GENERAL NOTES

1. THE NOTES ON THESE DRAWINGS ARE NOT INTENDED TO REPLACE SPECIFICATIONS. SEE SPECIFICATIONS FOR REQUIREMENTS IN ADDITION TO GENERAL NOTES. INCONSISTENCIES BETWEEN THESE DRAWINGS AND THE SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT PRIOR TO PROCEEDING WITH THE AFFECTED PORTION OF THE WORK.

2. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH JOB SPECIFICATIONS AND ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND SITE DRAWINGS. CONSULT THESE DRAWINGS FOR LOCATIONS AND DIMENSIONS OF OPENINGS, CHASES, INSERTS, REGLETS, SLEEVES, DEPRESSIONS, AND OTHER DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS.

3. ALL DIMENSIONS, EXISTING CONDITIONS, AND AS-BUILT CONDITIONS MUST BE VERIFIED IN THE FIELD. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK.

4. THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE ONLY AFTER THE STRUCTURAL WORK CONTAINED IN THE S- DRAWINGS IS COMPLETED. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO ENSURE THE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS OR TIEDOWNS. SUCH MATERIAL SHALL REMAIN THE PROPERTY OF THE CONTRACTOR AFTER COMPLETION OF THE PROJECT.

5. SECTIONS AND DETAILS SHOWN ON ANY STRUCTURAL DRAWINGS SHALL BE CONSIDERED TYPICAL FOR SIMILAR CONDITIONS AS DETERMINED BY THE STRUCTURAL ENGINEER. THE STRUCTURAL ENGINEER RESERVES THE RIGHT TO INTERPRET DETAILS TO ADDRESS OTHER PROJECT CONDITIONS.

6. PROVIDE AND INSTALL NECESSARY MATERIAL TO CONNECT ELEVATOR SUPPORT BEAMS AND GUIDE RAILS. LOCATION AND SIZE OF MEMBERS AND ANY INSERTS REQUIRED SHALL BE DETERMINED BY THE ELEVATOR MANUFACTURER.

7. THE CONTRACTOR SHALL SUBMIT COMPLETE SHOP DRAWINGS FOR ALL PARTS OF THE WORK, INCLUDING DESCRIPTION OF SHORING, AND CONSTRUCTION METHODS AND SEQUENCING WHERE APPLICABLE. NO PERFORMANCE OF THE WORK INCLUDING, BUT NOT LIMITED TO, DEMOLITION OF EXISTING STRUCTURE, OR FABRICATION OR ERECTION OF NEW STRUCTURAL ELEMENTS, SHALL COMMENCE WITHOUT REVIEW OF THE SHOP DRAWINGS BY THE ARCHITECT AND ENGINEER. SUBMIT ONE COPY AND ONE SEPIA. COPY WILL BE REVIEWED AND SEPIA WILL BE RETURNED. FOR SHOP DRAWINGS AND SUBMITTALS REQUIRED, REFERENCE THE PROJECT SPECIFICATION.

8. ALL APPLICABLE FEDERAL, STATE, AND MUNICIPAL REGULATIONS SHALL BE FOLLOWED, INCLUDING THE FEDERAL DEPARTMENT OF LABOR OCCUPATIONAL SAFETY AND HEALTH ACT.

9. IN ACCORDANCE WITH THE MAINE UNIFORM BUILDING AND ENERGY CODE/INTERNATIONAL BUILDING CODE (2009 EDITION, SECTION 1704.1), A STATEMENT OF SPECIAL INSPECTIONS IS REQUIRED AS A CONDITION FOR PERMIT ISSUANCE BY THE LOCAL CODE OFFICIAL. THIS STATEMENT SHALL INCLUDE A COMPLETE LIST OF MATERIALS AND WORK REQUIRING SPECIAL INSPECTIONS, THE INSPECTIONS TO BE PERFORMED AND A LIST OF THE INDIVIDUALS, APPROVED AGENCIES AND FIRMS INTENDED TO BE RETAINED FOR CONDUCTING SUCH INSPECTIONS.

10. REFERENCE THE PROJECT SPECIFICATIONS FOR ALL TESTING REQUIREMENTS.

DESIGN LOADS (UNFACTORED)

1. BUILDING CODE:
   MAINE UNIFORM BUILDING AND ENERGY CODE
   INTERNATIONAL BUILDING CODE, 2009 EDITION
   ASCE 7-05 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES

2. PLANK DESIGN DEAD LOADS:
   SUPERIMPOSED DEAD LOAD U.N.O.: 25 PSF
   MECHANICAL EQUIPMENT WEIGHT: PER PLANS
   8" HOLLOW CORE (ASSUMED): 56 PSF**

   * SEE PLANS FOR ADDITIONAL DEAD LOAD REQUIREMENTS AT PLANK
   ** NOTIFY ENGINEER IF IN EXCESS OF ASSUMED LOADS, REVISIONS MAY BE REQUIRED.

3. DESIGN FLOOR LIVE LOADS:
   ALL GROUND FLOOR SPACES (RETAIL / RESTAURANT / OFFICE): 100 PSF
   PUBLIC / MEETING ROOMS AND CORRIDORS SERVING: 100 PSF
   PRIVATE ROOMS AND CORRIDORS SERVING: 40 PSF
   STAIRS: 100 PSF
   MECHANICAL / ELECTRICAL / LAUNDRY ROOMS: 100 PSF

   * SEE PLANS FOR ADDITIONAL LIVE LOAD REQUIREMENTS AT PLANK

3. DESIGN ROOF SNOW LOAD:
   GROUND SNOW LOAD (Pg): 60 PSF
   SNOW EXPOSURE FACTOR (Ce): 1.0
   SNOW LOAD IMPORTANCE FACTOR (Is): 1.0
   SNOW LOAD THERMAL FACTOR (Ct): 1.1
   FLAT ROOF SNOW LOAD (Pf): 46 PSF + DRIFT

   * SEE PLANS FOR DRIFT REQUIREMENTS AT PLANK

4. DESIGN WIND LOAD:
   BASIC WIND SPEED: 100 MPH
   WIND LOAD IMPORTANCE FACTOR (Iw): 1.0
   WIND EXPOSURE: C
   INTERNAL PRESSURE COEFFICIENT: ±0.18
   COMPONENTS & CLADDING PER ASCE 7-05

5. DESIGN SEISMIC LOADS:
   EQUIVALENT LATERAL FORCE PROCEDURE
   SEISMIC OCCUPANCY CATEGORY: II
   SEISMIC IMPORTANCE FACTOR (Ie): 1.0
   MAPPED SPECTRAL RESPONSE ACCELERATIONS:
   Ss: 0.314
   S1: 0.077
   SEISMIC SITE CLASS: D
   SPECTRAL RESPONSE COEFFICIENTS:
   Sds: 0.324
   Sd1: 0.123
   SEISMIC DESIGN CATEGORY: B
   BASIC STRUCTURAL SYSTEM: BUILDING FRAME SYSTEM
   BASIC SEISMIC FORCE RESISTING SYSTEM:
   STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE (R=3)
   RESPONSE MODIFICATION FACTOR (R): 3.0
   SEISMIC RESPONSE COEFFICIENT (Cs): 0.108

FOUNDATION NOTES (BEDROCK SUPPORTED)

1. FOUNDATIONS HAVE BEEN DESIGNED IN ACCORDANCE WITH A REPORT ENTITLED "GEOTECHNICAL ENGINEERING SERVICES, PROPOSED HYATT PLACE HOTEL, UNION AND FORE STREET, PORTLAND, MAINE", PREPARED BY S.W. COLE ENGINEERING INC., DATED 10/11/2012. THE RECOMMENDATIONS OF THE REPORT ARE PART OF THIS WORK. REFER TO THIS REPORT FOR SPECIFIC RECOMMENDATIONS.

2. FOUNDATION DESIGN IS BASED ON FOUNDATIONS BEARING DIRECTLY ON, OR SOCKETED INTO, SOUND BEDROCK PER THE REQUIREMENTS OF THE GEOTECHNICAL REPORT. REFER TO THIS REPORT FOR SPECIFIC BEARING RECOMMENDATIONS.

3. ALLOWABLE BEARING CAPACITY 20,000 PSF

4. EXTEND BOTTOM OF EXTERIOR GRADE BEAMS AT LEAST 4.5 FEET BELOW THE FINAL EXTERIOR GRADE FOR PROTECTION AGAINST FROST.

5. NO FOUNDATION ELEMENTS SHALL BE PLACED UNTIL BEARING CONDITIONS HAVE BEEN OBSERVED AND APPROVED BY THE GEOTECHNICAL ENGINEER.

6. REFERENCE THE GEOTECHNICAL REPORT FOR ALL EXCAVATION, BACKFILL, COMPACTION, CONSTRUCTION DEWATERING AND PERMANENT DRAINAGE REQUIREMENTS.

7. SOILS EXPOSED AT THE BASE OF ALL SATISFACTORY FOUNDATION EXCAVATIONS SHOULD BE PROTECTED AGAINST ANY DETRIMENTAL CHANGE IN CONDITION, SUCH AS DISTURBANCE FROM RAIN OR FROST. SURFACE RUNOFF SHALL BE DRAINED AWAY FROM THE EXCAVATIONS AND NOT BE ALLOWED TO POND. FOUNDATION EXCAVATIONS SHALL BE ADEQUATELY PROTECTED FROM RAINFALL OR FREEZING CONDITIONS. GROUNDWATER SHOULD BE ANTICIPATED FOR EXCAVATIONS AND APPROPRIATE DEWATERING MEASURES SHALL BE EMPLOYED.

8. EXCAVATIONS FOR BUILDING CONSTRUCTION SHALL BE IN ACCORDANCE WITH OSHA REQUIREMENTS. BRACED EXCAVATIONS SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF MAINE. DO NOT UNDERMINE EXISTING FOUNDATIONS OF ANY ADJACENT STRUCTURES. REFER TO THE GEOTECHNICAL REPORT FOR ADDITIONAL AND/OR MORE SPECIFIC REQUIREMENTS.

CONCRETE NOTES

1. CONCRETE WORK SHALL CONFORM TO "ACI MANUAL OF CONCRETE PRACTICE", LATEST EDITION. THIS PUBLICATION IS AVAILABLE THROUGH THE AMERICAN CONCRETE INSTITUTE (248) 848-3800.

2. ALL CONCRETE FOR DRILLED PIERS, GRADE BEAMS, & FOOTINGS AT ROCK ANCHORS SHALL HAVE A 28-DAY COMPRESSIVE STRENGTH OF 5,000 PSI. ALL OTHER FOUNDATION ELEMENTS SHALL HAVE A 28-DAY COMPRESSIVE STRENGTH OF 3,000 PSI. U.N.O. EXTERIOR SLABS-ON-GRADE SHALL HAVE A 28-DAY COMPRESSIVE STRENGTH OF 4,500 PSI. ADDITIONAL CONCRETE MIX PERFORMANCE DATA INCLUDING AIR CONTENT, WATER-CEMENT RATIO, AGGREGATE SIZE, SLUMP, ETC. HAS BEEN INCLUDED IN THE PROJECT SPECIFICATIONS. SEE THE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.

3. CONCRETE SHALL NOT BE PLACED IN WATER OR ON FROZEN GROUND.

4. PROVIDE PVC SLEEVES WHERE PIPES PASS THROUGH EXTERIOR CONCRETE, OR SLABS.

5. REINFORCING BARS SHALL CONFORM TO ASTM A615 GRADE 60 DEFORMED BARS AND SHALL BE DETAILED, FABRICATED AND PLACED IN ACCORDANCE WITH ACI 315, LATEST EDITION.

6. FIBER REINFORCEMENT SHALL BE TYPE III SYNTHETIC VIRGIN HOMOPOLYMER POLYPROPYLENE FIBERS CONFORMING TO ASTM C1116.

7. MINIMUM CONCRETE PROTECTIVE COVERING FOR REINFORCEMENT, UNLESS NOTED OTHERWISE, SHALL BE AS FOLLOWS:

   A.SURFACES CAST AGAINST AND PERMANENTLY IN CONTACT WITH EARTH, 3.0"
   B.FORMED SURFACES IN CONTACT WITH EARTH OR EXPOSED TO WEATHER
   #5 BARS, 5/8" DIAMETER WIRE AND SMALLER, 1.5"
   #6 THROUGH #11 BARS, 2.0"
   C.SURFACES NOT IN CONTACT WITH EARTH OR EXPOSED TO WEATHER
   WALLS, SLABS, JOISTS #11 BARS AND SMALLER, 1.0"
   BEAMS, GIRDERS, AND COLUMNS; ALL REINFORCEMENT, 1.5"

9. REINFORCEMENT SHALL BE CONTINUOUS AROUND CORNERS AND AT INTERSECTIONS. PROVIDE LAPPED BARS AT NECESSARY SPLICES OR HOOKED BARS AT DISCONTINUOUS ENDS. PROVIDE TENSION LAP SPLICES PER THE SCHEDULE DRAWING S3.1, FOR ALL REINFORCING UNLESS OTHERWISE SHOWN ON PLAN.

10. WELDING OF REINFORCEMENT IS NOT PERMITTED

11. FOR ALL OPENINGS IN CONCRETE WALLS AND SLABS, PROVIDE SUPPLEMENTAL REINFORCING AROUND OPENING AS SHOWN ON THE CONTRACT DOCUMENTS TYPICAL DETAILS. NO PENETRATIONS SHALL BE MADE THROUGH FOOTINGS WITHOUT WRITTEN PERMISSION FROM ENGINEER.

12. CONSTRUCTION JOINTS SHOWN ON DRAWINGS ARE MANDATORY. OMISSIONS, ADDITIONS, OR CHANGES SHALL NOT BE MADE EXCEPT WITH THE SUBMITTAL OF A WRITTEN REQUEST TOGETHER WITH DRAWINGS OF THE PROPOSED JOINT LOCATIONS FOR APPROVAL OF THE STRUCTURAL ENGINEER. WHERE CONSTRUCTION JOINTS ARE NOT SHOWN, OR WHEN ALTERNATE LOCATIONS ARE PROPOSED, DRAWINGS SHOWING LOCATION OF CONSTRUCTION AND CONTROL JOINTS AND CONCRETE PLACING SEQUENCE SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW PRIOR TO PREPARATION OF THE REINFORCEMENT SHOP DRAWINGS. CONCRETE SHALL BE PLACED WITHOUT HORIZONTAL CONSTRUCTION JOINTS EXCEPT WHERE SHOWN OR NOTED. VERTICAL CONSTRUCTION JOINTS AND STOPS IN CONCRETE BEAMS/ GRADE BEAMS SHALL BE MADE AT MIDSPAN OR AT POINTS OF MINIMUM SHEAR, UNLESS NOTED OTHERWISE.

13. SPACING OF CONSTRUCTION JOINTS, UNLESS NOTED OTHERWISE SHALL BE AS FOLLOWS:
   A.FOOTINGS AND WALLS
   MAX LENGTH 40'-0" OR 15'-0" FROM ANY CORNER**
   B.SLABS CON GRADE
   SEE FOUNDATION PLAN

   ** EXCEED ONLY WHERE INTERMEDIATE CONTRACTION JOINTS ARE PROVIDED. MINIMUM OF 72 HOURS SHALL ELAPSE BETWEEN ADJACENT CONCRETE PLACEMENTS.

14. ANCHOR RODS SHALL BE HEADED RODS CONFORMING TO ASTM F1554, GRADE 36 KSI WELDABLE STEEL, UNLESS NOTED OTHERWISE ON DRAWINGS. ANCHOR RODS THAT ARE TO BE IN CONTACT WITH PRESSURE TREATED LUMBER SHALL BE HOT-DIPPED GALVANIZED.

15. ALL GROUT BENEATH BASE PLATES & BEARING PLATES SHALL BE "5-STAR" 5000-PSI NON-SHRINK GROUT BY U.S. GROUT CORP.

16. SLAB THICKNESSES INDICATED ON THE DRAWINGS ARE MINIMUMS. PROVIDE SUFFICIENT CONCRETE TO ACCOUNT FOR STRUCTURE DEFLECTION, SUBGRADE FLUCTUATIONS, AND TO OBTAIN THE SPECIFIED SLAB ELEVATION AT THE FLATNESS AND LEVELNESS INDICATED.

17. INSTALLATION OF REINFORCEMENT SHALL BE COMPLETED AT LEAST 24 HOURS PRIOR TO THE SCHEDULED CONCRETE PLACEMENT. NOTIFY ARCHITECT AND STRUCTURAL ENGINEER OF COMPLETION AT LEAST 24 HOURS PRIOR TO THE SCHEDULED COMPLETION OF THE INSTALLATION OF REINFORCEMENT.

18. ALL ITEMS TO BE EMBEDDED INTO CONCRETE SHALL BE INSTALLED PRIOR TO PLACEMENT OF CONCRETE. PROVIDE ADDITIONAL REINFORCEMENT AND/OR TEMPLATES AS REQUIRED TO ENSURE THE CORRECT POSITIONS OF EMBEDMENTS. "WET SETTING" OF EMBEDMENTS INTO CONCRETE IS STRICTLY PROHIBITED. EMBEDMENTS INCLUDE, BUT NOT BY LIMITATION, REINFORCEMENT, REINFORCING DOWELS, EMBEDDED PLATES, ANCHOR RODS, ANCHOR INSERTS, SLEEVES, LOAD TRANSFER PLATES, DIAMOND DOWELS, AND SHELF BULK HEADS.

PRECAST CONCRETE HOLLOW CORE PLANK

1. ALL WORK SHALL CONFORM TO THE LATEST EDITION OF THE FOLLOWING: ACI 318 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE", PCI MNL-116 "MANUAL FOR QUALITY CONTROL FOR PLANTS AND PRODUCTION OF PRECAST AND PRESTRESSED CONCRETE PRODUCTS" AND PCI "DESIGN HANDBOOK-PRECAST AND PRESTRESSED CONCRETE".

2. PRECAST HOLLOW CORE PLANK SHALL BE DESIGNED FOR THE LIVE, DEAD, AND SNOW LOADS AS INDICATED UNDER "DESIGN LOADS" THIS SHEET AND ELSEWHERE ON THE DRAWINGS.

3. CONCRETE STRENGTH SHALL BE MINIMUM 5000 PSI AT 28 DAYS.

4. ALL CONCRETE SHALL BE AIR ENTRAINED. MEET AIR ENTRAINING REQUIREMENTS PER ACI "SPECIFICATIONS FOR STRUCTURAL CONCRETE (ACI 301)", FOR CLASS OF CONCRETE AND APPLICATION.

5. PRESTRESSING TENDONS SHALL CONFORM WITH ASTM A416, GRADE 250.

6. COMPLETE SHOP DRAWINGS AND DESIGN CALCULATIONS STAMPED BY A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE STATE OF MAINE SHALL BE SUBMITTED TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR REVIEW PRIOR TO COMMENCEMENT OF THAT PORTION OF THE WORK.

7. COORDINATE WITH ALL ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWING FOR LOCATION OF CUTS AND PENETRATIONS. INDICATE LOCATION OF ALL OPENINGS ON SHOP DRAWINGS. INDICATE IF PENETRATIONS ARE SHOP OR FIELD CUT.

STRUCTURAL STEEL NOTES

1. STRUCTURAL STEEL FABRICATION, ERECTION, AND CONNECTION DESIGN SHALL CONFORM TO AISC "SPECIFICATION FOR THE DESIGN FABRICATIONS, AND ERECTION OF STRUCTURAL STEEL" LATEST EDITION, AND THE "CODE OF STANDARD PRACTICE", LATEST EDITION.

2. STRUCTURAL STEEL: STEEL PLATES, SHAPES, AND BARS, CONFORM TO ASTM A36 UNLESS NOTED OTHER WISE (U.N.O.). STRUCTURAL STEEL SHAPES DESIGNATED ON THE DRAWINGS FOR WIDE-FLANGE SECTIONS: ASTM A992 (ASTM A572 GRADE 50 WITH SPECIAL REQUIREMENTS PER AISC TECHNICAL BULLETIN #3 DATED MARCH, 1997)

3. STRUCTURAL TUBING: CONFORM TO ASTM A500 GRADE B46 KSI.

4. CONNECTION DESIGN FOR THIS PROJECT IS THE RESPONSIBILITY OF THE FABRICATOR. CONNECTION CALCULATIONS, SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF MAINE SHALL BE SUBMITTED WITH THE SHOP DRAWINGS FOR THIS PROJECT. SEE THE PROJECT SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.

5. FIELD CONNECTIONS SHALL BE BOLTED USING ASTM A325N HIGH STRENGTH BOLTS (U.N.O.) EXCEPT WHERE SLIP CRITICAL CONNECTIONS ARE REQUIRED AND NOTED BY A325 (SC) ON THE DRAWINGS. PROVIDE SLIP CRITICAL (SC) CONNECTIONS AT ALL MOMENT CONNECTIONS, BRACED FRAMES, RELIEVING ANGLES AND AS OTHERWISE NOTED. USE A490 BOLTS WHERE INDICATED.

6. WHERE WELDING IS INDICATED, ALL WELDING SHALL CONFORM TO AWS D1.1-LATEST EDITION. ELECTRODES SHALL CONFORM TO AWS A5.1 E70XX SERIES WITH PROPER ROD TO PRODUCE OPTIMUM WELD (LOW HYDROGEN)

7. SEE CONCRETE NOTES AND DRAWINGS FOR ANCHOR BOLT INFORMATION, TYP.

8. PROVIDE 3/8" MINIMUM STIFFENER PLATES EACH SIDE OF BEAM WEB AT BEAMS FRAMING OVER COLUMNS AND AT BEAMS SUPPORTING COLUMNS ABOVE.

9. PROVIDE ¼" THICK LEVELING PLATE UNDER ALL COLUMN BASE PLATES UNLESS OTHERWISE NOTED. LEVELING PLATES SHALL BE SET AND GROUTED PRIOR TO ERECTING COLUMNS.

10. PROVIDE ALL MISCELLANEOUS ANGLES, PLATES, ANCHOR BLOTS ETC., SHOWN ON ARCHITECTURAL DRAWINGS FOR SUPPORT OF BLOCKING, PARAPETS, FINISHES, ETC. COORDINATE WITH MISCELLANEOUS METAL FABRICATOR TO ENSURE COMPLETE COVERAGE OF ALL ITEMS.

11. PROVIDE L 4 x 4 x 1/2 PLANK SUPPORT ANGLE AS REQUIRED AT COLUMNS U.N.O., SEE DETAILS FOR ADDITIONAL REQUIREMENTS.

GIRDER-SLAB STRUCTURAL SYSTEM NOTES

1. THE OPEN WEB DISSYMMETRIC BEAM (DB) SHALL BE FABRICATED FROM ASTM A 992/A572, GRADE 50 STANDARD WIDE FLANGE SECTIONS WITH GRADE 50 FLAT BAR AT THE TOP FLANGE AND SHALL MEET AISC STANDARDS (EXCEPT FOR DEPTH, TOLERANCE ±1/8 INCH), CAMBER SHALL BE BUILT IN DURING ASSEMBLY OF THE DISSYMMETRIC BEAM.

2. ERECTOR IS RESPONSIBLE FOR DETERMINING AND PROVIDING ALL, SHORING AS NECESSARY TO ERECT THE SUPERSTRUCTURE, AS PART OF THE CONTRACTOR'S MEANS AND METHODS REQUIREMENTS. IN ADDITION, PROVIDE SHORING WHERE INDICATED FOR PRE-COMPOSITE STRENGTH.

3. MINIMUM BEARING OF PRECAST PRESTRESSED HOLLOW CORE SLAB UNITS ON DISSYMMETRIC BEAMS SHALL BE 2 INCHES. OPEN THE TOP OF EACH SLAB CORE FOR INSPECTION PRIOR TO GROUTING. REFER TO THE GIRDER-SLAB SYSTEM REFERENCE INFORMATION FOR ADDITIONAL REQUIREMENTS.

4. REINFORCING STEEL (ASTM A615, GRADE 60) SHALL BE PLACED THROUGH THE DISSYMMETRIC BEAM WEB OPENINGS INTO THE SLAB CORES.

5. CEMENTITIOUS GROUT (MIN. 4000 PSI) SHALL BE PLACED MONOLITHICALLY AROUND AND THROUGH THE DISSYMMETRIC BEAM WEB OPENINGS AND INTO THE SLAB CORES FILLED SOLID FOR A MINIMUM OF 8 INCHES, LEVEL TO THE SLAB SURFACE WITH A 9/16 INCH MINIMUM AVERAGE THICKNESS OVER THE TOP FLANGE (U.N.O.)

6. THE GIRDER-SLAB SYSTEM SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE UNDERWRITERS LABORATORIES, INC. FLOOR CEILING ASSEMBLY SPECIFIED BY THE ARCHITECT.

5. THE GIDER-SLAB SYSTEM AND D-BEAM GIRDERS SHALL BE PROVIDED BY STEEL CONTRACTORS AUTHORIZED BY GIRDER-SLAB TECHNOLOGIES LLC OF NJ IN CONFORMANCE WITH ITS DESIGN-GUIDE & DISTRIBUTION REQUIREMENTS. STEEL CONTRACTOR/DISTRIBUTOR CONTACT INFORMATION: 1-888-478-1100 OR WWW.GIRDER-SLAB.COM.

6. THE SUPPLIER OF THE GIRDER-SLAB SYSTEM SHALL PROVIDE TO THE PROJECT OWNER AND THE ARCHITECT A GIRDER-SLAB COMPLIANCE CERTIFICATE UPON COMPLETION OF SYSTEM ASSEMBLY AND CONSTRUCTION.

7. COMPLY WITH ALL APPLICABLE PROVISIONS OF THE STANDARDS AND CODES REFERENCED IN THE PROJECT SPECIFICATIONS.

METAL DECK

1. THE METAL ROOF AND FLOOR DECK SHALL BE FORMED OF STEEL SHEETS CONFORMING TO THE FOLLOWING STANDARDS:
   A. FLOOR DECKING: ASTM A1008, GRADE C, D OR ASTM A653, STRUCTURAL QUALITY, GRADE 40 OR HIGHER
   B. ROOF DECKING: ASTM A1008, GRADE C, D OR ASTM A653, STRUCTURAL QUALITY, GRADE 33 OR HIGHER
2. FLOOR AND ROOF DECK SHALL BE AS NOTED ON THE DRAWINGS (OR EQUIVALENT).
3. FOR DECK ATTACHMENTS, PENETRATIONS AND ACCESSORIES REFER TO SPECIFICATIONS.

CANAL 5 STUDIO
Architecture / Planning / Interior Design
207 553 2115
One Canal Plaza, Suite 888
Portland, Maine 04101
canal5studio.com

EAST BROWN COW MANAGEMENT, INC.

shinberg CONSULTING, LLC

Project Title

HYATT PLACE PORTLAND-OLD PORT

443 FORE STREET PORTLAND, ME

CSS Project No. 12013

BECKER STRUCTURAL ENGINEERS
75 York Street, Portland, Maine 04101
207.878.1838 ● beckerstructural.com

STATE OF MAINE
PAUL B. BECKER
NO. 6554
LICENSED PROFESSIONAL ENGINEER

Mark	Date	Description

Project Status

ISSUED FOR CONSTRUCTION

11/16/12

Drawing Title

GENERAL NOTES

Scale: ***NOT TO SCALE***

Drawing Number

S1.0