## STRUCTURAL GENERAL NOTES

## SI Inc. Job #: 10-0023 602 Congress Street Renovations

Portland, Maine 04101

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DESIGN LOADS: International Building Code; IBC 2006 Edition, and International Existing Buildings Code, except as noted Occupancy Category, Table 1604.5 II Standard

-									
oots:	Ground	Snow	Por			50 nst	(used for	drifting	calculations)
	Elat Roo	f Snow	Pf			34  psf	(used for	unning	careera donisj
	Snow Ex	rongure Factor	Ce	Table 16	0831	10			
	Show in	rpostance Eactor	Le Le	Table 16	00.5.1 04 5	1.0			
	Snow Th	permal Factor	Ct	Table 16	0832	1.0			
	onow n	rennar i actor,	91	Table It		1.0			
oors:									
	Resident	ial				40 psf			
	Office					50 psf			
	Partition	s in Offices				$20 \mathrm{psf}$			
	Corridor	rs & Public Spaces	, First Flo	or		100 psf			
	Corridor	s above first floor				80 psf			
	Storage .	Areas				125 psf			
	Commer	cial 1 <sup>st</sup> floor				100 psf			
						-			
Late	ral								
	Wind	IBC 1603.1.4, AS	CE 7-02		Analytic	Method			
		3 Second Gust V	elocity			100 mph			
		Importance Facto	or			1.0			
		Building Category	7 and Inte	mal Pres	sure Coeff	licient			
		IBC 1609.2, ASC	E Figure (	5-5		Enclosed	1		GCpi=0.18
		Exposure				С			
	Solomia	Lize Create				т			
	Seisinie	Use Gloup Importance Facto				1.0			
		Second Parameter Pacito	)r		Agaalam	1.0 fion		Coeffici	opt
			- · 1		7 CCCICIA			COEIIICI	
		Short Pe	enod		S <sub>s</sub>	0.50 g		$S_{DS}$	0.24 g
		One Sec	cond		S <sub>1</sub>	0.08 g		$S_{D1}$	0.09 g
		Soils Site Class		Table 16	515.1.1	D			
		Design Category		Table 16	516.3	В			
		Response Modifi	cation Co	efficient	R	1.5, Ordi	inary Plai	n Mason	ry Shear Walls
		Analysis Procedu	re			Equivale	nt Latera	l Force	

## FOUNDATION DESIGN:

Foundations are designed without an engineer's soil investigation. Foundation design criteria was assumed for purposes of foundation design and shall be confirmed by a soils engineer, at owner's expense, prior to construction. (This procedure may require revisions to foundation design, at additional expense to the owner, if soils engineer determines that such design criteria are inappropriate for this building site.)

--Footings--

Design of footings is based on Maximum allowable bearing pressure 2,000 psf

Bear on the natural undisturbed soil, or compacted structural fill, below frost depth.

## **REINFORCED CONCRETE:**

Design is based on "Building Code Requirements for Reinforced Concrete" (ACI 318). Concrete work shall conform to "Standard Specifications for Structural Concrete" (ACI 301). Structural concrete shall have the following properties:

	Intended Use	fc, psi	Max	Maximum	Slump	Entrained Air	Cement	Admixtures,
		28day	W/C	Aggregate	inches	Percent	Туре	Comments
		-	Ratio			$\pm 1.5\%$		
	footings	3,000	.6	³⁄₄" Stone	4		I/II	
	walls	4,000	.5	³⁄₄" Stone	4	5%	I/II	
	interior slabs on grade	3,500	.5	³⁄₄" Stone	4	3%	I/II	Fibermesh
Γ	Detailing, fabrication, and pla	cement of	f reinford	ing steel shal	l be in acco	rdance with the i	Manual of S	standard Practice for Detailing

Reinforced Concrete Structures (ACI 315-99). Welded wire fabric shall conform to ASTM A185.

Reinforcing bars shall conform to ASTM A615,

Grade 60,

except ties or bars shown to be field-bent, which shall be Grade 40.

Epoxy coated reinforcing bars shall conform to ASTM 775. Zinc coated (galvanized) reinforcing bars shall conform to ASTM 767.

Bars to be welded shall conform to ASTM 706.

At splices, lap bars 54 diameters unless noted otherwise.

At corners and intersections, make horizontal bars continuous or provide matching corner bars.

Around openings in walls and slabs, provide 2-#5, extending 2'-0 beyond edge of opening. In continuous members, splice top bars at mid-span and splice bottom bars over supports.

Provide intermittent shear keys at all construction joints and elsewhere as shown on the drawings.

Except as noted on the drawings, concrete protection for reinforcement in cast-in-place concrete shall be as follows:

3"

1-1/2"

3/4"

1-1/2"

Cast against and	permanently	exposed to earth
Exposed to earth	n or weather:	

- L'APO S	u 10	carui	OL	weat
#6 t	hroug	h #1	.8 b	ars

no ano agri ni o bato
#5 bar, $W31$ or D31 wire, and smaller
c. Not exposed to weather or in contact with ground:
Slabs walls, joists: #11 bar and smaller

Beams, columns:		
Primary	reinforcement	

Stirrups, ties, spira	ds 1-1/2"		
Fibremesh admixture shall be 100%	virgin polypropylene, fibrillated fibers a	s manufactured by Fibremesh C	Co. per ASTM C-1116 type 111

4.1.3 and ASTM C-1116 performance level one, 1.5 lbs per cubic yard of concrete.

Anchor bolts and rods for beam and column-bearing plates shall be placed with setting templates. All concrete work is subject to inspection by a qualified special inspector employed by the owner in accordance with IBC Section 1704.4.

STRUCTURAL MASONRY:
Design is based on Unit Strength Method
MSJC 2002, Section SC-1.4 B.2.
Compressive strength of masonry assembly used for design is 1500 psi, based on net-bedded area.
Hollow load-bearing concrete masonry (CMU) shall be lightweight units conforming to ASTM C90,
Grade N1, minimum compressive strength 1,900 psi based on average net area.
Facing brick shall conform to ASTM 216 Grade SW.
Building brick shall conform to ASTM C62 Grade SW.
Mortar shall be Type S or N conforming to ASTM C270.
Masonry cement shall not be used.
Provide full shoved mortar in all head and bed joints.

Admixtures shall not be added for any reason unless approved by the Architect. Except for lintels, bond beam units shall be produced from standard vertically voided units with pre-cut knockout cross walls. Grout used in masonry walls and block cells shall be:

coarse grout, as defined by ASTM C476, with a minimum cube strength = 2,000 psi. OR 3000 psi concrete using 3/8" diameter aggregate. placed by vibrating unless an approved self consolidating mix is used Lifts shall not exceed five feet in height

If grout pour height exceeds 5 feet, clean-out holes shall be provided.

Space continuous horizontal joint reinforcing at 16" maximum in all CMU walls. Joint reinforcing shall be welded type with 9 gage side-wires and 9 gage trussed or ladder cross wires.

Reinforcing bars shall be as for reinforced concrete except as noted.

At splices, lap bars 48 diameters. Provide reinforced grouted vertical cells

- at corners, ends of walls, jambs of openings, each side of vertical control joints, and
- at spacing 24" max. as noted on drawings. Reinforcement shall be secured against displacement prior to grouting
- by wire bar locators or other suitable devices at intervals not exceeding 200 bar diameters or 10 feet.

Where noted on the drawings, provide clearance between masonry and structural elements, or

Special Inspection is required by design. See Special Inspection Notes.

wrap steel with polyethylene film. Submit for review

Certificates for materials used in masonry construction indicating compliance with the contract documents

MSJC Level 2 Quality Assurance, MSJC Table 1.14.2

Prism and grout tests will be required

Prior to the start of masonry work

shall consist of five (5) masonry prisms.

Test specimens shall be made by the masons, at the direction of the owner's representative,

with materials and techniques currently being used in the wall. Specimens shall be protected and field cured for 48 hours before being transported to a testing agency.

The testing agent will be hired by the owner and shall be responsible for laboratory care and curing of specimens, testing, and reporting results to the owner, contractor, architect, and engineer in accordance with ASTM E447-92

## LOOSE LINTELS:

- Unless noted otherwise, provide loose lintels as follows: (One angle for each 4" of wall thickness to bear 6" minimum each end). Angle 3-1/2 x 3-1/2 x 1/4 Openings to 4'-0
- Openings 4'-1 to 5'-4: Angle 5  $x 3-1/2 \times 1/4$ Openings 5'-5 to 6'-6: Angle 6  $x 3-1/2 \times 5/16$

## STRUCTURAL STEEL:

Structural steel shall be detailed, fabricated, and erected in accordance with AISC Specifications, 2005, and Code of Standard Practice, 2000. Structural steel wide flange beams and "WT" shapes shall conform to ASTM A992.

Other rolled shapes, including plates, channels, and angles shall conform to ASTM A36. Hollow structural section (HSS) tube shapes shall conform to ASTM A500, Grade B, 46 ksi yield.

Pipe shapes shall conform to ASTM A53 Grade B.

Except as noted, framed beam connections shall be bearing-type with 3/4" diameter, snug tight, A325-N bolts, detailed in conformance with Part 4, Tables II and III, for 0.6 times the allowable uniform loads tabulated in Part 2 of the AISC Manual, 9th Edition. Install bolts in accordance with AISC "Specification for Structural Joints Using ASTM A325 or A490 Bolts", 1985.

All beams shall have full depth web stiffeners each side of webs above and below columns Anchor rods shall conform to ASTM F1554, Grade 36 (or high strength Gr 55 or Gr 105 as noted), with weldability supplement S1.

Welding shall be done by a certified welder in accordance with AISC and AWS specifications and recommendations using E70- electrodes. Where not specifically noted, minimum weld shall be 3/16" fillet by length of contact edge. All post-installed anchors shall have current National Evaluation Report, and shall be installed in accordance with the manufacturer's

requirements. Expansion anchors shall be approved "wedge" type unless specifically noted to be "sleeve" type.

Chemical anchors shall be approved epoxy or similar adhesive type and shall have current National Evaluation Report. Where base material is not solid, approved screen tubes shall be used.

Grout beneath column base and beam-bearing plates shall be minimum 28-day compressive strength of 7,500 psi,

approved pre-bagged, non-metallic, non-gaseous, bleed free,

non-shrink, when tested in accordance with ASTM C1107

Grade B or C at a flow cone fluid consistency of 20 to 30 seconds

Struc	tural Drawing Index
S-1.0	General Notes, Etc.
S-1.1	Basement/ Foundation Plan
S-1.2	First Floor Framing Plan
S-1.3	Second Floor Framing Plan
S-1.4	Third Floor Framing Plan
S-1.5	Fourth Floor Framing Plan
S-1.6	Roof Framing Plan
S-2.1	Sections
S-2.2	Sections

STRUCTURAL WOOD FRAMING: In-Grade Base Values have been used for design. 2x framing shall be S.P.F. S4S No. 2 and better unless noted. All lumber shall be 19% maximum moisture content, unless noted. Solid timber beams and posts shall be Douglas Fir-Larch No. 1. Studs shall be S.P.F. No. 2 and better. Top and bottom plates shall be S.P.F. No. 2 and better. Wood in contact with concrete shall be pressure-treated Southern Yellow Pine. Conventional light framing shall comply with IBC Section 2308. Except as noted otherwise, minimum nailing shall be provided as specified in IBC Table 2304.9.1 "Fastening Schedule" Plywood and oriented strand board (OSB) floor and roof sheathing shall be APA graded with panel identification index, thickness, and

nailing as noted on the drawings.

joists min. 4" at all floors to tie upper and lower stud walls together. Minimum height of sheathing panels shall be 16" to assure that plates are tied to studs.

or deformed shank) per 16". 12d nails are not acceptable. Provide solid blocking between joists under jamb studs of openings. All roof rafters, joists, trusses, beams shall be anchored to supports with metal framing anchors.

number and type of nails recommended by the manufacturer to develop the rated capacity. Note that heavy-duty hangers and skewed hangers may not be stocked locally and require special order from the factory. All beams and trusses shall be braced against rotation at points of bearing. Unless otherwise indicated, install two lengths of solid blocking x joist depth x 12 inches long in floor framing under column loads. Columns must have a continuous load path to foundation. Lead holes for lag screws shall be drilled in accordance with Table 6.23 of the AITC Timber Construction Manual, 5th edition.

PLANT FABRICATED / PRE-ENGINEERED WOOD FRAMING: Beams noted as ML or LVL on plan shall be 1-3/4" v Shall be plant-fabricated and manufactured by Tru Shall have the following minimum allowable desig

Fb = 2600 psi Fv = 285 psiBeams noted as PSL on plan shall be plant-fabricated Manufactured by Trus-Joist Co./ iLevel or equal,

Fb = 2900 psi Fv = 290 psi Fc(||) = 2900 psi  $Fc(\bot) = 750 \text{ psi}$  E = 2000 ksiBeams noted as TS or LSL on plan shall be plant-fabricated Manufactured by Trus-Joist Co. / iLevel or equal, and have the following minimum allowable design stresses: Fb = 1700 psi Fv = 400 psi Fc(||) = 1400 psi  $Fc(\perp) = 680 \text{ psi}$  E = 1300 ksi

## SHOP DRAWINGS:

Construction Documents are copyrighted and shall not be copied for use as erection plans or shop details. The General Contractor and his subcontractors shall submit in writing any requests to modify the plans or specifications. All shop and erection drawings shall be checked and stamped by the General Contractor prior to submission for Engineer's review. Unchecked submittals will be returned without review. Furnish one (1) reproducible and two (2) prints of shop and erection drawings to the Structural Engineer for review prior to fabrication

Reinforcing steel, Structural steel

Submit in a timely manner to permit ten (10) working days for review. Shop drawings submitted for review do not constitute "in writing" unless specific suggested changes are clearly marked. In any event, such changes by means of the shop drawing submittal process become the responsibility of the one initiating such change.

FIELD VERIFICATION OF EXISTING CONDITIONS: Contractor shall thoroughly inspect and survey existing structure to verify conditions that affect the work shown on the drawings. Contractor shall report any variations or discrepancies to the Owner and SI Inc before proceeding.

# STRUCTURAL ERECTION AND BRACING REQUIREMENTS:

The structural drawings illustrate the completed structure with elements in their final positions, properly supported and braced. These construction documents contain typical and representative details to assist the contractor. Details shown apply at all similar conditions unless otherwise indicated. Although due diligence has been applied to make the drawings as complete as possible, not every detail is illustrated, nor is every exceptional condition addressed.

All proprietary connections shall be installed in accordance with the manufacturers' recommendations. All work shall be accomplished in a workmanlike manner and in accordance with the applicable code and local ordinances. The general contractor is responsible for coordination of all work, including layout and dimension verification, materials coordination, shop drawing review, and the work of subcontractors.

Any discrepancies or omissions discovered in the course of the work shall be immediately reported to the architect for resolution. Continuation of work without notification of discrepancies relieves the architect and engineer from all consequences. Unless otherwise specifically indicated, the drawings do not describe methods of construction. The contractor, in the proper sequence, shall perform or supervise all work necessary to achieve the final completed structure, and to

protect the structure, workmen, and others during construction. adequate bracing is provided.

The architect and engineer bear no responsibility for the above items, and observation visits to the site do not in any way include inspection of them.



Nail wall sheathing with 8d commons at 6" o.c. at panel edges, and 12" o.c. at intermediate framing except as noted. Sheathing shall be continuous from bottom plate to top plate. Cut in "L" and "T" shapes around openings. Lap sheathing over rim

Minimum 3-8d per stud and nail plates with "edge nail" spacing. Sole plate at all perimeter walls and at designated shear walls shall be nailed as for braced panels with 3-16d x 3 1/2" long box nails (coated

Light gage framing anchors shown or required, shall be Simpson "Strong Tie" or equal Code approved connectors and installed with the

wide Laminated Veneer Lun	nber beam	s of the depth not	ed on plan
us-Joist Co./ Ilevel or equal,		-	-
zn stresses:			
Fc(  ) = 2460  psi	$Fc(\perp)$	=750  psi	E = 1800  ks
l			
and have the following mini	mum allo <sup>,</sup>	wable design stres	se s:
$E_{c}( 1 ) = 2900 \text{ nsi}$	$E_{c}(1)$	= 750  psi	E = 2000  ks

Such work shall include, but not be limited to, bracing, shoring for construction equipment, shoring for excavation, formwork, scaffolding, safety devices and programs of all kinds, support and bracing for cranes and other erection equipment. Do not backfill against basement or retaining walls until supporting slabs and floor framing are in place and securely anchored, unless

Temporary bracing shall remain in place until all floors, walls, roofs and any other supporting elements are in place.

77 Oak Street Portland, ME, 04101 p. 207-774-4614	Integrity www.structuralin.teg.com	Consulting Engineers, Inc. BUILD WITH CONFIDENCE © 2005 Supress Line grity Consulting Engineers, Inc.
DATE: 10/17/12	REVISION: AELEVATOR/	SHAFT W/ BASEMENT ACCESS SI# 1
Shwartz Building Renovation	602 Congress St.	Portland, ME 04101





# BASEMENT/FOUNDATION PLAN

NOTES:SCALE 1/4"=1'-0"1. VERIFY ALL EXISTING CONDITIONS IN FIELD, CONTACT SI Inc. IF

- CONDITIONS IN FIELD DO NOT MATCH PLAN 2. IF MEMBER IS NOT INDICATED AS <E>, <N> OR <R> MEMBER IS TO BE CONSIDERED A NEW, <N>, MEMBER
- 3. SEE ARCH. FOR DIMENSIONS AND ELEVATIONS OF NEW FLOOR PLANS
- 4. MECHANICALLY COMPACT ALL SOIL BELOW NEW FOOTINGS -TYP 5. SOILS ENGINEER TO APROVE ALL SUBGRADE PRIOR TO CONST.
- FRAMING PLAN SYMBOLS KEY WOOD POST  $(\mathbf{X})$ NUMBER OF WOOD STUDS IN POST BELOW COLUMN ABOVE THIS LEVEL COLUMN CONTINUOUS THROUGH THIS LEVEL ◄\_\_\_ TRUSS OR JOIST BEARING FLUSH FRAMED JOIST BEARING WITH HANGER INDICATES OVER FRAMING, SEE PLAN WOOD STUD BEARING WALL BELOW SHEAR WALL X''TNUMBER OF TRIM STUDS UNDER HEADER <u>"X"K</u> NUMBER OF KING STUDS ADJACENT TO HEADER <E> INDICATES EXISTING MEMBER TO REMAIN INDICATES NEW MEMBER <N> <R> INDICATES EXISTING MEMBER TO BE REMOVED

REVISION 3: 11/21/13

**S-1.1** 

AN

ΡL

BASEMENT



<u>"X"K</u>

<E>

<N> <R>

- NUMBER OF KING STUDS ADJACENT TO HEADER INDICATES EXISTING MEMBER TO REMAIN
- INDICATES NEW MEMBER
- INDICATES EXISTING MEMBER TO BE REMOVED

**REVISION 3: 11/21/13** 



- 1. VERIFY ALL EXISTING CONDITIONS IN FIELD, CONTACT SI Inc. IF CONDITIONS IN
- FIELD DO NOT MATCH PLAN 2. SEE ARCH. FOR DIMENSIONS AND ELEVATIONS OF NEW FLOOR PLANS
- 3. CONTRACTOR TO SHORE <E> FRAMING AS NEEDED
- 4. EXPOSE BRICK MASONRY PILASTERS @ MAIN LEVEL BELOW AND TUCK POINT ALL
- MASONRY AS NEEDED. ALL MASONRY TO BE SOLID AND SOUND AT COMPLETION OF WORK. 5. 30 TON JACK NEEDED TO MOVE BUILDING WALLS -TYP AT ALL JACKING LOCATIONS.



- LUS28 HGR -TYP @ <E> JOISTS TO <N> LEDGER, SHIM TIGHT AS NEEDED

 $< N > 9\frac{1}{2}$ " LVL LEDGER OVER POLY FASTENED TO <N> CMU WALL W/1/2"Øx6"EMBEDMENT EPOXY SET DOWELS @ 16" O.C. -TYP @ 2 SIDES.

- 8" CMU ELEVATOR SHAFT

- INDICATED INSIDE EDGE OF VERTICAL PLANE OF EXISTING BRICK WALL



					r-
D FLOOR FRAMING PLAN	Shwartz Building Renovation 602 Congress St. Portland, ME 04101	DATE: 12/05/12 SCALE: 1/4"=1'-0" REVISION: A FRONT & REAR OPENING ELEVATOR	Structural Integrity SI# 12-0088	77 Oak Street Portland, ME, 04101 p. 207-774-4614 f. 866-793-7835 www.strncturalin teg.com BUILD WITH CONFIDENCE @ 2005 Structural Integrity Consulting Engineers, Inc.	



# THIRD FLOOR FRAMING PLAN

## NOTES:

SCALE 1/4"=1'-0" 1. VERIFY ALL EXISTING CONDITIONS IN FIELD, CONTACT SI Inc. IF CONDITIONS IN

FIELD DO NOT MATCH PLAN 2. SEE ARCH. FOR DIMENSIONS AND ELEVATIONS OF NEW FLOOR PLANS

3. CONTRACTOR TO SHORE <E> FRAMING AS NEEDED

4. RUN CMU SHAFT LAYOUT VERTICAL TO ALL OTHER LEVELS FROM THIS INFO. IF BUILDING IS OUT OF PLUMB CONTACT SI Inc. AND ARCHITECT.



LU28R-18 HGR -TYP @ <E> JOISTS TO <N> (3) 11 1/2" LVL SHIM TIGHT AS NEEDED

- <N> (3) 11  $\frac{7}{8}$ " LVL LEDGER OVER POLY FASTENED TO CMU SHAFT W/ (2)  $\frac{7}{8}$ " Ø x 6" MIN. EMBED. DEPTH EPOXY SET DWL. LUS28 HGR -TYP @ <N> JOISTS TO <N> LEDGER, SHIM TIGHT AS NEEDED

|<N> (2) 7/8" Ø x 6" MIN. EMBED. DEPTH EPOXY SET DWL INTO GROUTED CMU @ 4" AND 12" FROM CORNER OF CMU SHAFT



Shwartz Building Renovation     DATE: 12/05/12       602 Congress St.     SCALE: 1/4"=1'-0"       FRONT & REAR     SCALE: 1/4"=1'-0"       Portland, ME 04101     OPENING ELEVATOR
Shwartz Building Renovation       DATE: 12/05/12         602 Congress St.       SCALE: 1/4"=1'-0"         Portland, ME 04101       REVISION: A FRONT & REAR
Shwartz Building Renovation 602 Congress St. Portland, ME 04101



# FO<sup>-</sup> NOTES

# FOURTH FLOOR FRAMING PLAN

SCALE 1/4"=1'-0"

1. VERIFY ALL EXISTING CONDITIONS IN FIELD, CONTACT SI Inc. IF CONDITIONS IN FIELD DO NOT MATCH PLAN

SEE ARCH. FOR DIMENSIONS AND ELEVATIONS OF NEW FLOOR PLANS
 CONTRACTOR TO SHORE <E> FRAMING AS NEEDED



LU28R-18 HGR -TYP @ <E> JOISTS TO <N> (3) 11 1/2" LVL SHIM TIGHT AS NEEDED

-<N> (3) 11  $\frac{7}{8}$ " LVL BEAM W/ 3" MIN. BEARING DIST. ON <N> (2) 2x 10 POST

— LUS28 HGR -TYP @ <N> JOISTS TO <N> LEDGER, SHIM TIGHT AS NEEDED

- <N> (2) 2x 10 POST W/ ½" EPOXY SET ANCHOR @ 16" O.C. VERT. SPACING W/ 6" MIN. EMBED. DEPTH INTO GROUTED CMU @ 4" MIN. EDGE DIST. FROM CORNER OF CMU SHAFT

- 8" CMU ELEVATOR SHAFT

— INDICATED INSIDE EDGE OF VERTICAL PLANE OF EXISTING BRICK WALL

– SEE GENERAL NOTES FOR LOOSE LINTELS -TYP





ND BOT @





	<ul> <li>-<e> BRICK MASONRY WALL/ PILASTER TO REMAIN -TYP</e></li> <li>-<n> STOREFRONT SYSTEM PER ARCH -TYP</n></li> <li>-CAP PER ARCH -TYP</li> <li>- FINISH PER ARCH -TYP</li> <li>- BOND BEAM @ TOP W/ (2) #4 BARS -TYP</li> <li>- FLASHING/ WATERPROOFING BY CONTRACTOR -TYP</li> <li>-<n> 8" CMU KNEE WALL W/ FULLY GROUTED CELLS AND #5 VERTS @ 24" EPOXY SET INTO <e> FND WALL 10" MIN</e></n></li> <li>-<e> SIDE WALK TO REMAIN -TYP</e></li> <li>-<e> GRANITE VENEER, RESET AS NEEDED -TYP</e></li> </ul>	AARON C. JONES No. 10968 CENSED SIONAL ENGLISHING 12/05/12	T/Oak Street         Structural         Pordand, ME, 0401         p. 207-174-4614         filtegrity         consulting Engineers. Inc.         SI# 12-0088
	– REPAIR TOP OF <e> FND WALL AS NEEDED TO RECIEVE CMU -TYP – <e> RUBBLE FND WALL TO REMAIN -TYP</e></e>		LR L
3	/4"=1'-0		`& RE∕ R
	– EXISTING ROOF FRAMING TO REMAIN -TYP		2 0" FRONT VATOI
\$	– 2x8 NAILER PLATE W/ 1/2" Ø CARRIAGE BOLTS @ 32"		DATE: 12/05/13 SCALE: 3/4"=1 REVISION: A OPENING ELE
	– <n> W14x30 INSTALLED TIGHT TO BOTTOM OF EXISTING ROOF FRAMING, SEE PLAN FOR LOCATION INFORMATION</n>		
\$	– CUT HOLE IN TOP AND BOTTOM PLATES AND FLOOR SHEATHING W/ 4" Ø HOLE SAW TO ACCOMMODATE 3" Ø PIPE COLUMN		
	– MSTC28 STRAP ACROSS THE FACE OF CUT TOP PLATES @ COLUMN -TYP (1) FACE OF TOP PLATES		tion
	– 3" Ø STD PIPE COLUMN FULL HEIGHT TO <e> DBL S24 @ SECOND LEVEL FLOOR FRAMING</e>		lova 01
	– 2 1/2" Ø x 6" STD PIPE SLEEVE -TYP @ JOINTS IN COL.		Rer ss St. 041(
4	– SEGMENT COLUMN AS NEEDED – <e> FLOOR FRAMING TO REMAIN -TYP</e>		ling mgre I, ME
3/16	THE 2" 2" TWO SIDES MIN - TYP		<b>3uild</b> 02 Cc rtlanc
_			rtz I 6( Po
5			hwa
	− <n> W8 (DROPPED)</n>		$\mathbf{N}$
3	/4"=1'-0		
/			
	3/8		
	— EXISTING 12 13/16" O.D. CONC FILLED STEEL COLUMN TO REMAIN		SZ
			CTIO
	— EXISTING 3" x 2'-0" SQ BASE PLATE TO REMAIN		SE
	$\land \Delta$		
3	/4"=1'-0		S-2.1
		<b>REVISION 3: 11/21/13</b>	



CONSTRUCTION

\* SEE ELEVATOR MANUFACTURER FOR GUIDE RAIL ATTACHMENT

€ OF ELEVATOR HOIST BEAM, VERIFY W/ ELEVATOR

## - 5" CONC SLAB ON 1.0C, 22GA CORRUGATED METAL DECKING W/

LIGHT GAUGE GALV. EDGE FORM

## - <N> 9 1/2" LVL LEDGER OVER POLY FASTENED TO CMU WALL W/ 1/2" Ø EPOXY SET DWL W/ 6" MIN EMBED. INTO HORIZ. BOND BEAM @ 16" O.C. -TYP

AROUND SHAFT PERIMETER @ ROOF

## - <N> 7 1/4" LVL LEDGER OVER POLY FASTENED TO CMU WALL W/ 1/2" Ø EPOXY SET DWL W/ 6" MIN EMBED. @ 16"

O.C. INTO HORIZ. BOND BEAM - TYP AROUND SHAFT PERIMETER @ FLOOR

## -BOND BEAM W/ (2) #4 BAR @ 8' MAX. & @ FLOOR/ROOF CONNECTIONS - TYP.

<sup>-</sup>L6x4x5/16 (LLV) W/ 5/8" Ø x4" W.A. W/  $4\frac{3}{8}$ " MIN. EMBED. INTO GROUTED CELLS @ 3' 6"

# -THRESHOLD BY ELEVATOR

FILL AND LEVELING PLATE,

# - <N> 7 1/4" LVL LEDGER OVER POLY FASTENED TO CMU WALL W/ 1/2" Ø EPOXY SET DWL W/ 6" MIN EMBED. @ 16"

O.C. INTO HORIZ. BOND BEAM - TYP AROUND SHAFT PERIMETER @ FLOOR

## - SEE GENERAL NOTES FOR LOOSE LINTELS -TYP

<E> MASONRY WALL W/ #5 x 10" EPOXY SET DOWELS @ 24", MATCH <N> CMU WALL REINFORCEMENT -



 $<\!\!\mathrm{E}\!\!>$  (2) C8 W/  $<\!\!\mathrm{N}\!\!>$  WELDED 1/2" DIA x 6" THREADED ROD @ 24" CAST INTO

-<N> #5 x 60" DOWELS EPOXY SET 8 " INTO CIP CONCRETE WALL @ 32" REINFORCEMENT IN CMU -TYP