

### GENERAL NOTES

- THE NOTES ON THESE DRAWINGS ARE NOT INTENDED TO REPLACE SPECIFICATIONS. SEE SPECIFICATIONS FOR REQUIREMENTS IN ADDITION TO GENERAL NOTES. INCONSISTENCIES BETWEEN THESE DRAWINGS AND THE SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT PRIOR TO PROCEEDING WITH THE AFFECTED PORTION OF THE WORK.
- STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH JOB SPECIFICATIONS AND ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND SITE DRAWINGS. CONSULT THESE DRAWINGS FOR LOCATIONS AND DIMENSIONS OF OPENINGS, CHASES, INSERTS, REGLETS, SLEEVES, DEPRESSIONS, AND OTHER DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS.
- ALL DIMENSIONS, EXISTING CONDITIONS, AND AS-BUILT CONDITIONS MUST BE VERIFIED IN THE FIELD. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK.
- THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE ONLY AFTER THE STRUCTURAL WORK CONTAINED IN THE S- DRAWINGS IS COMPLETED. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO INSURE THE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS OR TIEDOWNS. SUCH MATERIAL SHALL REMAIN THE PROPERTY OF THE CONTRACTOR AFTER COMPLETION OF THE PROJECT.
- SECTIONS AND DETAILS SHOWN ON ANY STRUCTURAL DRAWINGS SHALL BE CONSIDERED TYPICAL FOR SIMILAR CONDITIONS AS DETERMINED BY THE STRUCTURAL ENGINEER. THE STRUCTURAL ENGINEER RESERVES THE RIGHT TO INTERPRET DETAILS TO ADDRESS OTHER PROJECT CONDITIONS.
- PROVIDE AND INSTALL NECESSARY MATERIAL TO CONNECT ELEVATOR SUPPORT BEAMS AND GUIDE RAILS. LOCATION AND SIZE OF MEMBERS AND ANY INSERTS REQUIRED SHALL BE DETERMINED BY THE ELEVATOR MANUFACTURER.
- THE CONTRACTOR SHALL SUBMIT COMPLETE SHOP DRAWINGS FOR ALL PARTS OF THE WORK, INCLUDING DESCRIPTION OF SHORING, AND CONSTRUCTION METHODS AND SEQUENCING WHERE APPLICABLE. NO PERFORMANCE OF THE WORK INCLUDING, BUT NOT LIMITED TO, FABRICATION OR ERECTION OF NEW STRUCTURAL ELEMENTS, SHALL COMMENCE WITHOUT REVIEW OF THE SHOP DRAWINGS BY THE ARCHITECT AND ENGINEER. FOR SHOP DRAWINGS AND SUBMITTALS REQUIRED, REFERENCE THE PROJECT SPECIFICATIONS.
- ALL APPLICABLE FEDERAL, STATE, AND MUNICIPAL REGULATIONS SHALL BE FOLLOWED, INCLUDING THE FEDERAL DEPARTMENT OF LABOR OCCUPATIONAL SAFETY AND HEALTH ACT.
- IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE (2003 EDITION, SECTION 1704.1), A STATEMENT OF SPECIAL INSPECTIONS IS REQUIRED AS A CONDITION FOR PERMIT ISSUANCE BY THE LOCAL CODE OFFICIAL. THIS STATEMENT SHALL INCLUDE A COMPLETE LIST OF MATERIALS AND WORK REQUIRING SPECIAL INSPECTIONS, THE INSPECTIONS TO BE PERFORMED AND A LIST OF THE INDIVIDUALS, APPROVED AGENCIES AND FIRMS INTENDED TO BE RETAINED FOR CONDUCTING SUCH INSPECTIONS.
- REFERENCE THE PROJECT SPECIFICATIONS FOR ALL TESTING REQUIREMENTS.

### DESIGN LOADS

- BUILDING CODE:  
INTERNATIONAL BUILDING CODE, 2003 EDITION  
ASCE 7-02 MINIMUM DESIGN LOADS FOR BUILDINGS  
AND OTHER STRUCTURES.
- DESIGN FLOOR LIVE LOADS:  
LOBBIES & MEETING ROOM: 100 PSF  
FIRST FLOOR CORRIDOR: 100 PSF  
PRIVATE ROOMS & CORRIDORS SERVING: 40 PSF  
+ PARTITIONS  
STAIRS: 100 PSF  
ROOF: 100 PSF
- DESIGN ROOF SNOW LOAD:  
GROUND SNOW LOAD (Pg): 60 PSF  
SNOW EXPOSURE FACTOR (Ce): 1.0  
SNOW LOAD IMPORTANCE FACTOR (Is): 1.0  
SNOW LOAD THERMAL FACTOR (Ct): 1.1  
FLAT ROOF SNOW LOAD (Pf): 46 PSF + DRIFT
- DESIGN WIND LOAD:  
BASIC WIND SPEED: 100 MPH  
WIND LOAD IMPORTANCE FACTOR (Iw): 1.00  
WIND EXPOSURE: C  
INTERNAL PRESSURE COEFFICIENT: ±0.18  
COMPONENTS & CLADDING LOADS PER ASCE 7-02
- DESIGN SEISMIC LOADS:  
EQUIVALENT LATERAL FORCE PROCEDURE  
SEISMIC USE GROUP: I  
SEISMIC IMPORTANCE FACTOR (Ie): 1.0  
MAPPED SPECTRAL RESPONSE ACCELERATIONS:  
Ss: 0.369  
S1: 0.098  
SEISMIC SITE CLASS: C  
SPECTRAL RESPONSE COEFFICIENTS:  
Sds: 0.442  
Sd1: 0.167  
SEISMIC DESIGN CATEGORY: B  
BASIC STRUCTURAL SYSTEM: BUILDING FRAME SYSTEM  
BASIC SEISMIC FORCE RESISTING SYSTEM:  
ORDINARY STEEL CONCENTRICALLY BRACED FRAMES  
ORDINARY REINFORCED CONCRETE SHEARWALL  
RESPONSE MODIFICATION FACTOR (R): X: 3.0  
Y: 3.0  
NOTE: R OF 3 USED FOR STRUCTURAL STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE  
SEISMIC RESPONSE COEFFICIENT (Cs), X: 0.098  
Y: 0.098

### FLOWABLE FILL NOTES

- FLOWABLE FILL WORK SHALL CONFORM TO "CONTROLLED LOW STRENGTH MATERIALS (CLSM)", ACI 229-LATEST, AVAILABLE THROUGH THE AMERICAN CONCRETE INSTITUTE (248) 848-3800.
- FLOWABLE FILL MIX DESIGN: SUBMIT FLOWABLE FILL MIX DESIGN FOR REVIEW PRIOR TO WORK. PERFORMANCE DESIGN OF FLOWABLE FILL MIX SHALL MEET THE FOLLOWING PROPERTIES:  
28-DAY COMPRESSIVE STRENGTH: 2,000 PSI (MINIMUM)  
PROVIDE MATERIAL PROPERTIES ARE REQUIRED TO MAINTAIN WORKABILITY AND OBTAIN SPECIFIED STRENGTH.
- FLOWABLE FILL SHALL BE PROTECTED FROM FREEZING FOR A PERIOD OF 36 HOURS AFTER PLACEMENT.

### FOUNDATION NOTES (BEDROCK SUPPORTED)

- FOUNDATIONS HAVE BEEN DESIGNED IN ACCORDANCE WITH A REPORT ENTITLED "REPORT ON SUBSURFACE EXPLORATIONS AND FOUNDATION DESIGN RECOMMENDATIONS, THE PROPOSED WATERVIEW APARTMENTS AT BAYSIDE, 409 CUMBERLAND AVENUE, PORTLAND, MAINE", DATED MARCH 2005 WITH A SUPPLEMENTAL MEMORANDUM DATED JUNE 30, 2005, ENTITLED "SUPPLEMENTAL RECOMMENDATIONS - DRILLED SHAFT FOUNDATIONS". THE RECOMMENDATIONS OF THESE DOCUMENTS ARE PART OF THIS WORK. REFER TO THESE DOCUMENTS FOR SPECIFIC RECOMMENDATIONS.
- FOUNDATION DESIGN IS BASED ON SHALLOW SPREAD FOOTINGS AND DRILLED PIERS BEARING ON SUITABLE UNDISTURBED NATIVE BEDROCK OR FLOWABLE FILL EXTENDING TO UNDISTURBED NATIVE BEDROCK PER THE REQUIREMENTS OF THE GEOTECHNICAL REPORT. REFER TO THIS REPORT FOR SPECIFIC BEARING RECOMMENDATIONS.
- ALLOWABLE BEARING CAPACITY 35 TONS PER SQUARE FOOT.
- EXTEND BOTTOM OF EXTERIOR FOOTINGS AT LEAST 4.5 FEET BELOW THE FINAL EXTERIOR GRADE FOR PROTECTION AGAINST FROST.
- NO CONCRETE OR FLOWABLE FILL FOR BUILDING SUPPORT SHALL BE PLACED UNTIL SUBGRADES HAVE BEEN OBSERVED AND APPROVED BY THE GEOTECHNICAL ENGINEER.
- REFERENCE THE GEOTECHNICAL REPORT FOR ALL EXCAVATION, BACKFILL, COMPACTION, CONSTRUCTION DEWATERING AND PERMANENT DRAINAGE REQUIREMENTS.
- SOILS EXPOSED AT THE BASE OF ALL SATISFACTORY FOUNDATION EXCAVATIONS SHOULD BE PROTECTED AGAINST ANY DETRIMENTAL CHANGE IN CONDITION, SUCH AS DISTURBANCE FROM RAIN OR FROST. SURFACE RUNOFF SHOULD BE DRAINED AWAY FROM THE EXCAVATIONS AND NOT BE ALLOWED TO POND. FOUNDATION EXCAVATIONS AND SHOULD BE ADEQUATELY PROTECTED FROM RAINFALL OR FREEZING CONDITIONS. GROUNDWATER SHOULD BE ANTICIPATED FOR EXCAVATIONS AND APPROPRIATE DEWATERING MEASURES SHALL BE EMPLOYED.
- DRILLED PIER BEARING STRATA, CAPACITY AND EMBEDMENT DEPTH SHALL BE VERIFIED BY A QUALIFIED GEOTECHNICAL ENGINEER BEFORE PLACING CONCRETE. SEE SPECIFICATIONS FOR DETAILS.
- DRILLED PIERS SHALL BE DRILLED IN PLACE AND FILLED WITH CONCRETE. THE BED AND SHAFT SHALL BE LEVELED AND CLEARED OF ALL LOOSE MATERIAL BEFORE CONCRETE IS PLACED. THE SHAFT SHALL BE KEPT FREE OF WATER.
- THE TERMS DRILLED SHAFT AND DRILLED PIER ARE INTERCHANGEABLE FOR THESE DOCUMENTS.
- EXCAVATIONS FOR BUILDING CONSTRUCTION SHALL BE IN ACCORDANCE WITH OSHA REQUIREMENTS. BRACED EXCAVATIONS SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF MAINE. DO NOT UNDERMINE EXISTING FOUNDATIONS OF ANY ADJACENT STRUCTURES. REFER TO THE GEOTECHNICAL REPORT FOR ADDITIONAL AND/OR MORE SPECIFIC REQUIREMENTS.

### CONCRETE NOTES

- CONCRETE WORK SHALL CONFORM TO "ACI MANUAL OF CONCRETE PRACTICE", LATEST EDITION. THIS PUBLICATION IS AVAILABLE THROUGH THE AMERICAN CONCRETE INSTITUTE (248) 848-3800.
- ALL CONCRETE SHALL HAVE A 28-DAY COMPRESSIVE STRENGTH AS FOLLOWS:  
GRADE BEAM/FOOTING CONCRETE STRENGTH: 4,000 PSI.  
CONCRETE WALLS & PIERS (PILASTERS): 4,000 OR 5,000 PSI. (SEE DIAGRAM S1.1)  
DRILLED PIER FILL: 4,000 PSI  
INTERIOR SLABS-ON-GRADE EXCLUDING GARAGE SLAB: 3,000 PSI.  
PLANK TOPPING: 3,000 PSI.  
EXTERIOR SLAB-ON-GRADE/GARAGE SLAB: 5,000 PSI.  
EXTERIOR SITE RETAINING WALLS: 3,000 PSI.  
ADDITIONAL CONCRETE MIX PERFORMANCE DATA INCLUDING AIR CONTENT, WATER-CEMENT RATIO, AIR CONTENT, AGGREGATE SIZE, SLUMP, ETC. HAS BEEN INCLUDED IN THE PROJECT SPECIFICATIONS. SEE THE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.
- CONCRETE SHALL NOT BE PLACED IN WATER OR ON FROZEN GROUND.
- PROVIDE PVC SLEEVES WHERE PIPES PASS THROUGH EXTERIOR CONCRETE, OR SLABS.
- REINFORCING BARS SHALL CONFORM TO ASTM A615 GRADE 60 DEFORMED BARS AND SHALL BE DETAILED, FABRICATED AND PLACED IN ACCORDANCE WITH ACI 315, LATEST EDITION.
- WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185 AND BE PROVIDED IN FLAT SHEETS.
- MINIMUM CONCRETE PROTECTIVE COVERING FOR REINFORCEMENT, UNLESS NOTED OTHERWISE, SHALL BE AS FOLLOWS:  
A) SURFACES CAST AGAINST AND PERMANENTLY IN CONTACT WITH EARTH, 3.0"  
B) FORMED SURFACES IN CONTACT WITH EARTH OR EXPOSED TO WEATHER #5 BARS, 5/8" DIAMETER WIRE, AND SMALLER, 1.5"  
#6 THROUGH #11 BARS, 2.0"  
C) SURFACES NOT IN CONTACT WITH EARTH OR EXPOSED TO WEATHER WALLS, SLABS, JOISTS #11 BARS AND SMALLER, 1.0"  
BEAMS, GIRDERS, AND COLUMNS; ALL REINFORCEMENT, 1.5"
- REINFORCEMENT SHALL BE CONTINUOUS AROUND CORNERS AND AT INTERSECTIONS. PROVIDE LAPPED BARS AT NECESSARY SPLICES OR HOOKED BARS AT DISCONTINUOUS ENDS. PROVIDE TENSION LAP SPLICES PER THE SCHEDULE ON S2.1, FOR ALL REINFORCING UNLESS OTHERWISE SHOWN ON PLAN.
- WELDING OF REINFORCEMENT IS NOT PERMITTED.
- FOR ALL OPENINGS IN CONCRETE WALLS AND SLABS, PROVIDE SUPPLEMENTAL REINFORCING AROUND OPENING AS SHOWN ON THE CONTRACT DOCUMENTS TYPICAL DETAILS.
- CONSTRUCTION JOINTS SHOWN ON DRAWINGS ARE MANDATORY. OMISSIONS, ADDITIONS, OR CHANGES SHALL NOT BE MADE EXCEPT WITH THE SUBMITTAL OF A WRITTEN REQUEST TOGETHER WITH DRAWINGS OF THE PROPOSED JOINT LOCATIONS FOR APPROVAL OF THE STRUCTURAL ENGINEER. WHERE CONSTRUCTION JOINTS ARE NOT SHOWN, OR WHEN ALTERNATE LOCATIONS ARE PROPOSED, DRAWINGS SHOWING LOCATION OF CONSTRUCTION AND CONTROL JOINTS AND CONCRETE PLACING SEQUENCE SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW PRIOR TO PREPARATION OF THE REINFORCEMENT SHOP DRAWINGS. CONCRETE SHALL BE PLACED WITHOUT HORIZONTAL CONSTRUCTION JOINTS EXCEPT WHERE SHOWN OR NOTED. VERTICAL CONSTRUCTION JOINTS AND STOPS IN CONCRETE BEAMS/GRADE BEAMS SHALL BE MADE AT MIDSPAN OR AT POINTS OF MINIMUM SHEAR, UNLESS NOTED OTHERWISE.
- SPACING OF CONSTRUCTION JOINTS, UNLESS NOTED OTHERWISE SHALL BE AS FOLLOWS:  
A) FOOTINGS AND WALLS MAX LENGTH 40'-0" NOR 15'-0" FROM ANY CORNER\*\*  
B) SLABS ON GRADE SEE FOUNDATION PLAN
- EXCEED ONLY WHERE INTERMEDIATE CONTRACTION JOINTS ARE PROVIDED. MINIMUM OF 72 HOURS SHALL ELAPSE BETWEEN ADJACENT CONCRETE PLACEMENTS.
- ANCHOR RODS SHALL BE HEADED RODS CONFORMING TO ASTM F1554, GRADE AS NOTED ON DRAWINGS.
- ALL GROUT BENEATH BASE PLATES & BEARING PLATES SHALL BE "5-STAR" 5000-PSI NON-SHRINK GROUT BY U.S. GROUT CORP.

### STRUCTURAL STEEL NOTES

- STRUCTURAL STEEL FABRICATION, ERECTION, AND CONNECTION DESIGN SHALL CONFORM TO AISC "SPECIFICATION FOR THE DESIGN FABRICATION, AND ERECTION OF STRUCTURAL STEEL" 9TH EDITION, AND THE "CODE OF STANDARD PRACTICE, LATEST EDITION.
- STRUCTURAL STEEL: STEEL PLATES, SHAPES, AND BARS, CONFORM TO ASTM A36 UNLESS NOTED OTHERWISE (U.N.O.). STRUCTURAL STEEL SHAPES DESIGNATED ON THE DRAWINGS FOR WIDE-FLANGE SECTIONS: ASTM A992 (ASTM A572 GRADE 50 WITH SPECIAL REQUIREMENTS PER AISC TECHNICAL BULLETIN #3 DATED MARCH, 1997)
- STRUCTURAL TUBING: CONFORM TO ASTM A500 GRADE B46 K51.
- FIELD CONNECTIONS SHALL BE BOLTED USING 3/4" DIAMETER ASTM A325N HIGH STRENGTH BOLTS (U.N.O.) EXCEPT WHERE SLIP CRITICAL CONNECTIONS ARE REQUIRED AND NOTED BY A325 (SC) ON THE DRAWINGS. PROVIDE SLIP CRITICAL (SC) CONNECTIONS AT ALL MOMENT CONNECTIONS, BRACED FRAMES, RELIEVING ANGLES AND AS OTHERWISE NOTED. USE A490 BOLTS WHERE INDICATED.
- WHERE WELDING IS INDICATED, ALL WELDING SHALL CONFORM TO AWS D1.1-LATEST EDITION. ELECTRODES SHALL BE CONFORM TO AWS A5.1 E70XX SERIES WITH PROPER ROD TO PRODUCE OPTIMUM WELD (LOW HYDROGEN).
- SEE CONCRETE NOTES AND DRAWINGS FOR ANCHOR BOLT INFORMATION, TYP.
- PROVIDE 3/8" MINIMUM STIFFENER PLATES EACH SIDE OF BEAM WEB AT BEAMS FRAMING OVER COLUMNS AND AT BEAMS SUPPORTING COLUMNS ABOVE.
- PROVIDE 1/4" THICK LEVELING PLATE UNDER ALL COLUMN BASE PLATES UNLESS OTHERWISE NOTED. LEVELING PLATES SHALL BE SET AND GROUTED PRIOR TO ERECTING COLUMNS.
- PROVIDE ALL MISCELLANEOUS ANGLES, PLATES, ANCHORS, BOLTS, ETC., SHOWN ON ARCHITECTURAL DRAWINGS FOR SUPPORT OF BLOCKING, PARAPETS, FINISHES, ETC. COORDINATE WITH MISCELLANEOUS METAL FABRICATOR TO ENSURE COMPLETE COVERAGE OF ALL ITEMS.
- PROVIDE L 4 x 4 x 3/8 SLAB SUPPORT ANGLE AS REQUIRED AT COLUMNS WHERE STRUCTURAL MEMBERS DO NOT FRAME IN AT ALL FOUR SIDES.

### GIRDER-SLAB STRUCTURAL SYSTEM NOTES

- THE OPEN WEB DISSYMMETRIC BEAM (DB) SHALL BE FABRICATED FROM ASTM A 992/A572, GRADE 50 STANDARD WIDE FLANGE SECTIONS WITH GRADE 50 FLAT BAR AT THE TOP FLANGE AND SHALL MEET AISC STANDARDS (EXCEPT FOR DEPTH, TOLERANCE ±1/8 INCH), UNPAINTED UNLESS NOTED OTHERWISE. PROVIDE CAMBER WHERE SPECIFIED. CAMBER CAN BE BUILT IN DURING ASSEMBLY OF THE DISSYMMETRIC BEAM.
- ERECTOR IS RESPONSIBLE FOR DETERMINING AND PROVIDING ALL SHORING AS NECESSARY TO ERECT THE SUPERSTRUCTURE, AS PART OF THE CONTRACTOR'S MEANS AND METHODS REQUIREMENTS. IN ADDITION, PROVIDE SHORING WHERE REQUIRED FOR PRE-COMPOSITE STRENGTH.
- MINIMUM BEARING OF PRECAST PRESTRESSED HOLLOW CORE SLAB UNITS ON DISSYMMETRIC BEAMS SHALL BE 2 INCHES. OPEN THE TOP OF EACH SLAB CORE FOR INSPECTION PRIOR TO GROUTING. REFER TO THE GIRDER-SLAB SYSTEM REFERENCE INFORMATION FOR ADDITIONAL REQUIREMENTS.
- REINFORCING STEEL (ASTM A615, GRADE 60) SHALL BE PLACED THROUGH THE DISSYMMETRIC BEAM WEB OPENINGS INTO THE SLAB CORES.
- CEMENTITIOUS GROUT (MIN. 4000 PSI) SHALL BE PLACED MONOLITHICALLY AROUND AND THROUGH THE DISSYMMETRIC BEAM WEB OPENINGS AND INTO THE SLAB CORES FILLED SOLID FOR A MINIMUM OF 8 INCHES. LEVEL TO THE SLAB SURFACE WITH A 9/16 INCH MINIMUM AVERAGE THICKNESS OVER THE TOP FLANGE (U.N.O.) ATTAIN SPECIFIED STRENGTH OF GROUT PRIOR TO PLACEMENT OF PLANK TOPPING.
- THE GIRDER-SLAB SYSTEM SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE UNDERWRITERS LABORATORIES, INC. FLOOR CEILING ASSEMBLY SPECIFIED BY THE ARCHITECT.
- THE GIRDER-SLAB SYSTEM AND D-BEAM GIRDERS SHALL BE DISTRIBUTED AND ASSEMBLED BY STEEL CONTRACTORS AUTHORIZED BY GIRDER-SLAB TECHNOLOGIES LLC OF NJ IN CONFORMANCE WITH ITS DESIGN-GUIDE & DISTRIBUTION REQUIREMENTS. STEEL CONTRACTOR/DISTRIBUTOR CONTACT INFORMATION: 1-888-478-1100 OR WWW.GIRDER-SLAB.COM.
- THE DISTRIBUTOR OF THE GIRDER-SLAB SYSTEM SHALL PROVIDE TO THE PROJECT OWNER AND THE ARCHITECT A GIRDER-SLAB COMPLIANCE CERTIFICATE UPON COMPLETION OF SYSTEM ASSEMBLY AND CONSTRUCTION.
- COMPLY WITH ALL APPLICABLE PROVISIONS OF THE STANDARDS AND CODES REFERENCED IN THE PROJECT SPECIFICATIONS.

### METAL DECK

- THE METAL ROOF DECK SHALL BE FORMED OF STEEL SHEETS CONFORMING TO ASTM STANDARD A611.
- ROOF DECK SHALL BE AS NOTED ON THE DRAWINGS (OR EQUIVALENT).
- FOR DECK ATTACHMENTS, PENETRATIONS AND ACCESSORIES, REFER TO SPECIFICATIONS.

### PRECAST CONCRETE HOLLOW CORE PLANK AND STAIRS

- ALL WORK SHALL CONFORM TO THE LATEST EDITION OF THE FOLLOWING: ACI 318 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE", PCI MNL-116 "MANUAL FOR QUALITY CONTROL FOR PLANTS AND PRODUCTION OF PRECAST AND PRESTRESSED CONCRETE PRODUCTS" AND PCI "DESIGN HANDBOOK-PRECAST AND PRESTRESSED CONCRETE".
- PRECAST HOLLOW CORE PLANK AND STAIRS SHALL BE DESIGNED FOR THE LIVE LOADS AS INDICATED UNDER "DESIGN LOADS" THIS SHEET. DESIGN SHOULD INCLUDE FOR ALL DEAD LOADS DUE TO SELF WEIGHT AND APPLIED TOPPING AND TREATMENTS.
- CONCRETE STRENGTH SHALL BE MINIMUM 5000 PSI AT 28 DAYS.
- PRESTRESSING TENDONS SHALL CONFORM WITH ASTM A416, GRADE 250.
- COMPLETE SHOP DRAWINGS AND DESIGN CALCULATIONS STAMPED BY A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE STATE OF MAINE SHALL BE SUBMITTED TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR REVIEW PRIOR TO COMMENCEMENT OF THAT PORTION OF THE WORK.
- COORDINATE WITH ALL ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWING FOR LOCATION OF CUTS AND PENETRATIONS. INDICATE LOCATION OF ALL OPENINGS ON SHOP DRAWINGS. INDICATE IF PENETRATIONS ARE SHOP OR FIELD CUT.
- COORDINATE PLANK REQUIREMENTS WITH GIRDER-SLAB SYSTEM REQUIREMENTS.

### VEENEER LINTELS

- THE FOLLOWING MINIMUM LINTELS SHALL BE USED FOR VEENEER OPENING, UNO:

MASONRY OPENING	LINTEL SIZE
UP TO 4'-6"	L 4 x 4 x 5/16
4'-7" TO 8'-0"	L 6 x 4 x 5/16 (LLV)
8'-1" TO 12'-0"	L 6 x 4 x 3/8 (LLV)

- PROVIDE 8" OF BEARING AT EACH END OF ALL LINTELS.
- ALL STEEL ANGLE LINTELS SHALL BE HOT-DIPPED GALVANIZED.



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THE WATERVIEW  
AT BAYSIDE  
CONDOMINIUM

CUMBERLAND AVE

PORTLAND, MAINE

Project No: 2004.414 WVB

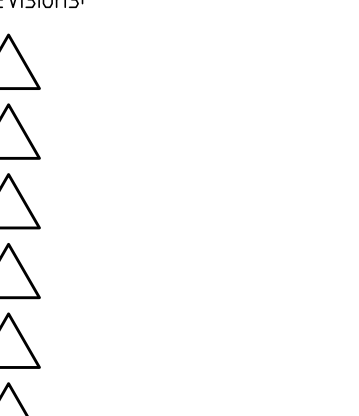
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