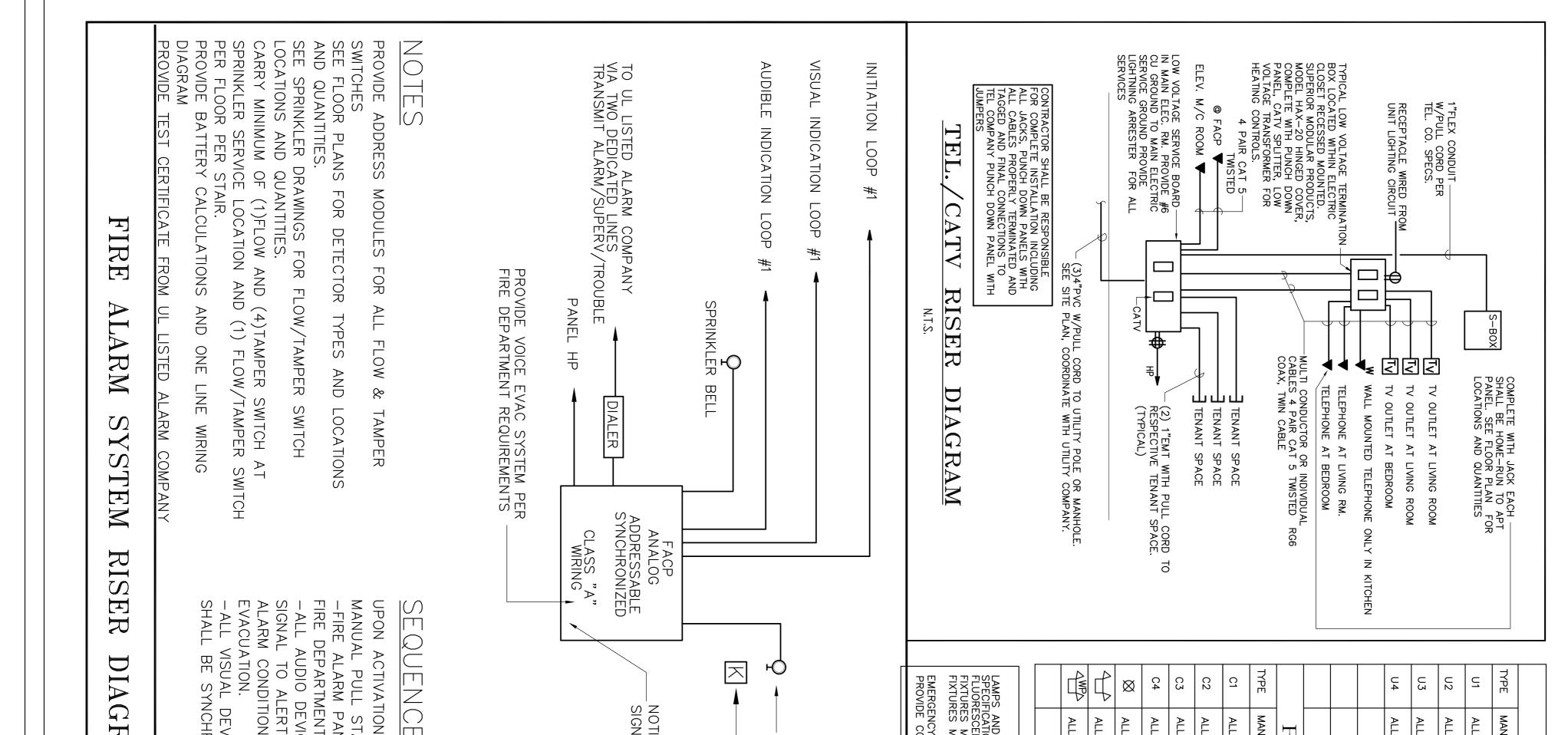




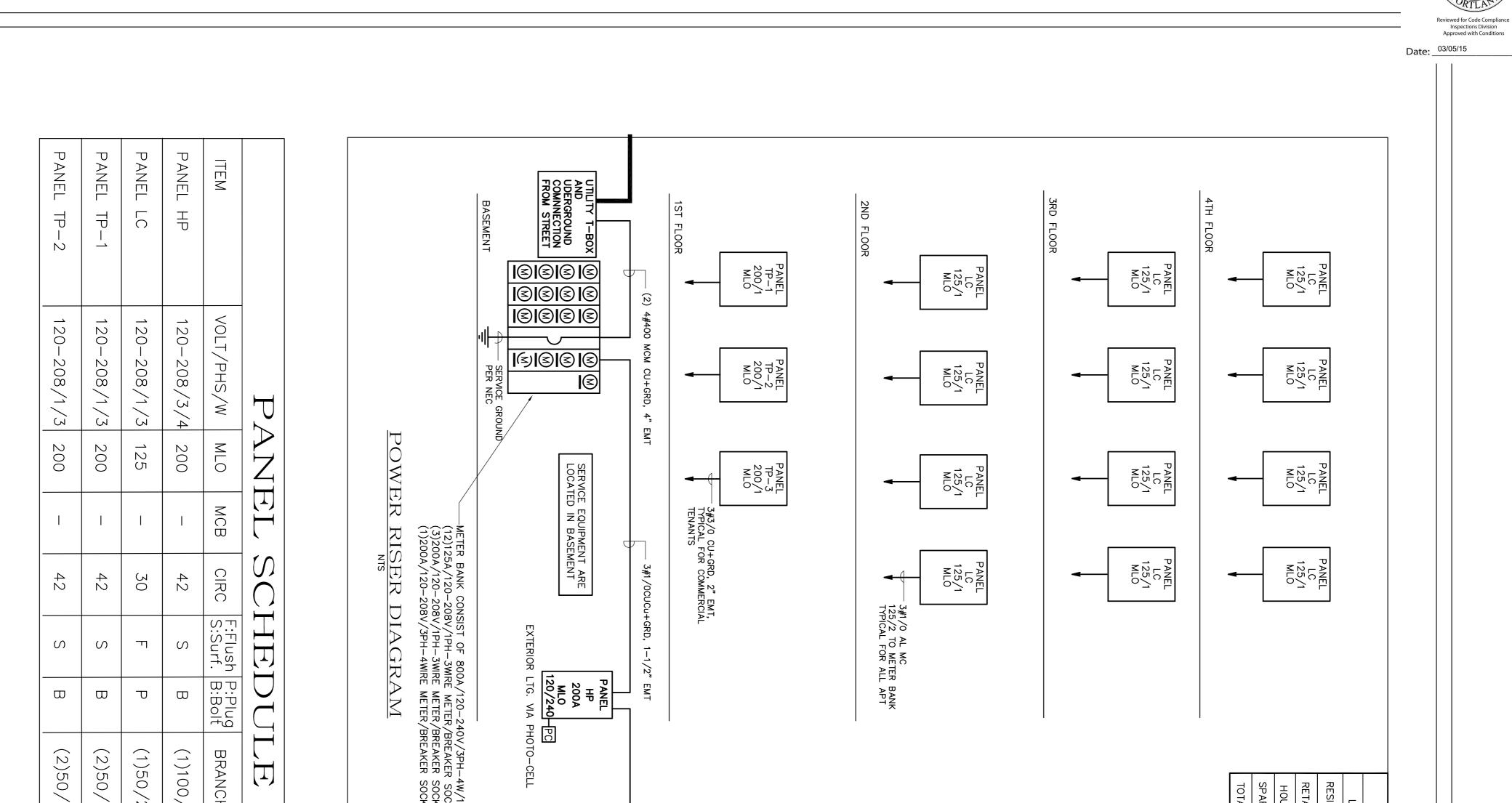








E-6			
scale: NTS	TELEPHONE ENTRY RISER DIAGRAM		RAM
ELECTRICAL		THEY	EVICES WILL ACTIVATE, HRONIZED TYPE.
5	TELEPHONE DIALER APARTMENT ENTRY PANEL HOUSING DEVICES INC, MODEL 1020-NSL MATCHING APARTMENT NUMBERS	FOR FOR FOR FULL	VICES WILL INITIATE AL, RT ALL OCCUPANTS FOF N IN THE BUILDING FO
	FOR EACH UNIT	TION CH CH ALL	NOF ANY FIRE DETECTOR, TATION OR FLOW SWITCH ANEL WILL TRIP AND CALL
5	SYSTEM SHALL BE COMPLETE WITH ALL NECESSARY RELAYS/TRANSFORMERS/OTHER COMPONENTS AND WIRED COMPLETE TO UNIT TELEPHONE LINES AS REQUIRED. THERE WILL BE NO SUBSCRIBER LINE IN THIS SYSTEM		
10	UNIVERSAL MOUNTING EXIT SIGN (DOUBLE FACED), WITHOUT ARROWS . EMERGENCY BATTERY UNIT WITH MOUNTING BRACKET AND VOLTMETER.		
15	TC TIME SWITCH PC PHOTO-CELL X UNIVERSAL MOUNTING EXIT SIGN (DOUBLE FACED), ARROWS AS INDICATED.	LL BE CLASS "A", STYLE "Z" L BE CLASS "A", STYLE "6".	SNAL LINE CIRCUIT SHALL
	TAN FLO		
	FACP FIRE ALARM CONTROL PANEL ANN FIRE ALARM ANNUNCIATOR		- RUTARY BEACON
\bigcirc	– HOMERUN JUNCTION		
	 AUTOMATIC DOOR OPERATOR DH MAGNETIC DOOR HOLDER 		
	E SWITCH) EIRE ALARM	SHALL HAVE INTEGRAL TEST SWITCHES BUILT INTO FIXTURE (NOT SEPARATE FEED TO BUILT-IN EMERGENCY BATTERY AS REQUIRED	CY LIGHTS SHALL HAVE INTEGRAL CONSTANT FEED TO BUILT-IN EM
RENOVATIONS	AWAY HOODS TO MAINTAIN RATINGS	ATIONS ABOVE FOR THE FIXTURE TYPE ONLY CENT FIXTURES SHALL HAVE ELECTRONIC BALLASTS THD LESS THAN %15 MOUNTED IN INSULATED CEILINGS, EC SHALL PROVIDE HOODS TO KEEP INSULATION , MOUNTED IN RATED CEILINGS (SEE ARCHITECTURAL DRAWINGS). EC SHALL PROVIDE F	TIONS ABOVE FOR THE FIXTURE DENT FIXTURES SHALL HAVE ELEC MOUNTED IN INSULATED CEILING: MOUNTED IN RATED CEILINGS (S
SHWARTZ BUILDING	AMS,	PLIANCE WITH LOCAL UTILITY COMPANY REBATE PRO	ND BALLASTS SHALL BE IN COMP
142 High Street Portland, Maine	$\sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{i$		
James Sterling AIA Architect	WP EMERGENCY LIGHT FOR EXTERIOR F FIRE ALARM PULL STATION (- REMO	
	D EXIT SIGN	120 – SELF POWERE	LLOWANCE \$100
	RY LIGHTS	1	LLOWANCE \$150
	SCONCES	Ι	
	TELEPHONE JACK COMPLETE W/JACK AND COVER, ('W' WAL	120 - COMMON ARE 120 - STAIRS	LLOWANCE \$150
		VOLT LAMP DESCRIPT	
	AREAS DUPLEX RECEPTACLE, 120V,18" AFF.,	HEDULE- COM	URE SC
	Φ ^{GFI} DUPLEX RECEPTACLE WITH GROU		
	St tour-way light switches St Time delay switch for unit bathroom ceiling fan D DUPLEX RECEPTACLE, 120V,18" AFF.		
	THREE	120 – LAUNDRY	LLOWANCE \$150
	ITS	– VANITY	
	CESSED LIGHTS	1	LLOWANCE \$150
E-MAIL zadeco@aol.com	/HALL/BEDROOM RECESSED		LLOWANCE \$150
140 BEACH STREET, BOSTON, MA 02111 TEL. (617) 338-4406 FAX. (617) 451-2540	AENTS ,	SCHEDULE-APA	TURE
ADDULIAI LD NG ENGINEERS			
ZADE ACCOCIATES IIC			



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CH BREAKERS 0/3, (4)30/2, (24)2 /2, (2)30/2, (6)20,	3#1/0 CU+GRD,2 1, 3#1/0 CU+GRD,2 1, 100A/3 FUSED W /AUX CONTRO W /AUX CONTRO W /AUX CONTRO W /AUX CONTRO NITERLO CONTRO NITERLO CONTRO OCKETS FOR APARTMENTS OCKETS FOR RETAILS OCKET FOR COMMON AREAS		LOAD BREAKDOWN LOAD KVA FACTOR ESIDENTIAL 372 0.41 ETAIL 48 1.00 PARE 50 1.00 OTAL 500 1.00
D/1, (8)20/	GRD,2 1/2" EMT 100A/3ø-SN/HD/ FUSED DISC. SWITCH W /AUXILIARY CONTACTS INTERLOCKED WITH CONTROL POWER EVATOR CONTROLLER EVATOR CONTROLLER REAS		R KVA 152 280 280
=SPARE =SPACE =SPACE	AL CONTROL DEVERS SHALL BE MIN 18" AWAY FROM CORNER REMOTE EXERGENCY LIGHTS AND EMERGENCY BATTERIES TO BE MOUNTED 6-8" AFF TO THE BOTTOM.	SPACES, INFORM ARCHIECT/RVIGNEER ABOUT ANT INFRINGEMENTS FRICK SUCH INSTALLATIONS OCCUR. -ELECTRIC ROOM DIMENSIONS, CONTRACTOR SHALL CONFIRM ROOM DIMENSIONS EQUIPMENT DIMENSIONS, CONTRACTOR SHALL CONFIRM ROOM DIMENSIONS PRIOR TO ORDERING EQUIPMENT 	 GENERAL POWER DISTRIBUTION NOTES BEFORE ORDERING ANY EQUIPMENT AND/OR START ANY CONSTRUCTION OR EXCAVATION ELECTRICAL CONTRACTOR AND/OR GENERAL CONTRACTOR SHALL CONTACT ELECTRIC/TELEPHONE/CATV COMPANIES FOR SERVICE POINT AND ROUTING, SERVICE AVAILABILITY. THIS ENGINEER HAS MADE APPLICATION TO UTILITY COMPANY BUT YET NO CONFRMATION HAS BEEN MADE BY THE UTILITY COMPANY. CONTRACTOR SHALL NOT PROCEED WITHOUT SUCH CONFIRMATION. -FEEDERS ARE SIZED BASED ON %3 VD. CONTRACTOR SHALL FOLLOW THE FOLLOWING CRITERIA. 50A, UP TO 100FT #6, INCREASE BY ONE SIZE FOR EVERY 30FT. 100A, UP TO 100FT #5, INCREASE BY ONE SIZE FOR EVERY 50 FT. 200A, UP TO 100FT #5, INCREASE BY ONE SIZE FOR EVERY 50 FT. 400A, UP TO 100FT #5, INCREASE BY ONE SIZE FOR EVERY 50 FT. 400A, UP TO 100FT #5, INCREASE BY ONE SIZE FOR EVERY 50 FT. 200A, UP TO 100FT #5, INCREASE BY ONE SIZE FOR EVERY 50 FT. 200A, UP TO 100FT #5, INCREASE BY ONE SIZE FOR EVERY 50 FT. 400A, UP TO 200FT #500, INCREASE BY ONE SIZE FOR EVERY 50 FT. -FOR SWITCHGEAR SHORT CIRCUIT RATINGS, SEE NOTES UNDER PANEL SCHEDULES. -PRIOR TO ORDERING ANY SWITCHGEAR ELECTRICAL CONTRACTOR SHALL CONTRACTORS, ENGINEERS APPROVAL IS GIVEN FOR QUALITY ONLY. -CONTRACTOR SHALL PASS THROUGH ELECTRIC ROOM OR ABOVE DEDICATES OTHER TRADE SHALL PASS THROUGH ELECTRIC ROOM OR ABOVE DEDICATES
		 D. All feeder conductors D. All feeder conductors A. The Contractor shall s to insure proper instal B. The Contract Drawings locations and arranger locations and darranger locations and detail d with contractor shall velevations and detail d with contract. Check, s transmitting to Architec E. This Contract. Check, s' transmitting to Architec E. This Contractor shall or fees and back charges that have jurisdiction. F. Material and equipmen Guarantee work in write or replace defective m damage caused in ma at no cost to the Owr H. Submit guarantee to <i>J</i> 	ELECTRICAL SPECIFICATIONS 1.1 General A. The General Conditio as part of the Elect B. The term "This Cont 1.2 Scope of Work A. The work under this material as specified a complete and reas shown for reference cription of the desig by other manufactur 1.3 Codes and Specifications A. The work shall be of of the State of MAS OSHA codes, Nationd B. All exposed wiring sin accordance with 1 C. All branch circuit co

ELECTRICAL ELECTRICAL	cections and transperent of the piping and wiring of equipment. The exact fuer-funces and defail trainings. Exact location of all trainings and becaments of the overal's representative prior to rough-m. Namit Stop Drawings. Exact location of all there as shall be confirmed a central. Check, stamp and marks with project name submittals before rarantiting to Antherit. Indicate devicins from Contract Documents. This Contractor shall give notices, file pins, obtain permits and leaders, and back charges, and obtain the reseasory approval from outprofile parameter work in which for one sub-risk parameters and a standard state and back charges. and obtain the reseasory repairs and replacements under guarantee to a contract bottom of the anticest of final accurate. The cost to the Ower.
140 BEACH STREET, BOSTON, MA 02111 TEL. (617) 338-4406 FAX. (617) 451-2540 E-MAIL zadeco@aol.com	<u>I SPECIFICATIONS</u> <u>Ird</u> <u>Tre</u> <u>The General Conditions and Drawings issued for this Project shall be considered specifications. The term "This Contractor" as used under this Section and wherever used on the Drawings shall mean the Electrical Contractor. The work under this Specification includes the furnishing of all labor and material as specified breain and as shown on the Drawings necessary to install a complete and ready for operation. Manufacturer's catalogue numbers are shown for reference purposes only. They are meent to provide a general description of the Electrical Codes and the local codes as most recently issued. Sy other manufacturers will be considered. Is and Specifications is and Specificational Electrical Codes and the local codes as most recently issued. State of MASSACHUSETTS and the local codes as most recently issued. SPAA codes, National Electrical Codes and NFPA. We have shall be conducted in accordance with the latest rules and regulations of the State of MASSACHUSETTS and the local codes as most recently issued. SPAA codes, National Electrical Codes and NFPA. We have the function shall be in electric metallic tubing. All concealed wiring shall be n accordance with local codes. We funct the design and operation. The second code is a noted XHHW insulation, 600V. It feeder conductors shall be copper, AWG size as noted XHHW insulation, 600V. In econtractor brawings are diagrammatic only and indicate the extent, general coations and arrangement of the piping and wiring of equipment. The exact coations and arrangement of the piping and wiring of equipments of the second and wiring of equipments of the extent general coations and arrangement of the piping and wiring of equipment. The exact coations and be conducted with Architectural Drawings and Documents of the piping and wiring of equipment. The exact coations and prevention of the piping and wiring of equipment. The exact coations and Documents of the piping and provings and Documents of the piping and provings and Documents of the piping </u>

Page



Jeanie Bourke - 600 Congress St. BP#2014-01674 Facade & Structural Modifications

From:	Jeanie Bourke
To:	aaron@structuralinteg.com; jleasure@ll-eng.com
Date:	10/22/2014 4:29 PM
Subject:	600 Congress St. BP#2014-01674 Facade & Structural Modifications
CC:	dan leo; james sterling

Hi Joe and Aaron,

Just following up, Joe, on our discussion last month about this project and oversight of the structural modifications in the tower per your plans. After further research, I located a Statement of Special Inspections submitted by Aaron, the original engineer of record. That scope of work was for the new elevator shaft and tower improvements on a previous permit.

Today I spoke with Matt at Structural Integrity to confirm oversight of their portion of the design on this project and they are aware that your firm, L& L Structural, has also provided structural design in the area of the tower.

I am confident that inspections and observations respective to each of the areas in your responsible charge will be performed, upon reasonable notification to inspect, by the owner, applicant or general contractor of this project. Can someone please forward this to Waldo Trott of Dirty Deeds?

Joe, for the above referenced permit to be issued, can you please submit to me a statement of observation and oversight per the designed plans under your charge.

I appreciate the oversight from both of you on this project, for your expertise and involvement during the lengthy renovations on this building. Per this and previous permit approvals, there are conditions that specify structural oversight, and it is expected you will be contacted appropriately for this follow up. This is in addition to the required inspections from the Building Inspections Department.

Thank you and let me know if you have any questions, Jeanie

Jeanie Bourke CEO/LPI/Plan Reviewer

City of Portland Planning & Urban Development Dept./ Inspections Division 389 Congress St. Rm 315 Portland, ME 04101 jmb@portlandmaine.gov Direct: (207) 874-8715 Office: (207) 874-8703 Permit status can be viewed at: http://www.portlandmaine.gov/792/Permit-Status

GENERAL NOTES:

- . The notes on the drawings are not intended to replace specifications. in addition to general notes. See specifications for requirements
- Structural drawings shall be used in conjunction with job specifications and architectural, mechanical,
- Structural drawings shall be used in conjunction with job specifications and architectural, mechanical, electrical, plumbing, and site drawings. Consult, openings, chases, inserts, reglets, sleeves, depressions, and other details not shown on structural drawings.
 All dimensions and conditions must be verified in the field. Any discrepancies shall be brought to the attention of the engineer before proceeding with the affected part of the work.
 Do not scale plans.
 Sections and details shown on any structural drawings shall be considered typical for similar conditions.

- 6. All propietary products shall be installed in accordance with the manufacturers written instructions.
- The structure is designed to be self supporting and stable after the erection is complete. It is the
- contractor's sole responsibility to determine erection procedures and sequencing to ensure the sofety of the building and its components during erection. This includes the addition of necessary shoring, sheeting temporary bracing, guys or tiedowns. Such material shall remain the property of the contractor after completion of the project 8. All applicable federal, state, and municipal regulations shall be followed, including the federal department of
- occupational safety and health act DESIGN LOADS:
- . Building code: IBC (2009) International Building Code.
- Design Live Loads: (Ground Snow load = 50 psf) Roof 45 psf (+ drift as applicable)
- Residential floor 40 psf Corridors, stairways, exits 100 psf
- 3. Design wind loads are based on exposure C using 100 mph basic wind speed. 4. Seismic Design per IBC 2009

STRUCTURAL STEEL NOTES:

- Structural steel fabrication, erection, and connection design shall conform to AISC "Specification for the design, fabrication, and erection of structural steel"-Ninth edition.
- 2 Structural steel
- a) Structural steel shall conform to ASTM A-36.
- b) Structural tubing shall conform to ASTM A-500 GR-B c) Structural pipe shall conform to ASTM A-53, TYPE E OR S
- The fabricator shall design connections for the reactions shown on the drawings or the maximum end reaction that can be produced by a laterally supported uniformly loaded beam for each given beam size and span.
- 4. Field connections shall be bolted using 3/4" diameter ASTM A325 high strength bolts except where field welding is indicated on the drawings.
- 5. All welding shall conform to AWS D1.1-Latest edition. Welding electrodes shall be E70XX.
- Structural Steel Primer Point. TNEMEC 10-99 Alkyd rust inhibitive primer, 2.0 to 3.5 mils dry thickness, or approved alternate. substantial stear name reasonable and approved alternate. Structural Steel Top Coat for steel permanently exposed to view. TNEMEC series 2 TNEMEC-GLOSS Enamel, 3.0 to 5.0 mils dry thickness, or approved alternate.
- 8. Complete shop drawings and schedules of all structural steel shall be prepared by the contractor and submitted to the engineer for review prior to commencement of that portion of the work. All accessories must be shown on the shop drawings. Submit (2) black line prints to the Engineer/Architect.

TIMBER FRAMING:

- 1. All Timber framing shall be in accordance with the AITC timber construction manual or the national design specification (NDS) - latest edition
- specification (NDS) latest edition
 2. Individual timber framing members shall be visually graded, minimum grade #2 Spruce-Pine-Fir (SPF), kiln dried to 19% maximum moisture content.
 3. Timber shall be southern yellow pine treated with ACQ water borne preservative in accordance with AWPA treatment C1 with 0.40 PCF retainage for items in contact with roofing, masonry or concrete with 0.60 PCF retainage for items in contact with earth.
- Metal connectors shall be used at all timber to timber connections or as noted on the design drawings. All metal connectors in contact with pressure treated timber shall be stainless steel.
 Provide Simpson H2.5A hurricane anchors where timber framing and/or trusses bear on bearing wall and the total based.
- Fronde Simpson Fiz.JA humanicale anchols where tumber framing and/of tasses bed of bearing wan and structural beams.
 Nails and screws not specified shall conform with IBC 2009. All nails and screws in contact with pressure treated timber shall be stainless steel.
 Provide ½" thick APA rated exterior wall sheathing fastened w/ 10d nails @ 4" o.c. at panel edges and 6" o.c. intermediate. Lap sheathing 1'-0" minimum over existing structure (Where applicable).
 Provide ½" thick APA rated roof sheathing fastened w/ 10d nails @ 6" o.c. at panel edges and
- 9. Provide ¾" thick APA rated floor sheathing fastened w/ construction adhesive and 10d ring shank nails @ 6" o.c. at panel edges and intermediate
- LVL indicated laminated veneer lumber beams and posts shall be manufactured by Boise Cascade or approved equal.

EXISTING 2×10 @ 16 (JOISTS (V.I.F.) (**) FASTEN BEAMS TOGETHER W/(4)}"@ THRU BOLTS MIN. VTYP.) * FXISTING 6"x9 TIMBER BEAM PED) (V.I.F. EXISTING HANGERS, (V.I.F.) A X8" TIMBS POST E (E) JAN LVI EXISTING 12" THICK (3 WYTHE) 3.22 X(E) W14 STEEL BEAM EXTERIOR BRICK MASONRY BEAM W/AM WALL (V.I.F.), REPAIR LOOSE ELEV. 13'-0" AFF (V.I.F.) BRICK AND RE-MORTAR W 34X117 LVI JOISTS AS REQUIRED (TYP.) BCAN BOH A 58¢ EXISTING 2x4 @ 16" O.C. INTERIOR -BEARING WALL (V.I.F.). ADD 2x4 FULL HEIGHT AT EACH STUD (TYP.) (ie: 2-2x4 @ 16" O.C. TYP.) 25 A.S.A.K. VERIFY (E) OR INSTALL 2-2x10 BLOCKING EXISTING 2"x9" BENEATH WALL ABOVE W/(2) SIMPSON A35 FRAMING ANGLES AT BOTH ENDS OF EACH PIECE (NOT SHOWN FOR CLARITY) (V.I.F.) (TYP.) ROOF JAN N JOISTS (V.),F.) 5 6x6 p. TEMP. SUPPORT EXISTING ROOF AND 3 52 TOWER STRUCTURE ABOVE AS REQ'D TO PLUMB CUT EXISTING ROOF JOISTS TO WITHIN %" FROM FACE OF NEW LVL BEAM AND FASTEN JOISTS TO LVL /(E) W8 ELEV. A. X BEAM W/SIMPSON U210R HANGERS AFF (V.I.F. INSTALL 51/2×91/2 LVL BEAM FLUSH FRAMED IN ROOF SYSTEM AT CENTER OF TOWER WALL AS SHOWN (TYP.) ે COPEN FOR SERVICE 1 52 EXISTING STEEL BEAM TO REMAIN (TYP.) HSS4×4×1 TUBE 3A S2 ¥√S2) (SEE SECTION COPE BOTTOM AND TOP FLANGE ON ONE SIDE OF W6 BEAMS AS REQ'D TO FIT. (SEE SECTION 1/S2 FOR CONN. OF W6 TO 4x4 TUBE (TYP.) بن في ا XXX ÂĴ INSTALL HSS4x4x4 TUBE STEEL BEAMS. SUPPORT ON EXISTING MC12 AT ONE END AND EXISTING S4 AT OPPOSITE END. FASTEN W/¼" FILLET WELD 2" MIN. LONG ON BOTH SIDES (TYP. AT BOTH ENDS). SEE SECTION 1/S2 FOR CONN. TO EXISTING W4 (TYP.) ی بنج بنج ^{بد}ی EXISTING S9 (DROPPED) STEEL BEAM (V.I.F.) T/STEEL ELEV. = 13'-0" AFF (V.I.F.) (= BOTTOM OF EXISTING ROOF JOISTS TYP. V.I.F.) EX MC12x35 (E) MC12x35 BOT STEEL ELEV. = 10'-9³" AFF (TO MATCH BOT/EXIST. ROTATED S4 STEEL BEAM) EXISTING S4x9.5 (V.I.F.) (BOT STEEL $ELEV. = 11' - 0\frac{1}{2}$ AFF) (V.I.F.) *** COPE BOTTOM FLANGE OF W6 ON BOTH SIDES OF BEAM TO FIT. SEE SECTION 1/S2 FOR CONN. (TYP.) EXISTING S4x9.5 (ROTATED W/FLANGES -VERTICAL) (V.I.F.) (BOT STEEL ELEV. = 10'-9²/₄" AFF) (V.I.F.) *** \overline{A} \ S2

(E) W6x20 BOT STEEL ELEV. =

10'-9³" AFF (TO MATCH BOT/MC12)

COPE BOTTOM FLANGE OF W6 ON BOTH SIDES OF

BEAM TO FIT. SEE SECTION 1/S2 FOR CONN. (TYP.)

EXISTING 12" THICK (3 WYTHE) EXTERIOR BRICK -

MASONRY WALL (V.I.F.). REPAIR LOOSE BRICK AND RE-MORTAR JOISTS AS REQUIRED (TYP.)

NOTES: 1. "*" INDICATES: SIMPSON TIE DOWNS (SEE SECTION 2/S2) (TYP. 4 PLACES) 2. "**" INDICATES: SEE STRUCTURAL DRAWING PREPARED BY STRUCTURAL INTEGRITY CONSULTING ENGINEERS, INC. FOR ADDITIONAL INFORMATION REGARDING THESE STRUCTURAL COMPONENTS. 2. "**" INDICATES: SEE STRUCTURAL DRAWING PREPARED BY STRUCTURAL INTEGRITY CONSULTING ENGINEERS, INC. FOR ADDITIONAL INFORMATION REGARDING THESE STRUCTURAL COMPONENTS. 2. "**" INDICATES: SEE STRUCTURAL DRAWING PREPARED BY STRUCTURAL INTEGRITY CONSULTING ENGINEERS, INC. FOR ADDITIONAL INFORMATION REGARDING THESE STRUCTURAL COMPONENTS. 2. "**" INDICATES: SEE STRUCTURAL DRAWING PREPARED BY STRUCTURAL INTEGRITY CONSULTING ENGINEERS, INC. FOR ADDITIONAL INFORMATION REGARDING THESE STRUCTURAL COMPONENTS. 2. "**" INDICATES: SEE STRUCTURAL DRAWING PREPARED BY STRUCTURAL INTEGRITY CONSULTING ENGINEERS, INC. FOR ADDITIONAL INFORMATION REGARDING THESE STRUCTURAL COMPONENTS. 3. "INDICATES: SEE STRUCTURAL DRAWING PREPARED BY STRUCTURAL INTEGRITY CONSULTING ENGINEERS, INC. FOR ADDITIONAL INFORMATION REGARDING THESE STRUCTURAL COMPONENTS. 3. "INDICATES: SEE STRUCTURAL DRAWING PREPARED BY STRUCTURAL INTEGRITY CONSULTING ENGINEERS, INC. FOR ADDITIONAL INFORMATION REGARDING THESE STRUCTURAL COMPONENTS. 3. "INDICATES: SEE STRUCTURAL DRAWING PREPARED BY STRUCTURAL COMPONENTS. 3. "INDICATES: SEE STRUCTURAL DRAWING PREPARED BY STRUCTURAL COMPONENTS. 3. "INDICATES: SEE STRUCTURAL DRAWING PREPARED BY STRUCTURAL COMPONENTS. 3. "INDICATES: SEE STRUCTURAL DRAWING PREPARED BY STRUCTURAL COMPONENTS. 3. "INDICATES: SEE STRUCTURAL DRAWING PREPARED BY STRUCTURAL COMPONENTS. 3. "INDICATES: STRUCTURAL DRAWING PREPARED BY STRUCTURAL COMPONENTS. 3. "INDICATES: SEE STRUCTURAL DRAWING PREPARED BY STRUCTURAL COMPONENTS. 3. "INDICATES: SEE STRUCTURAL DRAWING PREPARED BY STRUCTURAL PREPARED BY STRUCTURAL COMPONENTS. 3. "INDICATES: SEE STRUCTURAL DRAWING PREPARED BY STRUCTURAL PREP 2. **** INDICATES: CLEAN EXISTING 3. ***** INDICATES: CLEAN EXISTING 0. To 3.5 MILS DRY THICKNESS, OR APPROVED ALTERNATE AND TOP COAT TIME BRUSH FREE OF RUST, DEBRIS, OIL, AND GREASE TO BARE STEEL. COAT W/PRIMER PAINT TINEME TO-99 ALKYD RUST INHIBITIVE PRIMER, 2.0 TO 3.5 MILS DRY THICKNESS, OR APPROVED ALTERNATE AND TOP COAT TIMEME SRUSH FREE OF RUST, DEBRIS, OIL, AND GREASE TO BARE STEEL. COAT W/PRIMER PAINT TINEME TO-99 ALKYD RUST INHIBITIVE PRIMER, 2.0 TO 3.5 MILS DRY THICKNESS, OR APPROVED ALTERNATE AND TOP COAT TIMEME SRUSH FREE OF RUST, DEBRIS, OIL, AND GREASE TO BARE STEEL. COAT W/PRIMER PAINT TINEME TO-99 ALKYD RUST INHIBITIVE PRIMER, 2.0 TO 3.5 MILS DRY THICKNESS, OR APPROVED ALTERNATE AND TOP COAT TIMEME SRUSH SRUE OF RUST, DEBRIS, OIL, AND GREASE TO BARE STEEL. COAT W/PRIMER PAINT TINEME TO-99 ALKYD RUST INHIBITIVE PRIMER, 4. 'E'' INDICATES: EXISTING

1/2" = 1'-0"

3'-0" (V.I.F.)

EXISTING S4x9.5 (V.I.F.) (BOT

(E) MC12x35 BOT STEEL ELEV. =

ROTATED S4 STEEL BEAM)

PARTIAL ROOF FRAMING PLAN

10'-97" AFF (TO MATCH BOT/EXIST.

STEEL ELEV. = 11'-02" AFF) ***

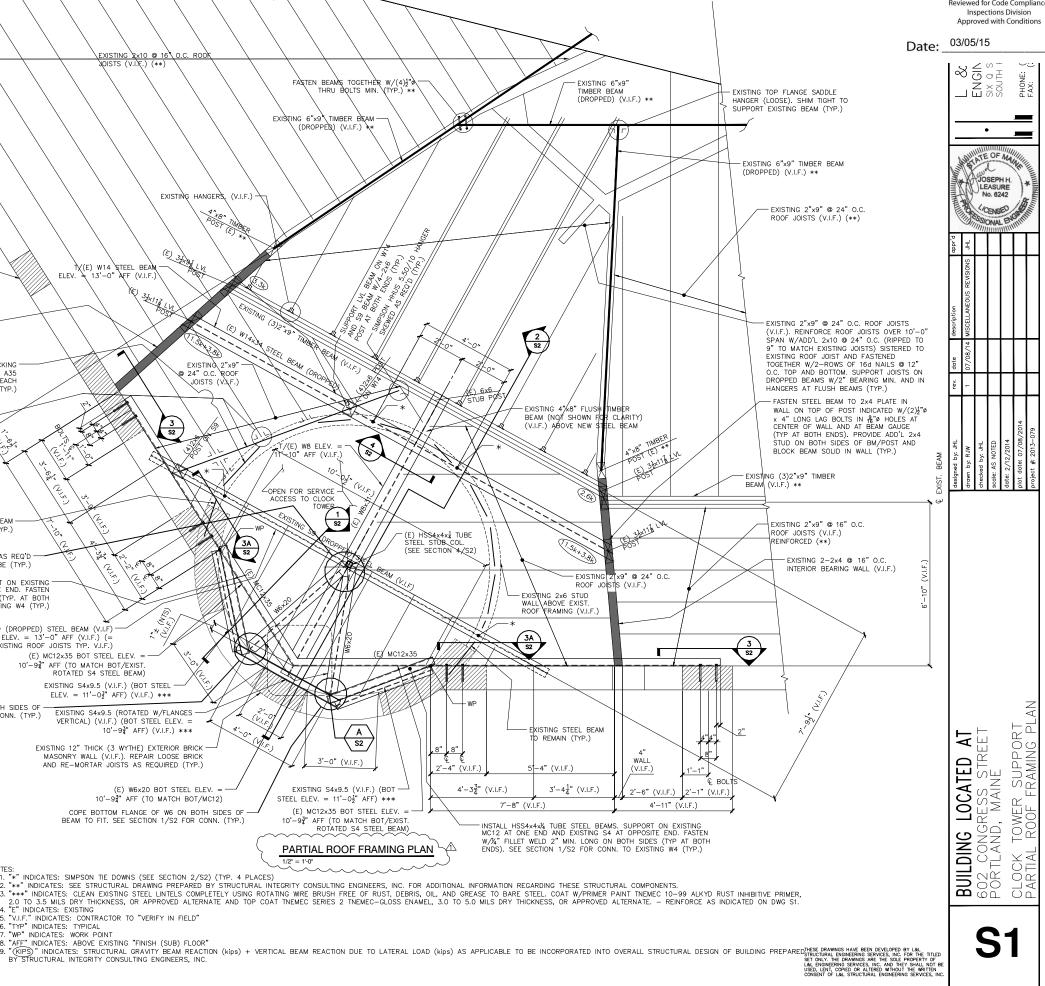
- "VI.F." INDICATES: ENSTING "VI.F." INDICATES: CONTRACTOR TO "VERIFY IN FIELD" "TYP" INDICATES: TYPICAL "WP" INDICATES: WORK POINT

- 3. "AFF" INDICATES: ABOVE EXISTING "FINISH (SUB) FLOOR

SKEWED AS REQ'D AND SHIMMED TIGHT



Reviewed for Code Compliance Inspections Division



2 52

3A 52

7'-8" (VIE)

8" 8"

-1

4'-3<u>3</u>" (V.I.F.)

STRUCTURAL GENERAL NOTES

SI Inc. Job #: 10-0023 602 Congress Street Renovations

Portland, Maine 04101

DESIGN LOADS: International Building Code; IBC 2006 Edition, and International Existing Buildings Code, except as noted Occupancy Category, Table 1604.5 II Standard

.oofs:	Snow im		Pg Pf Ce Is Ct	Table 16 Table 16 Table 16	04.5	50 psf 34 psf 1.0 1.0 1.0	(used for	drifting c	alculations)
loors:	Corridor Corridor Storage 1	s in Offices s & Public Spaces, s above first floor	First Floo	or		40 psf 50 psf 20 psf 100 psf 80 psf 125 psf 100 psf			
Later	ral Wind	IBC 1603.1.4, AS 3 Second Gust Ve Importance Facto Building Category	elocity r and Inter		Analytic sure Coeff	100 mph 1.0 ficient			
	Seismic	IBC 1609.2, ASC Exposure Use Group Importance Facto Spectral Response Short Pe	r	5-5	Accelera S _s	Enclosed C I 1.0 tion 0.30 g	1	Coefficie S _{DS}	GCpi=0.18 ent 0.24 g
		One Sec Soils Site Class Design Category Response Modific Analysis Procedur	ond cation Coe	Table 16 Table 16 efficient	S ₁ 15.1.1 16.3	0.08 g D B 1.5, Ordi	ina ry Plair nt Lateral	S _{D1} n Masonr	0.09 g y Shear Walls

FOUNDATION DESIGN:

Foundations are designed without an engineer's soil investigation. Foundation design criteria was assumed for purposes of foundation design and shall be confirmed by a soils engineer, at owner's expense, prior to construction. (This procedure may require revisions to foundation design, at additional expense to the owner, if soils engineer determines that such design criteria are inappropriate for this building site.)

--Footings--

Design of footings is based on

Maximum allowable bearing pressure 2,000 psf Bear on the natural undisturbed soil, or compacted structural fill, below frost depth.

REINFORCED CONCRETE:

Design is based on "Building Code Requirements for Reinforced Concrete" (ACI 318). Concrete work shall conform to "Standard Specifications for Structural Concrete" (ACI 301). Structural concrete shall have the following properties:

	Intended Use	fc, psi	Max	Maximum	Slump	Entrained Air	Cement	Admixtures,
		28day	W/C	Aggregate	inches	Percent	Туре	Comments
		-	Ratio			$\pm 1.5\%$		
	footings	3,000	.6	³/₄" Stone	4		I/II	
	walls	4,000	.5	³/₄" Stone	4	5%	I/II	
	interior slabs on grade	3,500	.5	³⁄₄" Stone	4	3%	I/II	Fibermesh
Т	Detailing, fabrication, and pla	cement of	^r reinforc	ing steel shal	l be in acco	rdance with the	Manual of S	tandard Practice for Detailing

Reinforced Concrete Structures (ACI 315-99). Welded wire fabric shall conform to ASTMA185.

Reinforcing bars shall conform to ASTM A615,

Grade 60,

except ties or bars shown to be field-bent, which shall be Grade 40.

Epoxy coated reinforcing bars shall conform to ASTM 775.

Zinc coated (galvanized) reinforcing bars shall conform to ASTM 767.

Bars to be welded shall conform to ASTM 706. At splices, lap bars 54 diameters unless noted otherwise.

At corners and intersections, make horizontal bars continuous or provide matching corner bars.

Around openings in walls and slabs, provide 2-#5, extending 2'-0 beyond edge of opening.

In continuous members, splice top bars at mid-span and splice bottom bars over supports.

Provide intermittent shear keys at all construction joints and elsewhere as shown on the drawings. Except as noted on the drawings, concrete protection for reinforcement in cast-in-place concrete shall be as follows:

xcept as noted on the drawings, concrete protection to	L D
a. Cast against and permanently exposed to earth	
b. Exposed to earth or weather:	

Expos	ed 1	to e	arth	or	weath
$-\hat{\pm}6t$	hre	uoł	,	81	ars

+0 tine	ugn #10 Dars	
#5 bar	W31 or D31 wire, and smalle	r

c. Not exposed to weather or in contact with ground: Slabs, walls, joists: #11 bar and smaller	3/4"
Beams, columns:	
Primary reinforcement	1 - 1/2''

Primary	reiniorcement	
Stirrups,	ties, spirals	

1-1/2"Fibremesh admixture shall be 100% virgin polypropylene, fibrillated fibers as manufactured by Fibremesh Co. per ASTM C-1116 type 111 4.1.3 and ASTM C-1116 performance level one, 1.5 lbs per cubic yard of concrete. Anchor bolts and rods for beam and column-bearing plates shall be placed with setting templates.

All concrete work is subject to inspection by a qualified special inspector employed by the owner in accordance with IBC Section 1704.4.

1 - 1/2"

STRUCTURAL MASONRY: Design is based on Unit Strength Method MSJC 2002, Section SC-1.4 B.2. Compressive strength of masonry assembly used for design is 1500 psi, based on net-bedded area. Hollow load-bearing concrete masonry (CMU) shall be lightweight units conforming to ASTM C90, Grade N1, minimum compressive strength 1,900 psi based on average net area. Facing brick shall conform to ASTM 216 Grade SW. Building brick shall conform to ASTM C62 Grade SW. Mortar shall be Type S or N conforming to ASTM C270. Masonry cement shall not be used. Provide full shoved mortar in all head and bed joints. Admixtures shall not be added for any reason unless approved by the Architect. Except for lintels, bond beam units shall be produced from standard vertically voided units with pre-cut knockout cross walls. Grout used in masonry walls and block cells shall be: coarse grout, as defined by ASTM C476, with a minimum cube strength = 2,000 psi. OR 3000 psi concrete using 3/8" diameter aggregate. placed by vibrating unless an approved self consolidating mix is used Lifts shall not exceed five feet in height If grout pour height exceeds 5 feet, clean-out holes shall be provided. Space continuous horizontal joint reinforcing at 16" maximum in all CMU walls. Joint reinforcing shall be welded type with 9 gage side-wires and 9 gage trussed or ladder cross wires. Reinforcing bars shall be as for reinforced concrete except as noted. At splices, lap bars 48 diameters. Provide reinforced grouted vertical cells at corners, ends of walls, jambs of openings, each side of vertical control joints, and at spacing 24" max. as noted on drawings. Reinforcement shall be secured against displacement prior to grouting by wire bar locators or other suitable devices at intervals not exceeding 200 bar diameters or 10 feet. Where noted on the drawings, provide clearance between masonry and structural elements, or wrap steel with polyethylene film. Submit for review

Certificates for materials used in masonry construction indicating compliance with the contract documents Special Inspection is required by design. See Special Inspection Notes.

MSIC Level 2 Quality Assurance, MSIC Table 1.14.2

Prism and grout tests will be required

Prior to the start of masonry work shall consist of five (5) masonry prisms.

Test specimens shall be made by the masons, at the direction of the owner's representative,

with materials and techniques currently being used in the wall.

Specimens shall be protected and field cured for 48 hours before being transported to a testing agency. The testing agent will be hired by the owner and shall be responsible for laboratory care and curing of specimens, testing, and reporting results to the owner, contractor, architect, and engineer in accordance with ASTM E447-92

LOOSE LINTELS:

- Unless noted otherwise, provide loose lintels as follows: (One angle for each 4" of wall thickness to bear 6" minimum each end).
- Openings to 4'-0 Angle $3-1/2 \ge 3-1/2 \ge 1/4$
- Openings 4'-1 to 5'-4 Angle 5 x 3 - 1/2 x 1/4Openings 5'-5 to 6'-6: Angle 6 $\times 3 - 1/2 \times 5/16$

STRUCTURAL STEEL:

Structural steel shall be detailed, fabricated, and erected in accordance with AISC Specifications, 2005, and Code of Standard Practice, 2000. Structural steel wide flange beams and "WT" shapes shall conform to ASTM A992.

- Other rolled shapes, including plates, channels, and angles shall conform to ASTM A36.
- Hollow structural section (HSS) tube shapes shall conform to ASTM A500, Grade B, 46 ksi yield.
- Pipe shapes shall conform to ASTM A53 Grade B.

Except as noted, framed beam connections shall be bearing-type with 3/4" diameter, snug tight, A325-N bolts, detailed in conformance with Part 4, Tables II and III, for 0.6 times the allowable uniform loads tabulated in Part 2 of the AISC Manual, 9th Edition. Install bolts in accordance with AISC "Specification for Structural Joints Using ASTM A325 or A490 Bolts", 1985.

All beams shall have full depth web stiffeners each side of webs above and below columns

Anchor rods shall conform to ASTM F1554, Grade 36 (or high strength Gr 55 or Gr 105 as noted), with weldability supplement S1. Welding shall be done by a certified welder in accordance with AISC and AWS specifications and recommendations using E70- electrodes. Where not specifically noted, minimum weld shall be 3/16" fillet by length of contact edge.

All post-installed anchors shall have current National Evaluation Report, and shall be installed in accordance with the manufacturer's requirements. Expansion anchors shall be approved "wedge" type unless specifically noted to be "sleeve" type.

Chemical anchors shall be approved epoxy or similar adhesive type and shall have current National Evaluation Report. Where base material is not solid, approved screen tubes shall be used.

Grout beneath column base and beam-bearing plates shall be minimum 28-day compressive strength of 7,500 psi,

approved pre-bagged, non-metallic, non-gaseous, bleed free,

non-shrink, when tested in accordance with ASTM C1107

Grade B or C at a flow cone fluid consistency of 20 to 30 seconds

Structural Drawing Index				
S-1.0	General Notes, Etc.			
S-1.1	Basement/ Foundation Plan			
S-1.2	First Floor Framing Plan			
S-1.3	Second Floor Framing Plan			
S-1.4	Third Floor Framing Plan			
S-1.5	Fourth Floor Framing Plan			
S-1.6	Roof Framing Plan			
S-2.1	Sections			
S-2.2	Sections			

STRUCTURAL WOOD FRAMING: In-Grade Base Values have been used for design. 2x framing shall be S.P.F. S4S No. 2 and better unless noted.

All lumber shall be 19% maximum moisture content, unless noted. Solid timber beams and posts shall be Douglas Fir-Larch No. 1. Studs shall be S.P.F. No. 2 and better. Top and bottom plates shall be S.P.F. No. 2 and better. Wood in contact with concrete shall be pressure-treated Southern Yellow Pine. Conventional light framing shall comply with IBC Section 2308. Except as noted otherwise, minimum nailing shall be provided as specified in IBC Table 2304.9.1 "Fastening Schedule" Plywood and oriented strand board (OSB) floor and roof sheathing shall be APA graded with panel identification index, thickness, and

nailing as noted on the drawings.

joists min. 4" at all floors to tie upper and lower stud walls together. Minimum height of sheathing panels shall be 16" to assure that plates are tied to studs.

or deformed shank) per 16". 12d nails are not acceptable. Provide solid blocking between joists under jamb studs of openings. All roof rafters, joists, trusses, beams shall be anchored to supports with metal framing anchors. Light gage framing anchors shown or required, shall be Simpson "Strong Tie" or equal Code approved connectors and installed with the

number and type of nails recommended by the manufacturer to develop the rated capacity. Note that heavy-duty hangers and skewed hangers may not be stocked locally and require special order from the factory. All beams and trusses shall be braced against rotation at points of bearing. Unless otherwise indicated, install two lengths of solid blocking x joist depth x 12 inches long in floor framing under column loads. Columns must have a continuous load path to foundation. Lead holes for lag screws shall be drilled in accordance with Table 6.23 of the AITC Timber Construction Manual, 5th edition.

Beams noted as ML or LVL on plan shall be 1-3/4" w Shall be plant-fabricated and manufactured by Tru Shall have the following minimum allowable desig Fb = 2600 psi Fv = 285 psi

Beams noted as PSL on plan shall be plant-fabricated Manufactured by Trus-Joist Co. / iLevel or equal, Fb = 2900 psi Fv = 290 psi

Beams noted as TS or LSL on plan shall be plant-fabr Manufactured by Trus-Joist Co. / iLevel or equal, and have the following minimum allowable design stresses: Fb = 1700 psi Fv = 400 psi Fc(||) = 1400 psi $Fc(\perp) = 680 \text{ psi}$ E = 1300 ksi

SHOP DRAWINGS:

Construction Documents are copyrighted and shall not be copied for use as erection plans or shop details. The General Contractor and his subcontractors shall submit in writing any requests to modify the plans or specifications. All shop and erection drawings shall be checked and stamped by the General Contractor prior to submission for Engineer's review. Unchecked submittals will be returned without review. Furnish one (1) reproducible and two (2) prints of shop and erection drawings to the Structural Engineer for review prior to fabrication

Reinforcing steel, Structural steel

Submit in a timely manner to permit ten (10) working days for review. Shop drawings submitted for review do not constitute "in writing" unless specific suggested changes are clearly marked. In any event, such changes by means of the shop drawing submittal process become the responsibility of the one initiating such change.

FIELD VERIFICATION OF EXISTING CONDITIONS: Contractor shall thoroughly inspect and survey existing structure to verify conditions that affect the work shown on the drawings. Contractor shall report any variations or discrepancies to the Owner and SI Inc before proceeding.

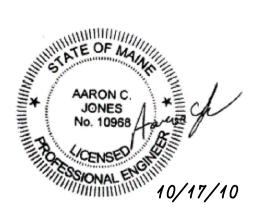
STRUCTURAL ERECTION AND BRACING REQUIREMENTS: The structural drawings illustrate the completed structure with elements in their final positions, properly supported and braced. These construction documents contain typical and representative details to assist the contractor. Details shown apply at all similar conditions unless otherwise indicated. Although due diligence has been applied to make the drawings as complete as possible, not every detail is illustrated, nor is every exceptional condition addressed.

All proprietary connections shall be installed in accordance with the manufacturers' recommendations. All work shall be accomplished in a workmanlike manner and in accordance with the applicable code and local ordinances. The general contractor is responsible for coordination of all work, including layout and dimension verification, materials coordination, shop drawing review, and the work of subcontractors.

Any discrepancies or omissions discovered in the course of the work shall be immediately reported to the architect for resolution. Continuation of work without notification of discrepancies relieves the architect and engineer from all consequences. Unless otherwise specifically indicated, the drawings do not describe methods of construction. The contractor, in the proper sequence, shall perform or supervise all work necessary to achieve the final completed structure, and to

protect the structure, workmen, and others during construction. Such work shall include, but not be limited to, bracing, shoring for construction equipment, shoring for excavation, formwork, scaffolding, safety devices and programs of all kinds, support and bracing for cranes and other erection equipment. Do not backfill against basement or retaining walls until supporting slabs and floor framing are in place and securely anchored, unless adequate bracing is provided.

Temporary bracing shall remain in place until all floors, walls, roofs and any other supporting elements are in place. The architect and engineer bear no responsibility for the above items, and observation visits to the site do not in any way include inspection of them.



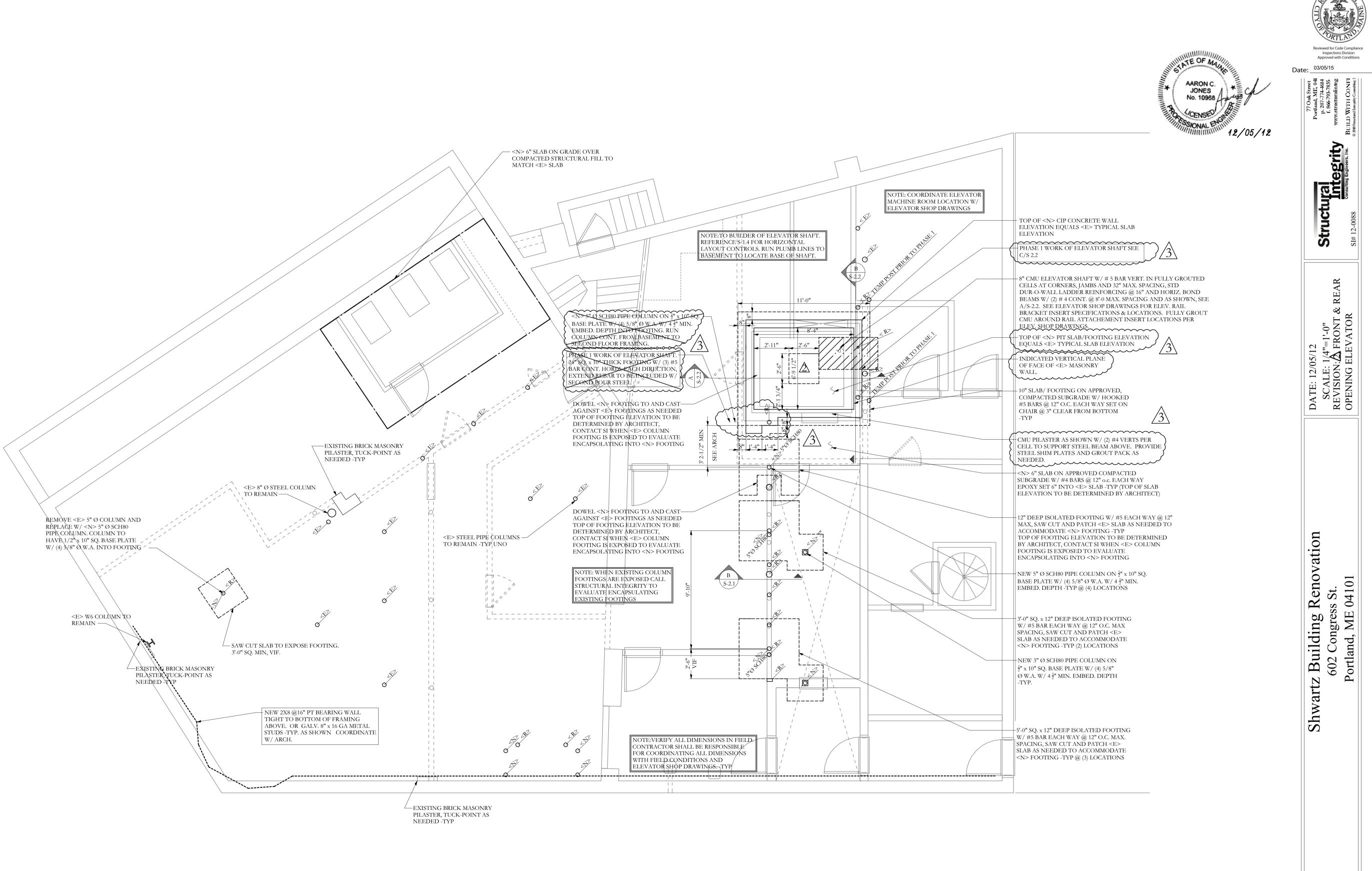
Nail wall sheathing with 8d commons at 6" o.c. at panel edges, and 12" o.c. at intermediate framing except as noted. Sheathing shall be continuous from bottom plate to top plate. Cut in "L" and "T" shapes around openings. Lap sheathing over rim

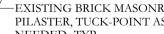
Minimum 3-8d per stud and nail plates with "edge nail" spacing. Sole plate at all perimeter walls and at designated shear walls shall be nailed as for braced panels with 3-16d x 3 1/2" long box nails (coated

PLANT FABRICATED / PRE-ENGINEERED WOOD FRAMING:

wide Laminated Veneer Lum rus-Joist Co./ Ilevel or equal,		is of the depth note	ed on plan
gn stresses:			
Fc() = 2460 psi	$Fc(\perp)$	=750 psi	E = 1800 ksi
1			
and have the following mini:	mum allo	wable design stress	es:
Fc() = 2900 psi	$Fc(\perp)$	$= 750 \mathrm{psi}$	E = 2000 ksi
pricated			

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Date	77 Oak Street Portland, ME, 041 D. 207-774 4614	പത	BUILD WITH CONFI © 2010 Sunctural Integrity Consulting 1	
	Cturi Intel	ourucuurar Integrity	Consulting Engineers. Inc. SI# 10-0023	
	DATE: 10/17/12	REVISION: AELEVATOR/	T ACCESS	
	Shwartz Building Renovation	602 Congress St.	Portland, ME 04101	
		GENERAL NOTES, Etc.		
	S-	1.()	







BASEMENT/FOUNDATION PLAN

NOTES: SCALE 1/4"=1'-0"

- 1. VERIFY ALL EXISTING CONDITIONS IN FIELD, CONTACT SI Inc. IF CONDITIONS IN FIELD DO NOT MATCH PLAN
- 2. IF MEMBER IS NOT INDICATED AS <E>, <N> OR <R> MEMBER IS TO BE CONSIDERED A NEW, <N>, MEMBER
- 3. SEE ARCH. FOR DIMENSIONS AND ELEVATIONS OF NEW FLOOR PLANS
- 4. MECHANICALLY COMPACT ALL SOIL BELOW NEW FOOTINGS TYP 5. SOILS ENGINEER TO APROVE ALL SUBGRADE PRIOR TO CONST.

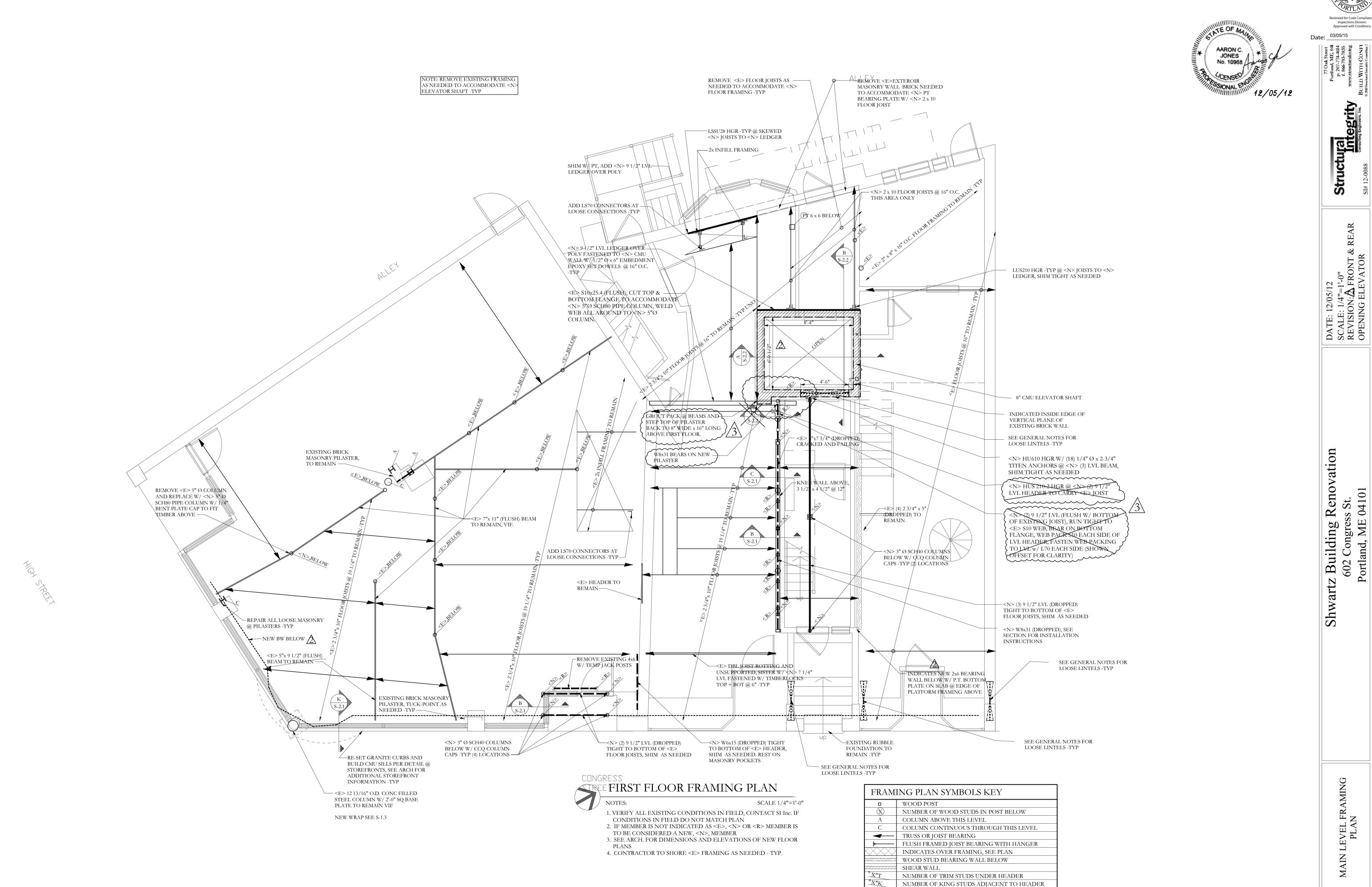
FRAMING PLAN SYMBOLS KEY			
	WOOD POST		
\mathbf{X}	NUMBER OF WOOD STUDS IN POST BELOW		
А	COLUMN ABOVE THIS LEVEL		
С	COLUMN CONTINUOUS THROUGH THIS LEVEL		
-	TRUSS OR JOIST BEARING		
<u> </u>	FLUSH FRAMED JOIST BEARING WITH HANGER		
	INDICATES OVER FRAMING, SEE PLAN		
	WOOD STUD BEARING WALL BELOW		
	SHEAR WALL		
<u>"X"T</u>	NUMBER OF TRIM STUDS UNDER HEADER		
<u>"X"K</u>	NUMBER OF KING STUDS ADJACENT TO HEADER		
<e></e>	INDICATES EXISTING MEMBER TO REMAIN		
<n></n>	INDICATES NEW MEMBER		
<r></r>	INDICATES EXISTING MEMBER TO BE REMOVED		

S-1.1

AN

PL

BASEMENT



INDICATES EXISTING MEMBER TO REMAIN INDICATES NEW MEMBER INDICATES EXISTING MEMBER TO BE REMOVED

<E>

< N >

<R>

REVISION 3: 11/21/13

S-1.2

MAIN LEVEL FRAMING PLAN

Inspections Division Approved with Conditions

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Structu

REAR

8

: 12/05/12 E: 1/4"=1'-0" SION: A FRONT & ING ELEVATOR

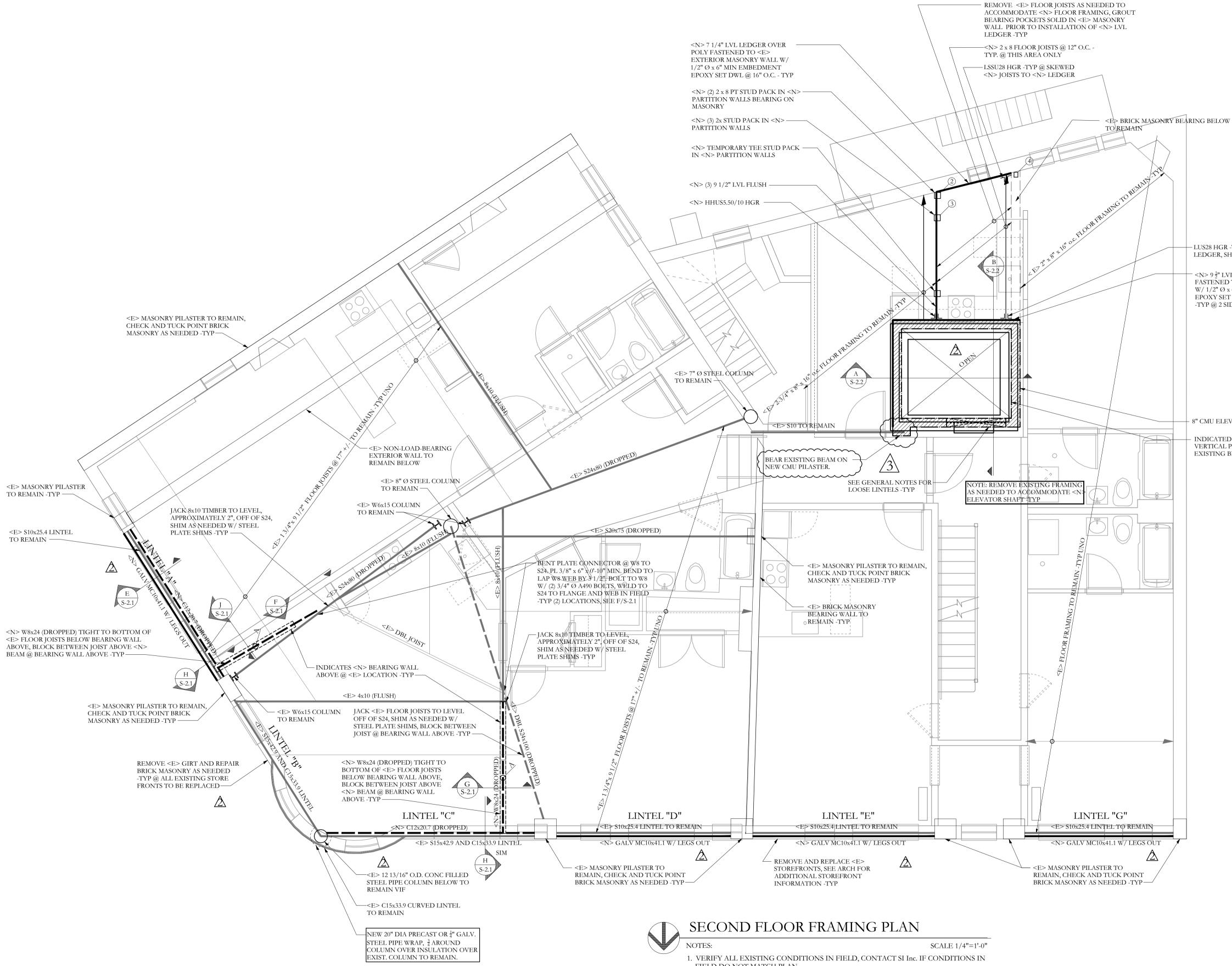
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ovation

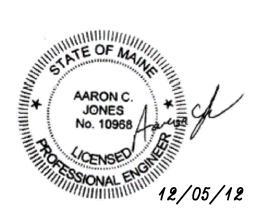
Shwartz

Rend ss St.

03/05/15



- FIELD DO NOT MATCH PLAN 2. SEE ARCH. FOR DIMENSIONS AND ELEVATIONS OF NEW FLOOR PLANS
- 3. CONTRACTOR TO SHORE <E> FRAMING AS NEEDED
- 4. EXPOSE BRICK MASONRY PILASTERS @ MAIN LEVEL BELOW AND TUCK POINT ALL
- MASONRY AS NEEDED. ALL MASONRY TO BE SOLID AND SOUND AT COMPLETION OF WORK. 5. 30 TON JACK NEEDED TO MOVE BUILDING WALLS - TYP AT ALL JACKING LOCATIONS.



-LUS28 HGR -TYP @ <E> JOISTS TO <N> LEDGER, SHIM TIGHT AS NEEDED

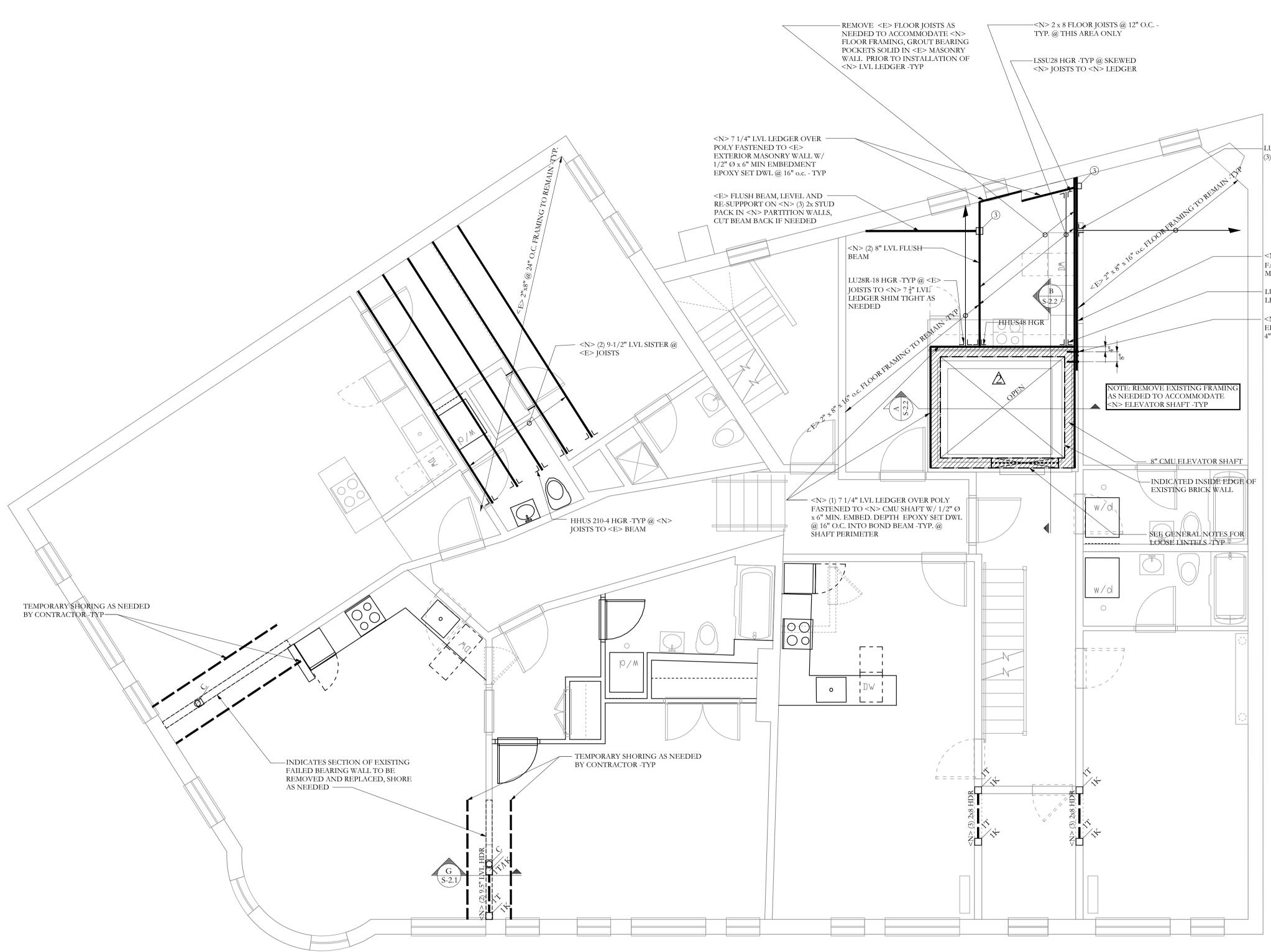
^{−−} <N> 9 ½" LVL LEDGER OVER POLY FASTENED TO <N> CMU WALL W/ 1/2" Ø x 6" EMBEDMENT EPOXY SET DOWELS @ 16" O.C. -TYP @ 2 SIDES.

- 8" CMU ELEVATOR SHAFT

- INDICATED INSIDE EDGE OF VERTICAL PLANE OF EXISTING BRICK WALL

ГКАМІ	NG PLAN SYMBOLS KEY
	WOOD POST
\mathbf{X}	NUMBER OF WOOD STUDS IN POST BELOW
А	COLUMN ABOVE THIS LEVEL
С	COLUMN CONTINUOUS THROUGH THIS LEVEL
-	TRUSS OR JOIST BEARING
<u>⊨</u>	FLUSH FRAMED JOIST BEARING WITH HANGER
	INDICATES OVER FRAMING, SEE PLAN
	WOOD STUD BEARING WALL BELOW
	SHEAR WALL
<u>"X"T</u>	NUMBER OF TRIM STUDS UNDER HEADER
" <u>X"K</u>	NUMBER OF KING STUDS ADJACENT TO HEADER
<e></e>	INDICATES EXISTING MEMBER TO REMAIN
<n></n>	INDICATES NEW MEMBER
<r></r>	INDICATES EXISTING MEMBER TO BE REMOVED

	Reviewed for Code Compliance Inspections Division Approved with Conditions	ANTER
Dat	Structural Bull.D WITH CONFI SI# 12-0088	
	DATE: 12/05/12 SCALE: 1/4"=1'-0" REVISION: A FRONT & REAR OPENING ELEVATOR	
	Shwartz Building Renovation 602 Congress St. Portland, ME 04101	
	SECOND FLOOR FRAMING PLAN	
_	S-1.3	





THIRD FLOOR FRAMING PLAN

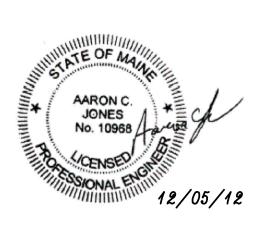
NOTES:

SCALE 1/4"=1'-0"

1. VERIFY ALL EXISTING CONDITIONS IN FIELD, CONTACT SI Inc. IF CONDITIONS IN FIELD DO NOT MATCH PLAN

2. SEE ARCH. FOR DIMENSIONS AND ELEVATIONS OF NEW FLOOR PLANS

- 3. CONTRACTOR TO SHORE <E> FRAMING AS NEEDED 4. RUN CMU SHAFT LAYOUT VERTICAL TO ALL OTHER LEVELS FROM THIS INFO.
- IF BUILDING IS OUT OF PLUMB CONTACT SI Inc. AND ARCHITECT.



LU28R-18 HGR -TYP @ <E> JOISTS TO <N> (3) 11 1/2" LVL SHIM TIGHT AS NEEDED

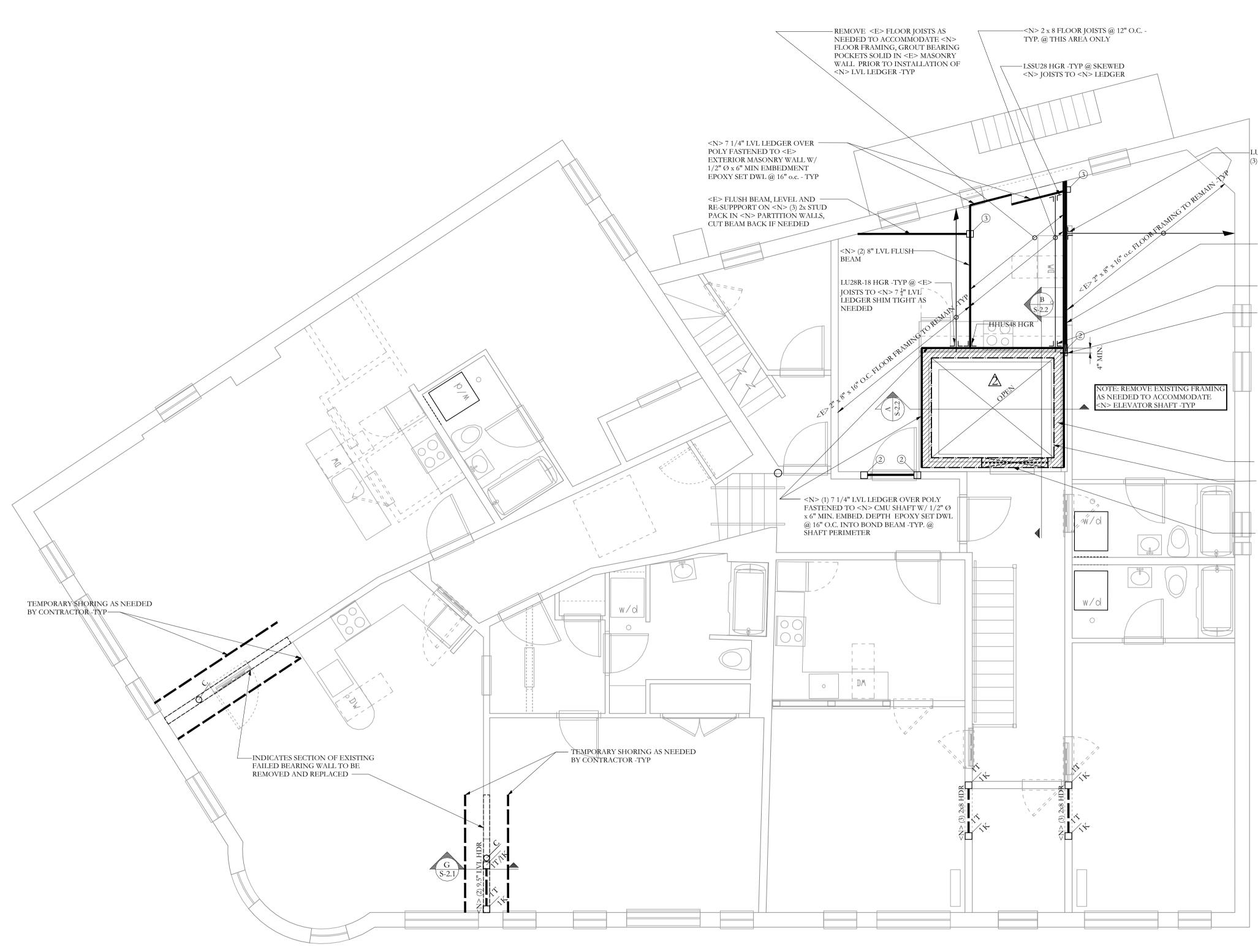
- <N> (3) 11 $\frac{7}{8}$ " LVL LEDGER OVER POLY FASTENED TO CMU SHAFT W/ (2) $\frac{7}{8}$ " Ø x 6" MIN. EMBED. DEPTH EPOXY SET DWL.

— LUS28 HGR -TYP @ <N> JOISTS TO <N> LEDGER, SHIM TIGHT AS NEEDED

— <N> (2) 7/8" Ø x 6" MIN. EMBED. DEPTH EPOXY SET DWL INTO GROUTED CMU @ 4" AND 12" FROM CORNER OF CMU SHAFT

FRAM	ING PLAN SYMBOLS KEY
	WOOD POST
\mathbf{X}	NUMBER OF WOOD STUDS IN POST BELOW
А	COLUMN ABOVE THIS LEVEL
С	COLUMN CONTINUOUS THROUGH THIS LEVEL
-	TRUSS OR JOIST BEARING
<u>⊨</u>	FLUSH FRAMED JOIST BEARING WITH HANGER
	INDICATES OVER FRAMING, SEE PLAN
	WOOD STUD BEARING WALL BELOW
	SHEAR WALL
<u>"X"T</u>	NUMBER OF TRIM STUDS UNDER HEADER
X''K	NUMBER OF KING STUDS ADJACENT TO HEADER
<e></e>	INDICATES EXISTING MEMBER TO REMAIN
<n></n>	INDICATES NEW MEMBER
<r></r>	INDICATES EXISTING MEMBER TO BE REMOVED

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	Structural Integrit	Consulting Engineers, Ine. SI# 12-0088
	DATE: 12/05/12 SCALE: 1/4"=1'-0" REVISION: A FRONT & REAR	OPENING ELEVATOR
	Shwartz Building Renovation 602 Congress St.	Portland, ME 04101
	THIRD FLOOR FRAMING PLAN	
	S-1.	4



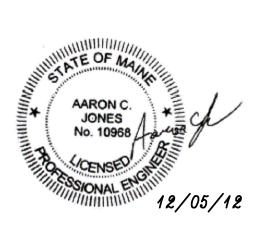
FOU NOTES:

FOURTH FLOOR FRAMING PLAN

SCALE 1/4"=1'-0"

 VERIFY ALL EXISTING CONDITIONS IN FIELD, CONTACT SI Inc. IF CONDITIONS IN FIELD DO NOT MATCH PLAN
 SEE ARCH. FOR DIMENSIONS AND ELEVATIONS OF NEW FLOOR PLANS

SEE ARCH. FOR DIMENSIONS AND ELEVATIONS OF NEW FLOOR PLANS
 CONTRACTOR TO SHORE <E> FRAMING AS NEEDED



LU28R-18 HGR -TYP @ <E> JOISTS TO <N> (3) 11 1/2" LVL SHIM TIGHT AS NEEDED

LUS28 HGR -TYP @ <N> JOISTS TO <N> LEDGER, SHIM TIGHT AS NEEDED

<N> (2) 2x 10 POST W/ ¹/₂" EPOXY SET
 ANCHOR @ 16" O.C. VERT. SPACING W/ 6"
 MIN. EMBED. DEPTH INTO GROUTED CMU
 @ 4" MIN. EDGE DIST. FROM CORNER OF
 CMU SHAFT

– 8" CMU ELEVATOR SHAFT

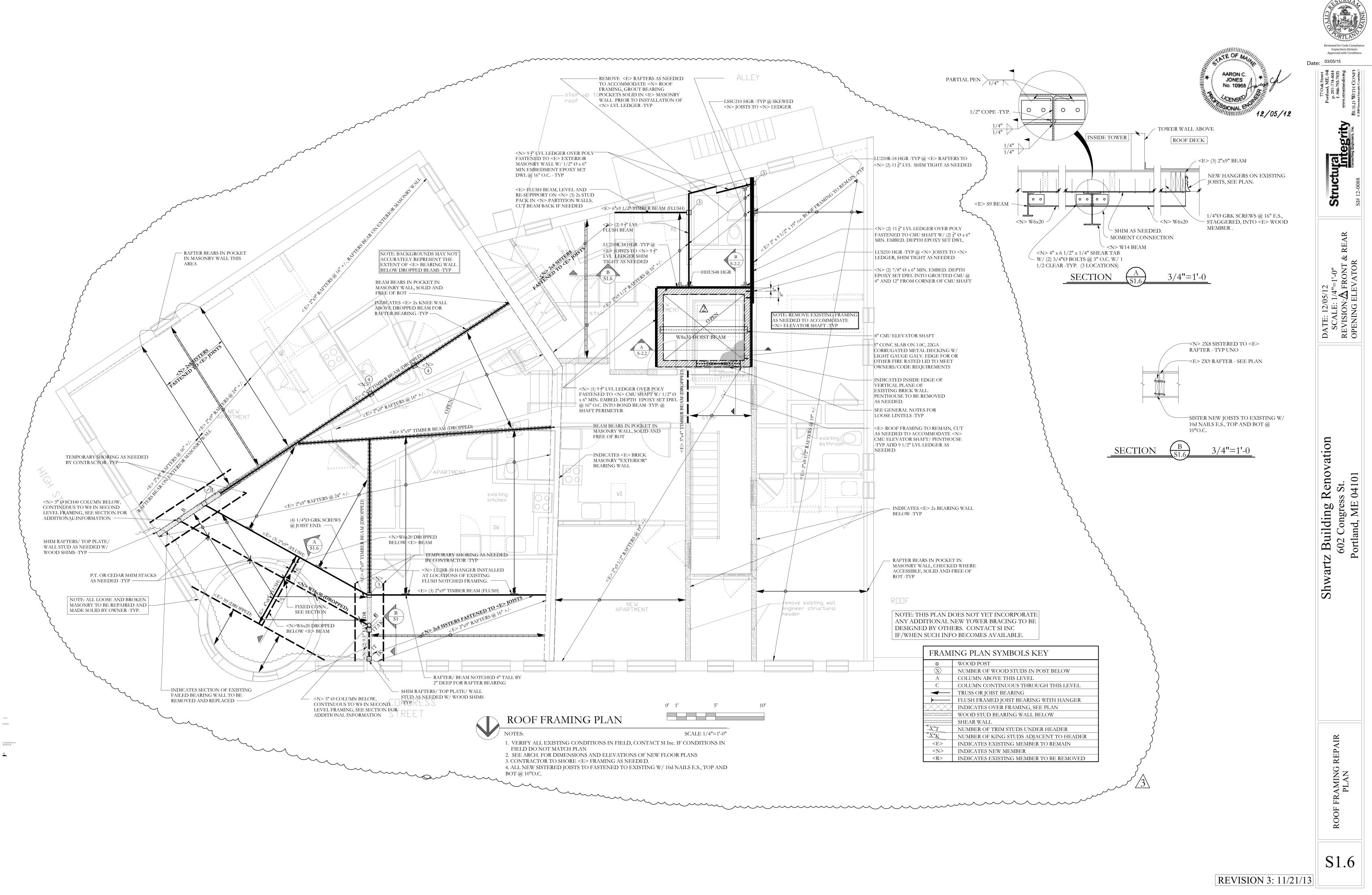
— INDICATED INSIDE EDGE OF VERTICAL PLANE OF EXISTING BRICK WALL

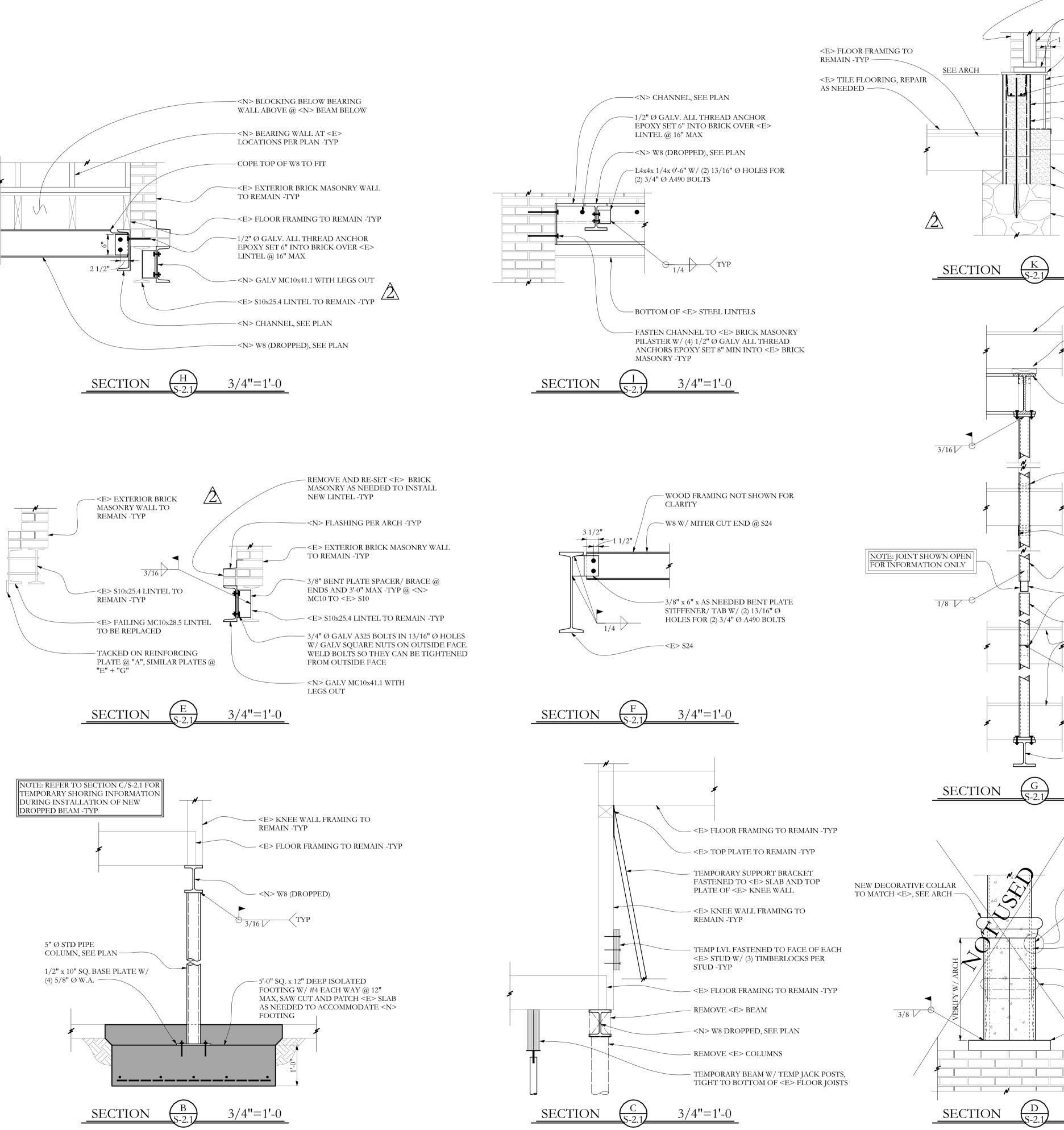
SEE GENERAL NOTES FOR LOOSE LINTELS -TYP

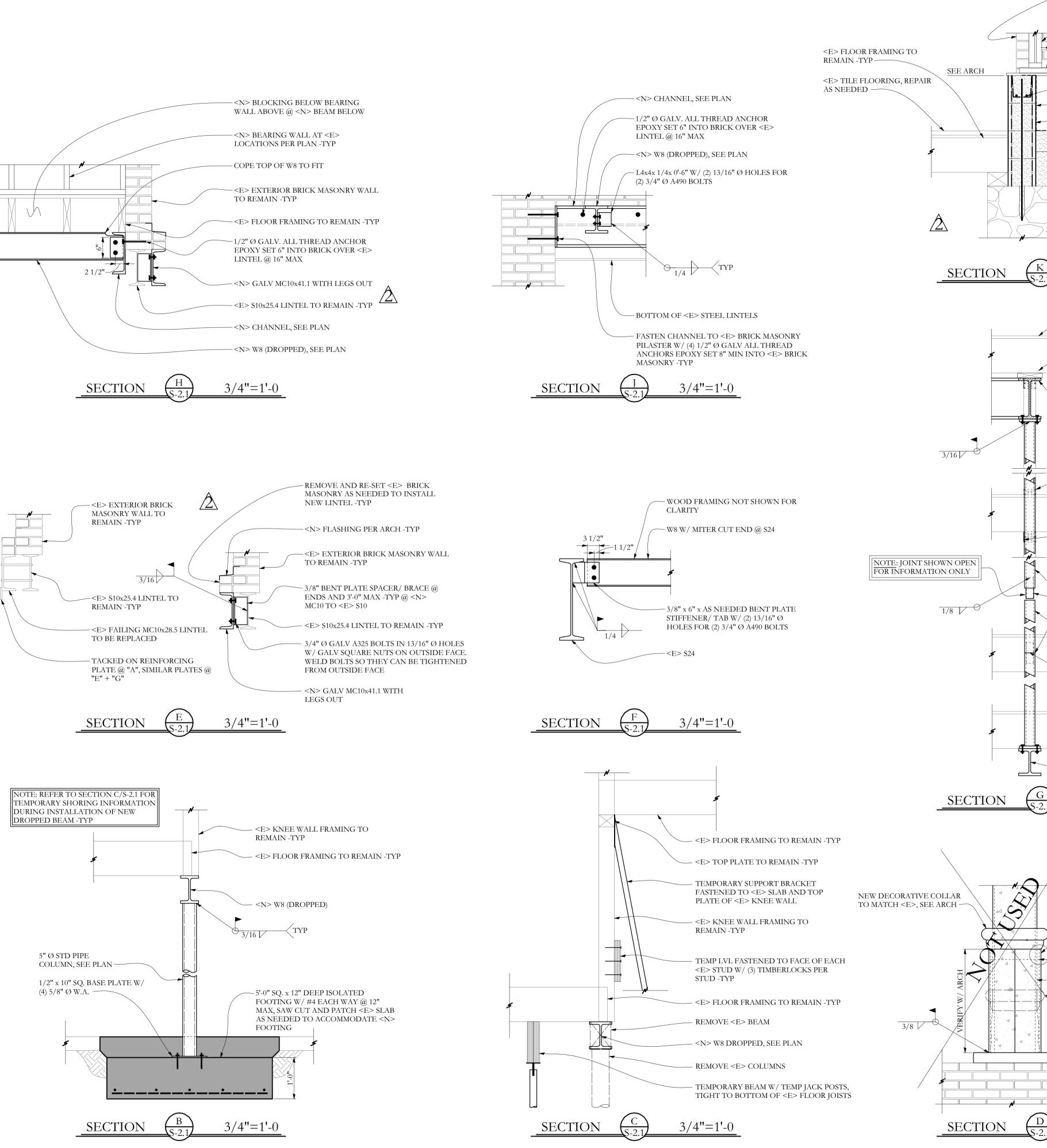
FRAM	ING PLAN SYMBOLS KEY
	WOOD POST
\mathbf{X}	NUMBER OF WOOD STUDS IN POST BELOW
А	COLUMN ABOVE THIS LEVEL
С	COLUMN CONTINUOUS THROUGH THIS LEVEL
	TRUSS OR JOIST BEARING
<u> </u>	FLUSH FRAMED JOIST BEARING WITH HANGER
	INDICATES OVER FRAMING, SEE PLAN
	WOOD STUD BEARING WALL BELOW
	SHEAR WALL
<u>"X"T</u>	NUMBER OF TRIM STUDS UNDER HEADER
<u>"X"K</u>	NUMBER OF KING STUDS ADJACENT TO HEADER
<e></e>	INDICATES EXISTING MEMBER TO REMAIN
<n></n>	INDICATES NEW MEMBER
<r></r>	INDICATES EXISTING MEMBER TO BE REMOVED

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	Structural Integrity SI# 12-0088	
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	FOURTH FLOOR FRAMING PLAN	
	S-1.5	

ESURGAL







-<E> BRICK MASONRY WALL/ PILASTER TO REMAIN - TYP

-<N> STOREFRONT SYSTEM PER ARCH -TYP

CAP PER ARCH -TYP

— FINISH PER ARCH - TYP

CONTRACTOR - TYP

– BOND BEAM @ TOP W/ (2) #4 BARS -TYP - FLASHING/ WATERPROOFING BY

-<N> 8" CMU KNEE WALL W/ FULLY GROUTED CELLS AND #5 VERTS @ 24" EPOXY SET INTO <E> FND WALL 10" MIN

— <E> GRANITE VENEER, RESET AS NEEDED -TYP

– REPAIR TOP OF <E> FND WALL AS NEEDED TO RECIEVE CMU -TYP

E> RUBBLE FND WALL TO REMAIN -TYP

3/4"=1'-0

– EXISTING ROOF FRAMING TO REMAIN -TYP

— 2x8 NAILER PLATE W/ 1/2" Ø CARRIAGE BOLTS @ 32"

- <N> W14x30 INSTALLED TIGHT TO BOTTOM OF EXISTING ROOF FRAMING, SEE PLAN FOR LOCATION INFORMATION

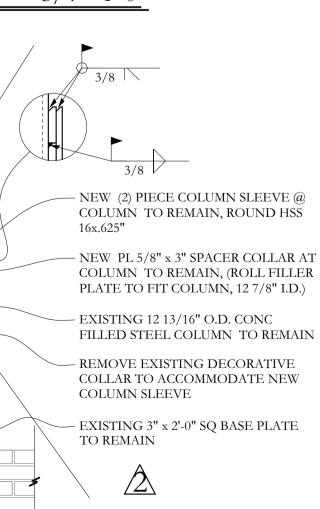
- CUT HOLE IN TOP AND BOTTOM PLATES AND FLOOR SHEATHING W/ 4" Ø HOLE SAW TO ACCOMMODATE 3" Ø PIPE COLUMN

– MSTC28 STRAP ACROSS THE FACE OF CUT TOP PLATES @ COLUMN -TYP (1) FACE OF TOP PLATES

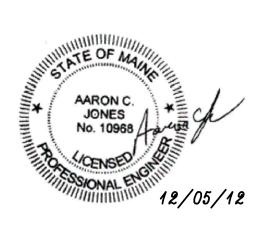
- 3" Ø STD PIPE COLUMN FULL HEIGHT TO <E> DBL S24 @ SECOND LEVEL FLOOR FRAMING -21/2" Ø x 6" STD PIPE SLEEVE -TYP @
- JOINTS IN COL. -SEGMENT COLUMN AS NEEDED
- UNO 3/16 = 2 TWO SIDES MIN - TYP

- < N > W8 (DROPPED)

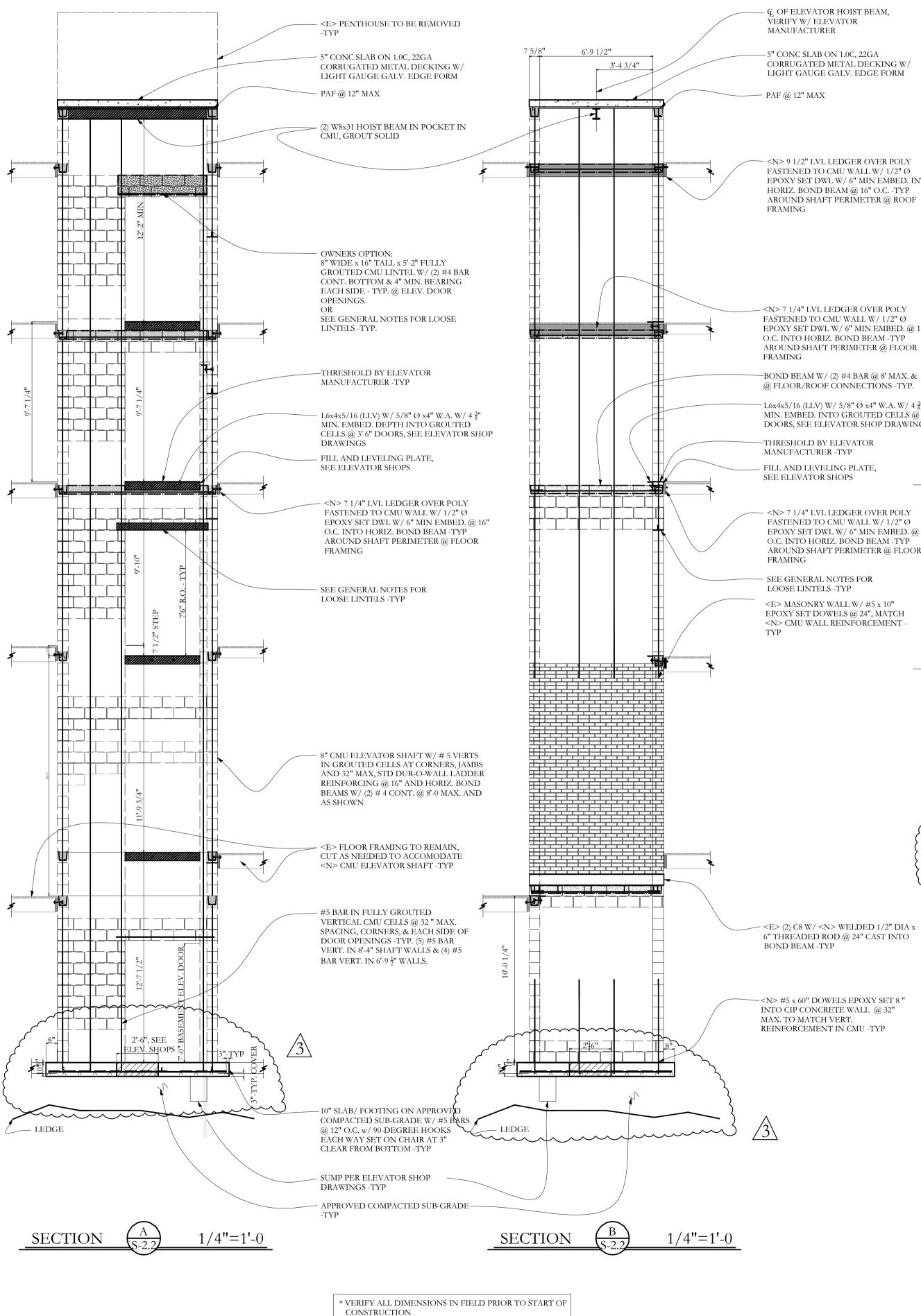
3/4"=1'-0



3/4"=1'-0



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	Structural Integrity SI# 12-0088	
	DATE: 12/05/12 SCALE: 3/4"=1'-0" REVISION: A FRONT & REAR OPENING ELEVATOR	
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	SECTIONS	
	S-2.1	



* SEE ELEVATOR MANUFACTURER FOR GUIDE

RAIL ATTACHMENT

€ OF ELEVATOR HOIST BEAM, VERIFY W/ ELEVATOR

-5" CONC SLAB ON 1.0C, 22GA CORRUGATED METAL DECKING W/ LIGHT GAUGE GALV. EDGE FORM

- <N> 9 1/2" LVL LEDGER OVER POLY FASTENED TO CMU WALL W/ 1/2" Ø EPOXY SET DWL W/ 6" MIN EMBED. INTO HORIZ. BOND BEAM @ 16" O.C. -TYP AROUND SHAFT PERIMETER @ ROOF

- <N> 7 1/4" LVL LEDGER OVER POLY FASTENED TO CMU WALL W/ 1/2" Ø EPOXY SET DWL W/ 6" MIN EMBED. @ 16" O.C. INTO HORIZ. BOND BEAM - TYP

-BOND BEAM W/ (2) #4 BAR @ 8' MAX. & @ FLOOR/ROOF CONNECTIONS -TYP.

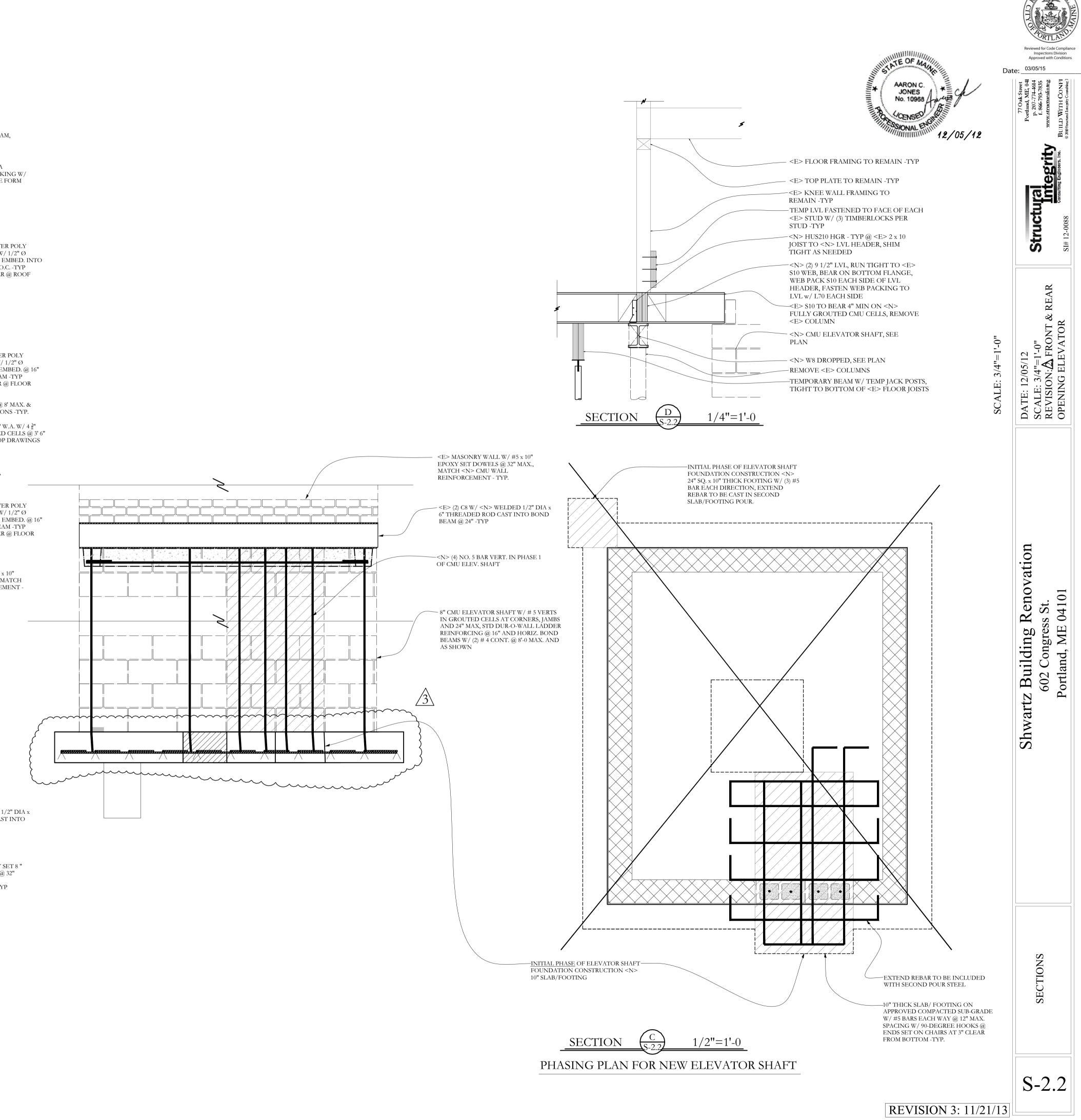
- L6x4x5/16 (LLV) W/ 5/8" Ø x4" W.A. W/ 4 $\frac{3}{8}$ " MIN. EMBED. INTO GROUTED CELLS @ 3' 6" DOORS, SEE ELEVATOR SHOP DRAWINGS

-THRESHOLD BY ELEVATOR

FILL AND LEVELING PLATE,

<N> 7 1/4" LVL LEDGER OVER POLY FASTENED TO CMU WALL W/ 1/2" Ø EPOXY SET DWL W/ 6" MIN EMBED. @ 16" O.C. INTO HORIZ. BOND BEAM - TYP AROUND SHAFT PERIMETER @ FLOOR

- SEE GENERAL NOTES FOR LOOSE LINTELS -TYP <E> MASONRY WALL W/ #5 x 10"



-<N> #5 x 60" DOWELS EPOXY SET 8 " INTO CIP CONCRETE WALL @ 32" REINFORCEMENT IN CMU -TYP

