Form # P 04

### DISPLAY THIS CARD ON PRINCIPAL FRONTAGE OF WORK

#### **CITY OF PORTLAND**

Please Read Application And Notes, If Any, Attached

This is to certify that\_

has permission to \_\_\_\_\_

Ne Tel & Tel Co State &/No

Add a 30' Extension to Existi

# PERMIT

Add 6

63' Tow

Permit Number 1057764

hes and 3 Panel Antennas

AT _45 Forest Ave	. 03	7. A001001 GTY OF FUTE AND
provided that the person or person the provisions of the Statutes the construction, maintenance a this department.	s of Name and of the same ances	g this permit shall comply with a of the City of Portland regulatings, and of the application on file in
Apply to Public Works for street line and grade if nature of work requires such information.	N ication inspect in must git and with a permission procuble this to ding or the thereof label or complete the R NOTICE IS REQUIRED.	A certificate of occupancy must be procured by owner before this building or part thereof is occupied.
OTHER REQUIRED APPROVALS		
Fire Dept		$\sim$ $\sim$ $\sim$ $\sim$
Appeal Board		lle least 7/8/93
Department Name	ENALTY FOR REMOVING THIS CA	Director - Building & Inspection Services

							PERMIT	: 14 <b>ED</b>		
Cit	y of Portland, Maine	- Building or Use	Permi	it Application	Pe	rmit No:	Issue Date:	CBL:	<del> </del>	
	Congress Street, 04101	U				03-0764		037 AC	001001	
	ation of Construction:	Owner Name:				er Address:		Phone:		
45	Forest Ave	Ne Tel & Tel	Co Stat	te &	Po E	Box 152206	PATY DE DOM	3 4 1 5		
Busi	ness Name:	Contractor Name	<del></del>		Contr	actor Address:	WITH THE PURI	Phone		
		Nortech			35 N	Norton Road T	'aunton	5038803	663	
Less	ee/Buyer's Name	Phone:			Permi	it Type:			Zone:	
					Ado	ditions - Com	nercial		B3/	
Past	Use:	Proposed Use:			Perm	nit Fee:	Cost of Work:	CEO District:	7 <b>B</b> 3C	
Co	mmunication Tower/Comm	ercial Communication	n Tow	er/Commercial		\$779.00	\$108,000.00	2		
					FIRE	E DEPT:	Approved INSI	PECTION:		
							Denied Use	Group:	Type: 2	
						-		2/9/	13	
									<u>س</u>	
1 -	osed Project Description:	(A) T		5.1			Ì	( ) [ ]	bu k	
1	d a 30' Extension to Existing nel Antennas	g 63 Tower. Add 6 Mi	crowav		Signa		Sign VITIES DISTRIC		-eup	
1 41	ici Antennas				PEDE	ISTRIAN ACTI	VII IES DISTRIC	I (P.A.D.)		
					Actio	on: Approv	ed Approved	w/Conditions	Denied	
					Signa	ature:		Date:		
	nit Taken By:	Date Applied For:				Zoning	Approval			
ga	<u>d</u>	06/27/2003		110 0		T a :		- HI		
1.	This permit application do	•	Spe	ecial Zone or Review	WS	Zonin	g Appeal	Historic Pre		
	Applicant(s) from meeting Federal Rules.	g applicable State and	S	horeland	Variance			Not in District or Landma		
2.	Building permits do not in septic or electrical work.	nclude plumbing,		/etland	Miscellaneous			Does Not Require Review		
3.	Building permits are void within six (6) months of the		☐ F	lood Zone	Conditional Use			Requires Review		
	False information may inv permit and stop all work			ubdivision	Q	Interpret	ation	Approved		
			☐ Si	ite Plan		Approve	d	Approved w	/Conditions	
			Maj	Minor MM	Denied			Denied	/	
			Date: Ū	Lill	3	Date:		Date:	$-\!\!/-$	
				/(11)	/					
								•		
			(	CERTIFICATIO	)N					
I he	reby certify that I am the ow	uner of record of the no				nosed work is	authorized by th	he owner of reco	rd and that	
I ha	we been authorized by the o adiction. In addition, if a pell have the authority to enter	wner to make this appliermit for work describe	ication d in the	as his authorized application is is:	agen	it and I agree t I certify that t	o conform to all he code official	applicable laws s authorized rep	of this resentative	
	permit.	,	1	,			•	· / I	-	

ADDRESS

### RESPONSIBLE PERSON IN CHARGE OF WORK, TITLE

SIGNATURE OF APPLICANT

DATE

PHONE

Permit No: Date Applied For: CBL: City of Portland, Maine - Building or Use Permit 03-0764 06/27/2003 389 Congress Street, 04101 Tel: (207) 874-8703, Fax: (207) 874-8716 037 A001001 Location of Construction: Owner Address: Owner Name: Phone: 45 Forest Ave Ne Tel & Tel Co State & Po Box 152206 **Business Name: Contractor Name:** Contractor Address: Phone Nortech 35 Norton Road Taunton (503) 880-3663 Phone: Lessee/Buyer's Name Permit Type: Additions - Commercial Proposed Project Description: Proposed Use: Communication Tower/Commercial Add a 30' Extension to Existing 63' Tower. Add 6 Microwave Dishes and 3 Panel Antennas Dept: Zoning Reviewer: Marge Schmuckal 07/01/2003 Status: Approved Approval Date: Ok to Issue: Note: ok under section 14-430 Approval Date: 07/09/2003 Dept: Building Status: Approved Reviewer: Mike Nugent Ok to Issue: 🗹 Note:



# Commercial Building Permit Application

n03-0764

If you or the property owner owes real estate or personal property taxes or user charges on any property within the City, payment arrangements must be made before permits of any kind are accepted.

Location/Address of Construction: 45 · 5!	5 Fores	t Amenul, Por	Han	d maine
Total Square Footage of Proposed Structure		Square Footage of Lot		
Tax Assessor's Chart, Block & Lot Chart# Block# Lot#  O 3 7 A DO 1	Owner: Yer 125 High Boston	izin NewEnglar Stract MA	vcl	Telephone:
Lessee/Buyer's Name (If Applicable)	CETICO POU VERIZUM N	ne, address & telephone: atmership d/bla ireless g larkway (503 gh MA UKSI 330	We	st Of ork: \$ 108,000 c: \$ 77 9,
Current Specific use:	tion to	wer		
Proposed Specific use:				
Project description: Addition of Shub (O) top tower. Ir as well as three (3) po added. Five lyisting he	n addit anel ai orn and	nim, six (6) i n tennas (micro ennas will b	MICTI Nave L Ne	) will be moved.
Contractor's name, address & telephone: AH	Δ	· ·	א ככ	
Who should we contact when the permit is read Mailing address: Wellman Association		Buer	M. Carlotte	50880-3663
70 Broadway St Westford Ma C		Pł	10ne:9	78 589: 9870
Please submit all of the information outli do so will result in the automatic denial of			Check	dist. Failure to
At the discretion of the Planning and Development D further information stop by the Building Inspections of	Department, add office, room 31	itional information may be re o City Hall or call 874-8703.	equired p	rior to permit approval. For
I hereby certify that I am the Owner of record of the named authorized by the owner to make this application as his/her if a permit for work described in this application is issued, I areas covered by this permit at any reasonable hour to enfor	authorized agent certify that the C	I agree to conform to all appli- ode Official's authorized represe	cable laws entative sh	of this jurisdiction. In addition,
Signature of applicant		Date:		

This is not a Permit; you may not commence any work until the Permit is issued.

Permit Fee: \$30.00 for the first \$1000.00 Construction Cost, \$7.00 per additional \$1000.00 cost

# **BUILDING PERMIT INSPECTION PROCEDURES** Please call 874-8703 or 874-8693 to schedule your inspections as agreed upon Permits expire in 6 months, if the project is not started or ceases for 6 months.

The Owner or their designee is required to notify the inspections office for the following inspections and provide adequate notice. Notice must be called in 48-72 hours in advance in order to schedule an inspection:

By initializing at each inspection time, y inspection procedure and additional feet Work Order Release" will be incurred in below.	
	be scheduled with your inspection team upon
	lopment Review Coordinator at 874-8632 must
also be contacted at this time, before any s	ite work begins on any project other than
single family additions or alterations.	
Footing/Building Location Inspe	ction: Prior to pouring concrete
Re-Bar Schedule Inspection:	Prior to pouring concrete
Foundation Inspection:	Prior to placing ANY backfill
Framing/Rough Plumbing/Electr	ical: Prior to any insulating or drywalling
Final/Certificate of Occupancy:	Prior to any occupancy of the structure or use. NOTE: There is a \$75.00 fee per inspection at this point.
you if your project requires a Certificate of inspection	certain projects. Your inspector can advise Occupancy. All projects DO require a final cur, the project cannot go on to the next
phiase, REGARDLESS OF THE NOTICE	OR CIRCUMSTANCES.
	ES MUST BE ISSUED AND PAID FOR,
REFORE THE SPACE MAY BE OCCU	PIED
X Mallo Naoi-	7/14/03
Signature of applicant/designee  Lu-  Signature of inspections Official	$\frac{\frac{7}{1}}{\frac{1}{1}} = \frac{\frac{7}{1}}{\frac{1}{1}} = \frac{\frac{7}{1}}{\frac{1}} = \frac{\frac{7}{1}}{\frac{1}}{\frac{1}} = \frac{\frac{7}{1}}{\frac{1}} = \frac{\frac{7}{1}}{\frac{1}}{\frac{1}} = \frac{\frac{7}{1}}{\frac{1}}{\frac{1}} = \frac{\frac{7}{1}}{\frac{1}} = \frac{\frac{7}{1}}{\frac$
CBL: <u>037-A - 00)</u> Building Permit #: <u>(</u>	03-0764

David S. Ho Manager - Transactions



**Verizon Communications** 125 High Street, Oliver 02339 Boston, MA 02110

June 26, 2003

City of Portland City Hall 389 Congress Street Portland, ME 04101

RE: Verizon Wireless Microwave Installation, 45-55 Forest Avenue, Portland. ME

Dear Sir/Madam:

Please be advised that Verizon Communications is the owner of the property located at 45-55 Forest Avenue, Portland, Maine. It is Verizon Wireless' intention to co-locate antennas at the property. Verizon Wireless is required to obtain all state, local and town approval prior to the installation of the additional equipment.

As owner of the property, permission is hereby granted to Verizon Wireless for the purpose of consummating any applications necessary to gain the required approvals or permits on the tower from the City of Portland.

**Verizon Communications** 

Authorized Agent:

(A copy of this letter shall have the same effect as the original)

Auch te . . . Engineers



n - Colorn Business Park Sychet Mayor 04210

> essiden genommunetary esince 1870

April 15, 2003

Mr. Robert Hogan (Faxed Copy Sent to 508-330-3405) Structures Consulting Group, Inc. 43 White Street, Suite 4 Belmont, MA 02478

Re: Verizon Wireless Building Structural Evaluation 45-55 Congress Street Portland, Maine 04101 Project No. 02180

Dear Mr. Hogan:

Harriman Associates has completed its review and evaluation of the existing building columns supporting the rooftop antenna tower at 45-55 Congress Street in Portland, Maine. Our evaluation was completed to confirm that additional base loads resulting from an antenna tower extension could be supported by the existing building columns. The evaluation concluded that the existing building columns are capable of withstanding the increased base loads caused by the addition of a 30 ft. extension to the existing antenna tower. The analysis was based primarily on information provided by All-Points Technology Corporation's Structural Analysis Report of the Antenna Tower as well as original construction drawings and calculations of the building. Evaluation of the tower and the grillage has been completed by All-Points Technology and is not a part of our scope.

The existing steel columns are built-up sections comprised of plates and angles riveted together. The size of the columns were determined from review of the original drawings as the columns are concrete encased and could not be inspected on site. Each existing column was extended above the roof line approximately 1'-6" and capped. The tower grillage has been attached to these column extensions for transfer of the tower loads into the building. The configuration of the columns considered in this study, as well as their locations, are outlined in Attachment 1.

From the tower evaluation report, maximum gravity (downward) load, maximum uplift load, and maximum shear load at the base of the tower was provided. The loads provided were 77.9 kip, 64.9 kip, and 10.4 kip, respectively. Also, original calculations documented a thoor dead load of 116 pounds per square foot (psf), and design live loads between 150 psf and 175 psf. A site investigation was conducted to verify the existing information. This investigation found that the 1<sup>th</sup> to 5<sup>th</sup> floors were mainly abandoned and the 6<sup>th</sup> floor had electrical data equipment on it. At this time, it did not appear that the actual floor loads are exceeding the design values

#### HARRIMAN ASSOCIATES

Mr. Robert Hogan Page 2 April 15, 2003

A complete analysis was performed on the existing columns based on the above loads. As stated, the analysis determined that the building columns could withstand the increased base loads of the tower plus extension. In order to restrain the anticipated shear loads, the additional 1" thick stiffener plates installed as part of the grillage framing were required to be used in the calculations. These plates "stiffen" the column section above the roof level and are necessary to keep the columns within design stresses. Since these plates are installed and modifications to the grillage are not planned, no additional work is required.

As a side note, we discovered a couple items during our research into the existing structure that we believe you would be interested in.

- 1. The building was originally designed for 9 stories with only / built
- 2. In 1953, the existing building was analyzed for the addition of a 100 it tall tower (the current tower is approximately 65 ft.) The building was found adequate to support this tower with its particular configuration of antennas. (Note: Any differences or modifications to the antenna configuration would change the resultant loads at the tower base and would require re-evaluation of the tower, tower support framing, and building columns).

We thank you for the opportunity to assist with this project. If we can be of further assistance, please do not hesitate to contact us.

Sincerely,

Harriman Associates

James Fortin, P.E.

ו. .... ממאבבוב "ממוממ (ב..... (מ

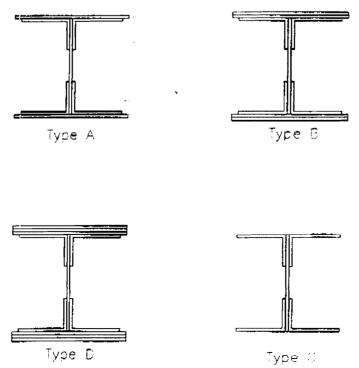
icf

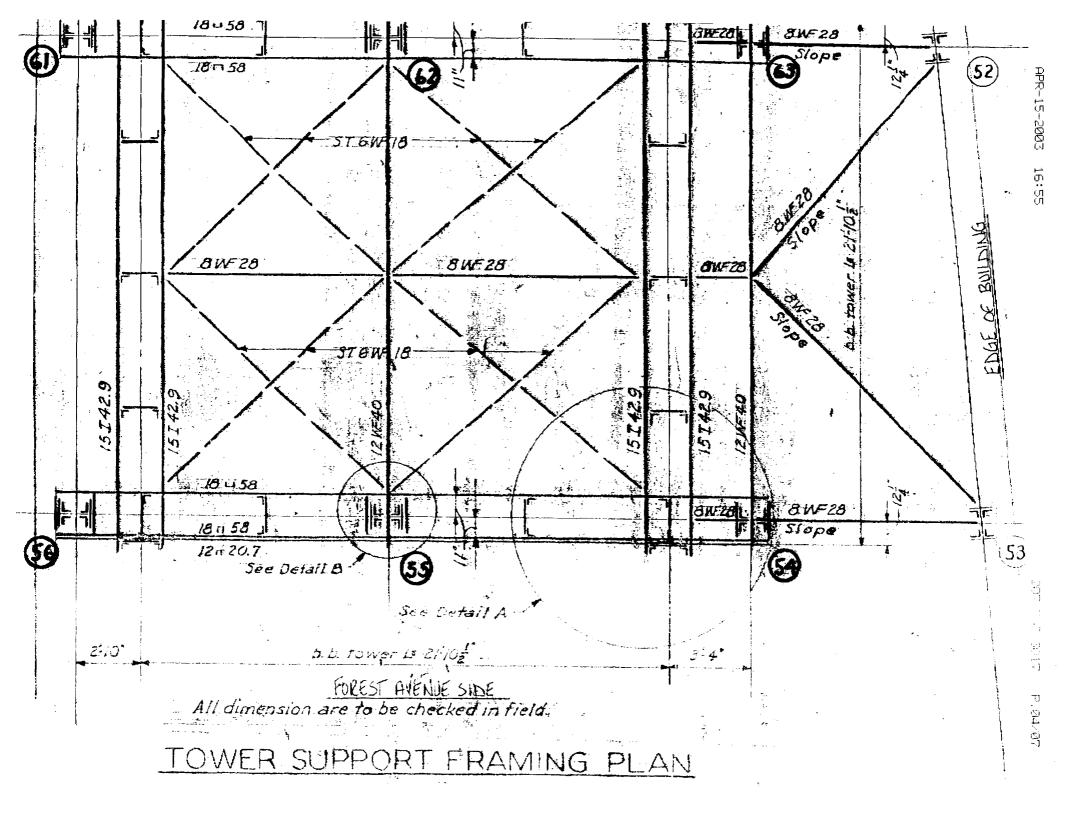
cc: Jim Seymour - Sebago Technics w/encs. (Faxed Copy Sent to 856-2206)

#### VERIZON WIRELESS BUILDING 45-55 CONGRESS STREET, PORTLAND MAINE ATTACHMENT 1: COLUMN CONFIGURATION TABLE

0-1	Calvara 454	
	1	Cedlumin #61
		Chianin #62
<u> </u>		12" x 3/8" Hick web plate
	4- 6x4x3/8" angles	न14" x3/8" काck flange plates
4- 6x4x3/8" angles		4- 6x4x3∜   angles
Type A		Type A
<del></del>	i '	12" x 3/8" thick web plate
	4- 6x4x3/8" angles	14" x 3/8" thick flange plates
4- 6x4x3/8" angles		4- 6x4x3/8 angles
Type A		уре А
$\perp$ 12" x 3/8" thick web plate	,	12" x 3/8" thick web plate
$14'' \times 3/8''$ thick flange plates		14" x 3/8" Hack flange plates
4- 6x4x1/2" thick angles	4- 6x4x3/8" angles	4- 6x4xt angles
Type A	Type A	± type ∧
12" x 3/8" thick web ptate	12" x 3/8" thick web plate	12" x 3/8" thick web plate
14" x 3/8" thick flange plates	14" x 3/8" thick flange plates	14" x 3/8" thick flange plates
4- 6x4x1/2" thick angles	4- 6x4x3/8" angles	'4- 6x4x1/2" angles
Туре А	Type A	ype A
12" x 3/8" thick web plate	12" x 3/8" thick web plate	12" x 5/8" thick web plate
14" x 1/2" thick flange plates	14" x 3/8" thick flange plates	114" x 1/2" thick flange plates
4- 6x4x1/2" thick angles	4- 6x4x1/2" angles	,4- 6x4x1′2″ angles
Туре А	Type A	Type A
12" x 3/8" thick web plate		12" x 5/8" thick web plate
14" $\times$ 1/2" thick flange plates	14" x 3/8" thick flange plates	14" x 1/2" thick flange plates
14" x 3/8" thick flange plates	14" x 3/8" thick flange plates	114" x 3/8" flick flange plates
4- 6x4x1/2" thick angles	4- 6x4x1/2" angles	14- 6x4x1/2" if gies
Type B	Type B	Type B
12" x 5/8" thick web plate	12" x 5/8" thick web plate	12" x 5/δ" trouk web plate
14" x 1/2" thick flange plates	14" x 3/8" thick flange plates	14" x 1/2" thick flange plates
14" x 3/8" thick flange plates	14" x 3/8" thick flange plates	(14" x 3-8" thick flange plates
14" x 3/8" thick flange plates	4- 6x4x1/2" angles	14" x 3/6" tinck flange plates
4- 6x4x1/2" thick angles	_	4- 6x4x1/2" angles
Type C	Type B	Type C
	12" x 3/8" thick web plate 14" x 3/8" thick flange plates 4- 6x4x3/8" angles Type A 12" x 3/8" thick flange plates 4- 6x4x1/2" thick angles Type A 12" x 3/8" thick web plate 14" x 3/8" thick web plate 14" x 3/8" thick flange plates 4- 6x4x1/2" thick angles Type A 12" x 3/8" thick web plate 14" x 1/2" thick flange plates 4- 6x4x1/2" thick flange plates 4- 6x4x1/2" thick flange plates 4- 6x4x1/2" thick flange plates 14" x 1/2" thick flange plates 14" x 3/8" thick flange plates 14" x 3/8" thick flange plates 14" x 1/2" thick flange plates 14" x 1/2" thick flange plates 14" x 3/8" thick flange plates	Column #55  12" x 3/8" thick web plate 14" x 3/8" thick flange plates 4- 6x4x3/8" angles Type A  12" x 3/8" thick web plate 14" x 3/8" thick flange plates 4- 6x4x3/8" angles Type A  12" x 3/8" thick flange plates 4- 6x4x3/8" angles Type A  12" x 3/8" thick flange plates 4- 6x4x3/8" angles Type A  12" x 3/8" thick flange plates 4- 6x4x1/2" thick angles Type A  12" x 3/8" thick flange plates 4- 6x4x1/2" thick angles Type A  12" x 3/8" thick flange plates 4- 6x4x1/2" thick angles Type A  12" x 3/8" thick web plate 14" x 3/8" thick flange plates 4- 6x4x1/2" thick flange plates 14" x 3/8" thick flange plates

Note: Type A to Type D are schematic representations of the column configuration and are not to scale







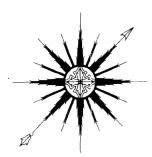
Picture 1: View of antenna and building from Forest Avenue.



Picture 2: 6th floor area.



Picture 3: Typical floor in area being evaluated.

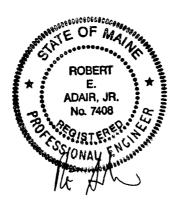


# ALL-POINTS TECHNOLOGY CORPORATION, P.C.

#### STRUCTURAL ANALYSIS REPORT 63' SELF-SUPPORTING TOWER PORTLAND, MAINE

Prepared for Structure Consulting Group, Inc.

September 5, 2002



APT Project #ME147110

# of 63' SELF-SUPPORTING TOWER PORTLAND, MAINE prepared for Structure Consulting Group

#### **EXECUTIVE SUMMARY:**

All-Points Technology Corp., P.C. (APT) performed an inspection and structural analysis of this 63-foot self-supporting roof-top tower. The analysis was performed with the addition of a 30-foot extension, six microwave dishes, and three square panel antennas. Five existing inactive conical horn antennas were assumed to be removed from the tower. Feed lines, support members, and appurtenances associated with the horns were also assumed to be removed. Existing platforms were assumed to remain in place.

Our analysis indicates the tower is capable of supporting the proposed extension and antennas. Evaluation of the building structure to support tower reactions is pending receipt of building drawings.

#### INTRODUCTION:

An inspection and structural analysis was performed on the above-mentioned communications tower by All-Points Technology Corporation, P.C. (APT) for Structure Consulting Group. The tower is located on the roof of the building at 45-55 Forest Avenue in Portland, Maine.

Robert E. Adair, P.E. inspected the tower on August 13, 2002 to record information regarding physical and dimensional properties of the structure and its appurtenances. Mr. Adair climbed the structure in its entirety to compile data necessary to perform the structural analysis.

The structure is a 63-foot painted galvanized steel, self-supporting tower of unknown manufacturer (possibly Andrew or LeBlanc). The analysis was performed with a 30-foot tower extension and the following antenna inventory:

Antenna	Elev.	Mount	Coax.
6' high performance dish	180'	Pipe	EW-52
1' square panel	180'	Pipe	1-5/8"
6' dish, no radome	175'	Pipe	EW-63
6' high performance dish	170'	Pipe	EW-90
7' omnidirectional whip	170'	Wide flange extension	7/8"
2' square panel	165'	Pipe	1-5/8"
(2) omnidirectional antennas	162'	On platform face	(2) 1/2"
8' omnidirectional whip	161'	Pipe extension	7/8"
Super Stationmaster whip	160'	Pipe extension	7/8"
6' high performance dish	160'	Pipe	EW-52
2' square panel	160'	Pipe	1-5/8"
2' high performance dish	158'	5' x 6' frame on platform	(2) 3/8"
1' square panel	158'	On above frame	3/8"
12' high performance dish	157'	6' x 3' frame on platform	2-1/2" solid
7' omnidirectional whip	156'	Pipe extension	1/2"
Quad yagi	156'	Pipe on platform	7/8"
Dual yagi	152'	Platform	1/2"
6' dish with radome	150'	Pipe	1-5/8"
Dual yagi	149'	Platform	7/8"
Dual yagi	144'	Pipe on platform	7/8"
12' high performance dish	144'	Platform	2-1/2" solid
4' dish with radome	127'	Pipe	EW-90
12' high performance dish	111'	Platform	EW-52

Elevations listed are above ground level and assumes tower base is 87' AGL. Proposed antennas are depicted in **bold** text.

#### FIELD INSPECTION:

- **General Condition:** The tower, a galvanized steel structure, appeared to be in very good condition. No signs of movement or overstress of the tower were observed.
- **Bolted Connections of Lattice Bracing:** Connections were visually inspected to the maximum extent practicable. All connections that were observed appeared to be sound, with no loose or missing bolts noted.

- **Antenna Connections:** Antenna mounting hardware was in generally good condition, with corrosion-resistant hardware and galvanized members prevalent.
- **Splice Connections:** Observed splice connections were in good condition. One missing splice bolt was observed at the 100' elevation (13' elev. on tower) on the climbing leg. The remaining splice bolts at this elevation are within allowable loads.

#### **STRUCTURAL ANALYSIS:**

#### Methodology:

The structural analysis was done in accordance with EIA/TIA-222-F, <u>Structural Standards for Steel Antenna Towers and Antenna Supporting Structures</u>; and the American Institute of Steel Construction (AISC), <u>Manual of Steel Construction</u>, <u>Allowable Stress Design</u>, Ninth Edition.

The analysis was conducted using a wind speed of 80 miles per hour and one-half inch of radial ice over the entire structure and all appurtenances. The EIA/TIA Standard requires a minimum wind speed of 80 miles per hour for Cumberland County, Maine. The tower was analyzed by calculating the resultant wind loading and associated maximum bending moments, shear forces, and axial loads. The moments and forces were used to calculate stresses in leg and bracing members, which were compared to allowable stresses according to AISC.

Two loading conditions were evaluated in accordance with EIA/TIA-222-F to determine the tower's capacity. The more demanding of the two cases is used to calculate the tower capacity:

- Case 1 = Wind Load (without ice) + Tower Dead Load
- Case 2 = 0.75 Wind Load (with ice) + Ice Load + Tower Dead Load

In addition, the TIA/EIA standard permits a one-third increase in allowable stresses for towers less than 700-feet tall. Allowable stresses of tower members were increased by one-third when computing the load capacity values shown below.

#### **Analysis:**

Analysis of the tower was conducted in accordance with the criteria outlined herein with proposed antennae as previously described.

Our analysis determined the existing tower is capable of supporting the extension and additional proposed antennas. The following table summarizes the results of the analysis based on compressive stresses of individual leg members:

**Tower Capacity** 

Elevation	Capacity
0-13'	49%
13'-25'	41%
25'-38'	32%
38'-50'	31%
50'-63'	19%
63'-78''	18%
78'-93'	6%

#### **Bracing Members:**

Bracing is generally installed in a X-brace configuration, with each compression member paired with a corresponding tension member. Diagonal bracing was evaluated by calculating bracing members' allowable compression and tension forces and assessing each tower section's ability to resist calculated shear forces.

Bracing members were determined to be adequate to support the proposed loads.

#### **Base Support:**

Evaluation of the existing base support frame, comprised of MC18 x 58 channels and W16 x 31 steel beams, was also performed. Our calculations indicate the base frame easily supports reactions generated by the tower with the proposed extension and antennas.

Base reactions imposed with the additional antennas were calculated as follows:

Tension: 64.9 kips Compression: 77.9 kips Total Shear: 41.6 kips

#### **CONCLUSIONS AND RECOMMENDATIONS:**

Our structural analysis indicates the 63-foot self-supporting roof-top tower located at 45-55 Forest Avenue in Portland, Maine is capable of supporting the 30-foot tower extension and antenna loading proposed by Verizon Wireless.

#### **LIMITATIONS:**

This analysis is based on the tower being properly installed, members in new condition, required members in place, and required bolts in place. The tower inspection conducted on August 13, 2002 confirmed these conditions.

All-Points Technology Corporation, P.C. (APT) is not responsible for any modifications completed prior to or hereafter which APT is not or was not directly involved. Modifications include but are not limited to:

- 1. Replacing or strengthening bracing members.
- 2. Reinforcing vertical members in any manner.
- 3. Adding or relocating stabilizers.
- 4. Installing antenna mounting gates or side arms.
- 5. Extending tower.

APT hereby states that this document represents the entire report and that it assumes no liability for any factual changes that may occur after the date of this report. All representations, recommendations, and conclusions are based upon the information contained and set forth herein. If you are aware of any information which is contrary to that which is contained herein, or you are aware of any defects arising from the original design, material, fabrication and erection deficiencies, you should disregard this report and immediately contact APT. APT disclaims all liability for any representation, recommendation, or conclusion not expressly stated herein.

# Appendix A

Tower Drawings

150 OLD WESTSIDE ROAD NORTH CONWAY, NH 03860 PHONE/FAX: (603) 356-5214 MOBILE: (603) 496-5853 www.allpointstech.com



#### **EXISTING TOWER**

SHEET: 1 OF 1

SCALE: 1" = 10" DRAWN BY: REA **DATE: 21 AUG 02** APT JOB #ME147110 veri onwireless

**Structure Consulting Group** 43 White Street, Suite 4 Belmont, MA 02478

**VERIZON PROJECT #** 

**63' SELF-SUPPORTING TOWER** 45-55 FOREST AVENUE PORTLAND, MAINE

L2.5 x 2 x 1/4 ) 3/4"Ø

(14)

C6 x 10.5 (14) 3/4"Ø

Not applicable

5 × 1/4 (14)

L3 x 2.

(14) 3/4"Ø

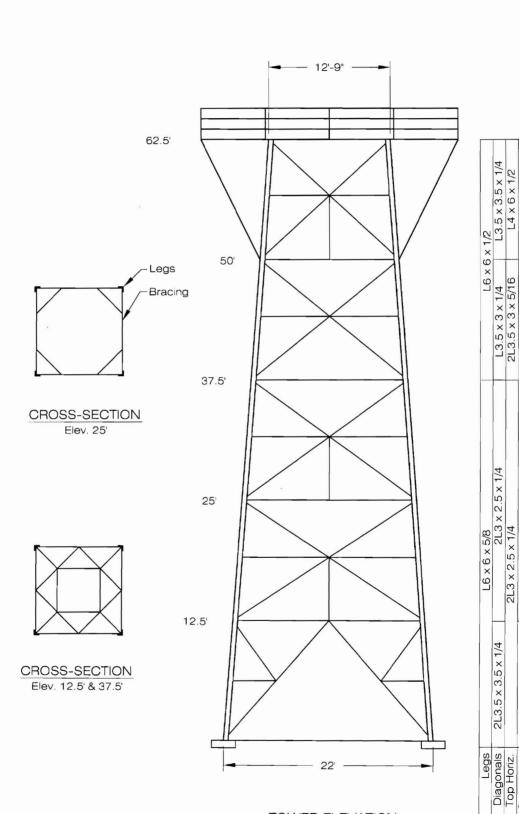
.5 × 1/4 × 3/16

L2.5 × 2.5 L2 × 2 × 3

Diagonals
Top Horiz.
Sub- Diag.
Mid Horiz.
Splice Bolts

2L3.5 × 3.5 × 1/4

2L3 × 2.5 × 1/4



TOWER ELEVATION

Scale: 1" = 10'

150 OLD WESTSIDE ROAD NORTH CONWAY, NH 03860 PHONE/FAX: (603) 356-5214 MOBILE: (603) 496-5853 www.allpointstech.com



#### **TOWER EXTENSION**

SHEET: 1 OF 1

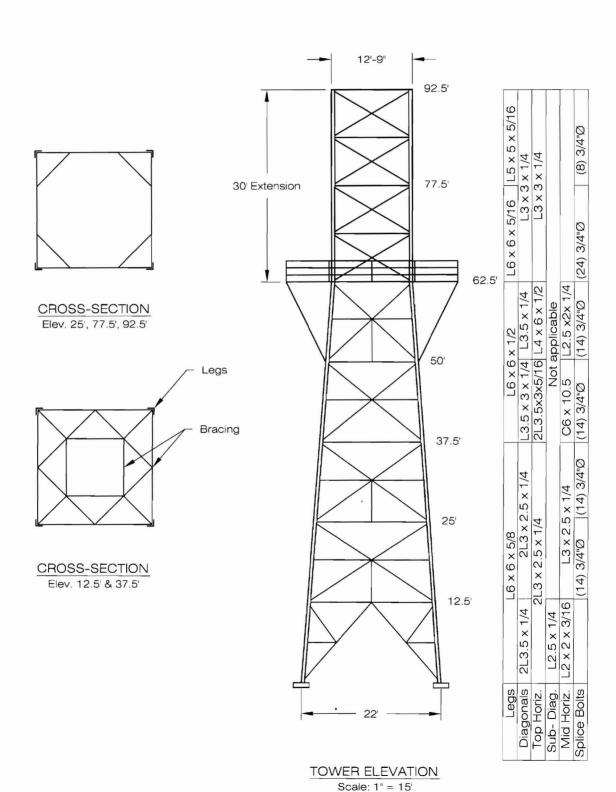
SCALE: 1" = 15' DRAWN BY: REA

DATE: 21 AUG 02 APT JOB #ME147110

veri onwireless

Structure Consulting Group 43 White Street, Suite 4 Belmont, MA 02478 **VERIZON PROJECT #** 

93' SELF-SUPPORTING TOWER 45-55 FOREST AVENUE PORTLAND, MAINE



# Appendix B

Photographs

# VERIZON WIRELESS 63' SELF-SUPPORTING TOWER PORTLAND, MAINE

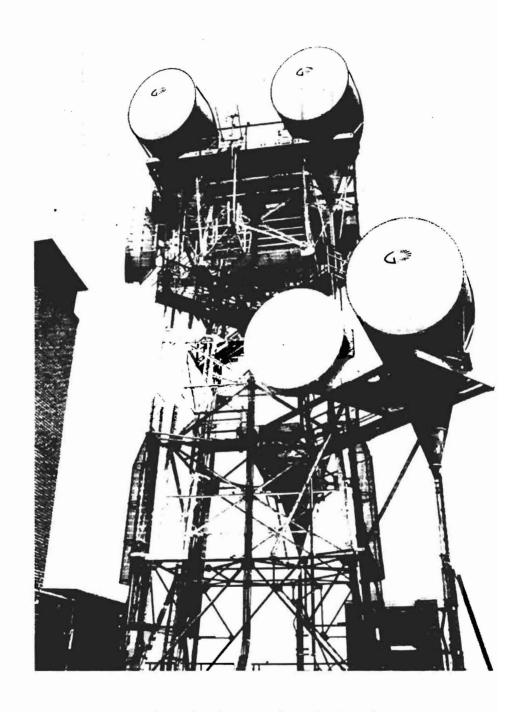


Photo showing overview of 63' rooftop self-supporting tower.

Photos taken August 13, 2002 by All-Points Technology Corp., P.C.

# VERIZON WIRELESS 63' SELF-SUPPORTING TOWER PORTLAND, MAINE

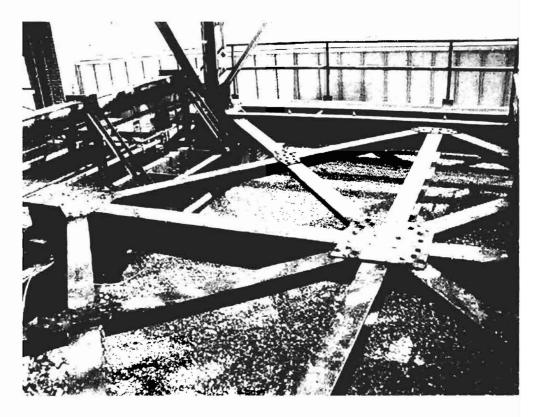


Photo of tower base and support frame.

# Appendix C

Calculations

150 Old Westside Road North Conway, NH 03860 (603) 356-5214

Client:

**Structure Consulting Group** 

Job:

Portland, ME

Calculated By:

R. Adair

Job No.: **ME147110** 

19-Aug-02

Date:

#### General Information

Tower Manufacturer Andrew? Tower Type Self-supporting Tower Total Height of Tower 180 ft. Wind Speed EIA-TIA: Cumberland County 80 mph. Radial Ice 0.5 in. 25% Reduction for ice yes (yes or no) 1/3 increase for allowable loads yes (yes or no) Number of faces 4 faces

Antenna Force Calculations based on EIA/TIA-222-F, using the following formulas:

Force on discrete appurtenance: F=Qz\*Gh\*Ca\*A

Force on microwave antennae:  $F=Cr^*A^*Gh^*Kz^*V^2$ , where  $Cr=((Ca^2)+(Cs^2))^2(1/2)$ 

Gh=.65+.60/(h/33)^(1/7) =

V as specified EIA-222-F

 Fy
 36 ksi

 E (Modulus of Elasticity)
 29000 ksi

 Fb
 0.6

 K
 1

 Tower taper
 0.15 ft/ft

						Leg Propert	ies
Section No.	Section Length	Leg Spread @ Base of section	Leg Size (Description)	Width of Leg to Wind	Area	r <sub>z</sub>	Unbraced Lengths
Bldg	87.0	50.00					
1	12.5	22.00	6x6x5/8	6.00	7.11	1.180	78
2	12.5	20.15	6x6x5/8	6.00	7.11	1.180	78
3	12.5	18.30	6x6x5/8	6.00	7.11	1.180	78
4	12.5	16.45	6x6x1/2	6.00	5.75	1.180	81
5	12.5	14.60	6x6x1/2	6.00	5.75	1.180	65
6	15.0	12.75	6x6x5/16	6.00	3.65	1.200	90
7	15.0	12.75	5x5x5/16	5.00	3.03	0.994	90
Тор		12.75					
	180						

Gh= 1.12

150 Old Westside Road North Conway, NH 03860 (603) 356-5214

Client:

**Structure Consulting Group** 

Job: Calculated By: Portland, ME

R. Adair

Job No.: **ME147110** 

Date: 19-Aug-02

Section		1				typ	e		
	Ag =	270	sf	z =	93.25	ft			
		Quantity Per						Wt. (lbs.)	
		Face	Length (ft.)	Width (in.)	Area (sf)	Area w/ ice	Wt. Per ft.	Tower	Wt. (lbs.) Ice
Round Members	<u>s</u>								
		0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
					0.0	0.0		0.0	0.0
Flat Members									
Leg		2	12.5	6.0	12.5	14.6	24.2	1209.7	252.8
Diagonal		2	16.3	3.5	9.5	12.2	11.6	1514.9	812.6
Horizontal		1	19.7	3.0	4.9	6.6	9.0	707.4	427.9
Sub-Diagonal		2	6.9	2.5	2.9	4.0	4.1	227.0	258.3
Sub-Horizontal		2	4.5	2.0	1.5	2.3	2.4	88.0	140.3
Section		2				typ	е		<u> </u>
	Ag =	247	sf	z =	105.75	ft			
								Wt. (lbs.)	
		3	Length (ft.)	Width (in.)	Area (sf)	Area w/ ice	Wt. Per ft.	Tower	Wt. (lbs.) Ice
Round Members	<u>s</u>								
		0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
					0.0	0.0		0.0	0.0
Flat Members									
Leg		2	12.5	6.0	12.5	14.6	24.2	1209.7	252.8
Diagonal		2	22.4	3.0	11.2	15.0	9.0	1615.1	977.0
Horizontal		1	17.8	3.0	4.5	5.9	9.0	640.8	387.6
Horizontal		1	18.7	2.0	3.1	4.7	3.2	238.9	291.3
Vertical		1	6.5	2.5	1.4	1.9	4.1	106.6	121.3
Section		3			_	typ	<u> </u>		
	Ag =	223	sf	z =	118	ft			
		Quantity Per						Wt. (lbs.)	
		Face	Length (ft.)	Width (in.)	Area (sf)	Area w/ ice	Wt. Per ft.	Tower	Wt. (lbs.) Ice
Round Members	<u> </u>								, ,
		0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
					0.0	0.0		0.0	0.0
Flat Members									
Leg		2	12.5	6.0	12.5	14.6	24.2	1209.7	252.8
Diagonal		2	20.9	3.0	10.5	13.9	9.0	1505.1	910.5
Horizontal		1	17.8	3.0	4.5	5.9	9.0	640.8	387.6
Horizontal		1	16.9	2.0	2.8	4.2	3.2	215.3	262.5
		1	6.5	2.5		1.9	4.1	106.6	202.0

150 Old Westside Road North Conway, NH 03860 (603) 356-5214

Client:

**Structure Consulting Group** 

Job:

Portland, ME

Calculated By:

R. Adair

Job No.: **ME147110** 

Date: 19-Aug-02

Section		4				typ	e		
	Ag =	200	sf	z =	131	ft			_
		Quantity Per						Wt. (lbs.)	
		Face	Length (ft.)	Width (in.)	Area (sf)	Area w/ ice	Wt. Per ft.	Tower	Wt. (lbs.) Ice
Round Members	<u> </u>								
		0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Flat Members									
Leg		2	12.5	6.0	12.5	14.6	19.6	978.3	252.8
Diagonal		2	19.4	3.5	11.3	14.6	5.4	839.5	967.3
Horizontal		1	16.0	3.0	4.0	5.3	9.0	574.2	347.4
Horizontal		1	15.0	2.0	2.5	3.8	3.2	191.7	233.7
Vertical		1	6.5	2.5	1.4	1.9	4.1	106.6	121.3
Section		5				type	e		
	Ag =	177	sf	z =	143.25	ft			
		Quantity Per						Wt. (lbs.)	
		Face	Length (ft.)	Width (in.)	Area (sf)	Area w/ ice	Wt. Per ft.	Tower	Wt. (lbs.) Ice
Round Members	<u> </u>								
		0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Flat Members									
Leg		2	12.5	6.0	12.5	14.6	19.6	978.3	252.8
Diagonal		2	18.0	3.5	10.5	13.5	5.8	836.5	897.4
Horizontal		1	12.3	20.0	20.4	21.4	53.7	2629.7	1562.6
Horizontal		1	13.2	2.5	2.7	3.8	3.6	190.8	123.0
Vertical		1	6.5	2.5	1.4	1.9	4.1	106.6	60.7
Section						type	ê		
	An =	<b>6</b>		7 =	157				
	Ag =	199	sf	z =	157	ft		\\(\Delta/t\) (lbc.)	
	Ag =	199 Quantity Per				ft		Wt. (lbs.)	\\\/t \(\l)\co
Round Members		199	sf Length (ft.)	z = Width (in.)	157 Area (sf)		Wt. Per ft.	Wt. (lbs.) Tower	Wt. (lbs.) Ice
Round Members		199 Quantity Per Face	Length (ft.)	Width (in.)	Area (sf)	ft Area w/ ice	Wt. Per ft.	Tower	, ,
		199 Quantity Per				ft			Wt. (lbs.) Ice
Flat Members		199 Quantity Per Face	Length (ft.)	Width (in.)	Area (sf)	ft Area w/ ice 0.0	Wt. Per ft.	Tower	0.0
lat Members		199 Quantity Per Face 0	Length (ft.) 0.0 15.0	Width (in.) 0.0 6.0	Area (sf) 0.0 15.0	ft Area w/ ice 0.0 17.5	Wt. Per ft. 0.0 12.4	Tower 0.0 745.2	0.0
Flat Members Leg Diagonal		199 Quantity Per Face 0 2 4	Length (ft.) 0.0 15.0 13.6	Width (in.) 0.0 6.0 3.0	Area (sf) 0.0 15.0 13.6	ft Area w/ ice 0.0 17.5 18.2	Wt. Per ft. 0.0 12.4 4.9	745.2 1068.2	0.0 303.3 1186.9
Flat Members Leg Diagonal		199 Quantity Per Face 0	Length (ft.) 0.0 15.0	Width (in.) 0.0 6.0	Area (sf) 0.0 15.0	ft Area w/ ice 0.0 17.5	Wt. Per ft. 0.0 12.4	Tower 0.0 745.2	0.0
Flat Members Leg Diagonal Horizontal	<u>.</u>	199 Quantity Per Face  0 2 4 2	Length (ft.) 0.0 15.0 13.6 11.8	Width (in.)  0.0  6.0  3.0  3.0	Area (sf) 0.0 15.0 13.6 5.9	ft  Area w/ ice  0.0  17.5  18.2  7.8	Wt. Per ft. 0.0 12.4 4.9 4.9	745.2 1068.2	0.0 303.3 1186.9
Flat Members Leg Diagonal Horizontal		199 Quantity Per Face 0 2 4 2 7 198	Length (ft.) 0.0 15.0 13.6	Width (in.) 0.0 6.0 3.0	Area (sf) 0.0 15.0 13.6	ft  Area w/ ice  0.0  17.5  18.2  7.8	Wt. Per ft. 0.0 12.4 4.9 4.9	745.2 1068.2 460.6	0.0 303.3 1186.9
Flat Members Leg Diagonal Horizontal	<u>.</u>	199 Quantity Per Face  0 2 4 2 7 198 Quantity Per	Length (ft.)  0.0  15.0  13.6  11.8	Width (in.)  0.0  6.0  3.0  3.0	Area (sf) 0.0 15.0 13.6 5.9	ft Area w/ ice 0.0 17.5 18.2 7.8	Wt. Per ft. 0.0 12.4 4.9 4.9	Tower 0.0 745.2 1068.2 460.6 Wt. (lbs.)	0.0 303.3 1186.9 255.9
Flat Members Leg Diagonal Horizontal  Section	Ag =	199 Quantity Per Face 0 2 4 2 7 198	Length (ft.) 0.0 15.0 13.6 11.8	Width (in.)  0.0  6.0  3.0  3.0	Area (sf) 0.0 15.0 13.6 5.9	ft  Area w/ ice  0.0  17.5  18.2  7.8	Wt. Per ft. 0.0 12.4 4.9 4.9	745.2 1068.2 460.6	0.0 303.3 1186.9
Flat Members Leg Diagonal Horizontal  Section	Ag =	199 Quantity Per Face  0 2 4 2  7 198 Quantity Per Face	Length (ft.)  0.0  15.0  13.6  11.8  sf  Length (ft.)	Width (in.)  0.0  6.0  3.0  3.0  z =  Width (in.)	Area (sf)  0.0  15.0  13.6  5.9  172  Area (sf)	ft Area w/ ice 0.0 17.5 18.2 7.8  type ft Area w/ ice	Wt. Per ft.  0.0  12.4  4.9  4.9  Wt. Per ft.	745.2 1068.2 460.6 Wt. (lbs.) Tower	0.0 303.3 1186.9 255.9 Wt. (lbs.) Ice
Flat Members Leg Diagonal Horizontal  Section  Round Members	Ag =	199 Quantity Per Face  0 2 4 2 7 198 Quantity Per	Length (ft.)  0.0  15.0  13.6  11.8	Width (in.)  0.0  6.0  3.0  3.0	Area (sf) 0.0 15.0 13.6 5.9	ft Area w/ ice 0.0 17.5 18.2 7.8	Wt. Per ft. 0.0 12.4 4.9 4.9	Tower 0.0 745.2 1068.2 460.6 Wt. (lbs.)	0.0 303.3 1186.9 255.9
Flat Members Leg Diagonal Horizontal  Section  Round Members	Ag =	199 Quantity Per Face  0 2 4 2 7 198 Quantity Per Face 0	Length (ft.)  0.0  15.0  13.6  11.8  sf  Length (ft.)  0.0	Width (in.)  0.0  6.0  3.0  3.0  z =  Width (in.)  0.0	Area (sf)  0.0  15.0  13.6  5.9  172  Area (sf)  0.0	ft Area w/ ice 0.0 17.5 18.2 7.8  type ft Area w/ ice 0.0	Wt. Per ft.  0.0  12.4 4.9 4.9  Wt. Per ft.  0.0	Tower 0.0 745.2 1068.2 460.6  Wt. (lbs.) Tower 0.0	0.0 303.3 1186.9 255.9 Wt. (lbs.) Ice
Flat Members Leg Diagonal Horizontal  Section  Round Members Flat Members	Ag =	199 Quantity Per Face  0 2 4 2 7 198 Quantity Per Face 0 2	Length (ft.)  0.0  15.0  13.6  11.8  sf  Length (ft.)  0.0  15.0	Width (in.)  0.0  6.0  3.0  3.0  z =  Width (in.)  0.0  5.0	Area (sf)  0.0  15.0 13.6 5.9  172  Area (sf)  0.0 12.5	ft Area w/ ice 0.0 17.5 18.2 7.8  type ft Area w/ ice	Wt. Per ft.  0.0  12.4  4.9  4.9  Wt. Per ft.	745.2 1068.2 460.6 Wt. (lbs.) Tower	0.0 303.3 1186.9 255.9 Wt. (lbs.) Ice
Flat Members Leg Diagonal Horizontal  Section  Round Members Flat Members Leg Diagonal	Ag =	199 Quantity Per Face  0 2 4 2 7 198 Quantity Per Face 0 2 4 2 4 4 4 4 7 198 Quantity Per Face 0	Length (ft.)  0.0  15.0  13.6  11.8  sf  Length (ft.)  0.0  15.0  13.6	Width (in.)  0.0  6.0  3.0  3.0  2 =  Width (in.)  0.0  5.0  3.0	Area (sf)  0.0  15.0  13.6  5.9  172  Area (sf)  0.0  12.5  13.6	ft Area w/ ice 0.0 17.5 18.2 7.8  type ft Area w/ ice 0.0	Wt. Per ft.  0.0  12.4 4.9 4.9  Wt. Per ft.  0.0	Tower 0.0 745.2 1068.2 460.6  Wt. (lbs.) Tower 0.0	0.0 303.3 1186.9 255.9 Wt. (lbs.) Ice 0.0
Round Members  Flat Members  Leg  Diagonal  Horizontal  Section  Round Members  Flat Members  Leg  Diagonal  Horizontal	Ag =	199 Quantity Per Face  0 2 4 2 7 198 Quantity Per Face 0 2	Length (ft.)  0.0  15.0  13.6  11.8  sf  Length (ft.)  0.0  15.0	Width (in.)  0.0  6.0  3.0  3.0  z =  Width (in.)  0.0  5.0	Area (sf)  0.0  15.0 13.6 5.9  172  Area (sf)  0.0 12.5	ft  Area w/ ice  0.0  17.5  18.2  7.8  type  ft  Area w/ ice  0.0  15.0	Wt. Per ft.  0.0  12.4 4.9 4.9  Wt. Per ft.  0.0  10.3	Tower 0.0 745.2 1068.2 460.6  Wt. (lbs.) Tower 0.0 618.6	0.0 303.3 1186.9 255.9 Wt. (lbs.) Ice 0.0 256.7

150 Old Westside Road North Conway, NH 03860 (603) 356-5214

Client:

**Structure Consulting Group** 

Job:

Portland, ME

Calculated By:

R. Adair

Job No.: **ME147110** 

Date: 19-Aug-02

#### Antennas

		Coeff.			Area (no	Area_	Force (no	Force	Weight	Weight			
<u>Type</u>	Elev. (z)	(C)	<u>Kz</u>	Qz	ice)	(ice)	ice)	(ice)	(no ice)	(w/ ice)			
7' whip	170	1.2	1.60	26.17	2.9	4.0	103	142	75	125			
(2) Omnis	162	1.2	1.58	25.81	0.5	8.0	17	29	60	100			
8' whip	161	1.2	1.57	25.77	2.9	4.0	99	139	75	125			
SS whip	160	1.2	1.57	25.72	5.1	7.4	178	256	100	150			
7' whip	156	1.2	1.56	25.54	2.3	3.3	77	112	75	125			
1' square panel	158	1.2	1.56	25.63	1.0	1.2	34	40	50	75			
Quad yagi	156	1.2	1.56	25.54	2.7	6.0	92	206	60	100			
Dual yagi	152	1.2	1.55	25.35	2.9	5.6	99	190	60	100			
Dual yagi	149	1.2	1.54	25.20	2.9	5.6	99	189	60	100			
Dual yagi	144	1.2	1.52	24.96	1.3	3.3	45	112	40	60			
			1.00	16.38			0	0					
Platform	150	1.2	1.54	25.25	26.8	37.0	909	1257	2200	3000			
Platform	137	1.2	1.50	24.61	15.0	21.7	496	717	900	1500			
Platform	131	1.2	1.48	24.29	43.7	57.0	1427	1863	1600	2400			
Platform	112	1.2	1.42	23.23	35.0	45.0	1094	1406	1500	2200			
			1.00	16.38			0	0					
			1.00	16.38			0	0					
			1.00	16.38			0	0					
			1.00	16.38			0	0					
Dishes											Azimuth	Ca	<u>Cs</u>
12' HP dish	157	0.00323	1.56	0.0032	120.3	121.9	4353	4410	900	1600		0.0032	0.0000
2' HP dish	158	0.00194	1.56	0.0019	4.1	4.4	89	96	50	80		0.0014	0.0014
12' HP dish	144	0.00323	1.52	0.0032	120.3	121.9	4246	4303	900	1600		0.0032	0.0000
12' HP dish	111	0.00289	1.41	0.0029	120.3	121.9	3524	3571	900	1600		0.0028	0.0008
		0.00000	1.00	0.0000			0	0		,,,,,		0.0020	0.000
		0.00000	1.00	0.0000			0	0					
		0.00000	1.00	0.0000			0	0					
Proposed Antennas	:												
6' HP dish	180	0.00324	1.62	26.60	28.3	29.5	1068	1113	250	500	228	0.0032	0.0003
6' HP dish	160	0.00324	1.57	25.72	28.3	29.5	1033	1076	250	500	228	0.0032	
4' dish w/radome	127	0.00027	1.47	24.08	14.2	14.8	146	152	150	250	330	0.0032	0.0003
6' HP dish	170	0.00037	1.60	26.17	28.3	29.5	1052	1096	250	500	230	0.0003	0.0003
6' dish w/radome	150	0.00324	1:54	25.25	28.3	29.5	452	471	250 250	500	230 287	0.0032	0.0003
2' square panel	165	1.4	1.58	25.25	4.0	4.3	163	177	50 50	100	201	0.0000	0.0012
2' square panel	160	1.4	1.57	25.72	4.0	4.3	161	175	50 50	100			
1' square panel	180	1.4	1.62	26.60	1.0	1.2	42	49	25	50			
6' dish				_0.00		1	7=		20				

Antenna Info.

150 Old Westside Road North Conway, NH 03860 (603) 356-5214

Client:

**Structure Consulting Group** 

Job: Calculated By: Portland, ME

R. Adair

Job No.:

ME147110

Date:

19-Aug-02

#### **Existing Wind Load Without Ice**

	Midpoint	Areas				Factors										Section		
Section	Height	Gross	Flats	Rounds	Ae	Aa	Df	Dr	Ca	Rr	Kz	Qz	Gh	е	Cf	Wind Load	Length	Uniform Load
1	93.25	269.7	31.3	0.0	31.3	17.68	1	1	1.2	0.58	1.35	22.05	1.12	0.12	2.90	2769 lbs.	12.5	222 lbs/ft.
2	105.75	246.6	32.6	0.0	32.6	17.59	1	1	1.2	0.58	1.39	22.85	1.12	0.13	2.84	2913 lbs.	12.5	233 lbs/ft.
3	118.25	223.4	31.6	0.0	31.6	16.51	1	1	1.2	0.58	1.44	23.59	1.12	0.14	2.80	2865 lbs.	12.5	229 lbs/ft.
4	130.75	200.3	31.7	0.0	31.7	16.51	1	1	1.2	0.58	1.48	24.28	1.12	0.16	2.74	2903 lbs.	12.5	232 lbs/ft.
5	143.25	177.2	47.5	0.0	47.5	15.38	1	1	1.2	0.61	1.52	24.92	1.12	0.27	2.38	3681 lbs.	12.5	294 lbs/ft.
6	157	198.8	34.5	0.0	34.5	11.82	1	1	1.2	0.59	1.56	25.58	1.12	0.17	2.69	3064 lbs.	15.0	204 lbs/ft.
7	172	197.5	32.0	0.0	32.0	5.51	1	1	1.2	0.58	1.60	26.26	1.12	0.16	2.73	2764 lbs.	15.0	184 lbs/ft.

#### Existing Wind Load With Ice

	Midpoint			Areas				Factors	;								Section	
Section	Height	Gross	Flats	Rounds	Ae	Ai	Df	Dr	Ca	Rr	Kz	Qz	Gh	е	Cf	Wind Load	Length	Uniform Load
1	93.25	269.7	39.7	0.0	39.7	30.70	1	1	1.2	0.58	1.35	22.05	1.12	0.15	2.78	3638 lbs.	12.5	291 lbs/ft.
2	105.75	246.6	42.0	0.0	42.0	30.57	1	1	1.2	0.58	1.39	22.85	1.12	0.17	2.70	3845 lbs.	12.5	308 lbs/ft.
3	118.25	223.4	40.6	0.0	40.6	29.01	1	1	1.2	0.59	1.44	23.59	1.12	0.18	2.66	3773 lbs.	12.5	302 lbs/ft.
4	130.75	200.3	40.1	0.0	40.1	29.01	1	1	1.2	0.59	1.48	24.28	1.12	0.20	2.59	3781 lbs.	12.5	302 lbs/ft.
5	143.25	177.2	55.3	0.0	55.3	27.15	1	1	1.2	0.62	1.52	24.92	1.12	0.31	2.26	4407 lbs.	12.5	353 lbs/ft.
6	157	198.8	43.5	0.0	43.5	20.13	1	1	1.2	0.59	1.56	25.58	1.12	0.22	2.53	3854 lbs.	15.0	257 lbs/ft.
7	172	197.5	41.0	0.0	41.0	7.22	1	1	1.2	0.59	1.60	26.26	1.12	0.21	2.57	3357 lbs.	15.0	224 lbs/ft.

Wind Loads Page 1

150 Old Westside Road North Conway, NH 03860 (603) 356-5214

Client:

**Structure Consulting Group** 

Job:

Portland, ME

Calculated By: R. Adair

Job No.:

ME147110

Date:

19-Aug-02

#### Proposed Wind Load Without Ice

	Midpoint			Areas			Facto	ors									Section	
Section	Height	Gross	Flats	Rounds	Ae	Aa	Df	Dr	Ca	Rr	Kz	Qz	Gh	е	Cf	Wind Load	Length	Uniform Load
1	93.25	269.7	31.3	0.0	31.3	26.57	1	1	1.2	0.58	1.35	22.05	1.12	0.12	2.90	3032 lbs.	12.5	243 lbs/ft.
2	105.75	246.6	32.6	0.0	32.6	26.48	1	1	1.2	0.58	1.39	22.85	1.12	0.13	2.84	3186 lbs.	12.5	255 lbs/ft.
3	118.25	223.4	31.6	0.0	31.6	25.40	1	1	1.2	0.58	1.44	23.59	1.12	0.14	2.80	3147 lbs.	12.5	252 lbs/ft.
4	130.75	200.3	31.7	0.0	31.7	24.85	1	1	1.2	0.58	1.48	24.28	1.12	0.16	2.74	3176 lbs.	12.5	254 lbs/ft.
5	143.25	177.2	47.5	0.0	47.5	23.59	1	1	1.2	0.61	1.52	24.92	1.12	0.27	2.38	3956 lbs.	12.5	316 lbs/ft.
6	157	198.8	34.5	0.0	34.5	19.67	1	1	1.2	0.59	1.56	25.58	1.12	0.17	2.69	3335 lbs.	15.0	222 lbs/ft.
7	172	197.5	32.0	0.0	32.0	9.38	1_	1_	1.2	0.58	1.60	26.26	1.12	0.16	2.73	2900 lbs.	15.0	193 lbs/ft.

#### Proposed Wind Load With Ice

	Midpoint			Areas				Factors	5								Section	
Section	Height	Gross	Flats	Rounds	Ae	Ai	Df	Dr	Ca	Rr	Kz	Qz	Gh	е	Cf _	Wind Load	Length	Uniform Load
1	93.25	269.7	39.7	0.0	39.7	44.28	1	1	1.2	0.58	1.35	22.05	1.12	0.15	2.78	4040 lbs.	12.5	323 lbs/ft.
2	105.75	246.6	42.0	0.0	42.0	44.15	1	1	1.2	0.58	1.39	22.85	1.12	0.17	2.70	4262 lbs.	12.5	341 lbs/ft.
3	118.25	223.4	40.6	0.0	40.6	42.59	1	1	1.2	0.59	1.44	23.59	1.12	0.18	2.66	4204 lbs.	12.5	336 lbs/ft.
4	130.75	200.3	40.1	0.0	40.1	41.62	1	1	1.2	0.59	1.48	24.28	1.12	0.20	2.59	4193 lbs.	12.5	335 lbs/ft.
5	143.25	177.2	55.3	0.0	55.3	39.52	1	1	1.2	0.62	1.52	24.92	1.12	0.31	2.26	4822 lbs.	12.5	386 lbs/ft.
6	157	198.8	43.5	0.0	43.5	32.00	1	1	1.2	0.59	1.56	25.58	1.12	0.22	2.53	4262 lbs.	15.0	284 lbs/ft.
7	172	197.5	41.0	0.0	41.0	13.02	1 _	1	1.2	0.59	1.60	26.26	1.12	0.21	2.57	3562 lbs.	15.0	237 lbs/ft.

Wind Loads

150 Old Westside Road North Conway, NH 03860 (603) 356-5214

Client:

**Structure Consulting Group** 

Job:

Portland, ME

Job No.: ME147110

Calculated By:

R. Adair

Date: 19-Aug-02

#### Uplift Due to Moment Minus 1/4 Dead & Ice Loads

<u></u>	Existing			Proposed	1	
	W <sub>o</sub> -DL	.75W <sub>i</sub> -DL-I	$W_i$ -DL-I	W₀-DL	.75W <sub>i</sub> -DL-I	W <sub>I</sub> -DL-I
Elev.	Force	Force	Force	Force	Force	Force
87	49.9	38.5	55.8	64.9	51.0	72.6
100	40.2	30.3	44.4	53.7	41.5	59.4
112	30.5	22.7	33.4	42.3	32.4	46.4
125	21.4	15.1	22.9	31.3	23.3	33.7
137	12.4	8.2	12.9	20.2	14.5	21.3
150	4.9	2.8	4.9	10.2	7.0	10.5
165	0.6	0.1	0.4	2.4	1.5	2.3

#### Tension in Anchor Bolts - Shear in Splice Bolts

		<b>Existing</b> W₀-DL	.75W <sub>I</sub> -DL-I	W <sub>I</sub> -DL-I	<b>Proposed</b> W <sub>o</sub> -DL	.75W <sub>i</sub> -DL-I	W <sub>I</sub> -DL-I
Elev.	# of Bolts	Force/Bolt	Force/Bolt	Force/Bolt	Force/Bolt	Force/Bolt	Force/Bolt
87	14	3.57	2.75	3.99	4.63	3.64	5.18
100	14	2.87	2.17	3.17	3.83	2.97	4.24
112	14	2.18	1.62	2.39	3.02	2.31	3.31
125	14	1.53	1.08	1.63	2.24	1.66	2.41
137	14	0.89	0.58	0.92	1.44	1.04	1.52
150	24	0.21	0.12	0.20	0.43	0.29	0.44
165	8	0.08	0.01	0.05	0.30	0.19	0.29

#### Shear in Anchor Bolts - Tension in Splice Bolts

			Existing		Proposed				
Elev.	Bolt Size (dia.)	W <sub>o</sub>	.75W <sub>1</sub>	W	W <sub>o</sub>	.75W	Wı		
87	3/4	0.68	0.61	0.82	0.74	0.67	0.90		
100	3/4	0.63	0.56	0.75	0.69	0.62	0.83		
112	3/4	0.49	0.45	0.59	0.55	0.50	0.66		
125	3/4	0.44	0.39	0.53	0.49	0.44	0.59		
137	3/4	0.36	0.31	0.41	0.45	0.39	0.52		
150	3/4	0.12	0.11	0.15	0.18	0.15	0.20		
165	3/4	0.09	0.08	0.11	0.19	0.16	0.22		

150 Old Westside Road North Conway, NH 03860 (603) 356-5214

Client:

**Structure Consulting Group** 

Job:

Portland, ME

R. Adair

Job No.: **ME147110** 

Date: 19-Aug-02

#### **Evaluation of Bracing Members**

Center Bolted?

Yes

Yield Strength (F<sub>Y</sub>):

Calculated By:

36 ksi

 $C^{C} =$ 

126.1

Section	Member	K Value	Length (ft.)	Area (in. <sup>2</sup> )	r <sub>x</sub> (in.)	r <sub>z</sub> (in.)	kL/r <sub>X</sub>	kL/r <sub>z</sub>
1	2L3.5 x 3.5 x1/4	1.0	16.32	3.380	1.090	0.694	134.8	141.1
2	2L3 x 2.5 x 1/4	1.0	22.43	2.620	0.945	0.528	213.6	254.9
3	2L3 x 2.5 x 1/4	1.0	20.90	2.620	0.945	0.528	199.1	237.5
4	L3.5 x 3 x 1/4	1.0	19.43	1.560	1.110	0.631	157.6	184.8
5	L3.5 x 3.5 x1/4	1.0	18.03	1.690	1.090	0.694	148.8	155.9
6	L3 x 3 x 1/4	1.0	13.63	1.440	0.930	0.592	131.9	138.1
7	L3 x 3 x 1/4	1.0	13.63	1.440	0.930	0.592	131.9	138.1

Section	All. Tens. (k)	F <sub>a</sub> (ksi)	All. Comp. (k)	Brace Angle	All. Shear (k)	Act. Shear (k)	Stress Ratio
1	73.01	7.50	25.34	0.55	167.41	40.89	24%
2	56.59	2.30	6.02	0.33	118.63	40.89	34%
3	56.59	2.65	6.93	0.36	119.00	35.18	30%
4	33.70	4.37	6.82	0.41	74.32	35.18	47%
5	36.50	6.15	10.39	0.38	87.20	27.65	32%
6	31.10	7.83	11.28	0.53	33.83	24.79	73%
7	31.10	7.83	11.28	0.53	33.83	19.96	59%

150 Old Westside Road North Conway, NH 03860 (603) 356-5214

Client:

**Structure Consulting Group** 

Job:

Portland, ME

Job No.: **ME147110** 

Calculated By:

R. Adair

Date: 19-Aug-02

#### **Evaluation of Leg Members**

						Existing		Propose	d
Section	Size	KI/r	Сс	Fa allow	133% Allow	D+W <sub>o</sub>	D+.75WI+I	D+W <sub>o</sub>	D+.75WI+I
1	6x6x5/8	66.10	126.04	16.83	22.43	8.78	8.26	10.95	10.18
2	6x6x5/8	66.10	126.04	16.83	22.43	7.24	6.84	9.20	8.56
3	6x6x5/8	66.10	126.04	16.83	22.43	5.52	5.21	7.25	6.72
4	6x6x1/2	68.64	126.04	16.57	22.09	5.04	4.80	6.84	6.37
5	6x6x1/2	55.08	126.04	17.89	23.85	3.09	2.96	4.51	4.21
6	6x6x5/16	75.00	126.04	15.90	21.20	2.26	2.25	3.81	3.62
7	5x5x5/16	90.54	126.04	14.13	18.85	0.45	0.46	1.11	1.07

#### **Percent Capacity**

	Existing				Propose	ed	Maximum			
Section	Base Elev.	D+W <sub>o</sub>	D+.75WI+I	Secondary	D+W <sub>o</sub>	D+./5WI+I	Secondary	Existing	Proposed	
1	87	39%	37%	0%	49%	45%	0%	39%	49%	
2	100	32%	30%	0%	41%	38%	0%	32%	41%	
3	112	25%	23%	0%	32%	30%	0%	25%	32%	
4	125	23%	22%	0%	31%	29%	0%	23%	31%	
5	137	13%	12%	0%	19%	18%	0%	13%	19%	
6	150	11%	11%	0%	18%	17%	0%	11%	18%	
7	165	2%	2%	0%	6%	6%	0%	2%	6%	

#### Maximum Reactions:

Uplift:

64.9 kips

Compression:

77.9 kips

Shear:

41.6 kips

# VERIZON WIRELESS (TENANT)

400 FRIBERG PARKWAY WESTBOROUGH, MA 01581 (508) 330-3343

# 45-55 FOREST AVENUE TOWER SITE

FOR THE:

INSTALLATION OF 9 NEW MICROWAVE ANTENNA TO BE MOUNTED ON THE MODIFIED 93' SHELF SUPPORT VERIZON NEW ENGLAND MICRO-WAVE TOWER.

#### PROJECT SUMMARY

SITE ADDRESS: 45-55 FOREST AVENUE

PORTLAND, MAINE 04101

OWNER/LESSOR

VERIZON NEW ENGLAND, INC.

5 DAVIS FARM ROAD PORTLAND, MAINE 04103

LESSEE:

VERIZON WIRELESS

400 FRIBERG PARKWAY WESTBOROUGH, MA 01581

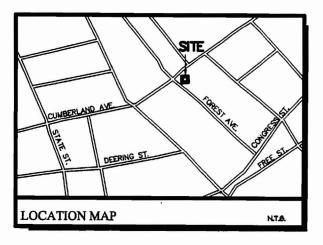
ASSESSORS MAP: TAX MAP 37, LOT 1

CURRENT ZONING: DOWN TOWN BUSINESS B3 & B3C

LATITUDE: N 43°-39'-21.0" LONGITUDE: W 70°-15'-50.0"

TOWER BASE ELEVATION: ASSUMED 87'

R STRUCTURAL REPORT



#### SITE DIRECTIONS:

ROM EXIT I OF THE MAINE TURNFIKE, TAKE INTERSTATE 89 NORTH ASTERLY TO EXIT 6A, TAKE INTERSTATE 289 NORTHEASTERLY TO ORTLAND EXIT 6, ROREST AVENUE EAST EXIT, TAKE A RIGHT ONTO OREST AVENUE AND PROCESSED SOUTHEASTERLY FOR 1/2 MILE TO TH TIE AT THE INTERSECTION OF FOREST AVENUE AND CUMBERLAND TREET.

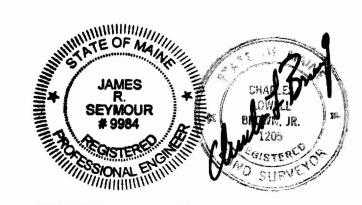
#### SHEET INDEX

1 of 4 COVER SHEET

2 of 4 SITE PLAN

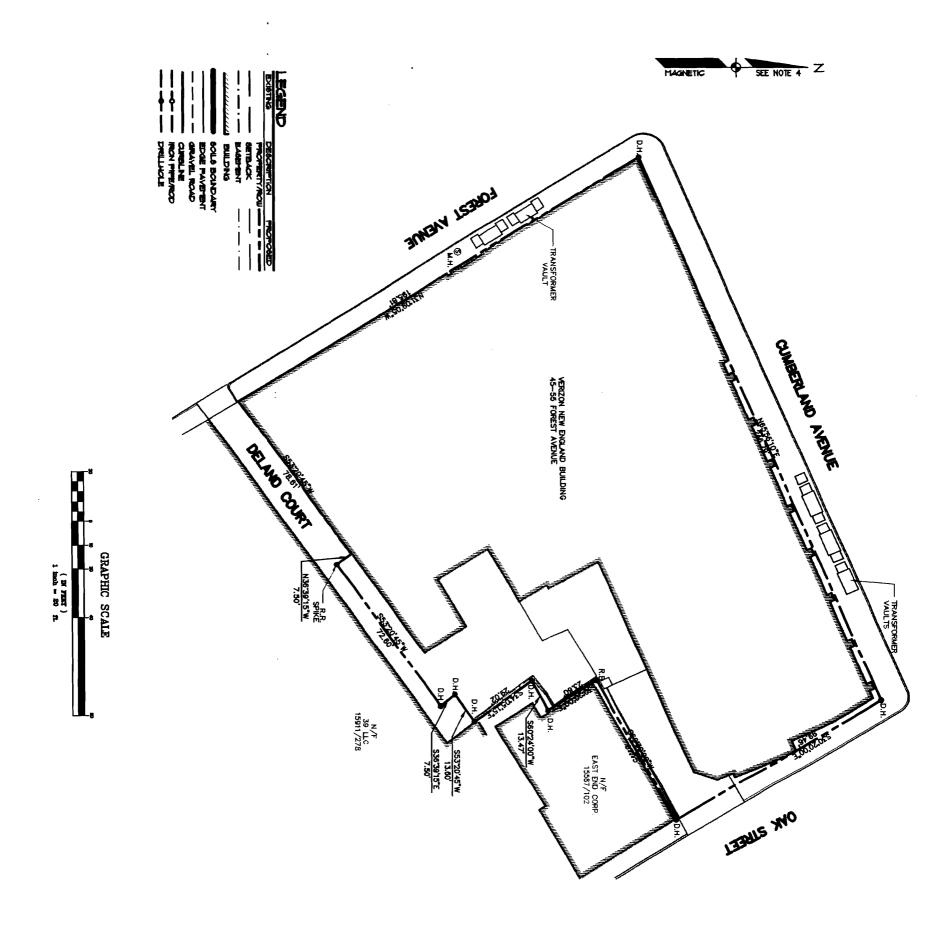
3 of 4 FLOOR PLAN

4 of 4 TOWER PROFILE / WITH EXTENSION





1 of 4 COVER SHEET PROJECT TITLE: PORTLAND TOWER SITE ISSUE DATE: APRIL 17, 2003 PROJECT NO.: 02342



2. THE PROPERTY IS SHOUN AS LOT IN BLOCK A OF THE CITY OF PORTLAND MANICIPAL TAX MAP NO. ST.

3. THIS LOT IS LOCATED IN 2 DOWN TOWN BUSINESS ZONNG DISTRICTS B-3 AND B-3C.

STE PLAN
OF:
PORTLAND TOWER SITE
45-55 FOREST AVENUE
PORTLAND, MANE 04103
FOR:
VERIZON WIRELESS
400 FRIBERG PARKWAY
WESTBOROUGH, MA 01581

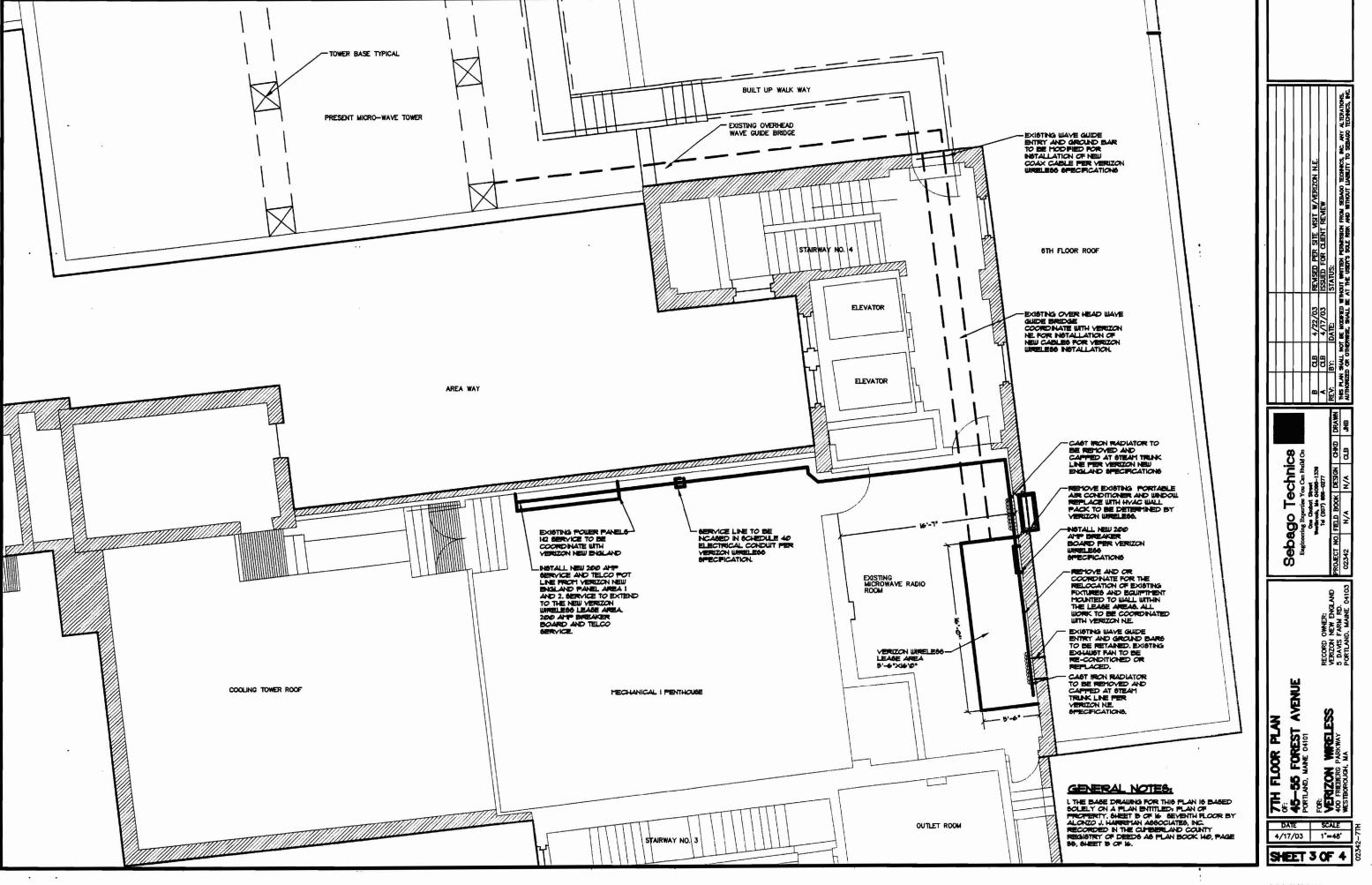
RECORD OWNER: VERIZON NEW ENGLAND 5 DAVIS FARM RD. PORTLAND, MAINE 04103

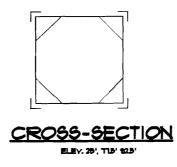
Sebago Technics Engineering Expertise You Can Build On

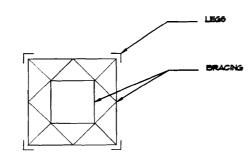
4	Heatbrook, Me 04 Tel (207) 806	1096-1339			Ŀ
PROJECT NO.	FIELD BOOK	DESIGN	CHKD	DRAWN	ŀ
02342	N/A	N/A	a.	JNB	L

<u> </u>	αв	4/17/03	ISSUED FOR CLIENT REVIEW	1
REV:	BY:	DATE:	STATUS:	
THIS PLA AUTHORI	N SHALL I ZED OR OT	NOT BE MODIFIED I	NITHOUT WRITTEN PERMISSION FROM SEBAGO TECHNICS, INC. ANY ALTERATIONS, E AT THE USER'S SOLE RISK AND WITHOUT LIABILITY TO SEBAGO TECHNICS, INC.	

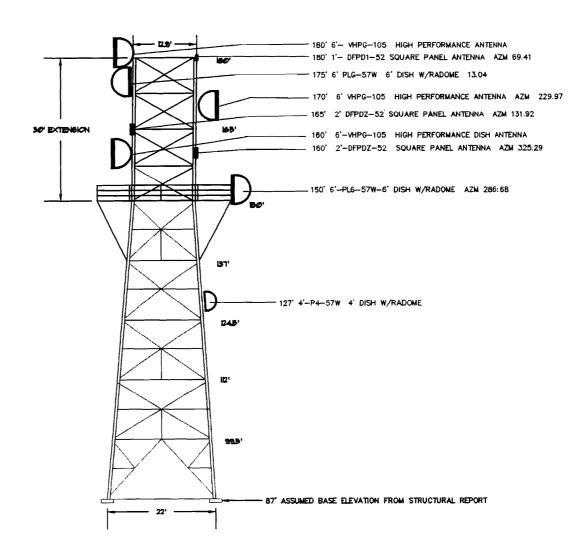
06/27/2003







CROSS-SECTION



## TOWER ELEVATION SCALE: 1'-10'

#### NOTE:

ONLY PROPOSED DISH INSTALLATIONS ARE SHOUN THE HEIGHT OF EACH ANTENNA IS BASED ON THE STRUCTURAL ANALSIS REPORT BY ALL POINTS TECHNOLOGY CORPARATION, P.C. DATED SEPTEMBER B, 2002, FOR LOCATION OF EXISTING ANTENNAS AND CURRENT 63" HIGH SELF SUPPORT TOWER, SEE STRUCTURAL REPORT.

				A   CLB   4/17/03   ISSUED FOR CLIENT REVIEW	REV: BY: DATE: STATUS:	CONTRACTOR OF THE CONTRACTOR PROPERTY OF THE CONTRACTOR OF THE CASE OF THE CAS	AUTHORIZED OR OTHERWISE, SAALL BE AT THE USER'S SOLE RISK AND WITHOUT LIABILITY TO SERVICE INC.	
	Sebado Technica	Engineering Expense You Can Build On	Weetbrook, Me 04086-1339	Tel (207) 856-0277	MANAGE CALL INCIDENTIAL PARCEL CALL TOTAL COM	TRUCK! NO. TIELD BOOK DESIGN CITY DICKING	02342 N/A N/A CLB JNB	
TOWER DROPHE ANTH EXTENSION	STANK THE PART OF	45-55 FOREST AVENUE	POR ILAME, MAINE UFICE		WENGEN WENGERSO	II 400 FRIBERG PARKWAY	WESTBOROUGH, MA 01581	

**06/27/2003** 



#### CITY OF PORTLAND, MAINE

#### **Department of Building Inspections**

1 1 1 20

_	to free to	20 /
Received from Sub-Caraba	<u>.u /51/2</u>	cares
Location of Work	to the section	26. Aug.
Cost of Construction \$	<u> </u>	Site Plan (U2)
Other		
CBL: 37.4 00/		
Check #:	Total Coll	ected s 7776

### THIS IS NOT A PERMIT

No work is to be started until PERMIT CARD is actually posted upon the premises. Acceptance of fee is no guarantee that permit will be granted. PRESERVE THIS RECEIPT. In case permit cannot be granted the amount of the fee will be refunded upon return of the receipt less \$10.00 or 10% whichever is greater.

WHITE - Applicant's Copy YELLOW - Office Copy PINK - Permit Copy