## WHITE ENGINEERING, LLC P.O. Box 878 Glen, N.H. 03838

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Client: S.W. Cole Engineering, Inc. Report: 004

**Project:** 169 Newbury Street **SWCE Project #:** 16-0910 **Date:** November 22, 2016

Subject: Site Inspection of Structural Steel

We visited the site on this date as requested to perform structural steel inspections on the Luminato project located at 169 Newbury St. in Portland, ME. Upon arrival we met with the project superintendent for Landry French. Our actions and observations were as follows:

- Bolted connections on the 2<sup>nd</sup> Floor framing were inspected for proper tightening.
- Anchoring of the deck support angles at the 2<sup>nd</sup> Floor framing was inspected.
- Welding of the HSS bracing between the 1<sup>st</sup> and 2<sup>nd</sup> Floors was visually inspected.
- Layout, welding and fastening of the 1<sup>st</sup> and 2<sup>nd</sup> Floor composite deck were inspected. Work was still being performed on pour stops and closures at each floor at the time of this inspection.
- Layout and welding of the 1<sup>st</sup> and 2<sup>nd</sup> Floor shear studs were inspected.
- Visual inspections were performed on the plated moment connections at the 1<sup>st</sup> Floor framing.
- Visual inspection and ultrasonic testing were performed on the moment connections at the 1<sup>st</sup> and 2<sup>nd</sup> floor that were welded at the time of this visit. Inspections were performed in accordance to AWS D1.1. See the attached report for locations tested.
- The missing weld previously reported on the HSS brace member on Brace A between lines 4 and 5 from grade to the 1<sup>st</sup> Floor was added. The welds were visually inspected.

All inspections performed above appeared acceptable in accordance to AWS, AISC, RCSC and contract documents except as noted below:

1. Seven shear studs were found to have unacceptble welds at the 1<sup>st</sup> Floor framing.

While we were on site we had followed up on discrepancies noted during a site visit by Veitas and Veitas on 11/29/16. Our observations were as follows:

- Item 2-Not addressed at this time.
- Item 3-Welds were increased to larger than the minimum<sup>1</sup>/<sub>4</sub>" and found to be acceptable.
- Item 4-Clip was cut back and re-welded on the left side. Weld was found to be acceptable.
- Item 5-Not addressed at this time.
- Item 6-Clip was cut back and re-welded at the left side and weld size was increased on the right side. Welds were found to be acceptable.
- Item 7-Clip was cut back and re-welded at C/3.8 and weld size was increased at C/4.8. Welds were found to be acceptable.

Item 8-Clip was cut back and re-welded on the left side. Welds were found to be acceptable. Item 9-See report above.

The project superintendent as well as the erector foreman was notified of our findings.

**Inspector;** Michael Bump CWI#07091231

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## **ULTRASONIC TEST REPORT**

Date: December 2, 2016 Report: 004

Client: S.W. Cole Engineering, Inc.

Transducer: 70 degree 2.25 mhz

Project: Newbury Street 16-0910 Ultrasonic Unit: Olympus Epoch 600

Address: **Portland, ME**Test Method Standard: **AWS D1.1** 

Welding contractor: American Aerial Services Acceptance Standard: AWS D1.1 table 6.2

Description of Joint: **Butt** Material: **ASTM A992** 

Weld Identification:	Acceptable	Rejectable	Transducer Angle	From Face	Leg 1-2-3				Defect			Distance		
						<b>W</b> Indication Level	<b>B</b> Reference Level	A Attenuation Level	<b>D</b> Indication	Length	Angular Distance (sound path)	Depth from "A" surface	"X"	"Y"
2 <sup>nd</sup> Floor Moment														
Conn.s														
D/4	X		70	A	1-3		62							
1st Floor Conn.s														
A/5.9	X		70	A	1-3		62							
B/5.9	X													

Remarks: All field welded connections were tested at each location listed above. Top and bottom flanges were tested at each connection listed.

Technician: Michael Bump Level: II