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GENERAL NOTES:

1. REFERENCE ARCHITECTURAL PLANS FOR DIMENSIONS NOT SHOWN. REFERENCE MECHANICAL, ELECTRICAL, AND ARCHITECTURAL PLANS FOR SIZES AND LOCATIONS OF WALL AND SLAB OPENINGS, DUCTS, PIPING, CURBS, AND EQUIPMENT PADS. IN THE EVENT OF A CONFLICT BETWEEN THE DRAWINGS, SPECIFICATIONS, OR NOTES ON THE DRAWINGS, THE ENGINEER SHALL BE NOTIFIED PRIOR TO CONSTRUCTION.
2. EXISTING DIMENSIONS AND CONDITIONS ARE FOR REFERENCE ONLY. CONTRACTOR SHALL VERIFY ALL EXISTING CONSTRUCTION AND DIMENSIONS IN THE FIELD PRIOR TO CONSTRUCTION OR FABRICATION. ALL DISCREPANCIES SHALL BE REPORTED TO THE ENGINEER PRIOR TO COMMENCING WORK.
3. THE CONTRACTOR SHALL NOTIFY THE ENGINEER IF DEVIATIONS OR CHANGES ARE REQUIRED TO THE CONTRACT DOCUMENTS OR APPROVED SHOP DRAWINGS DUE TO INTERFERENCES, FABRICATION ERRORS, OR OTHER CAUSES.
4. THE STRUCTURE IS SELF-SUPPORTING AND STABLE AFTER THE ENTIRE BUILDING IS COMPLETELY CONSTRUCTED. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR ERECTION PROCEDURES AND SEQUENCING DURING CONSTRUCTION AND ERECTION TO PROVIDE AND ENSURE LOCAL AND OVERALL STABILITY OF THE BUILDING AND ITS COMPONENTS DURING CONSTRUCTION AND ERECTION. THE CONTRACTOR SHALL RETAIN A LICENSED STRUCTURAL ENGINEER TO DESIGN TEMPORARY BRACING/SHORING AND DETERMINE WHERE THE TEMPORARY BRACING/SHORING IS NEEDED.
5. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ERECTION PROCEDURES, SEQUENCING AND FOR COMPLYING WITH ALL APPLICABLE SAFETY REGULATIONS DURING THE WORK.
6. ONE ELECTRONIC COPY OR TWO SETS OF HARD COPIES OF SHOP DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER. ONE HARD COPY WILL BE RETURNED TO THE CONTRACTOR AND ONE HARD COPY WILL BE RETAINED BY THE ENGINEER.
7. REFERENCE THE PROJECT SPECIFICATIONS FOR MATERIAL, WORKMANSHIP AND ADDITIONAL INFORMATION NOT COVERED IN THESE NOTES (WHERE APPLICABLE)

DESIGN CRITERIA:

1. BUILDING CODES:
   INTERNATIONAL BUILDING CODE (IC), 2009 EDITION
   ASCE 7-05 MINIMUM DESIGN LOADS FOR BUILDING AND OTHER STRUCTURES
   MAINE UNIFORM BUILDING AND ENERGY CODE
2. LIVE LOADS:
   FIRST FLOOR LIVING AREAS = 40 PSF
   SECOND FLOOR SLEEPING AREAS = 30 PSF
3. SNOW LOADS:
   GROUND SNOW LOAD (Pg) = 60 PSF
   SNOW EXPOSURE FACTOR (Ce) = 1.0
   SNOW LOAD IMPORTANCE FACTOR (Is) = 1.0
   THERMAL FACTOR (Ct) = 1.1
   FLAT ROOF SNOW LOAD (Pf) = 46.2 PSF + DRIFT
4. WIND LOADS:
   BASIC WIND SPEED = 100 MPH
   IMPORTANCE FACTOR (Iw) = 1.0
   WIND EXPOSURE B
   MAIN WINDFORCE-RESISTING SYSTEM (INCLUDES WINDWARD + LEEWARD) = 18 PSF
   COMPONENTS & CLADDING – PER ASCE 7-05

FOUNDATION NOTES:

1. FOUNDATIONS HAVE BEEN DESIGNED USING A PRESUMED ALLOWABLE BEARING PRESSURE PER TABLE 1806.2 OF THE INTERNATIONAL BUILDING CODE BASED ON TYPICAL SOILS FOUND IN THIS AREA. IF CLAY, MUD, ORGANIC SILT, PEAT OR UNPREPARED FILL IS FOUND DURING CONSTRUCTION, NOTIFY ENGINEER IMMEDIATELY, AS THE ALLOWABLE LOADS USED IN DESIGN WILL NEED TO BE VERIFIED BY A GEOTECHNICAL ENGINEER. CASCO BAY ENGINEERING RECOMMENDS PROCURING A GEOTECHNICAL ENGINEER TO VERIFY EXISTING SOIL CONDITIONS.
2. ALLOWABLE SOIL BEARING CAPACITY USED IN DESIGN = 2,000 PSF
3. MINIMUM FROST DEPTH COVER = 4'-6" FOR EXTERIOR FOOTINGS BELOW FINAL EXTERIOR GRADE.
4. EXCAVATION, BACKFILL, COMPACTION, GRADATION REQUIREMENTS, FOUNDATION DRAINAGE AND PERMANENT DEWATERING REQUIREMENTS SHALL BE PROVIDED BY A GEOTECHNICAL ENGINEER.
5. CONCRETE SLABS ON GRADE SHALL BE CONSTRUCTED ON A MINIMUM 12" THICK LAYER OF PROPERLY COMPACTED STRUCTURAL FILL, UNLESS OTHERWISE DIRECTED BY A GEOTECHNICAL ENGINEER.
6. FOUNDATIONS SHALL BEAR ON UNDISTURBED NATIVE SOIL, UNLESS NOTED OTHERWISE. THE CONTRACTOR SHALL NOTIFY THE GEOTECHNICAL AND STRUCTURAL ENGINEER IF ANY UNSUITABLE SOILS ARE ENCOUNTERED PRIOR TO PLACING FOUNDATIONS.
7. FOUNDATION WALLS AND SLAB-ON-GRADES SHALL REACH THEIR FULL 28 DAY COMPRESSIVE STRENGTH PRIOR TO BACKFILLING. THE CONTRACTOR SHALL PROVIDE TEMPORARY SHORING/BRACING FOR WALLS WHEN BACKFILL IS PLACED PRIOR TO CONCRETE ACHIEVING ITS FULL 28 DAY STRENGTH. CONTRACTOR SHALL PROVIDE TEMPORARY SHORING/BRACING FOR WALLS AND OTHER STRUCTURAL ELEMENTS PRIOR TO INSTALLATION OF PERMANENT BRACING/FLOOR/STRUCTURE.
8. PROTECT FOUNDATIONS FROM FROST AND KEEP BOTTOM OF TRENCH DRY DURING CONSTRUCTION. IF GROUNDWATER IS ENCOUNTERED NEAR OR ABOVE THE BASE OF THE FOOTINGS, EXCAVATIONS SHALL BE DEWATERED DURING CONSTRUCTION. SURFACE WATER SHALL BE DIVERTED AWAY FROM EXCAVATIONS.
9. DO NOT UNDERMINE EXISTING FOUNDATIONS OF ADJACENT STRUCTURES. CONTRACTOR SHALL BE RESPONSIBLE FOR THE SHORING, BRACING AND UNDERPINNING OF EXISTING STRUCTURES DURING EXCAVATION, BACKFILLING, AND CONSTRUCTION. CONTRACTOR SHALL SLOPE EXCAVATIONS TO ACHIEVE SOIL STABILITY.

CONCRETE NOTES:

1. ALL WORK SHALL CONFORM TO IC 2009 REFERENCED EDITIONS OF "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" (ACI 318) AND "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" (ACI 301).
2. REQUIRED CONCRETE PARAMETERS ARE AS FOLLOWS:

LOCATION	MAX W/C RATIO	f'c	AIR-ENTRAINMENT
INT. WALLS/ELEV. SLABS	.52	3,000 PSI	2% ± 1½%
FOUNDATIONS, FOOTINGS, & FOUNDATION WALLS	.45	4,500 PSI	6% ± 1½%
INT. SLAB-ON-GRADE	.45	4,500 PSI	2% ± 1½%
EXT. SLAB-ON-GRADE	.45	4,500 PSI	6% ± 1½%

   WHERE: W/C = WATER TO CEMENT RATIO AND
   f'c = COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS

   USE PORTLAND CEMENT TYPE II, IN CONFORMANCE WITH ASTM 150
   AIR ENTRAINING ADMIXTURES SHALL CONFORM TO ASTM C 260
   ADMIXTURES SHALL CONFORM TO ASTM C 494
   FLY ASH USED AS ADMIXTURES SHALL CONFORM TO ASTM C 618
3. MAXIMUM AGGREGATE SIZE SHALL BE ¾", IN CONFORMANCE WITH ASTM C33.
4. CALCIUM CHLORIDE OR ADMIXTURES CONTAINING CALCIUM CHLORIDE IS NOT PERMITTED.
5. MAXIMUM SLUMP AFTER THE ADDITION OF A WATER-REDUCING ADMIXTURE IS 6 INCHES.
6. CONTRACTOR SHALL NOT PLACE CONCRETE ON FROZEN GROUND OR IN WATER. ADEQUATE EQUIPMENT SHALL BE PROVIDED FOR HEATING CONCRETE MATERIALS AND PROTECTING CONCRETE DURING NEAR-FREEZING OR FREEZING WEATHER. REFERENCE ACI 306, AS NOTED ABOVE, FOR RECOMMENDATIONS FOR COLD WEATHER CONCRETING.
7. VERTICAL CONSTRUCTION JOINTS IN WALLS SHALL NOT EXCEED A SPACING OF 40 FEET, U.N.O.
8. ANCHOR BOLTS SHALL BE HEADED RODS AND CONFORM TO ASTM F1554, GRADE 36 KSI WELDABLE STEEL, U.N.O. ON DRAWINGS. PROVIDE GALVANIZED ANCHOR BOLTS WHERE IN CONTACT WITH PRESSURE TREATED LUMBER.
9. REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60, DEFORMED BARS.
10. WELDED WIRE FABRIC (WWF) SHALL CONFORM TO ASTM A185 AND BE PROVIDED IN FLAT SHEETS. PROVIDE ADEQUATE SUPPORT FOR WWF TO ENSURE PROPER LOCATION WITHIN SLAB DURING CONCRETE PLACEMENT.
11. MINIMUM CONCRETE COVER FOR REINFORCEMENT SHALL BE AS FOLLOWS:
    A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH — 3 INCHES
    B. FORMED CONCRETE IN CONTACT WITH EARTH OR EXPOSED TO WEATHER — 2 INCHES
    C. CONCRETE NOT EXPOSED TO EARTH OR WEATHER IN SLABS & WALLS — 1½ INCHES
12. WELDING OF REINFORCEMENT IS NOT PERMITTED.
13. PROVIDE NON-SHRINK GROUT BENEATH LEVELING PLATES & BEARING PLATES w/ MINIMUM COMPRESSIVE STRENGTH OF 7,000 PSI AT 28 DAYS.
14. PROVIDE CONTINUOUS REINFORCEMENT AT ALL CORNERS AND INTERSECTIONS, SEE TYPICAL FOUNDATION WALL DETAILS ON FOUNDATION DETAILS SHEET.
15. REINFORCING BARS AND ALL EMBEDDED ITEMS, INCLUDING ANCHOR BOLTS, MUST BE ACCURATELY PLACED AND ADEQUATELY SECURED BEFORE CONCRETE IS PLACED. "WET SETTING" OF COLUMN ANCHOR BOLTS INTO CONCRETE IS STRICTLY PROHIBITED.
16. UNLESS NOTED ON DRAWINGS, FOLLOW ACI STANDARDS FOR LAP SPLICE LENGTHS OF REINFORCING BARS.

REBAR LAP SPLICE TABLE

BAR SIZE	#3	#4	#5	#6	#7	#8	#9
3000 PSI CONCRETE	18"	24"	30"	36"	48"	56"	64"
4500 PSI CONCRETE	16"	20"	24"	30"	40"	48"	54"

STRUCTURAL STEEL NOTES:

1. STRUCTURAL STEEL WORK SHALL CONFORM TO IC 2009 REFERENCED EDITIONS OF AISC "SPECIFICATION FOR THE DESIGN FABRICATIONS, AND ERECTION OF STRUCTURAL STEEL" AND THE "CODE OF STANDARD PRACTICE"
2. STRUCTURAL STEEL MEMBERS SHALL BE IN CONFORMANCE WITH THE FOLLOWING:

WIDE FLANGE SHAPES AND TEES	ASTM A992
ANGLES, PLATES, CHANNELS	ASTM A36, Fy=36 KSI (U.N.O.)
SQUARE/RECTANGULAR HSS	ASTM A500, GRADE B, Fy=46 KSI
ROUND HSS	ASTM A500, GRADE B, Fy=42 KSI
STEEL PIPE	ASTM A53, TYPE E OR S, GRADE B, Fy=35 KSI

3. SHOP DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW PRIOR TO COMMENCING FABRICATION.
4. FIELD CONNECTIONS SHALL UTILIZE MINIMUM ¾" DIAMETER A325 HIGH STRENGTH BOLTS, U.N.O. BOLTED CONNECTIONS SHALL BE SLIP CRITICAL AT ALL MOMENT FRAMES, BRACED FRAMES, AND AT ADDITIONAL LOCATIONS INDICATED "SC" IN THE DRAWINGS. SLIP CRITICAL CONNECTIONS SHALL UTILIZE LOAD INDICATOR WASHERS OR TENSION CONTROL BOLTS. USE A490 BOLTS WHERE INDICATED ON DRAWINGS.
5. CONTRACTOR IS RESPONSIBLE FOR DESIGN OF CONNECTIONS NOT ALREADY DETAILED ON STRUCTURAL DRAWINGS. CONTRACTOR SHALL SUBMIT DESIGN STAMPED BY A STRUCTURAL ENGINEER LICENSED IN THE STATE WHERE PROJECT IS LOCATED PRIOR TO COMMENCING FABRICATION.
6. WELDING SHALL CONFORM TO AWS D1.1. USE LOW-HYDROGEN SMAW ELECTRODES WITH MINIMUM TENSILE STRENGTH OF 70 KSI.
7. PROVIDE ¼" LEVELING PLATES UNDER ALL COLUMN BASE PLATES, U.N.O. LEVELING PLATES SHALL BE SET AND GROUTED PRIOR TO COLUMN ERECTION.
8. ALL STRUCTURAL STEEL NOT EXPOSED TO WEATHER SHALL RECEIVE ONE COAT OF STANDARD SHOP PRIMER, U.N.O.
9. SEE DRAWINGS AND CONCRETE NOTES FOR ANCHOR BOLT INFORMATION.

WOOD NOTES:

1. ALL TIMBER FRAMING SHALL BE IN ACCORDANCE WITH IC 2009 REFERENCED EDITIONS OF THE AITC TIMBER CONSTRUCTION MANUAL AND AF&PA NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION (NDS).
2. ALL FRAMING SHALL BE SPRUCE-PINE-FIR, No.2 OR BETTER U.N.O. AND HAVE A MAXIMUM MOISTURE CONTENT OF 19%.
3. ALL WOOD IN CONTACT WITH MASONRY OR CONCRETE OR EXPOSED TO WEATHER SHALL BE PRESSURE TREATED (PT) SOUTHERN YELLOW PINE.
4. WHERE "LVL" IS NOTED ON DRAWINGS, PROVIDE LAMINATED VENEER LUMBER, WHICH HAS THE FOLLOWING MINIMUM ALLOWABLE STRESSES:

   Fb = 2600 PSI — Fc = 2510 PSI (PARALLEL TO GRAIN)
   Fv = 285 PSI — Fc = 750 PSI (PERPENDICULAR TO GRAIN)
   Ft = 1555 PSI — E = 2,000,000 PSI
5. WHERE "PSL" IS NOTED ON DRAWINGS, PROVIDE PARALLAM STRAND LUMBER, WHICH HAS THE FOLLOWING MINIMUM ALLOWABLE STRESSES:

   Fb = 2900 PSI — Fc = 2900 PSI (PARALLEL TO GRAIN)
   Fv = 290 PSI — Fc = 750 PSI (PERPENDICULAR TO GRAIN)
   Ft = 2025 PSI — E = 2,000,000 PSI
6. ALL ENGINEERED LUMBER THAT IS EXPOSED TO WEATHER SHALL BE WOLMANIZED.
7. ALL FLOOR SHEATHING SHALL BE ¾" TONGUE AND GROOVE, GLUED AND NAILED TO FLOOR FRAMING WITH 8d RINK SHANK NAILS AT 6" o.c. AT SUPPORTED PANEL EDGES, 12" o.c. AT INTERMEDIATE SUPPORTS UNLESS NOTED OTHERWISE ON DRAWINGS.
8. ALL ROOF SHEATHING (5/8") AND WALL SHEATHING (1/2") SHALL BE APA PERFORMANCE-RATED. ATTACH TO SUPPORTED PANEL EDGES WITH 8d NAILS AT 6" o.c. AND AT INTERMEDIATE SUPPORTS WITH 8d NAILS AT 12" o.c. U.N.O. SEE DRAWINGS FOR MORE STRINGENT NAILING REQUIREMENTS AT WOOD SHEAR WALLS.
9. SHEATHING SHALL BE ORIENTED WITH LONG DIMENSION PERPENDICULAR TO THE SUPPORTS AND BE CONTINUOUS OVER TWO OR MORE SUPPORTS. STAGGER ALL JOINTS & PROVIDE ADEQUATE JOINT SPACING (1/8" TYP) AS RECOMMENDED BY MANUFACTURER.
10. PROVIDE FULL DEPTH BLOCKING AT ENDS AND INTERIOR SUPPORTS OF ALL JOISTS AND RAFTERS WHERE JOISTS AND RAFTERS FRAME OVER SUPPORTS. PROVIDE 1x3 DIAGONAL BRIDGING OR FULL DEPTH SOLID BLOCKING FOR EACH 8'-0" OF SPAN FOR ALL JOISTS AND RAFTERS.
11. WHERE BEAMS ARE LABELED ON PLAN, DO NOT SPLICE BEAM NOR ANY PLY OF BEAM BETWEEN SUPPORTS.
12. ALL CONNECTION HARDWARE SHALL BE BY SIMPSON STONG-TIE (OR APPROVED EQUIVALENT) AND SHALL BE HOP-DIPPED GALVANIZED. HARDWARE IN CONTACT WITH PRESSURE TREATED (PT) LUMBER SHALL BE GALVANIZED G185 (ZMAX). REFER TO MANUFACTURERS LITERATURE FOR PROPER INSTALLATION GUIDELINES.
13. FASTENERS USED IN CONTACT WITH PRESSURE TREATED (PT) LUMBER SHALL BE HOT-DIPPED GALVANIZED, STAINLESS STEEL, OR OTHER FINISH APPROVED BY ENGINEER.
14. ALIGN COLUMNS SUCH THAT COLUMNS BEAR CONTINUOUSLY TO FOUNDATION SUPPORT. INSTALL ADDITIONAL SOLID BLOCKING WITHIN FLOOR PACKAGE TO PROVIDE CONTINUITY OF LOAD PATH.
15. PROVIDE HORIZONTAL BLOCKING FOR ALL LOAD BEARING WALLS AT 4'-0" O.C. VERTICAL, MAXIMUM.
16. SUBMIT SHOP DRAWINGS FOR ALL PREFABRICATED WOOD JOISTS AND WALL PANELS TO ENGINEER FOR REVIEW PRIOR TO CONSTRUCTION.

MASONRY NOTES:

1. ALL MASONRY CONSTRUCTION AND MATERIALS SHALL CONFORM TO IC 2009 REFERENCED EDITIONS OF THE BUILDING CODE REQUIREMENTS AND SPECIFICATION FOR MASONRY STRUCTURES (ACI 530 AND ACI 530.1)
2. CONCRETE MASONRY UNITS (CMU) SHALL BE ASTM C90 GRADE N, TYPE 1 STANDARD WEIGHT WITH MINIMUM PRISM STRENGTH (f'm) = 1,500 PSI
3. CMU SHALL BE STANDARD UNITS WITH NOMINAL FACE DIMENSIONS (LENGTH = 16 INCHES, HEIGHT = 8 INCHES, WIDTH AS INDICATED)
4. MORTAR SHALL CONFORM TO ASTM C270, TYPE M OR S. PLACE MASONRY WHILE MORTAR IS SOFT AND PLASTIC. TYPE N MORTAR MAY BE USED FOR ABOVE GRADE MASONRY VENEER OR INTERIOR NON-LOAD BEARING APPLICATIONS.
5. GROUT SHALL CONFORM WITH ASTM C476 WITH MINIMUM COMPRESSIVE STRENGTH = 3,000 PSI
6. VERTICAL REINFORCEMENT, BOND BEAM REINFORCEMENT AND REINFORCEMENT IN LINTELS SHALL CONFORM TO ASTM A615, GRADE 60 DEFORMED BARS.
7. STANDARD LAP LENGTH FOR GRADE 60 MASONRY REINFORCEMENT SHALL BE 48 BAR DIAMETERS OR AS SHOWN ON DRAWINGS, WHICHEVER IS GREATER. PROVIDE CORNER BARS TO MATCH HORIZONTAL REINFORCEMENT.
8. HORIZONTAL REINFORCEMENT SHALL BE LADDER TYPE, STANDARD CLASS, GALVANIZED WITH 9 GAGE SIDE RODS AND 9 GAGE CROSS TIES. BEGIN HORIZONTAL REINFORCEMENT AT TOP OF FIRST COURSE AND EVERY OTHER COURSE, U.N.O. LAP HORIZONTAL REINFORCEMENT 6 INCHES, MIN.
9. ALL UNITS SHALL BE LAID IN RUNNING BOND, UNLESS NOTED OTHERWISE. PROVIDE FULL MORTAR COVERAGE ON HORIZONTAL AND VERTICAL FACES. EXPOSED JOINTS SHALL BE TOOLED CONCAVE, UNEXPOSED JOINTS SHALL BE STRUCK FLUSH. PROVIDE CORNER BLOCKS AND END BLOCKS TO FINISH ALL 90 DEGREE CORNERS AND OPENINGS.
10. PROVIDE VERTICAL CONTROL JOINTS IN WALLS AT A MAXIMUM SPACING OF 24'-0" AND AT APPROXIMATELY ½ WALL HEIGHT FROM WALL INTERSECTIONS.
11. PROVIDE LINTELS AT WALL PENETRATIONS AS SHOWN ON DRAWINGS. USE LINTEL-TYPE BLOCKS AND PROVIDE MINIMUM 8" OF BEARING AT EACH END OF LINTEL U.N.O.
12. FIELD PENETRATIONS THROUGH CMU WALLS SHALL NOT BE MADE THROUGH BOND BEAMS, LINTELS OR GROUTED CELLS.
13. SECURE ALL CMU WALL SUPPORTED FIXTURES, EQUIPMENT, ETC. TO CMU WALL PER STRUCTURAL DRAWINGS AND MANUFACTURER'S RECOMMENDATIONS. DO NOT USE EXPANSION ANCHORS.

ABBREVIATION:

AB	ANCHOR BOLT	L	ANGLE
ABV	ABOVE	LL	DOUBLE ANGLE
ADDL	ADDITIONAL	LB	POUND
ARCH	ARCHITECT	LF	LINEAR FOOT
&	AND	LLH	LONG LEG HORIZONTAL
		LLV	LONG LEG VERTICAL
B/FTG, BOF	BOTTOM OF FOOTING		
BLDG	BUILDING	MAX	MAXIMUM
BM	BEAM	MECH	MECHANICAL
BOT	BOTTOM	MFR	MANUFACTURER
BRG	BEARING	MIN	MINIMUM
BTWN	BETWEEN	MISC	MISCELLANEOUS
C	STRUCTURAL STEEL CHANNEL	NF	NEAR FACE
CANT	CANTILEVER	NO	NUMBER
CIP	CAST-IN-PLACE CONCRETE	NS	NEAR SIDE
CJ	CONTROL JOINT	NTS	NOT TO SCALE
CL	CENTERLINE		
CLR	CLEAR	OC	ON CENTER
CMU	CONCRETE MASONRY UNIT	OF	OUTSIDE FACE
CNJ	CONSTRUCTION JOINT	OPNG	OPENING
COL	COLUMN	OPP	OPPOSITE
CONC	CONCRETE		
CONN	CONNECTION	P	PIER DESIGNATION
CONT	CONTINUOUS	PL	PLATE
CONTR	CONTRACTOR	PP	PARTIAL PENETRATION WELD
CP	COMPLETE PENETRATION WELD	PREFAB	PREFABRICATED
CY	CUBIC YARD	PSF	POUNDS PER SQUARE FOOT
		PSI	POUNDS PER SQUARE INCH
DIA	DIAMETER		
DIM	DIMENSION	REINF	REINFORCING STEEL
DISCONT	DISCONTINUOUS	REQ, REQD	REQUIRED
DWG	DRAWING	RD	ROOF DRAIN
(E), EX, EXIST	EXISTING	SC	SLIP CRITICAL
EA	EACH	SECT	SECTION
EF	EACH FACE	SHEATH	SHEATHING
EL, ELEV	ELEVATION	SIM	SIMILAR
EQ	EQUAL	SOG	SLAB-ON-GRADE
EQUIP	EQUIPMENT	SPAC	SPACING
ES	EACH SIDE	SPECS	SPECIFICATIONS
EW	EACH WAY	SS	STAINLESS STEEL
EXP	EXPANSION	STD	STANDARD
EXT	EXTERIOR	STIFF	STIFFENER
		STL	STEEL
F	FOOTING DESIGNATION	STR	STRAIGHT
FDN	FOUNDATION	STRUCT	STRUCTURAL
FF	FINISH FLOOR		
FLG	FLANGE	T	TOP
FLR	FLOOR	T&B	TOP AND BOTTOM
FT	FOOT	TOC, T/CONC	TOP OF CONCRETE
FTG	FOOTING	T/FTG, TOF	TOP OF FOOTING
FV	FIELD VERIFY	TEMP	TEMPERATURE
		T/SHELF	TOP OF SHELF
G	GAGE	T/SLAB	TOP OF SLAB
GALV	GALVANIZED	T/STL	TOP OF STEEL
		T/WALL	TOP OF WALL
HOR, HORIZ	HORIZONTAL	TS	STRUCTURAL TUBING
HSS	HOLLOW STRUCTURAL SHAPE	TYP	TYPICAL
HT	HEIGHT		
		UNO	UNLESS NOTED OTHERWISE
IF	INSIDE FACE		
IN	INCH	VER, VERT	VERTICAL
INFO	INFORMATION	VIF	VERIFY IN FIELD
JT	JOINT	W	STRUCTURAL STEEL WIDE FLANGE
		w/	WITH
K	KIP (1 KIP = 1,000 LBS)	w/O	WITHOUT
KSI	KIPS PER SQUARE INCH	WP	WORK POINT
		WT	WEIGHT
		WWF	WELDED WIRE FABRIC

Reviewed for Code Compliance
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Portland, Maine
Date: 08/31/17

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PRELIMINARY
NOT FOR CONSTRUCTION

33 HAMPSHIRE STREET
PORTLAND, MAINE

PROPOSED HOSTEL

ISSUED

No.	DESCRIPTION	DR. BY	CKD. BY	DATE
A	ISSUE FOR PERMIT	SD	KB	6-7-17

SHEET TITLE:
STRUCTURAL NOTES

DESIGNED:	KB
DRAWN:	SD
DATE:	06-07-17
PROJECT NUMBER:	17-089

S0.0