

DISPLAY THIS CARD ON PRINCIPAL FRONTAGE OF WORK CITY OF PORTLAND

Please Read Application And Notes, If Any, Attached

BUILDING INSPECTION PERMIT

PERMIT ISSUED
Permit Number: 061030
NOV - 9 2006
CITY OF PORTLAND

This is to certify that PORTLAND ARCHITECTURAL SALVAGE INC/TBD
has permission to Connector Addition will incorporate new elevator steel stairs & new lobby
AT 131 PREBLE ST

provided that the person or persons firm or corporation accepting this permit shall comply with all of the provisions of the Statutes of the State and of the Ordinances of the City of Portland regulating the construction, maintenance and use of buildings and structures, and of the application on file in this department.

Apply to Public Works for street line and grade if nature of work requires such information.

Classification of inspection must be given and when permission is granted before this building or part thereof is started or otherwise closed-in. FOUR NOTICES REQUIRED

A certificate of occupancy must be procured by owner before this building or part thereof is occupied.

OTHER REQUIRED APPROVALS
Fire Dept. Greg Cass See Contracting 9-19-06
Health Dept. _____
Appeal Board _____
Other _____
Department Name _____

Jeanie Banker 11/8/06
Director - Building & Inspection Services

PENALTY FOR REMOVING THIS CARD

City of Portland, Maine - Building or Use Permit

389 Congress Street, 04101 Tel: (207) 874-8703, Fax: (207) 874-8716

Permit No: 06-1030	Date Applied For: 07/13/2006	CBL: 034 I001001
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Location of Construction: 131 PREBLE ST	Owner Name: PORTLAND ARCITECTURAL SA	Owner Address: 919 CONGRESS ST	Phone:
Business Name:	Contractor Name: TBD	Contractor Address:	Phone:
Lessee/Buyer's Name	Phone:	Permit Type: Additions - Commercial	

Proposed Use: Retail Warehouse- 3 Story connector addition connecting 131 Preble Street & 123 Kennebec St. Addition will incorporate new elevator, steel staircase & new lobby	Proposed Project Description: Connector Addition will incorporate new elevator, steel staircase & new lobby
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Dept: Zoning	Status: Approved with Conditions	Reviewer: Ann Machado	Approval Date: 07/20/2006	Ok to Issue: <input checked="" type="checkbox"/>
Note: 1) This permit is being approved on the basis of plans submitted. Any deviations shall require a separate approval before starting that work.				
Dept: Building	Status: Pending	Reviewer:	Approval Date:	Ok to Issue: <input type="checkbox"/>
Note:				
Dept: Fire	Status: Approved	Reviewer: Cptn Greg Cass	Approval Date: 09/19/2006	Ok to Issue: <input checked="" type="checkbox"/>
Note: 1) All work as drawn. Sprinkler, Fire Alarm , and Standpipe certification letters; for entire structure shall be required.				

Comments: 9/22/2006-mjn: left message w/ designer, need statement of special inspections and 2 hour fire separation assemblies with openings protected. 10/24/2006-ldobson: Todd Dana dropped additional information off rerouted permit to MJN. Lannie 11/6/2006-jmb: Todd Dana called to see if we received the fax of the new special inspection form for IBC 2003....no we have not. 9/19/2006-dmartin: Recieved additional info took permit off hold and routed w/ info to Fire 11/8/2006-jmb: Received updated statement of special inspections 8/14/2006-gg: received granted site plan exemption. /gg
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General Building Permit Application

If you or the property owner owes real estate or personal property taxes or user charges on any property within the City, payment arrangements must be made before permits of any kind are accepted.

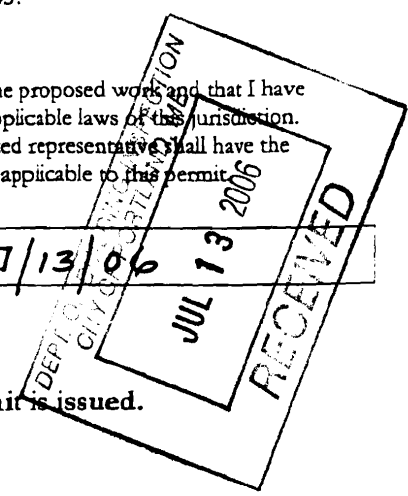
Location/Address of Construction: 123-129 Kennebec Street		
Total Square Footage of Proposed Structure 1480 Sq. feet		Square Footage of Lot 15,588 Sq. feet
Tax Assessor's Chart, Block & Lot Chart# Block# Lot# 34 I 1	Owner: TOD DANA / ALICE DUNN P.O. Box 169 Portland, Me. 04112	Telephone: 207.671.5566
Lessee/Buyer's Name (If Applicable) Eric Stark (Designer) 776-5227	Applicant name, address & telephone: Tod Dana P.O. Box 169 Portland, Me. 04112 207.761.8280 (Fax)	Cost Of Work: \$ 175,000.- Fee: \$ 1,845 C of O Fee: \$ 75.-
Current Specific use: Retail/Warehouse 671.5566 (Phone)		
Proposed Specific use: same		
Project description: 3 story connector addition, connecting 131 Preble St and 123-129 Kennebec St. Addition will incorporate new elevator, steel staircase, and new entrance lobby.		
Contractor's name, address & telephone: TBD		
Who should we contact when the permit is ready: _____		
Mailing address: _____ Phone: _____		

Please submit all of the information outlined in the Commercial Application Checklist. Failure to do so will result in the automatic denial of your permit.

In order to be sure the City fully understands the full scope of the project, the Planning and Development Department may request additional information prior to the issuance of a permit. For further information visit us on-line at www.portlandmaine.gov, stop by the Building Inspections office, room 315 City Hall or call 874-8703.

I hereby certify that I am the Owner of record of the named property, or that the owner of record authorizes the proposed work and that I have been authorized by the owner to make this application as his/her authorized agent. I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in this application is issued, I certify that the Code Official's authorized representative shall have the authority to enter all areas covered by this permit at any reasonable hour to enforce the provisions of the codes applicable to this permit.

Signature of applicant: [Signature]	Date: 7/13/06
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This is not a permit; you may not commence ANY work until the permit is issued.



CITY OF PORTLAND
BUILDING CODE CERTIFICATE
389 Congress St., Room 315
Portland, Maine 04101

TO: Inspector of Buildings City of Portland, Maine
Department of Planning & Urban Development
Division of Housing & Community Service

FROM: _____

RE: Certificate of Design

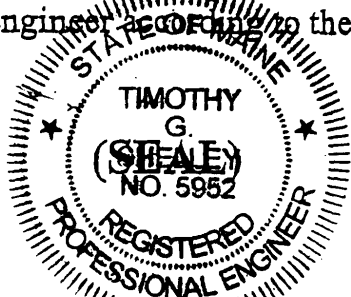
DATE: 7/13/06

These plans and/ or specifications covering construction work on:

123-129 Kennebec St.

"Kennebec Street Addition"

Have been designed and drawn up by the undersigned, a Maine registered Architect / Engineer, according to the 2003 International Building Code and local amendments.



As per Maine State Law:

\$50,000.00 or more in new construction, repair expansion, addition, or modification for Building or Structures, shall be prepared by a registered design Professional.

Signature: Timothy G. Shelley

Title: PROFESSIONAL ENGINEER

Firm: SHELLEY ENGINEERING

Address: 90 BRIDGE ST.
WESTBROOK, ME
04092

Statement of Special Inspections

Project: KENNEBEC ST. ADDITION
Location: 123-129 KENNEBEC ST,
Owner:

Design Professional in Responsible Charge: TIMOTHY G. SHELLEY, P.E.

This *Statement of Special Inspections* is submitted as a condition for permit issuance in accordance with the Special Inspection and Structural Testing requirements of the Building Code. It includes a schedule of Special Inspection services applicable to this project as well as the name of the Special Inspection Coordinator and the identity of other approved agencies to be retained for conducting these inspections and tests. This *Statement of Special Inspections* encompass the following disciplines:

- Structural Mechanical/Electrical/Plumbing
 Architectural Other: _____

The Special Inspection Coordinator shall keep records of all inspections and shall furnish inspection reports to the Building Official and the Registered Design Professional in Responsible Charge. Discovered discrepancies shall be brought to the immediate attention of the Contractor for correction. If such discrepancies are not corrected, the discrepancies shall be brought to the attention of the Building Official and the Registered Design Professional in Responsible Charge. The Special Inspection program does not relieve the Contractor of his or her responsibilities.

Interim reports shall be submitted to the Building Official and the Registered Design Professional in Responsible Charge.

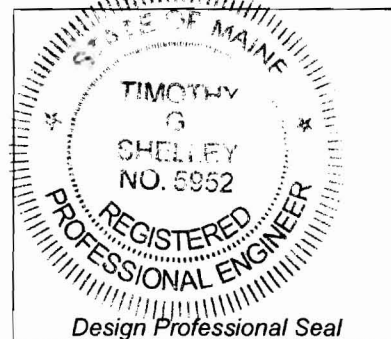
A *Final Report of Special Inspections* documenting completion of all required Special Inspections, testing and correction of any discrepancies noted in the inspections shall be submitted prior to issuance of a Certificate of Use and Occupancy.

Job site safety and means and methods of construction are solely the responsibility of the Contractor.

Interim Report Frequency: _____ or per attached schedule.

Prepared by:

TIMOTHY G. SHELLEY
(type or print name)



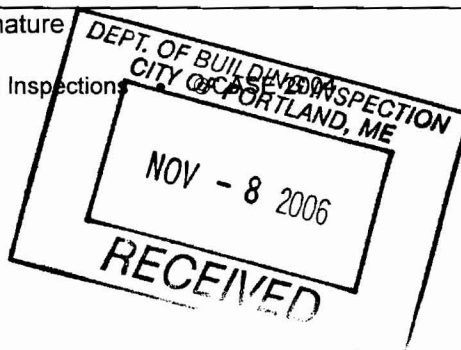
Timothy G. Shelley 11.6.06
Signature Date

Owner's Authorization:

Building Official's Acceptance:

J. H. M. 11.6.06
Signature Date

Signature _____ Date _____



City of Portland, Maine - Building or Use Permit Application

389 Congress Street, 04101 Tel: (207) 874-8703, Fax: (207) 874-8716

Permit No: 06-1030	Issue Date:	CBL: 034 I001001
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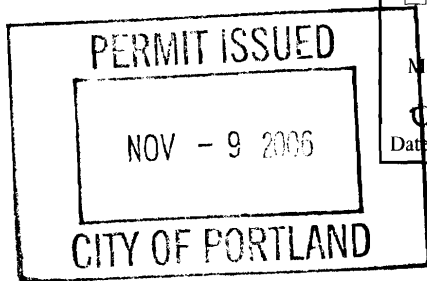
Location of Construction: 131 PREBLE ST	Owner Name: PORTLAND ARCITECTURAL SA	Owner Address: 919 CONGRESS ST	Phone:
Business Name:	Contractor Name: TBD	Contractor Address:	Phone
Lessee/Buyer's Name	Phone:	Permit Type: Additions - Commercial	Zone: 57

Past Use: Retail Warehouse	Proposed Use: Retail Warehouse- 3 Story connector addition connecting 131 Preble Street & 123 Kennebec St. Addition will incorporate new elevator, steel staircase & new lobby	Permit Fee: \$1,845.00	Cost of Work: \$175,000.00	CEO District: 1
Proposed Project Description: Connector Addition will incorporate new elevator, steel staircase & new lobby		FIRE DEPT: <input checked="" type="checkbox"/> Approved <input type="checkbox"/> Denied <i>See Conditions</i>	INSPECTION: Use Group: <i>A/W</i> Type: <i>3</i> <i>FBC-2003</i> Signature: <i>JMB 11/8/06</i>	

PEDESTRIAN ACTIVITIES DISTRICT (P.A.D.)	
Action: <input type="checkbox"/> Approved <input type="checkbox"/> Approved w/Conditions <input type="checkbox"/> Denied	Signature: _____ Date: _____

Permit Taken By: Idobson	Date Applied For: 07/13/2006	Zoning Approval
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<ol style="list-style-type: none"> This permit application does not preclude the Applicant(s) from meeting applicable State and Federal Rules. Building permits do not include plumbing, septic or electrical work. Building permits are void if work is not started within six (6) months of the date of issuance. False information may invalidate a building permit and stop all work.. 	Special Zone or Reviews <input type="checkbox"/> Shoreland <input type="checkbox"/> Wetland <input type="checkbox"/> Flood Zone <input type="checkbox"/> Subdivision <input type="checkbox"/> Site Plan Maj <input type="checkbox"/> Minor <input type="checkbox"/> MM <input type="checkbox"/> <i>OK w/conditions</i> Date: <i>7/20/06</i> <i>AKM</i>	Zoning Appeal <input type="checkbox"/> Variance <input type="checkbox"/> Miscellaneous <input type="checkbox"/> Conditional Use <input type="checkbox"/> Interpretation <input type="checkbox"/> Approved <input type="checkbox"/> Denied Date: _____	Historic Preservation <input checked="" type="checkbox"/> Not in District or Landmark <input type="checkbox"/> Does Not Require Review <input type="checkbox"/> Requires Review <input type="checkbox"/> Approved <input type="checkbox"/> Approved w/Conditions <input type="checkbox"/> Denied <i>AKM</i> Date: _____
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CERTIFICATION

I hereby certify that I am the owner of record of the named property, or that the proposed work is authorized by the owner of record and that I have been authorized by the owner to make this application as his authorized agent and I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in the application is issued, I certify that the code official's authorized representative shall have the authority to enter all areas covered by such permit at any reasonable hour to enforce the provision of the code(s) applicable to such permit.

SIGNATURE OF APPLICANT	ADDRESS	DATE	PHONE
RESPONSIBLE PERSON IN CHARGE OF WORK, TITLE		DATE	PHONE

Schedule of Inspection and Testing Agencies

This Statement of Special Inspections / Quality Assurance Plan includes the following building systems:

- | | |
|--|--|
| <input checked="" type="checkbox"/> Soils and Foundations | <input type="checkbox"/> Spray Fire Resistant Material |
| <input checked="" type="checkbox"/> Cast-in-Place Concrete | <input type="checkbox"/> Wood Construction |
| <input type="checkbox"/> Precast Concrete | <input type="checkbox"/> Exterior Insulation and Finish System |
| <input checked="" type="checkbox"/> Masonry | <input type="checkbox"/> Mechanical & Electrical Systems |
| <input checked="" type="checkbox"/> Structural Steel | <input type="checkbox"/> Architectural Systems |
| <input type="checkbox"/> Cold-Formed Steel Framing | <input type="checkbox"/> Special Cases |

Special Inspection Agencies	Firm	Address, Telephone, e-mail
1. Special Inspection Coordinator TIMOTHY SHELLEY, P.E.	SHELLEY ENGINEERING	90 BRIDGE ST WESTBROOK ME 04092 854.5465 TIM@SHELLEYENGINEERING.COM
2. Inspector PETER FICKETT, P.E.	" "	PETERFC " "
3. Inspector BILL P		
4. Testing Agency BILL PETERLEIN, PE	SUMMIT GEOENGINEERING SERVICES	640 MAIN ST. LEWISTON, ME 04240 795-6009 BPETERLEIN@SUMMITENV.COM
5. Testing Agency		
6. Other		

Note: The inspectors and testing agencies shall be engaged by the Owner or the Owner's Agent, and not by the Contractor or Subcontractor whose work is to be inspected or tested. Any conflict of interest must be disclosed to the Building Official, prior to commencing work.

Quality Assurance Plan

Quality Assurance for Seismic Resistance

Seismic Design Category B

Quality Assurance Plan Required (Y/N) No

Description of seismic force resisting system and designated seismic systems:

REINFORCED MASONRY SHEAR WALLS

Quality Assurance for Wind Requirements

Basic Wind Speed (3 second gust) 100 mph

Wind Exposure Category B

Quality Assurance Plan Required (Y/N) No

Description of wind force resisting system and designated wind resisting components:

REINFORCED MASONRY SHEAR WALLS

Statement of Responsibility

Each contractor responsible for the construction or fabrication of a system or component designated above must submit a Statement of Responsibility.

Qualifications of Inspectors and Testing Technicians

The qualifications of all personnel performing Special Inspection and testing activities are subject to the approval of the Building Official. The credentials of all Inspectors and testing technicians shall be provided if requested.

Key for Minimum Qualifications of Inspection Agents:

When the Registered Design Professional in Responsible Charge deems it appropriate that the individual performing a stipulated test or inspection have a specific certification or license as indicated below, such designation shall appear below the *Agency Number* on the Schedule.

PE/SE	Structural Engineer – a licensed SE or PE specializing in the design of building structures
PE/GE	Geotechnical Engineer – a licensed PE specializing in soil mechanics and foundations
EIT	Engineer-In-Training – a graduate engineer who has passed the Fundamentals of Engineering examination

American Concrete Institute (ACI) Certification

ACI-CFTT	Concrete Field Testing Technician – Grade 1
ACI-CCI	Concrete Construction Inspector
ACI-LTT	Laboratory Testing Technician – Grade 1&2
ACI-STT	Strength Testing Technician

American Welding Society (AWS) Certification

AWS-CWI	Certified Welding Inspector
AWS/AISC-SSI	Certified Structural Steel Inspector

American Society of Non-Destructive Testing (ASNT) Certification

ASNT	Non-Destructive Testing Technician – Level II or III.
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International Code Council (ICC) Certification

ICC-SMSI	Structural Masonry Special Inspector
ICC-SWSI	Structural Steel and Welding Special Inspector
ICC-SFSI	Spray-Applied Fireproofing Special Inspector
ICC-PCSI	Prestressed Concrete Special Inspector
ICC-RCSI	Reinforced Concrete Special Inspector

National Institute for Certification in Engineering Technologies (NICET)

NICET-CT	Concrete Technician – Levels I, II, III & IV
NICET-ST	Soils Technician - Levels I, II, III & IV
NICET-GET	Geotechnical Engineering Technician - Levels I, II, III & IV

Exterior Design Institute (EDI) Certification

EDI-EIFS	EIFS Third Party Inspector
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Other

Item	Agency # (Qualif.)	Scope
1. Shallow Foundations	PE/GE # 4 SUMMIT	Inspect soils below footings for adequate bearing capacity and consistency with geotechnical report. Inspect removal of unsuitable material and preparation of subgrade prior to placement of controlled fill
2. Controlled Structural Fill	PE/GE # A SUMMIT	Perform sieve tests (ASTM D422 & D1140) and modified Proctor tests (ASTM D1557) of each source of fill material. Inspect placement, lift thickness and compaction of controlled fill. Test density of each lift of fill by nuclear methods (ASTM D2922) Verify extent and slope of fill placement.
3. Deep Foundations	PE/GE	Inspect and log pile driving operations. Record pile driving resistance and verify compliance with driving criteria. Inspect piles for damage from driving and plumbness. Verify pile size, length and accessories. Inspect installation of drilled pier foundations. Verify pier diameter, bell diameter, lengths, embedment into bedrock and suitability of end bearing strata.
4. Load Testing	N/A	
4. Other:		

Cast-in-Place Concrete

Item	Agency # (Qualif.)	Scope
1. Mix Design	#4 ACI-CCI ICC-RCSI	Review concrete batch tickets and verify compliance with approved mix design. Verify that water added at the site does not exceed that allowed by the mix design.
2. Material Certification		
3. Reinforcement Installation	#1 SEI ACI-CCI ICC-RCSI	Inspect size, spacing, cover, positioning and grade of reinforcing steel. Verify that reinforcing bars are free of form oil or other deleterious materials. Inspect bar laps and mechanical splices. Verify that bars are adequately tied and supported on chairs or bolsters
4. Post-Tensioning Operations	N/A ICC-PCSI	Inspect placement, stressing, grouting and protection of post-tensioning tendons. Verify that tendons are correctly positioned, supported, tied and wrapped. Record tendon elongations.
5. Welding of Reinforcing	N/A AWS-CWI	Visually inspect all reinforcing steel welds. Verify weldability of reinforcing steel. Inspect preheating of steel when required.
6. Anchor Rods	#1 SEI	Inspect size, positioning and embedment of anchor rods. Inspect concrete placement and consolidation around anchors.
7. Concrete Placement	#1 SEI ACI-CCI ICC-RCSI	Inspect placement of concrete. Verify that concrete conveyance and depositing avoids segregation or contamination. Verify that concrete is properly consolidated.
8. Sampling and Testing of Concrete	#4 SUMMIT ACI-CFTT ACI-STT	Test concrete compressive strength (ASTM C31 & C39), slump (ASTM C143), air-content (ASTM C231 or C173) and temperature (ASTM C1064).
9. Curing and Protection	#4 SUMMIT ACI-CCI ICC-RCSI	Inspect curing, cold weather protection and hot weather protection procedures.
10. Other:		

Masonry

Required Inspection Level: 1 2

Item	Agency # (Qualif.)	Scope
1. Material Certification		
2. Mixing of Mortar and Grout	ICC-SMSI	<i>Inspect proportioning, mixing and retempering of mortar and grout.</i>
3. Installation of Masonry	#1 SEI ICC-SMSI	<i>Inspect size, layout, bonding and placement of masonry units.</i>
4. Mortar Joints	#1 ICC-SMSI	<i>Inspect construction of mortar joints including tooling and filling of head joints.</i>
5. Reinforcement Installation	#1 ICC-SMSI AWS-CWI	<i>Inspect placement, positioning and lapping of reinforcing steel. Inspect welding of reinforcing steel.</i>
6. Prestressed Masonry	ICC-SMSI	<i>Inspect placement, anchorage and stressing of prestressing bars.</i>
7. Grouting Operations	#1 ICC-SMSI	<i>Inspect placement and consolidation of grout. Inspect masonry clean-outs for high-lift grouting.</i>
7. Weather Protection	ICC-SMSI	<i>Inspect cold weather protection and hot weather protection procedures. Verify that wall cavities are protected against precipitation.</i>
9. Evaluation of Masonry Strength	ICC-SMSI	<i>Test compressive strength of mortar and grout cube samples (ASTM C780). Test compressive strength of masonry prisms (ASTM C1314).</i>
10. Anchors and Ties	#1 ICC-SMSI	<i>Inspect size, location, spacing and embedment of dowels, anchors and ties.</i>
11. Other:		

Item	Agency # (Qualif.)	Scope
1. Fabricator Certification/ Quality Control Procedures <input type="checkbox"/> Fabricator Exempt	AWS/AISC- SSI ICC-SWSI	<i>Review shop fabrication and quality control procedures.</i>
2. Material Certification	AWS/AISC- SSI ICC-SWSI	<i>Review certified mill test reports and identification markings on wide-flange shapes, high-strength bolts, nuts and welding electrodes</i>
3. Open Web Steel Joists		<i>Inspect installation, field welding and bridging of joists.</i>
4. Bolting	#1 SEI AWS/AISC- SSI ICC-SWSI	<i>Inspect installation and tightening of high-strength bolts. Verify that splines have separated from tension control bolts. Verify proper tightening sequence. Continuous inspection of bolts in slip-critical connections.</i>
5. Welding	#1 SEI AWS-CWI ASNT	<i>Visually inspect all welds. Inspect pre-heat, post-heat and surface preparation between passes. Verify size and length of fillet welds.</i> <i>Ultrasonic testing of all full-penetration welds.</i>
6. Shear Connectors	AWS/AISC- SSI ICC-SWSI	<i>Inspect size, number, positioning and welding of shear connectors. Inspect studs for full 360 degree flash. Ring test all shear connectors with a 3 lb hammer. Bend test all questionable studs to 15 degrees.</i>
7. Structural Details	#1 PE/SE	<i>Inspect steel frame for compliance with structural drawings, including bracing, member configuration and connection details.</i>
8. Metal Deck	#1 AWS-CWI	<i>Inspect welding and side-lap fastening of metal roof and floor deck.</i>
9. Other:		

FROM DESIGNER: TIMOTHY G SHELLY
 DATE: 7.10.06
 Job Name: KENNEBEC STREET ADDITION
 Address of Construction: 125 KENNEBEC ST, PORTLAND ME

2003 International Building Code

Construction project was designed according to the building code criteria listed below:

Building Code and Year IBC 2003 Use Group Classification(s) RETAIL / WAREHOUSE
 Type of Construction MASONRY / STEEL
 Will the Structure have a Fire suppression system in Accordance with Section 903.3.1 of the 2003 IRC _____
 Is the Structure mixed use? _____ if yes, separated or non separated (see Section 302.3) _____
 Supervisory alarm system? _____ Geotechnical/Soils report required? (See Section 1802.2) _____

STRUCTURAL DESIGN CALCULATIONS

No Submitted for all structural members
 (108.1, 108.1.1)

No
N/A

Live load reduction
 (1603.1.1, 1607.9, 1607.10)
 Roof live loads (1603.1.2, 1607.11)

DESIGN LOADS ON CONSTRUCTION DOCUMENTS
 (1603)

Roof snow loads (7603.7.1, 1608)

Uniformly distributed floor live loads (7603.11, 1607)	Floor Area Use	Loads Shown
	<u>100 PSF</u>	<u>YES</u>
	_____	_____
	_____	_____
	_____	_____
	_____	_____

Ground snow load, P_g (1608.2)
 If $P_g > 10$ psf, flat-roof snow load, P_f
 (1608.3)
 If $P_g > 10$ psf, snow exposure factor, C_e
 (Table 1608.3.1)
 If $P_g > 10$ psf, snow load importance
 factor, I_s (Table 1604.5)
 Roof thermal factor, C_t (Table 1608.3.2)
 Sloped roof snowload, P_s (1608.4)

Wind loads (1603.1.4, 1609)

SHEAR WALL

N/A Design option utilized (1609.1.1, 1609.6)
 " Basic wind speed (1609.3)
 " Building category and wind importance
 factor, I_w (Table 1604.6, 1609.5)
 " Wind exposure category (1609.4)
 " Internal pressure coefficient (ASCE 7)
 " Component and cladding pressures
 (1609.1.1, 1609.6.2.2)
 " Main force wind pressures (7603.1.1,
 1609.6.2.1)

2.5
S.F.M
30'

Seismic design category (1616.3)
 Basic seismic-force-resisting system
 (Table 1617.8.2)
 Response modification coefficient, R ,
 and deflection amplification factor, C_d
 (Table 1617.8.2)
 Analysis procedure (1616.6, 1617.5)
 Design base shear (1617.4, 1617.5.1)

Earthquake design data (1603.1.5, 1614-1623)

S.F.M Design option utilized (1614.1)
2 Seismic use group ("Category")
 (Table 1604.5, 1616.2)
1 Spectral response coefficients, S_{DS} &
 S_{D1} (1615.1)
1 Site class (1615.1.5)

Flood loads (1603.1.8, 1612)
N/A Flood hazard area (1612.3)
50' Elevation of structure

Other loads
N/A Concentrated loads (1607.4)
N/A Partition loads (1607.5)
N/A Impact loads (1607.8)
N/A Misc. loads (Table 1607.9, 1607.9.1,
 1607.7, 1607.12, 1607.13, 1610,
 1611, 2404)



CITY OF PORTLAND
BUILDING CODE CERTIFICATE
389 Congress St., Room 315
Portland, Maine 04101

ACCESSIBILITY CERTIFICATE

Designer: ERIC STARK

Address of Project: 125 KENNEBEC ST. PORTLAND, ME

Nature of Project: 3 STORY CONNECTOR ADDITION
STEEL FRAME, NEW ELEVATOR
& STAIR, MASONRY SHAFT WALL

The technical submissions covering the proposed construction work as described above have been designed in compliance with applicable referenced standards found in the Maine Human Rights Law and Federal Americans with Disability Act.

Signature: [Handwritten Signature]

Title: OWNER / DESIGNER

Firm: ERIC STARK / architecture

Address: 71 BECKETT ST #1

PORTLAND ME 04101

Phone: 207 776 5227

(SEAL)

NOTE: If this project is a new Multi Family Structure of 4 units or more, this project must also be designed in compliance with the Federal Fair Housing Act. On a separate submission, please explain in narrative form the method of compliance.

Eric Stark
architecture

71 Beckett Street #1 Portland Maine 04101
telephone/207 776 5227 fax/207 871 5063
ericstark@verizon.net

19 September 2006

Mr. Mike Nugent
Planning & Development Department
City of Portland, Maine
389 Congress Street, Room 308
Portland, ME 04101

Dear Mike,

Please see attached drawings which are meant to address the issues you raised with the previously submitted set of July 11, 2006.

1. Architect's stamp required - We have enlisted the services of registered architect Leeland Hulst, who has reviewed and augmented the drawings.
2. HVAC information - the drawings now show heat monitors in the space of the addition. There will not be any other cooling, nor duct work penetrating the fire rated enclosure of the stair.
3. Life Safety drawings - per the request of Capt. Greg Cass, we have added these to the set. (I have included a second set for Capt. Cass' review).
4. Geo-technological information - we have enlisted the services of Summit Engineering. The boring will take place this Friday, September 22nd. We should have the soils report soon after that which I will share with you and Tim Shelley, structural engineer.

Please let me know what if anything else the city of Portland requires in order to grant a permit for our project. Thanks for you help.

Regards,



Eric Stark

new construction
remodels/additions
home purchase
consultation
interiors + furniture

October 23, 2006

Mike,

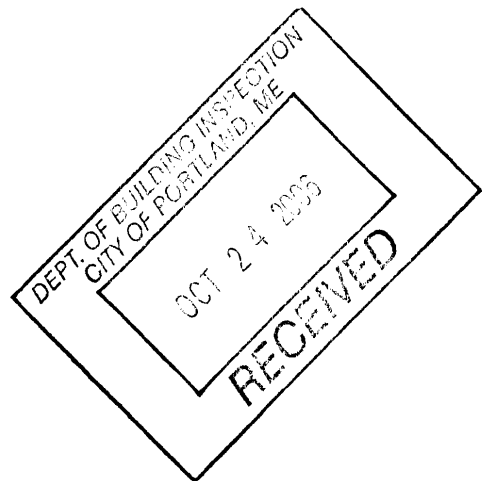
In reference to the project at 125 Kennebec Street, enclosed are the following items you requested:

1. Geotechnical Report ✓
2. Statement of Special Inspections ✓
3. UL Assemblies for new construction ✓

JK
JMB
10/8/06

Please let me know if you have any questions or require further info.

Eric Stark
776-5227



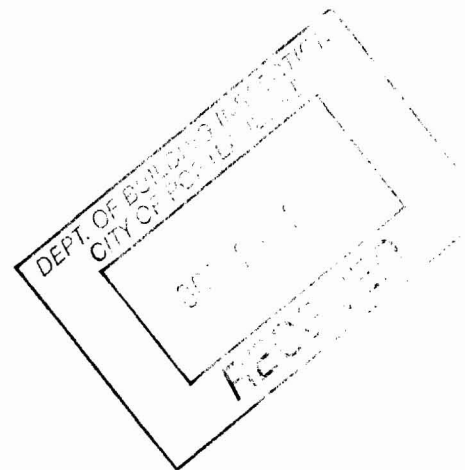


Geotechnical Report

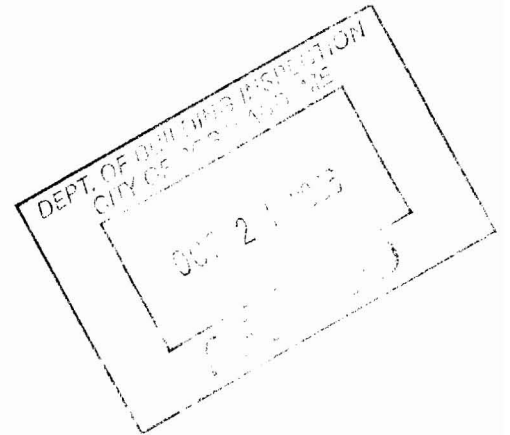
**Elevator/Stairwell Addition
123 Kennebec Street
Portland, Maine**

Prepared for:
Tod Dana

Summit Job Number 17102
October 2006



640 MAIN STREET LEWISTON, MAINE 04240 TEL: (207) 795-6009 FAX: (207) 795-6128
P.O. BOX 4698 AUGUSTA MAINE 04330 TEL: (207) 621-8334 FAX: (207) 626-9094
GEOTECHNICAL ENGINEERING - CONSTRUCTION MATERIALS TESTING - GEOENVIRONMENTAL



October 17, 2006
Summit #17102

Tod Dana
c/o Asia West
219 Commercial Street
Portland, Maine 04101

Reference: Geotechnical Engineering Evaluation
Proposed Elevator/Stairwell Addition, 123 Kennebec Street, Portland, Maine

Dear Tod;

We have completed the geotechnical investigation for the proposed addition at the site referenced above. Our scope of services included performing 1 test boring at the site and preparing this letter summarizing our findings and geotechnical recommendations.

1.0 Project and Site

The project will consist of constructing a new elevator/stairwell addition to the former mattress factory located between Preble Street and Elm Street in Portland, Maine. Currently the site is developed with two existing wood framed buildings and surrounding pavement areas.

The elevator/stairwell building has dimensions of approximately 12 feet by 35 feet. We understand that the elevator portion, 10 feet by 12 feet, will be constructed on a structural mat foundation, anticipated to be 24 inches thick. The average contact pressure at the base of the elevator mat is approximately 1,500 psf. The remaining exterior walls and any interior columns will be constructed on isolated or continuous spread footings. We understand that the bottom of the elevator slab will be approximately 6 feet below the existing grade. We also understand that the addition foundation will not be attached to any existing building foundations.

2.0 Exploration and Subsurface Conditions

Summit observed the subsurface conditions at the site with the drilling of 1 boring on September 22, 2006. The boring was drilled by Northern Test Borings under contract to and the direction of Summit. The boring was drilled a depth of 26 feet, then rod probed to refusal at a depth of 58.7 feet, using 4" casing with rotary wash. Split spoon samples were obtained continuous to a depth of 16 feet and then at selected intervals. Vane shear tests were performed in the boring at

selected depths. An undisturbed tube samples was obtained at a depth of 20 feet. The boring was drilled at or near the center of the elevator portion of the addition. A log of the boring is attached. The boring was located by others prior to drilling by Summit.

Due to some uncertainty of exact locations for on-site underground utilities, Dig Smart of Maine was contracted by Summit to verify the boring location was free from subsurface utilities.

Addition exploration at the site included the excavation of test hole adjacent to the existing building, performed by others and observed by Summit, on October 6, 2006. The test pit was excavated to expose the existing footings.

The soil encountered in the boring consisted of 1.5 inches of pavement over 12.5 feet of sandy fill, over 35.5 feet of silty clay, over a loose to compact glacial marine sand. Refusal was encountered at a depth of 58.7 feet. We believe that refusal was on bedrock.

The *sandy fill* is described as brown to dark brown sand with a little silt, grading to a silty sand with a little clay and ash debris, grading to a brown sand with a little gravel and silt. This soil was generally loose and wet below a depth of 8.5 feet. The fill soil is visually classified as SM or SM-SP in accordance with the USCS. SPT-N values for the fill ranged from 4 blows per foot (bpf) to 8 bpf and averaged 6 bpf, indicating loose soil conditions.

The *silty clay deposit* grades from a firm olive-gray slightly mottled silty clay to a very soft gray silty clay. This layer was 35.5 feet thick. SPT-N values ranged from 1 bpf to 5 bpf. Moisture contents were determined at various depths as follows.

DEPTH	MOISTURE CONTENT
13 ft	36.0%
14 ft	40.9%
24 ft	45.9%

Vane shear tests taken in the boring resulted in the following undrained shear strengths:

DEPTH	MOISTURE CONTENT
18 ft	870 psf
19 ft	925 psf
22 ft	545 psf
23 ft	650 psf

An Atterberg limit (ASTM D4318) performed on a sample of the soft silty clay at a depth of 24 to 26 feet, resulted in a liquid limit of 49 and a plasticity index of 27. The silty clay soil is classified as CL in accordance with the USCS.

The *glacial marine sand* was encountered at a depth of 48 feet. Spoon samples were not obtained for direct examination of the soil. Blow counts on the probe sampler ranged from hydraulic push to 48 blows per foot.

Refusal, presumed to be bedrock, was encountered at a depth of 58.7 feet. Samples of the refusal material were not obtained.

Groundwater was at a depth of 8.5 feet, based on moisture changes observed in the samples obtained in the boring. Mottling in the upper layer of the silty clay soil indicates that groundwater elevations will likely fluctuate seasonally during wet and dry periods.

4.0 Foundation Recommendations

A. General

Based on the anticipated elevator footing depth, the proposed mat will be constructed within the existing granular fill. The soil below the fill consists of stiff silty clay over soft silty clay. The bottom of the mat is estimated to be 6 feet below the existing ground surface. Due to the looseness and potential variability of the existing fill, improvement of the subgrade soil beneath the mat is recommended. With the subgrade soil improvement, the existing soils will be suitable to support the mat footing as proposed. Interior and exterior column and wall footings in other areas of the addition can be constructed on conventional spread footing foundations.

B. Allowable Bearing Pressure

The footings for the building and elevator mat footing can be proportioned using an allowable bearing pressure of 1,500 psf. Settlement for the mat was estimated in the upper fill layer and the upper stiff and lower soft silty clay layers. For the proposed load and presumed soil bearing conditions, the total computed settlement of the elevator mat was calculated to be 0.5 inches. Settlement from the interior footings and exterior walls will be less than this. Differential settlement between the elevator mat and the footings will be tolerable for the structure.

This allowable bearing pressure is based on the following conditions.

- Exterior continuous footings, interior isolated footings, and the elevator mat are constructed on 12-inch minimum thickness of $\frac{3}{4}$ inch crushed stone placed over the existing proofrolled sandy fill soil, to create a uniform condition immediately beneath the footings. The crushed stone should be compacted using a vibratory roller. Proofrolling should consist of making 3 passes in an east-west direction and 3 passes in a north-south direction using a vibratory compactor.
- Soft, loose and otherwise unsuitable soil pockets, if present, are removed and replaced with crushed stone.

Seasonal groundwater is anticipated to be below the footings. If groundwater is present above the base of the footings, the excavations should be dewatered prior to constructing the footings.

C. Frost Protection

Based on the required frost protection depth, spread footings for the exterior walls and interior columns should be constructed at a minimum depth of 4 feet below the exterior finished grade. The 12 inches of crushed stone can be included in the frost depth, allowing the footing to be constructed 3 feet below the finished grade. This frost penetration depth is based on a design air-freezing index of 1,250-degree days for the Portland area. We recommend that the footings be backfilled with Foundation Backfill. The portion of Foundation Backfill passing the 3 inch sieve should meet the following gradation specification:

FOUNDATION BACKFILL	
Sieve Size	Percent finer
3 inch	100
No. 40	0 to 70
No. 200	0 to 5

Reference: MDOT 703.20, Granular Borrow (modified)

The maximum particle size should be limited to 6 inches. The Foundation Backfill should be compacted to a minimum of 95 percent of its maximum dry density, determined in accordance with ASTM D1557.

D. Groundwater Control

Observations of the soil samples indicate that groundwater was at a depth of 8.5 feet below the existing ground surface. This depth is below the anticipated footing depths. Based on this a perimeter footing drain is not strictly necessary. Having said that, we note that it is also generally good practice to install underdrains to account for unobserved conditions or future changes in local hydrogeology.

Underdrains, if used, should consist of 4 inch rigid perforated PVC surrounded by 6 inches of $\frac{3}{4}$ inch crushed stone wrapped in geotextile filter fabric. We recommend that the ground surface slope away from the building and the ground surface on the exposed sides of the addition be sealed with pavement or a low permeability soil to reduce infiltration of roof and building wall water runoff into the Foundation Backfill.

In order to prevent frost action from lifting concrete entrance pads and potentially blocking entrances, we recommend that where they are connected to the building, concrete entrance pads be constructed on frost walls. The footings should be at least 4 feet below the finished grade.

E. Seismic Design

The soils at the site are categorized as Site Class E, Soft Soil Profile, for foundations constructed on soil in accordance with the 2006 International Building Code.

F. Building Slab

We recommend the building addition slab be constructed on a minimum 12-inch thick layer of Structural Backfill soil. The portion passing the 3 inch sieve meet the following gradation specifications:

STRUCTURAL BACKFILL	
Sieve Size	Percent finer
3 inch	100
1/4 inch	0 to 70
No. 200	0 to 10

Reference: MDOT Specification 703.20, Gravel Borrow

The maximum particle size should be limited to 6 inches. The Structural Backfill should be placed in 8 to 12-inch lifts and should be compacted to 95 percent of its maximum dry density determined in accordance with ASTM D1557. The existing fill subgrade soil should be proofrolled prior to placing the Structural Fill.

For the conditions described above, the slabs can be designed using a subgrade modulus of 150 pci.

The slab will be above the groundwater and no drains are necessary.

5.0 Earthwork Considerations

Based on visual observations of the existing fill it will not meet gradation requirements for Foundation Backfill or Structural Backfill.

Excavations below 4 feet should be sloped no greater than 1.5H to 1V. This slope is based on the current OSHA Excavation Guidelines.

The maximum angle of repose of the existing fill soil is estimated to be 1H:1V. Excavated sidewalls steeper than this will collapse, potentially undermining the existing footings. Based on this, shoring of the existing building superstructure will be necessary where excavations extend below the existing footings. Voids beneath the existing foundations created by sloughing soil, should be filled with lean concrete or flowable fill.

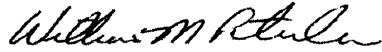
6.0 Closure

Our recommendations are based on professional judgment and generally accepted principles of geotechnical engineering. Some changes in subsurface conditions from those presented in this report may occur. Should these conditions differ materially from those described in this report, Summit should be notified so that we can re-evaluate our recommendations.

We recommend that a qualified geotechnical consultant be retained to monitor and test soil materials used during construction. Summit would welcome the opportunity to provide this service.

We appreciate the opportunity to serve you during this phase of your project. If there are any questions or additional information is required, please do not hesitate to call.

Sincerely yours,
Summit Geoengineering Services,



William M. Peterlein, P.E.
Principal Geotechnical Engineer



EXPLORATION REPORT COVER SHEET

The exploration report has been prepared by the geotechnical engineer from both field and laboratory data. Differences between field logs and exploration reports may exist.

It is common practice in the soil and foundation engineering profession that field logs and laboratory data sheets not be included in engineering reports, because they do not represent the engineer's final opinion as to appropriate descriptions for conditions encountered in the exploration and testing work. The field logs will be retained in our office for review. Results of laboratory tests are generally shown on the borings logs or are described in the text of the report as appropriate.

Drilling and Sampling Symbols:

SS = Split Spoon	Hyd = Hydraulic advance of probes
ST = Shelby Tube – 2" OD, disturbed	WOH = Weight of Hammer
UT = Shelby Tube – 3" OD, undisturbed	WOR = Weight of Rod
HSA = Hollow Stem Auger	GS = Grain Size Data
CS = Casing – size as noted	PI = Plasticity Index
Sv = Vane Shear	LL = Liquid Limit
PP = Pocket Penetrometer	w = Natural Water Content
RX = Rock Core – size as noted	USCS = unified Soil Classification System

Water Level Measurements:

Water levels indicated on the boring logs are the levels measured in the boring at the times indicated. In pervious soils, the indicated elevations are considered reliable groundwater levels. In impervious soils, the accurate determination of groundwater elevations may not be possible, even after several days of observations; additional evidence of groundwater elevations via observation or monitoring wells must be sought.

Gradation Description and Terminology:

Boulders: Over 8 inches	Trace: Less than 5%
Cobbles: 8 inches to 3 inches	Little: 5% to 15%
Gravel: 3 inches to No.4 sieve	Some: 15% to 25%
Sand: No.4 to No. 200 sieve	Silty, Sandy, etc.: Greater than 25%
Silt: No. 200 sieve to 0.005 mm	
Clay: less than 0.005 mm	

Density of Granular Soils and Consistency of Cohesive Soils:

CONSISTENCY OF COHESIVE SOILS		DENSITY OF GRANULAR SOILS	
SPT N-value blows/ft	Consistency	SPT N-value blows/ft	Relative Density
0 to 2	Very Soft	0 to 3	Very Loose
3 to 4	Soft	4 to 9	Loose
5 to 8	Firm	10 to 29	Compact
9 to 16	Stiff	30 to 49	Dense
17 to 32	Very Stiff	50 to 80	Very Dense
>32	Hard		

SUMMIT GEOENGINEERING SERVICES 640 Main Street Lewiston, Maine 04240					SOIL BORING LOG				Boring #: B-1	
Project: Asia West Elevator Shaft Portland, Maine					Project #: 17102 Sheet: 1 of 3 Prep by: JSIH					
Drilling Co: Northern Test Borings					Ground Elevation:					
Foreman: Mike Nadeau					Reference:					
Summit: Craig Coolidge, E.I.T.					Date started: 9/22/2006 Date Comp: 9/22/2006					
DRILLING METHOD		SAMPLER			GROUND WATER DEPTH					
Vehicle: ATV Model: Diedrich D-50 Method: 4" Casing		Type: 24" SS Hammer: 140 LB Fall: 30"			Date	Depth	Elevation	Comments		
					9/22/2005	8.5		Observed Moisture Change		
Depth (ft.)	SAMPLE DATA				ENGINEERING DESCRIPTION				GEOLOGIC DESCRIPTION	
	No.	Pen/Rec (in.)	Depth (ft)	Blows						
1	S-1	24/3	0 - 2	3	Brown SAND, little Silt, damp, SM-SF grading to dark brown silty SAND with little Debris, damp, loose, SM				Bit. Pavement = 1.5" FILL	
				4						
				3						
2				4	Black to dark brown silty SAND, little Clay trace Ash Debris, damp/moist, loose, SV				4" Casing (140 lb hammer)	
	S-2	24/3	2 - 4	2						
				2						
3				3	Same as above				<u>Depth</u> <u>Blows</u> 0'-10' Push 10'-11' 24 11'-12' 24 12'-13' 24	
				8						
	S-3	24/4	4 - 6	5						
4				4	Brown SAND, little Gravel, little to trace Sil damp, compact, SF				13'-14' 24 14'-15' 24	
				4						
				4						
5	S-4	24/20	6 - 8	5	Dark brown SAND with little Gravel anc Silt, wet, loose, SM-SP				2 1/4" HSA to 14', switch to RW	
				3						
				5						
6				3	Dark brown to brown SAND, little Gravel anc Silt, wet, loose, SM-SP					
				2						
				2						
7	S-5	24/3	8 - 10	5	Same as above					
				2						
				2						
8	S-6	24/16	10 - 12	2	Gray silty CLAY, little black Sand, wet, CI Firm olive gray slightly mottled silty CLAY, moist soft gray with black streaks silty CLAY, soft wet, CL				PP=4000 to 3000 psf w = 36.0% PP=500 psf or less w = 40.9% GLACIAL MARINE	
				2						
				3						
9	S-7	24/24	12 - 14	4	Sv = 870 psf, 220 psf remold @ 18.7					
				2						
				2						
10				2	Sv = 925 psf, 165 psf remold @ 19.7					
				2						
				2						
11	S-8	24/24	14 - 16	1	very soft gray silty CLAY, wet, CI					
				1						
				1						
12				2						
				1						
				1						
13				2						
				1						
				1						
14				2						
				1						
				1						
15				2						
				1						
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17				2						
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18				2						
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				1						
19				2						
				1						
				1						
20				2						
				1						
				1						
21	UT-1	30/24	20 - 22.5	Push						
				Push						
				Push						

SUMMIT GEOENGINEERING SERVICES

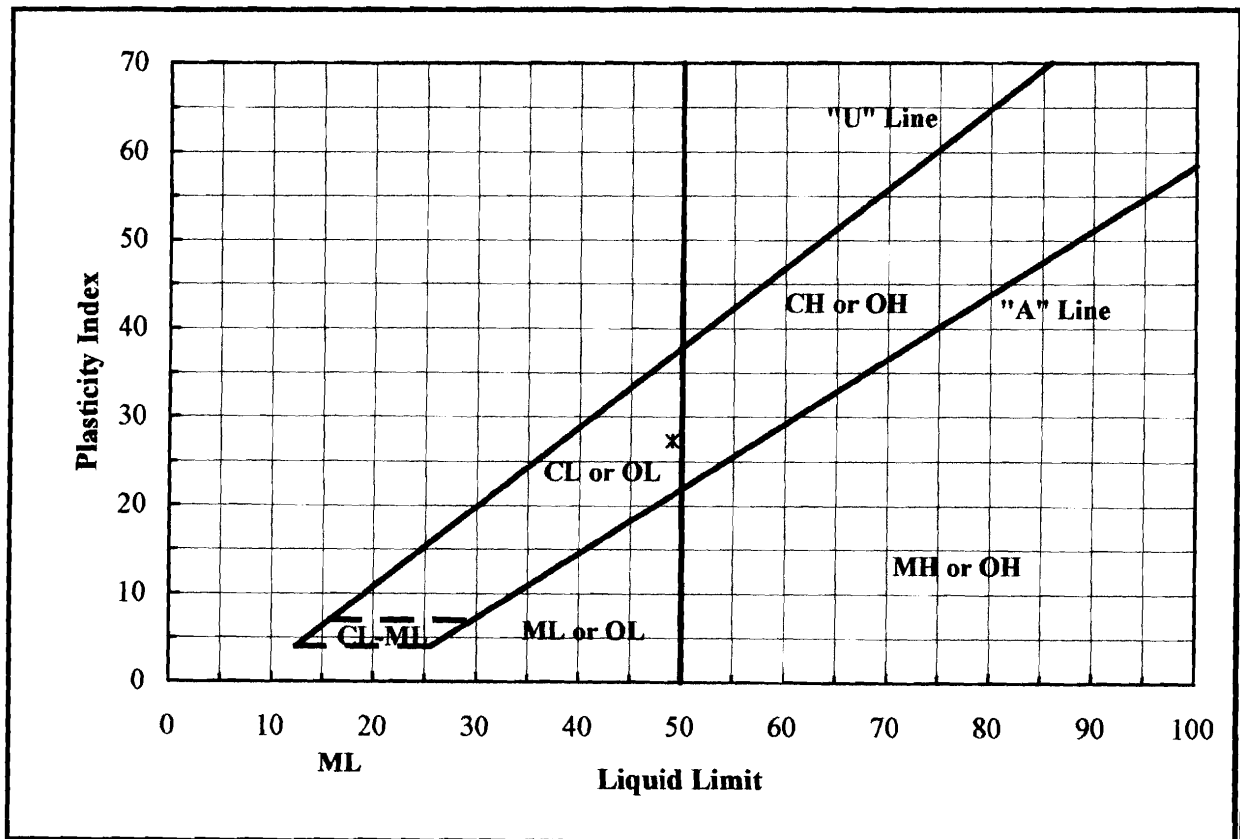
434 Cony Road, Augusta, Maine 04330
Phone: (207) 621-8334 Fax: (207) 626-9094

ATTERBERG LIMIT TEST - ASTM D4318

PROJECT NAME:	Elevator Shaft	PROJECT #:	17102
CLIENT:	Asia West	SAMPLE #:	S1
SOIL DESCRIPTION:	Clay	DATE:	38992
INTENDED USE:	Soil Investigation	SOURCE:	B1, S9, 24' to 26'
		TECHNICIAN:	Sullivan

DATA

Source	Depth	LL	PL	PI	Classification
B1, S9, 24' to 26'		49	22	27	CL Lean Clay



Notes:

Reviewed: Darrell A. Gilman, CMT Manager
Sent: 10/2/2006

Eric Stark
architecture

71 Beckett Street #1 Portland Maine 04101
telephone/207 776 5227 fax/207 871 5063
ericstark@verizon.net

22 October 2006

125 Kennebec Street
UL assemblies for new construction

All assemblies are 2 hour rated by UL

Steel Stud

V438 – two layers 5/8" Fire Shield gypsum wallboard base layer screw attached vertically to both sides steel studs @ 24" on center. Vertical joints staggered.

T1771 (based on WP1711) – first layer 5/8" Fire-Shield Gypsum Wallboard screw attached vertically both sides 3 5/8" steel studs, spaced 24" on center. Second layer laminated vertically both sides. Vertical joints staggered.

Wood Stud

U301 – two layers of 5/8" Fire-Shield Gypsum Wallboard nail applied to 2x wood framing 16" on center. Boards may be applied horizontally or vertically with all joints staggered.

Steel Column

X528 - base layer of 5/8" Fire-Shield Gypsum Wallboard furred from column by 1 5/8" metal studs at each corner. Face layer 1/2" Fire-Shield C Wallboard.

CMU block shaft

U905 - Concrete Blocks - Various designs. Classification D-2 (2 hr). Mortar - Blocks laid in full bed of mortar, nom. 3/8 in. thick, of not less than 2-1/4 and not more than 3-1/2 parts of clean sharp sand to 1 part Portland cement (proportioned by volume) and not more than 50 percent hydrated lime (by cement volume). Vertical joints staggered.

new construction
remodels/additions
home purchase
consultation
interiors + furniture

Mike Nugent - 123-129 Kennebec St.

From: Mike Nugent
To: ericstark@verizon.net
Subject: 123-129 Kennebec St.

need:

- 1) Statement of Special Inspections (see section 1704 of the code)
- 2) This stairway and elevator shaft need to have a 2 hr fire separation assembly, please provide UL listings for these assemblies

City of Portland, Maine - Building or Use Permit

389 Congress Street, 04101 Tel: (207) 874-8703, Fax: (207) 874-8716

Permit No: 06-1030	Date Applied For: 07/13/2006	CBL: 034 I001001
------------------------------	--	----------------------------

Location of Construction: 131 PREBLE ST	Owner Name: PORTLAND ARCITECTURAL SA	Owner Address: 919 CONGRESS ST	Phone:
Business Name:	Contractor Name: TBD	Contractor Address:	Phone:
Lessee/Buyer's Name	Phone:	Permit Type: Additions - Commercial	

Proposed Use: Retail Warehouse- 3 Story connector addition connecting 131 Preble Street & 123 Kennebec St. Addition will incorporate new elevator, steel staircase & new lobby	Proposed Project Description: Connector Addition will incorporate new elevator, steel staircase & new lobby
--	---

Dept: Zoning **Status:** Approved with Conditions **Reviewer:** Ann Machado **Approval Date:** 07/20/2006
Note: **Ok to Issue:**

1) This permit is being approved on the basis of plans submitted. Any deviations shall require a separate approval before starting that work.

Dept: Building **Status:** Pending **Reviewer:** **Approval Date:** **Note:** **Ok to Issue:**

Dept: Fire **Status:** Approved **Reviewer:** Cptn Greg Cass **Approval Date:** 09/19/2006
Note: **Ok to Issue:**

1) All work as drawn. Sprinkler, Fire Alarm , and Standpipe certification letters; for entire structure shall be required.

Comments:

9/22/2006-mjn: left message w/ designer, need statement of special inspections and 2 hour fire separation assemblies with openings protected.

9/19/2006-dmartin: Recieved additional info took permit off hold and routed w/ info to Fire

8/14/2006-gg: received granted site plan exemption. /gg

SEAM

Structural Engineering Association of Maine

STATEMENT OF SPECIAL INSPECTIONS

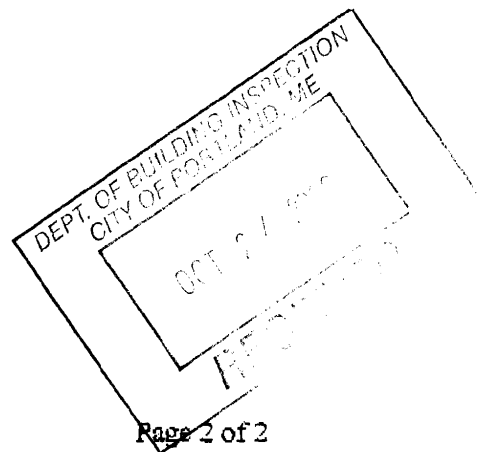
PROJECT: KENNEBEC ST ADDITION

STRUCTURAL ENGINEER OF RECORD: TIM SHELLEY / SHELLEY ENGR
NAME FIRM
90 BRIDGE ST, WESTBROOK, ME
ADDRESS 04092

ARCHITECT OF RECORD: LEE HULST, LELAND HULST, ARCH. SERVICES
NAME FIRM
278 SPRING ST, PORTLAND ME
ADDRESS 04102

Following is the List of Agents selected for performance of Special Inspections for this project.

1. Special Inspector: TIM SHELLEY NAME SHELLEY ENGINEERING FIRM
2. Testing Laboratory: SUMMIT ENGINEERING,
3. Testing Laboratory: _____
4. _____
5. _____
6. _____
7. _____
8. _____
9. _____
10. _____





APPLICATION FOR EXEMPTION FROM SITE PLAN REVIEW

Tod Dunn
Applicant

July 13
Application Date

PO Box 169 NW 04112
Applicant's Mailing Address

Kennelwood Commercial
Project Name/Description

Consultant/Agent/Phone Number

123-179 Kennelwood
Address of Proposed Site

131 Peble

CBL: EA I-1

Description of Proposed Development:

connector building

Please Attach Sketch/Plan of Proposal/Development

Criteria for Exemptions:

See Section 14-523 (4) on back side of form

- a) Within Existing Structures; No New Buildings, Demolitions or Additions
- b) Footprint Increase Less Than 500 Sq. Ft.
- c) No New Curb Cuts, Driveways, Parking Areas
- d) Curbs and Sidewalks in Sound Condition/Comply with ADA
- e) No Additional Parking/ No Traffic Increase
- f) No Stormwater Problems
- g) Sufficient Property Screening
- h) Adequate Utilities

Applicant's Assessment (Yes, No, N/A)	Planning Office Use Only
_____	✓
_____	✓
_____	✓
_____	✓
_____	✓
_____	✓
_____	✓
_____	✓
_____	✓