Please Read Application And Notes, If Any,

Attached

This is to certify that $\qquad$ Ne Tel \& Tel Co State \&/No has permission to $\qquad$ Add a 30' Extension to Existi

AT -45 Forest Ave
provided that the person or persons, of the provisions of the Statutes of N the construction, maintenance and $u$ this department.

Apply to Public Works for street line and grade if nature of work requires such information.

OTHER REQUIRED APPROVALS
Fire Dept.
Health Dept. $\qquad$
Appeal Board $\qquad$
Other $\qquad$

Permit Numbergatiatar ION
 hes and 3 Panel Antennas

037 A001001 CHM Mryme m or epuration epting this permit shall comply with all ne and of th_nces of the City of Portland regulating of buildings and si. tures, and of the application on file in


A certificate of occupancy must be procured by owner before this building or part thereof is occupied.

City of Portland, Maine - Building or Use Permit Application 389 Congress Street, 04101 Tel: (207) 874-8703, Fax: (207) 874-8716

| PRPRME $\therefore \therefore \mathrm{O}$ |  |  |
| :---: | :---: | :---: |
| Permit No: 03-0764 | Issue Date: 1f: | $\begin{aligned} & \text { CBL: } \\ & 037 \text { A001001 } \end{aligned}$ |



## CERTIFICATION

I hereby certify that I am the owner of record of the named property, or that the proposed work is authorized by the owner of record and that I have been authorized by the owner to make this application as his authorized agent and I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in the application is issued, I certify that the code official's authorized representative shall have the authority to enter all areas covered by such permit at any reasonable hour to enforce the provision of the code(s) applicable to such permit.

## DATE

PHONE

| City of Portland, Maine - Building or Use Permit 389 Congress Street, 04101 Tel: (207) 874-8703, Fax: (207) 874-8716 |  |  | $\begin{array}{\|l} \hline \text { Permit No: } \\ 03-0764 \end{array}$ | Date Applied For: 06/27/2003 | CBL: 037 A001001 |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Location of Construction: 45 Forest Ave | Owner Na Ne Tel |  | Owner Address: <br> Po Box 152206 |  | Phone: |
| Business Name: | Contracto Nortech |  | Contractor Address: 35 Norton Road T | unton | Phone (503) 880-3663 |
| Lessee/Buyer's Name | Phone: |  | Permit Type: <br> Additions - Comm | ercial |  |
| Proposed Use:  <br> Communication Tower/Commercial Proposed Project Description: <br> Add a 30' Extension to Existing 63' <br> Dishes and 3 Panel Antennas |  |  |  |  |  |
| Dept: Zoning Status: Approved <br> Note: ok under section 14-430 |  | Reviewer: | Marge Schmucka | Approval D | te: 07/01/2003 <br> Ok to Issue: |
| Dept: Building Status: Note: | Approved | Reviewer: | : Mike Nugent | Approval D | te: 07/09/2003 <br> Ok to Issue: |

## Commercial Building Permit Application

## $03-0744$

If you or the property owner owes real estate or personal property taxes or user charges on any property within the City, payment arrangements must be made before permits of any kind are accepted.


Please submit all of the information outlined in the Residential Application Checklist. Failure to do so will result in the automatic denial of your permit.

At the discretion of the Planning and Development Department, additional information may be required prior to permit approval. For further information stop by the Building Inspections office, room 315 City Hall or call 874-8703.

I hereby certify that I am the Owner of record of the named property, or that the owner of record authorizes the proposed work and that I have been authorized by the owner to make this application as his/her authorized agent. I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in this application is issued, I certify that the Code Official's authorized representative shall have the authority to enter all areas covered by this permit at any reasonable hour to enforce the provisions of the codes applicable to this permit.


Date:
Permit Fee: $\$ 39.00$ for the first $\$ 1000.00$ Construction Cost, $\$ 7.00$ per additional $\$ 1000.00$ cost
This is not a Permit; you may not commence any work until the Permit is issued.

# BUILDING PERMIT INSPECTION PROCEDURES Please call 874-8703 or 874-8693 to schedule your inspections as agreed upon 

Permits expire in 6 months, if the project is not started or ceases for 6 months.
The Owner or their designee is required to notify the inspections office for the following inspections and provide adequate notice. Notice must be called in 48-72 hours in advance in order to schedule an inspection:

By initializing at each inspection time, you are agreeing that you understand the inspect/raprocedure and additional fees from a "Stop Work Order" and "Stop Work Order Release" will be incurred if the procedure is not followed as stated below.

Pre-construction Meeting: Must be scheduled with your inspection team upon receipt this permit. Jay Reynolds, Development Review Coordinator at 874-8632 must also be contacted at this time, before any site work begins on any project other than single family additions or alterations.


Framing/Rough Plumbing/Electrical:
Prior to pouring concrete Prior to pouring concrete Prior to placing ANY backfill Prior to any insulating or drywalling

Final/Gertiffeaternancy:
Prior to any occupancy of the structure or use. NOTE: There is a $\$ 75.00$ fee perinspection this point.

Certificate of Occupancy is not required for certain projects. Your inspector can advise you if your project requires a Certificate of Occupancy. All projects DO require a final inspection
$\checkmark$ If any of the inspections do not occur, the project cannot go on to the next phase, REGARDLESS OF THE NOTICE OR CIRCUMSTANCES.
 REFORETHE SPACE MAY BE OCCUPIED


CBL: 037-A - OO 1 Building Permit\#: 03-0764

David S. Ho
Manager - Transactions

Varizon Commumications 125 High Street, Oliver 02339 Boston, MA 02110

June 26, 2003

City of Portland
City Hall
389 Congress Street
Portland, ME 04101
RE: Verizon Wireless Microwave Installation, 45-55 Forest Avenue, Portland. ME

## Dear Sir/Madam:

Please be advised that Verizon Communications is the owner of the property located at 45-55 Forest Avenue, Porland, Maine. It is Verizon Wireless' intention to co-locate antennas at the property. Verizon Wireless is required to obtain all state, local and town approval prior to the installation of the additional equipment.

As owner of the property, permission is hereby granted to Verizon Wireless for the purpose of consummating any applications necessary to gain the required approvals or permits on the tower from the City of Portland


## (A copy of this letter shall have the same effect as the original)

April 15, 2003

Mr. Robert Hogan
(Faxed Copy Sent to 508-330-3405)
Structures Consulting Group, Inc.
43 White Street, Suite 4
Belmont, MA 02478

## Re: Verizon Wireless Building <br> Structural Evaluation <br> 45-55 Congress Street <br> Portland, Maine 04101 <br> Project No. 02180

## Dear Mr. Hogan:

Harriman Associates has completed its review and evaluation of the existing building columns supporting the rooftop antenna tower at 45-55 Congress Street in Porland, Maint. Our evaluation was completed to confirm that additional base loads resulting Irom an antenna tower extension could be supported by the existing building columns. The evaluation concluded that the existing building columns are capable of withstanding the increased base loads caused by the addition of a 30 ft . extension to the existing antenna tower. The analysis a as based primarily on information provided by All-Points Technology Corporation's Structural Anathsis Report of the Antenna Tower as well as original construction drawings and calculations, o! the building Evaluation of the tower and the grillage has been completed by All-Points Technoiogy and is not a part of our scope.

The existing steel columns are built-up sections comprised of plates and angles niveted together. The size of the columns were determined from review of the original drawings as the columns are concrete encased and could not be inspected on site. Each existing cotumn was estended above the roof line approximately $1^{\prime}-6 "$ and capped. The tower grillage has been whached to these column extensiors for transfer of the tower loads into the building. The conisuration of the columns considered in this study, as well as their locations, are outlined in setachment 1 .

From the tower evaluation report, maximum gravity (downward) load, mixamm uplift load, and maximum shear load at the base of the tower was provided. The loads prowded were 7.7 .9 kip , 64.9 kip , and 10.4 kip , respecrively. Also, original calculations docume nted a thoor dead load of 116 pounds per square foot (psf), and design live loads between 150 pst ani 175 psi. A site investigation was conducted to venify the existing information. This invertigenon found that the $1^{\text {st }}$ to $5^{\text {th }}$ floors were mainly abandoned and the $6^{\text {th }}$ floor had electrical data sumpment on it. At this time, it did not appear that the actual floor loads are exceeding the dessen values

Mr. Robert Hogan<br>Page 2<br>April 15, 2003

A complete analysis was performed on the existing columns based on the atwe loads. As stated, the analysis determined that the building columns could withstand the incr-a-d lase loads of the tower plus extension. In order to restrain the anticipated shear loads, the adiuiondil 1" thick stiffener plates installed as part of the grillage framing were required to be used in the calculations. These plates "stiffen" the column section above the roof level and are neces, ary to keep the columns within design stresses. Since these plates are installed and moditi ations to the grillage are not planned, no additional work is required.

As a side note, we discovered a couple items during our research into the existing structure that we believe you would be interested in.

1. The building was originally designed for 9 stories with only, buil:
2. In 1953, the existing building was analyzed for the addition of a 100 it hii cower (the current tower is approximately 65 ft .) The building was found adequate to supper this tower with its particular configuration of antennas. (Note: Any differences or modific tuns wo the antenna configuration would change the resultant loads at the tower base ard $\%$ wid require re-evaluation of the tower, tower support framing, and building columns).

We thank you for the opportunity to assist with this project. If we can be fiurher assistance, please do not hesitate to contact us.

Sincerely,
Harriman Associates


James Fortin, P.E.
jcf
cc: Jim Seymour - Sebago Technics w/encs. (Faxed Copy Sent to 856200

VERIZON WIRELESS BUILDING
45-55 CONGRESS STREET, PORTLAND MAINE ATTACHMENT 1: COLUMN CONFIGURATION TABLE

|  | Column \#56 Column \#55 | Column \#54 Column \# |  |
| :---: | :---: | :---: | :---: |
| 6th Fioor | $12^{\prime \prime} \times 3 / 8^{\prime \prime}$ thick web plate $14^{\prime \prime} \times 3 / 8^{\prime \prime}$ thick flange plates <br> 4- $8 \times 4 \times 3 / 8^{\prime \prime}$ angles <br> Type A | $\begin{gathered} 12^{\prime \prime} \times 3 / 8^{\prime \prime} \text { thick weo plate } \\ 4-6 \times 4 \times 3 / 8^{\prime \prime} \text { angles } \\ \text { Type } \mathrm{D} \end{gathered}$ |  |
| 5th Floor | $12^{\prime \prime} \times 3 / 8^{\prime \prime}$ thick web plate $14^{\prime \prime} \times 3 / 8^{\prime \prime}$ thick flange plates 4- $6 \times 4 \times 3 / 3^{\prime \prime}$ angles Type A | $\begin{gathered} 12^{\prime \prime} \times 3 / 8^{\prime \prime} \text { thick web plate } \\ 4-6 \times 4 \times 3 / 8^{\prime \prime} \text { angles } \\ \text { Type } D \end{gathered}$ |  |
| 4th Fioor | $12^{\prime \prime} \times 3 / 8^{\prime \prime}$ thick web plate $14^{\prime \prime} \times 3 / 8^{\prime \prime}$ thick flange plates 4- $6 \times 4 \times 1 / 2^{2}$ thick angles Type A | $12^{\prime \prime} \times 3 / 8^{\prime \prime}$ thick web plate $14^{\prime \prime} \times 3 / 8^{\prime \prime}$ thick flange plates <br> 4- $6 \times 4 \times 3 / 8^{\prime \prime}$ angles <br> Type A | ' 12 " $\times 3.8$ "rick weo plate <br> $14^{\prime \prime} \times 3$ ह" $\cdot$ wk tange plates <br> $4-5 \times 4 \%$ ngle <br> 1 ype $=$ |
| 3ra Floor | $12^{\prime \prime} \times 3 / 8^{\prime \prime}$ thick web plate $14^{\prime \prime} \times 3 / 8^{\prime \prime}$ thick flange plates <br> 4- $6 \times 4 \times 1 / 2$ " thick angles Type A | $12^{\prime \prime} \times 3 / 8^{\prime \prime}$ thick weo plate $14^{\prime \prime} \times 3 / 8^{\prime \prime}$ thick flange plates 4- $6 \times 4 \times 3 / 8$ " angles Type A | ```12"x \e"max web plate 14"\times3.8":n.ck fagge plates 4-6\times4\times7/2 mogies yoe 4``` |
| 2nd Floor | $12^{\prime \prime} \times 3 / 8^{\prime \prime}$ thick web plate $14^{\prime \prime} \times 1 / 2^{\prime \prime}$ thick flange plates <br> 4- $6 \times 4 \times 1 / 2^{\prime \prime}$ thick angles Type A | $12^{\prime \prime} \times 3 / 8^{\prime \prime}$ thick web plate $14^{\prime \prime} \times 3 / 8^{\prime \prime}$ thick flange plates <br> 14- $6 \times 4 \times 1 / 2^{\prime \prime}$ angles <br> Type A | $12^{\prime \prime} \times 5$ sis" trak web plate <br> 14 " $\times 12$ 'mek flange plates <br> c. $6 \times 4 \times 1$ "? angles <br> - yoe $A$ |
| 1st Floor | $12^{\prime \prime} \times 3 / 8^{\prime \prime}$ thick web plate $14^{\prime \prime} \times 1 / 2^{\prime \prime}$ thick flange plates $14^{\prime \prime} \times 3 / 8^{\prime \prime}$ ihick flange plates <br> 4- $6 \times 4 \times 1 / 2$ " thick angles <br> Type B | $12^{\prime \prime} \times 3 / 8^{\prime \prime}$ thick web plate 14 " $\times 3 / 8$ " thick tange plates $14^{\prime \prime} \times 3 / 8$ " thick flange plates <br> 4. $6 \times 4 \times 1 / 2^{\prime \prime}$ angies <br> Type 8 | $12^{\prime \prime} \times 5 /{ }^{-1}$ " wk web plate <br>  <br> $14^{\prime \prime} \times 39^{\prime \prime}$ : farge piates <br> 14-6x, <br> ye E |
| Basement Leve! | $12^{\prime \prime} \times 5 / 8^{\prime \prime}$ thick web plate $14^{\prime \prime} \times 1 / 2^{\prime \prime}$ thick flange plates $14^{\prime \prime} \times 3 / 8$ " thick flange plates $14^{\prime \prime} \times 3 / 8^{\prime \prime}$ thick flange plates 4- $6 \times 4 \times 1 / 2$ " thick angles Type C | $12^{\prime \prime} \times 5 / 8$ " thick web plate $14^{\prime \prime} \times 3 / 8^{\prime \prime}$ thick flange plates $14^{\prime \prime} \times 3 / 8^{\prime \prime}$ thick flange plates 4. $6 \times 4 \times 1 / 2$ angles <br> Type 8 |  |

Note: Type A to Type D are schematic representations of the column configuration and are no: to scale



Tyoe E




Picture $1:$ View of antonna and building from Forest Avenue.


Picture 2: 6th floor area.


Picture 3: Typical floor in area being evaluated.

# ALL-POINTS TECHNOLOGY CORPORATION, P.C. 

## STRUCTURAL ANALYSIS REPORT 63' SELF-SUPPORTING TOWER PORTLAND, MAINE

Prepared for
Structure Consulting Group, Inc.

September 5, 2002


APT Project \#ME147110

# STRUCTURAL ANALYSIS REPORT <br> of <br> 63' SELF-SUPPORTING TOWER <br> PORTLAND, MAINE <br> prepared for <br> Structure Consulting Group 

## EXECUTIVE SUMMARY:

All-Points Technology Corp., P.C. (APT) performed an inspection and structural analysis of this 63 -foot self-supporting roof-top tower. The analysis was performed with the addition of a 30 -foot extension, six microwave dishes, and three square panel antennas. Five existing inactive conical horn antennas were assumed to be removed from the tower. Feed lines, support members, and appurtenances associated with the horns were also assumed to be removed. Existing platforms were assumed to remain in place.

Our analysis indicates the tower is capable of supporting the proposed extension and antennas. Evaluation of the building structure to support tower reactions is pending receipt of building drawings.

## INTRODUCTION:

An inspection and structural analysis was performed on the above-mentioned communications tower by All-Points Technology Corporation, P.C. (APT) for Structure Consulting Group. The tower is located on the roof of the building at 45-55 Forest Avenue in Portland, Maine.

Robert E. Adair, P.E. inspected the tower on August 13, 2002 to record information regarding physical and dimensional properties of the structure and its appurtenances. Mr. Adair climbed the structure in its entirety to compile data necessary to perform the structural analysis.

The structure is a 63 -foot painted galvanized steel, self-supporting tower of unknown manufacturer (possibly Andrew or LeBlanc). The analysis was performed with a 30 -foot tower extension and the following antenna inventory:

## All-Points Technology Corporation, P.C.

| Antenna | Elev. | Mount | Coax. |
| :---: | :---: | :---: | :---: |
| 6' high performance dish | 180' | Pipe | EW-52 |
| 1' square panel | 180, | Pipe | 1-5/8" |
| 6' dish, no radome | 175' | Pipe | EW-63 |
| 6' high performance dish | 170, | Pipe | EW-90 |
| 7' omnidirectional whip | 170 ' | Wide flange extension | 7/8" |
| 2' square panel | 165' | Pipe | 1-5/8" |
| (2) omnidirectional antennas | 162' | On platform face | (2) $1 / 2{ }^{\prime \prime}$ |
| 8' omnidirectional whip | 161, | Pipe extension | 7/8" |
| Super Stationmaster whip | 160' | Pipe extension | 7/8" |
| 6' high performance dish | 160' | Pipe | EW-52 |
| 2' square panel | 160' | Pipe | 1-5/8" |
| 2' high performance dish | 158' | 5' x 6' frame on platform | (2) $3 / 8$ " |
| 1' square panel | 158' | On above frame | 3/8" |
| 12' high performance dish | 157' | $6^{\prime} \times 3$ ' frame on platform | 2-1/2" solid |
| 7' omnidirectional whip | 156, | Pipe extension | 1/2" |
| Quad yagi | 156' | Pipe on platform | 7/8" |
| Dual yagi | 152' | Platform | 1/2" |
| 6' dish with radome | 150, | Pipe | 1-5/8" |
| Dual yagi | 149, | Platform | 7/8" |
| Dual yagi | 144, | Pipe on platform | 7/8" |
| 12' high performance dish | 144, | Platform | 2-1/2" solid |
| 4' dish with radome | 127, | Pipe | EW-90 |
| 12' high performance dish | 111' | Platform | EW-52 |

Elevations listed are above ground level and assumes tower base is $87^{\prime}$ AGL. Proposed antennas are depicted in bold text.

## FIELD INSPECTION:

- General Condition: The tower, a galvanized steel structure, appeared to be in very good condition. No signs of movement or overstress of the tower were observed.
- Bolted Connections of Lattice Bracing: Connections were visually inspected to the maximum extent practicable. All connections that were observed appeared to be sound, with no loose or missing bolts noted.

All-Points Technology Corporation, P.C.

- Antenna Connections: Antenna mounting hardware was in generally good condition, with corrosion-resistant hardware and galvanized members prevalent.
- Splice Connections: Observed splice connections were in good condition. One missing splice bolt was observed at the $100^{\prime}$ elevation (13' elev. on tower) on the climbing leg. The remaining splice bolts at this elevation are within allowable loads.


## STRUCTURAL ANALYSIS:

## Methodology:

The structural analysis was done in accordance with EIA/TIA-222-F, Structural Standards for Steel Antenna Towers and Antenna Supporting Structures; and the American Institute of Steel Construction (AISC), Manual of Steel Construction, Allowable Stress Design, Ninth Edition.

The analysis was conducted using a wind speed of 80 miles per hour and one-half inch of radial ice over the entire structure and all appurtenances. The EIA/TIA Standard requires a minimum wind speed of 80 miles per hour for Cumberland County, Maine. The tower was analyzed by calculating the resultant wind loading and associated maximum bending moments, shear forces, and axial loads. The moments and forces were used to calculate stresses in leg and bracing members, which were compared to allowable stresses according to AISC.

Two loading conditions were evaluated in accordance with EIA/TIA-222-F to determine the tower's capacity. The more demanding of the two cases is used to calculate the tower capacity:

- Case $1=$ Wind Load (without ice) + Tower Dead Load
- Case $2=0.75$ Wind Load (with ice) + Ice Load + Tower Dead Load

In addition, the TIA/EIA standard permits a one-third increase in allowable stresses for towers less than 700 -feet tall. Allowable stresses of tower members were increased by one-third when computing the load capacity values shown below.

## All-Points Technology Corporation, P.C.

## Analysis:

Analysis of the tower was conducted in accordance with the criteria outlined herein with proposed antennae as previously described.

Our analysis determined the existing tower is capable of supporting the extension and additional proposed antennas. The following table summarizes the results of the analysis based on compressive stresses of individual leg members:

## Tower Capacity

| Elevation | Capacity |
| :---: | :---: |
| $0-13^{\prime}$ | $\mathbf{4 9 \%}$ |
| $13^{\prime}-25^{\prime}$ | $\mathbf{4 1 \%}$ |
| $25^{\prime}-38^{\prime}$ | $\mathbf{3 2 \%}$ |
| $38^{\prime}-50^{\prime}$ | $\mathbf{3 1 \%}$ |
| $50^{\prime}-63^{\prime}$ | $\mathbf{1 9 \%}$ |
| $63^{\prime}-78^{\prime}{ }^{\prime}$ | $\mathbf{1 8 \%}$ |
| $78^{\prime}-93^{\prime}$ | $\mathbf{6 \%}$ |

## Bracing Members:

Bracing is generally installed in a X-brace configuration, with each compression member paired with a corresponding tension member. Diagonal bracing was evaluated by calculating bracing members' allowable compression and tension forces and assessing each tower section's ability to resist calculated shear forces.

Bracing members were determined to be adequate to support the proposed loads.

## Base Support:

Evaluation of the existing base support frame, comprised of MC18 $\times 58$ channels and W16 x 31 steel beams, was also performed. Our calculations indicate the base frame easily supports reactions generated by the tower with the proposed extension and antennas.

Base reactions imposed with the additional antennas were calculated as follows:

[^0]| Tension: | 64.9 kips |
| :--- | ---: |
| Compression: | 77.9 kips |
| Total Shear: | 41.6 kips |

## CONCLUSIONS AND RECOMMENDATIONS:

Our structural analysis indicates the 63 -foot self-supporting roof-top tower located at 4555 Forest Avenue in Portland, Maine is capable of supporting the 30 -foot tower extension and antenna loading proposed by Verizon Wireless.

## LIMITATIONS:

This analysis is based on the tower being properly installed, members in new condition, required members in place, and required bolts in place. The tower inspection conducted on August 13, 2002 confirmed these conditions.

All-Points Technology Corporation, P.C. (APT) is not responsible for any modifications completed prior to or hereafter which APT is not or was not directly involved. Modifications include but are not limited to:

1. Replacing or strengthening bracing members.
2. Reinforcing vertical members in any manner.
3. Adding or relocating stabilizers.
4. Installing antenna mounting gates or side arms.
5. Extending tower.

APT hereby states that this document represents the entire report and that it assumes no liability for any factual changes that may occur after the date of this report. All representations, recommendations, and conclusions are based upon the information contained and set forth herein. If you are aware of any information which is contrary to that which is contained herein, or you are aware of any defects arising from the original design, material, fabrication and erection deficiencies, you should disregard this report and immediately contact APT. APT disclaims all liability for any representation, recommendation, or conclusion not expressly stated herein.

## All-Points Technology Corporation, P.C.

150 Old Westside Road North Conway, NH 03860
(603) 356-5214

711 North Mountain Road Newington, CT 06111
(860) 953-4444

## Appendix A

Tower Drawings

| All-Points Technology Corp., P.C. | EXISTING TOWER |  | veri Onvieless | VERIZON PROJECT\# |
| :---: | :---: | :---: | :---: | :---: |
| 150 OLD WESTSIDE ROAD NORTH CONWAY, NH 03860 PHONE/FAX: (603) 356-5214 MOBILE: (603) 496-5853 www.allipointstech.com | SHEET: 1 OF 1 |  |  |  |
|  | SCALE: $1^{\prime \prime}=10^{\prime}$ | DRAWN BY: REA | g Group | 45-55 FOREST AVENUE |
|  | DATE: 21 AUG 02 | APT JOB \#ME1471 | 43 White Street, Suite 4 | PORTLAND, MAINE |



Scale: $1^{1 "=10}$

| All-Points Technology Corp., P.C. <br> 150 OLD WESTSIDE ROAD NORTH CONWAY, NH 03860 PHONE/FAX: (603) 356-5214 MOBILE: (603) 496-5853 www.allpointstech.com | TOWER EXTENSION |  | veri onwreess <br> Structure Consulting Group 43 White Street, Suite 4 Belmont, MA 02478 | VERIZON PROJECT \# |
| :---: | :---: | :---: | :---: | :---: |
|  | SHEET: 1 OF 1 |  |  | 93' SELF-SUPPORTING TOWER 45-55 FOREST AVENUE PORTLAND, MAINE |
|  | SCALE: $1^{\prime \prime}=15^{\circ}$ | DRAWN BY: REA |  |  |
|  | DATE: 21 AUG 02 | APT JOB \#ME147110 |  |  |



## Appendix B

Photographs

63' SELF-Supporting TOWER
Portland, Maine


Photo showing overview of $63^{\prime}$ rooftop self-supporting tower.

63' SELF-SUPPORTING TOWER Portland, Maine


Photo of tower base and support frame.

## Appendix C

Calculations

```
All-Points Technology Corp., P.C.
150 Old Westside Road
North Conway, NH }0386
(603) 356-5214
```

Client: Structure Consulting Group
Job: Portland, ME Job No.: ME147110
Calculated By: R. Adair
Date: 19-Aug-02

General Information

| Tower Manufacturer | Andrew? |
| :---: | :---: |
| Tower Type | Self-supporting Tower |
| Total Height of Tower | 180 ft . |
| Wind Speed EIA-TIA: Cumberland County | 80 mph . |
| Radial Ice | 0.5 in. |
| 25\% Reduction for ice | yes (yes or no) |
| $1 / 3$ increase for allowable loads | yes (yes or no) |
| Number of faces | 4 faces |
| Antenna Force Calculations based on EIATTIA-222-F, using the following formulas: |  |
| Force on discrete appurtenance: $\mathrm{F}=\mathrm{Qz}^{*} \mathrm{Gh}^{*} \mathrm{Ca}^{*} \mathrm{~A}$ |  |
| Force on microwave antennae: $\mathrm{F}=\mathrm{Cr}^{*} \mathrm{~A}^{*} \mathrm{Gh}^{*} \mathrm{~K} z^{*} \mathrm{~V}^{\wedge} 2$, where $\mathrm{Cr}=\left((\mathrm{Ca} 2)+\left(\mathrm{Cs}^{\wedge} 2\right)\right)^{\wedge}(1 / 2)$ |  |
| $\mathrm{Gh}=.65+.60 /(\mathrm{h} / 33)^{\wedge}(1 / 7)=$ | $\mathrm{Gh}=1.12$ |
| $V$ as specified EIA-222-F |  |
| Fy | 36 ksi |
| E (Modulus of Elasticity) | 29000 ksi |
| Fb | 0.6 |
| K | 1 |
| Tower taper | $0.15 \mathrm{ft} / \mathrm{ft}$ |


| Section No. | Section Length | Leg Spread @ Base of section | Leg Size (Description) | Width of Leg to Wind | Leg Properties |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | Area | $\mathrm{r}_{2}$ | Unbraced Lengths |
| Bldg | 87.0 | 50.00 |  |  |  |  |  |
| 1 | 12.5 | 22.00 | $6 \times 6 \times 5 / 8$ | 6.00 | 7.11 | 1.180 | 78 |
| 2 | 12.5 | 20.15 | $6 \times 6 \times 5 / 8$ | 6.00 | 7.11 | 1.180 | 78 |
| 3 | 12.5 | 18.30 | $6 \times 6 \times 5 / 8$ | 6.00 | 7.11 | 1.180 | 78 |
| 4 | 12.5 | 16.45 | $6 \times 6 \times 1 / 2$ | 6.00 | 5.75 | 1.180 | 81 |
| 5 | 12.5 | 14.60 | $6 \times 6 \times 1 / 2$ | 6.00 | 5.75 | 1.180 | 65 |
| 6 | 15.0 | 12.75 | $6 \times 6 \times 5 / 16$ | 6.00 | 3.65 | 1.200 | 90 |
| 7 | 15.0 | 12.75 | $5 \times 5 \times 5 / 16$ | 5.00 | 3.03 | 0.994 | 90 |
| Top |  | 12.75 |  |  |  |  |  |
|  | 180 |  |  |  |  |  |  |

All-Points Technology Corp., P.C.
150 Old Westside Road
North Conway, NH 03860
(603) 356-5214

Client:
Structure Consulting Group
Job:
Portland, ME
Job No.: ME147110
R. Adair

Date: 19-Aug-02
Tower Summary

| Section | 1 | type |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathrm{Ag}=$ | 270 | sf |  | 93.25 | $f$ | Wt (bs) |  |  |
|  | Quantity Per |  |  |  |  |  |  |  |
|  | Face | Length (ft.) | Width (in.) | Area (sf) | Area w/ ice | Wt. Per ft. | Tower | Wt. (lbs.) Ice |
| Round Members |  |  |  |  |  |  |  |  |
|  | 0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
|  |  |  |  | 0.0 | 0.0 |  | 0.0 | 0.0 |
| Flat Members |  |  |  |  |  |  |  |  |
| Leg | 2 | 12.5 | 6.0 | 12.5 | 14.6 | 24.2 | 1209.7 | 252.8 |
| Diagonal | 2 | 16.3 | 3.5 | 9.5 | 12.2 | 11.6 | 1514.9 | 812.6 |
| Horizontal | 1 | 19.7 | 3.0 | 4.9 | 6.6 | 9.0 | 707.4 | 427.9 |
| Sub-Diagonal | 2 | 6.9 | 2.5 | 2.9 | 4.0 | 4.1 | 227.0 | 258.3 |
| Sub-Horizontal | 2 | 4.5 | 2.0 | 1.5 | 2.3 | 2.4 | 88.0 | 140.3 |
| Section | 2 |  |  |  |  |  |  |  |
| $\mathrm{Ag}=$ | 247 | sf |  | 105.75 |  |  |  |  |
|  |  |  |  |  |  |  | Wt. (lbs.) |  |
|  | 3 | Length (ft.) | Width (in.) | Area (sf) | Area w/ ice | Wt. Per ft. | Tower | Wt. (lbs.) Ice |
| Round Members |  |  |  |  |  |  |  |  |
|  | 0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
|  |  |  |  | 0.0 | 0.0 |  | 0.0 | 0.0 |
| Flat Members |  |  |  |  |  |  |  |  |
| Leg | 2 | 12.5 | 6.0 | 12.5 | 14.6 | 24.2 | 1209.7 | 252.8 |
| Diagonal | 2 | 22.4 | 3.0 | 11.2 | 15.0 | 9.0 | 1615.1 | 977.0 |
| Horizontal | 1 | 17.8 | 3.0 | 4.5 | 5.9 | 9.0 | 640.8 | 387.6 |
| Horizontal | 1 | 18.7 | 2.0 | 3.1 | 4.7 | 3.2 | 238.9 | 291.3 |
| Vertical | 1 | 6.5 | 2.5 | 1.4 | 1.9 | 4.1 | 106.6 | 121.3 |
| Section | 3 |  |  |  | typ |  |  |  |
| $\mathrm{Ag}=$ | 223 | sf | z | 118 |  |  |  |  |
|  | Quantity Per |  |  |  |  |  | Wt. (lbs.) |  |
|  | Face | Length (ft.) | Width (in.) | Area (sf) | Area w/ ice | Wt. Per ft. | Tower | Wt. (lbs.) Ice |
| Round Members |  |  |  |  |  |  |  |  |
|  | 0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
|  |  |  |  | 0.0 | 0.0 |  | 0.0 | 0.0 |
| Flat Members |  |  |  |  |  |  |  |  |
| Leg | 2 | 12.5 | 6.0 | 12.5 | 14.6 | 24.2 | 1209.7 | 252.8 |
| Diagonal | 2 | 20.9 | 3.0 | 10.5 | 13.9 | 9.0 | 1505.1 | 910.5 |
| Horizontal | 1 | 17.8 | 3.0 | 4.5 | 5.9 | 9.0 | 640.8 | 387.6 |
| Horizontal | 1 | 16.9 | 2.0 | 2.8 | 4.2 | 3.2 | 215.3 | 262.5 |
| Vertical | 1 | 6.5 | 2.5 | 1.4 | 1.9 | 4.1 | 106.6 | 121.3 |

All-Points Technology Corp., P.C.
150 Old Westside Road
North Conway, NH 03860
(603) 356-5214

Client:
Structure Consulting Group
Job:
Portland, ME
Job No.: ME147110
Date: 19-Aug-02


All-Points Technology Corp., P.C.
150 Old Westside Road
North Conway, NH 03860
(603) 356-5214

| Client: | Structure Consulting Group |  |
| :--- | :--- | ---: |
| Job: | Portland, ME | Job No.: ME147110 |
| Calculated By: | R. Adair | Date: 19-Aug-02 |
| Antennas |  |  |



| 6' HP dish | 180 | 0.00324 | 1.62 | 26.60 | 28.3 | 29.5 | 1068 | 1113 | 250 | 500 | 228 | 0.0032 | 0.0003 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 6' HP dish | 160 | 0.00324 | 1.57 | 25.72 | 28.3 | 29.5 | 1033 | 1076 | 250 | 500 | 228 | 0.0032 | 0.0003 |
| 4 ' dish w/radome | 127 | 0.00097 | 1.47 | 24.08 | 14.2 | 14.8 | 146 | 152 | 150 | 250 | 330 | 0.0003 | 0.0009 |
| 6. HP dish | 170 | 0.00324 | 1.60 | 26.17 | 28.3 | 29.5 | 1052 | 1096 | 250 | 500 | 230 | 0.0032 | 0.0003 |
| 6 6' dish w/radome | 150 | 0.00145 | 1.54 | 25.25 | 28.3 | 29.5 | 452 | 471 | 250 | 500 | 287 | 0.0008 | 0.0012 |
| 2' square panel | 165 | 1.4 | 1.58 | 25.95 | 4.0 | 4.3 | 163 | 177 | 50 | 100 |  |  |  |
| 2' square panel | 160 | 1.4 | 1.57 | 25.72 | 4.0 | 4.3 | 161 | 175 | 50 | 100 |  |  |  |
| 1' square panel | 180 | 1.4 | 1.62 | 26.60 | 1.0 | 1.2 | 42 | 49 | 25 | 50 |  |  |  |
| 6 ' dish | 175 | 0.00228 | 1.61 | 26.39 | 28.3 | 29.5 | 745 | 776 | 250 | 500 | 13 | 0.0021 | 0.0009 |

Antenna Info.

All-Points Technology Corp., P.C.
150 Old Westside Road
North Conway, NH 03860
(603) 356-5214

| Client: | Structure Consulting Group |  |  |
| :--- | :--- | ---: | ---: |
| Job: | Portland, ME | Job No.: | ME147110 |
| Calculated By: | R. Adair | Date: | 19-Aug-02 |

Existing Wind Load Without Ice

| Section | Midpoint Height | Areas |  |  |  |  | Factors |  |  | Rr | Kz | Qz | Gh | e | Cf | Wind | Load | Section Length | Uniform | Load |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Gross | Flats | Rounds | Ae | Aa | Df | Dr | Ca |  |  |  |  |  |  |  |  |  |  |  |
| 1 | 93.25 | 269.7 | 31.3 | 0.0 | 31.3 | 17.68 | 1 | 1 | 1.2 | 0.58 | 1.35 | 22.05 | 1.12 | 0.12 | 2.90 | 2769 | lbs. | 12.5 | 222 | lbs/ft. |
| 2 | 105.75 | 246.6 | 32.6 | 0.0 | 32.6 | 17.59 | 1 | 1 | 1.2 | 0.58 | 1.39 | 22.85 | 1.12 | 0.13 | 2.84 | 2913 | lbs. | 12.5 | 233 | $\mathrm{lbs} / \mathrm{ft}$. |
| 3 | 118.25 | 223.4 | 31.6 | 0.0 | 31.6 | 16.51 | 1 | 1 | 1.2 | 0.58 | 1.44 | 23.59 | 1.12 | 0.14 | 2.80 | 2865 | lbs. | 12.5 | 229 | lbs/ft. |
| 4 | 130.75 | 200.3 | 31.7 | 0.0 | 31.7 | 16.51 | 1 | 1 | 1.2 | 0.58 | 1.48 | 24.28 | 1.12 | 0.16 | 2.74 | 2903 | lbs. | 12.5 | 232 | $\mathrm{lbs} / \mathrm{ft}$. |
| 5 | 143.25 | 177.2 | 47.5 | 0.0 | 47.5 | 15.38 | 1 | 1 | 1.2 | 0.61 | 1.52 | 24.92 | 1.12 | 0.27 | 2.38 | 3681 | lbs. | 12.5 | 294 | lbs/ft. |
| 6 | 157 | 198.8 | 34.5 | 0.0 | 34.5 | 11.82 | 1 | 1 | 1.2 | 0.59 | 1.56 | 25.58 | 1.12 | 0.17 | 2.69 | 3064 | lbs. | 15.0 | 204 | lbs/ft. |
| 7 | 172 | 197.5 | 32.0 | 0.0 | 32.0 | 5.51 | 1 | 1 | 1.2 | 0.58 | 1.60 | 26.26 | 1.12 | 0.16 | 2.73 | 2764 | lbs. | 15.0 | 184 | lbs/ft. |

Existing Wind Load With Ice

| Section | Midpoint Height | Areas |  |  |  |  | Factors |  |  | Rr | Kz | Qz | Gh | e | Cf | Wind | Load | Section Length | Uniform | Load |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Gross | Flats | Rounds | Ae | Ai | Df | Dr | Ca |  |  |  |  |  |  |  |  |  |  |  |
| 1 | 93.25 | 269.7 | 39.7 | 0.0 | 39.7 | 30.70 | 1 | 1 | 1.2 | 0.58 | 1.35 | 22.05 | 1.12 | 0.15 | 2.78 | 3638 |  | 12.5 | 291 | lbs/ft. |
| 2 | 105.75 | 246.6 | 42.0 | 0.0 | 42.0 | 30.57 | 1 | 1 | 1.2 | 0.58 | 1.39 | 22.85 | 1.12 | 0.17 | 2.70 | 3845 | lbs. | 12.5 | 308 | lbs/ft. |
| 3 | 118.25 | 223.4 | 40.6 | 0.0 | 40.6 | 29.01 | 1 | 1 | 1.2 | 0.59 | 1.44 | 23.59 | 1.12 | 0.18 | 2.66 | 3773 | lbs. | 12.5 | 302 | lbs/ft. |
| 4 | 130.75 | 200.3 | 40.1 | 0.0 | 40.1 | 29.01 | 1 | 1 | 1.2 | 0.59 | 1.48 | 24.28 | 1.12 | 0.20 | 2.59 | 3781 | lbs. | 12.5 | 302 | lbs/ft. |
| 5 | 143.25 | 177.2 | 55.3 | 0.0 | 55.3 | 27.15 | 1 | 1 | 1.2 | 0.62 | 1.52 | 24.92 | 1.12 | 0.31 | 2.26 | 4407 | lbs. | 12.5 | 353 | lbs/ft. |
| 6 | 157 | 198.8 | 43.5 | 0.0 | 43.5 | 20.13 | 1 | 1 | 1.2 | 0.59 | 1.56 | 25.58 | 1.12 | 0.22 | 2.53 | 3854 | lbs. | 15.0 | 257 | lbs/ft. |
| 7 | 172 | 197.5 | 41.0 | 0.0 | 41.0 | 7.22 | 1 | 1 | 1.2 | 0.59 | 1.60 | 26.26 | 1.12 | 0.21 | 2.57 | 3357 | lbs. | 15.0 | 224 | lbs/ft. |

## All-Points Technology Corp., P.C.

150 Old Westside Road
North Conway, NH 03860
(603) $356-5214$

| Client: | Structure Consulting Group |  |  |
| :--- | :--- | ---: | :--- |
| Job: | Portland, ME | Job No.: | ME147110 |
| Calculated By: | R. Adair | Date: | 19-Aug-02 |

## Proposed Wind Load Without Ice

| Section | $\begin{array}{r\|} \hline \begin{array}{c} \text { Midpoint } \\ \text { Height } \end{array} \\ \hline \end{array}$ | Areas |  |  |  |  | Factors |  |  | Rr | Kz | Qz | Gh | e | Cf | Wind | Load | Section Length | Uniform | Load |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Gross | Flats | Rounds | Ae | Aa | Df | Dr | Ca |  |  |  |  |  |  |  |  |  |  |  |
| 1 | 93.25 | 269.7 | 31.3 | 0.0 | 31.3 | 26.57 | 1 | 1 | 1.2 | 0.58 | 1.35 | 22.05 | 1.12 | 0.12 | 2.90 | 3032 |  | 12.5 | 243 | lbs/ft. |
| 2 | 105.75 | 246.6 | 32.6 | 0.0 | 32.6 | 26.48 | 1 | 1 | 1.2 | 0.58 | 1.39 | 22.85 | 1.12 | 0.13 | 2.84 | 3186 | lbs. | 12.5 | 255 | lbs/ft. |
| 3 | 118.25 | 223.4 | 31.6 | 0.0 | 31.6 | 25.40 | 1 | 1 | 1.2 | 0.58 | 1.44 | 23.59 | 1.12 | 0.14 | 2.80 | 3147 |  | 12.5 | 252 | $\mathrm{lbs} / \mathrm{ft}$. |
| 4 | 130.75 | 200.3 | 31.7 | 0.0 | 31.7 | 24.85 | 1 | 1 | 1.2 | 0.58 | 1.48 | 24.28 | 1.12 | 0.16 | 2.74 | 3176 | lbs. | 12.5 | 254 | $\mathrm{lbs} / \mathrm{ft}$. |
| 5 | 143.25 | 177.2 | 47.5 | 0.0 | 47.5 | 23.59 | 1 | 1 | 1.2 | 0.61 | 1.52 | 24.92 | 1.12 | 0.27 | 2.38 | 3956 |  | 12.5 | 316 | $\mathrm{lbs} / \mathrm{ft}$. |
| 6 | 157 | 198.8 | 34.5 | 0.0 | 34.5 | 19.67 | 1 | 1 | 1.2 | 0.59 | 1.56 | 25.58 | 1.12 | 0.17 | 2.69 | 3335 | lbs. | 15.0 | 222 | $\mathrm{lbs} / \mathrm{ft}$. |
| 7 | 172 | 197.5 | 32.0 | 0.0 | 32.0 | 9.38 | 1 | 1 | 1.2 | 0.58 | 1.60 | 26.26 | 1.12 | 0.16 | 2.73 | 2900 | lbs. | 15.0 | 193 | $\mathrm{lbs} / \mathrm{ft}$. |

## Proposed Wind Load With Ice

| Section | Mid point Height | Areas |  |  |  |  | Factors |  |  | Rr | Kz | Qz | Gh | e | Cf | Wind | Load | Section <br> Length | Uniform | Load |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Gross | Flats | Rounds | Ae | Ai | Df | Dr | Ca |  |  |  |  |  |  |  |  |  |  |  |
| 1 | 93.25 | 269.7 | 39.7 | 0.0 | 39.7 | 44.28 | 1 | 1 | 1.2 | 0.58 | 1.35 | 22.05 | 1.12 | 0.15 | 2.78 | 4040 | lbs. | 12.5 | 323 | lbs/ft. |
| 2 | 105.75 | 246.6 | 42.0 | 0.0 | 42.0 | 44.15 | 1 | 1 | 1.2 | 0.58 | 1.39 | 22.85 | 1.12 | 0.17 | 2.70 | 4262 | lbs. | 12.5 | 341 | lbs/ft. |
| 3 | 118.25 | 223.4 | 40.6 | 0.0 | 40.6 | 42.59 | 1 | 1 | 1.2 | 0.59 | 1.44 | 23.59 | 1.12 | 0.18 | 2.66 | 4204 | lbs. | 12.5 | 336 | lbs/ft. |
| 4 | 130.75 | 200.3 | 40.1 | 0.0 | 40.1 | 41.62 | 1 | 1 | 1.2 | 0.59 | 1.48 | 24.28 | 1.12 | 0.20 | 2.59 | 4193 |  | 12.5 | 335 | lbs/ft. |
| 5 | 143.25 | 177.2 | 55.3 | 0.0 | 55.3 | 39.52 | 1 | 1 | 1.2 | 0.62 | 1.52 | 24.92 | 1.12 | 0.31 | 2.26 | 4822 | lbs. | 12.5 | 386 | lbs/ft. |
| 6 | 157 | 198.8 | 43.5 | 0.0 | 43.5 | 32.00 | 1 | 1 | 1.2 | 0.59 | 1.56 | 25.58 | 1.12 | 0.22 | 2.53 | 4262 | lbs. | 15.0 | 284 | $\mathrm{lbs} / \mathrm{ft}$. |
| 7 | 172 | 197.5 | 41.0 | 0.0 | 41.0 | 13.02 | 1 | 1 | 1.2 | 0.59 | 1.60 | 26.26 | 1.12 | 0.21 | 2.57 | 3562 | lbs. | 15.0 | 237 | lbs/ft. |

## All-Points Technology Corp., P.C.

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Client:
Job:
Calculated By:

Structure Consulting Group
Portland, ME
R. Adair

Job No.: ME147110
Date: 19-Aug-02

## Uplift Due to Moment Minus 1/4 Dead \& Ice Loads

|  | Existing $W_{o}-D L$ | .75Wi-DL-1 | W 1 -DL-I | Proposed <br> $\mathrm{W}_{0}$-DL | . $75 \mathrm{~W}_{1}$-DL-I | W1-DL-I |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Elev. | Force | Force | Force | Force | Force | Force |
| 87 | 49.9 | 38.5 | 55.8 | 64.9 | 51.0 | 72.6 |
| 100 | 40.2 | 30.3 | 44.4 | 53.7 | 41.5 | 59.4 |
| 112 | 30.5 | 22.7 | 33.4 | 42.3 | 32.4 | 46.4 |
| 125 | 21.4 | 15.1 | 22.9 | 31.3 | 23.3 | 33.7 |
| 137 | 12.4 | 8.2 | 12.9 | 20.2 | 14.5 | 21.3 |
| 150 | 4.9 | 2.8 | 4.9 | 10.2 | 7.0 | 10.5 |
| 165 | 0.6 | 0.1 | 0.4 | 2.4 | 1.5 | 2.3 |

## Tension in Anchor Bolts - Shear in Splice Bolts

|  |  | Existing <br> $\mathrm{W}_{0}$-DL | .75W --DL-1 | W,-DL-I | Proposed <br> $W_{0}$-DL | .75W1-DL-I | W1-DL-I |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Elev. | \# of Bolts | Force/Bolt | Force/Bolt | Force/Bolt | Force/Bolt | Force/Bolt | Force/Bolt |
| 87 | 14 | 3.57 | 2.75 | 3.99 | 4.63 | 3.64 | 5.18 |
| 100 | 14 | 2.87 | 2.17 | 3.17 | 3.83 | 2.97 | 4.24 |
| 112 | 14 | 2.18 | 1.62 | 2.39 | 3.02 | 2.31 | 3.31 |
| 125 | 14 | 1.53 | 1.08 | 1.63 | 2.24 | 1.66 | 2.41 |
| 137 | 14 | 0.89 | 0.58 | 0.92 | 1.44 | 1.04 | 1.52 |
| 150 | 24 | 0.21 | 0.12 | 0.20 | 0.43 | 0.29 | 0.44 |
| 165 | 8 | 0.08 | 0.01 | 0.05 | 0.30 | 0.19 | 0.29 |

Shear in Anchor Bolts - Tension in Splice Bolts

|  |  | Existing |  |  | Proposed |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Elev. | Bolt Size (dia.) | $W_{0}$ | $.75 W_{1}$ | $W_{1}$ | $W_{0}$ | $.75 W_{1}$ | $W_{1}$ |
| 87 | $3 / 4$ | 0.68 | 0.61 | 0.82 | 0.74 | 0.67 | 0.90 |
| 100 | $3 / 4$ | 0.63 | 0.56 | 0.75 | 0.69 | 0.62 | 0.83 |
| 112 | $3 / 4$ | 0.49 | 0.45 | 0.59 | 0.55 | 0.50 | 0.66 |
| 125 | $3 / 4$ | 0.44 | 0.39 | 0.53 | 0.49 | 0.44 | 0.59 |
| 137 | $3 / 4$ | 0.36 | 0.31 | 0.41 | 0.45 | 0.39 | 0.52 |
| 150 | $3 / 4$ | 0.12 | 0.11 | 0.15 | 0.18 | 0.15 | 0.20 |
| 165 | $3 / 4$ | 0.09 | 0.08 | 0.11 | 0.19 | 0.16 | 0.22 |

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| Job: | Portland, ME | Job No.: ME147110 |
| Calculated By: | R. Adair | Date: 19-Aug-02 |

Evaluation of Bracing Members

| Center Bolted? | Yes |  |
| :--- | :---: | :--- |
| Yield Strength $\left(\mathrm{F}_{\mathrm{Y}}\right):$ | 36 ksi | $\mathrm{C}_{\mathrm{C}}=$ |


| Section | Member | $\mathbf{K}$ Value | Length (ft.) | Area (in. ${ }^{2}$ ) | $\mathbf{r}_{\mathbf{x}}$ (in.) | $\mathbf{r}_{\mathbf{z}}$ (in.) | $\mathbf{k L} / \mathbf{r}_{\mathbf{x}}$ | $\mathbf{k L} / \mathbf{r}_{\mathbf{z}}$ |
| :---: | :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | $2 \mathrm{~L} 3.5 \times 3.5 \times 1 / 4$ | 1.0 | 16.32 | 3.380 | 1.090 | 0.694 | 134.8 | 141.1 |
| 2 | $2 \mathrm{~L} 3 \times 2.5 \times 1 / 4$ | 1.0 | 22.43 | 2.620 | 0.945 | 0.528 | 213.6 | 254.9 |
| 3 | $2 \mathrm{~L} 3 \times 2.5 \times 1 / 4$ | 1.0 | 20.90 | 2.620 | 0.945 | 0.528 | 199.1 | 237.5 |
| 4 | L3.5 $\times 3 \times 1 / 4$ | 1.0 | 19.43 | 1.560 | 1.110 | 0.631 | 157.6 | 184.8 |
| 5 | L3.5 $\times 3.5 \times 1 / 4$ | 1.0 | 18.03 | 1.690 | 1.090 | 0.694 | 148.8 | 155.9 |
| 6 | L3 $\times 3 \times 1 / 4$ | 1.0 | 13.63 | 1.440 | 0.930 | 0.592 | 131.9 | 138.1 |
| 7 | L3 $\times 3 \times 1 / 4$ | 1.0 | 13.63 | 1.440 | 0.930 | 0.592 | 131.9 | 138.1 |


| Section | All. Tens. (k) | $\mathbf{F}_{\mathbf{a}} \mathbf{( k s i )}$ | All. Comp. (k) | Brace Angle | All. Shear (k) | Act. Shear (k) | Stress Ratio |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 73.01 | 7.50 | 25.34 | 0.55 | 167.41 | 40.89 | $24 \%$ |
| 2 | 56.59 | 2.30 | 6.02 | 0.33 | 118.63 | 40.89 | $34 \%$ |
| 3 | 56.59 | 2.65 | 6.93 | 0.36 | 119.00 | 35.18 | $30 \%$ |
| 4 | 33.70 | 4.37 | 6.82 | 0.41 | 74.32 | 35.18 | $47 \%$ |
| 5 | 36.50 | 6.15 | 10.39 | 0.38 | 87.20 | 27.65 | $32 \%$ |
| 6 | 31.10 | 7.83 | 11.28 | 0.53 | 33.83 | 24.79 | $73 \%$ |
| 7 | 31.10 | 7.83 | 11.28 | 0.53 | 33.83 | 19.96 | $59 \%$ |

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North Conway，NH 03860
（603）356－5214

| Client： | Structure Consulting Group |  |
| :--- | :--- | ---: |
| Job： | Portland，ME | Job No．：ME147110 |
| Calculated By： | R．Adair | Date：19－Aug－02 |

## Evaluation of Leg Members

|  |  |  |  |  |  | Existing |  | Proposed |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Section | Size | K／／r | Cc | Fa allow | 133\％Allow | D＋W。 | D＋．75WI＋1 | D＋W。 | D＋．75Wl＋1 |
| 1 | 6x6x5／8 | 66.10 | 126.04 | 16.83 | 22.43 | 8.78 | 8.26 | 10.95 | 10.18 |
| 2 | $6 \times 6 \times 5 / 8$ | 66.10 | 126.04 | 16.83 | 22.43 | 7.24 | 6.84 | 9.20 | 8.56 |
| 3 | $6 \times 6 \times 5 / 8$ | 66.10 | 126.04 | 16.83 | 22.43 | 5.52 | 5.21 | 7.25 | 6.72 |
| 4 | $6 \times 6 \times 1 / 2$ | 68.64 | 126.04 | 16.57 | 22.09 | 5.04 | 4.80 | 6.84 | 6.37 |
| 5 | $6 \times 6 \times 1 / 2$ | 55.08 | 126.04 | 17.89 | 23.85 | 3.09 | 2.96 | 4.51 | 4.21 |
| 6 | $6 \times 6 \times 5 / 16$ | 75.00 | 126.04 | 15.90 | 21.20 | 2.26 | 2.25 | 3.81 | 3.62 |
| 7 | $5 \times 5 \times 5 / 16$ | 90.54 | 126.04 | 14.13 | 18.85 | 0.45 | 0.46 | 1.11 | 1.07 |

## Percent Capacity

|  | Existing |  |  | Proposed |  |  | Maximum |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Section | Base Elev． | D＋W。 | D＋．75W1＋1 | Secondary | D＋W。 | D＋．／5W1＋1 | Secondary | Existing | Proposed |
| 1 | 87 | 39\％ | 37\％ | 0\％ | 49\％ | 45\％ | 0\％ | 39\％ | 49\％ |
| 2 | 100 | 32\％ | 30\％ | 0\％ | 41\％ | 38\％ | 0\％ | 32\％ | 41\％ |
| 3 | 112 | 25\％ | 23\％ | 0\％ | 32\％ | 30\％ | 0\％ | 25\％ | 32\％ |
| 4 | 125 | 23\％ | 22\％ | 0\％ | 31\％ | 29\％ | 0\％ | 23\％ | 31\％ |
| 5 | 137 | 13\％ | 12\％ | 0\％ | 19\％ | 18\％ | 0\％ | 13\％ | 19\％ |
| 6 | 150 | 11\％ | 11\％ | 0\％ | 18\％ | 17\％ | 0\％ | 11\％ | 18\％ |
| 7 | 165 | 2\％ | 2\％ | 0\％ | 6\％ | 6\％ | 0\％ | 2\％ | 6\％ |

## Maximum Reactions：

| Uplift： | 64.9 kips |
| ---: | ---: |
| Compression： | 77.9 kips |
| Shear： | 41.6 kips |

## VERIZON WIRELESS (Tenant)

400 FRIBERG PARKWAY
WESTBOROUGH, MA 01581
(508) 330-3343


## 45-55 FOREST AVENUE TOWER SITE



FOR THE:
INSTALLATION OF 9 NEW MICROWAVE ANTENNA TO BE MOUNTED ON THE MODIFIED 93' SHELF SUPPORT VERIZON NEW ENGLAND MICRO-WAVE TOWER.

## PROJECT SUMMARY

SITE ADDRESS: 45-55 FOREST AVENUE

OWNER/LESSOR VERIZON NEW ENGLAND, INC.
5 DAVIS FARM ROAD
PORTLAND, MAINE 04103
LESSEE:
VERIZON WIRELESS
400 FRIBERG PARKWAY
WESTBOROUGH, MA 01581
ASSESSORS MAP: TAX MAP 37, LOT 1
CURRENT ZONING: DOWN TOWN BUSINESS B3 \& B3C
LATITUDE: N 43º $39^{\circ}-21.0^{\prime \prime}$
LONGITUDE: W 70º $15^{\circ}-50.0^{\prime \prime}$
TOWER BASE ELEVATION:ASSUMED 87'



1 of 4 COVER SHEET
PROJECT TITLE: PORTLAND TOWER SITE PROJECT NO. 02342




## CITY OF PORTLAND, MAINE

## Department of Building Inspections



Received from
Location of Work

Cost of Construction
Permit Fee


Building (IL) Other $\qquad$

CBL: $\qquad$
Check \#: $\square$ Total Collected \$ $\qquad$

## THIS IS NOT A PERMIT

No work is to be started until PERMIT CARD is actually posted upon the premises. Acceptance of fee is no guarantee that permit will be granted. PRESERVE THIS RECEIPT. In case permit cannot be granted the amount of the fee will be refunded upon return of the receipt less $\$ 10.00$ or $10 \%$ whichever is greater.

WHITE - Applicant's Copy
YELLOW - Office Copy
PINK - Permit Copy


[^0]:    All-Points Technology Corporation, P.C.
    150 Old Westside Road

