THE FOLLOWING BUILDING CODES AND STANDARDS SHALL BE REFERENCED DURING CONSTRUCTION: EDITION OF THE IBC INTERNATIONAL BUILDING CODE AMERICAN SOCIETY OF CIVIL ENGINEERS, MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES AMERICAN CONCRETE INSTITUTE SPECIFICATION FOR STRUCTURAL CONCRETE AMERICAN INSTITUTE OF STEEL CONSTRUCTION AMERICAN CONCRETE INSTITUTE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE AMERICAN SOCIETY OF TESTING AND MATERIALS NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION BY NATIONAL FOREST PRODUCTS ASSOCIATION, 2005. REFERENCE ARCHITECTURAL PLANS FOR DIMENSIONS NOT SHOWN. REFERENCE MECHANICAL, ELECTRICAL, AND ARCHITECTURAL PLANS FOR SIZES AND LOCATIONS OF WALL AND SLAB OPENINGS, DUCTS, PIPING, CURBS, AND EQUIPMENT PADS. IN THE EVENT OF A CONFLICT BETWEEN THE DRAWINGS, SPECIFICATIONS, OR NOTES ON THE DRAWINGS. THE ENGINEER SHALL BE NOTIFIED PRIOR TO CONSTRUCTION. EXISTING DIMENSIONS AND CONDITIONS ARE FOR REFERENCE ONLY. CONTRACTOR SHALL VERIFY ALL EXISTING CONSTRUCTION AND DIMENSIONS IN THE FIELD PRIOR TO CONSTRUCTION OR FABRICATION. ALL DISCREPANCIES SHALL BE REPORTED TO THE ENGINEER PRIOR TO COMMENCING WORK. THE CONTRACTOR SHALL NOTIFY THE ENGINEER IF DEVIATIONS OR CHANGES ARE REQUIRED TO THE CONTRACT DOCUMENTS OR APPROVED SHOP DRAWINGS DUE TO INTERFERENCES, FABRICATION ERRORS, OR OTHER CAUSES. THE STRUCTURE IS SELF-SUPPORTING AND STABLE AFTER THE ENTIRE BUILDING IS COMPLETELY CONSTRUCTED. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR ERECTION PROCEDURES AND SEQUENCING DURING CONSTRUCTION AND ERECTION TO PROVIDE AND ENSURE LOCAL AND OVERALL STABILITY OF THE BUILDING AND ITS COMPONENTS DURING CONSTRUCTION AND ERECTION. THE CONTRACTOR SHALL RETAIN A LICENSED STRUCTURAL ENGINEER TO DESIGN TEMPORARY BRACING/SHORING AND DETERMINE WHERE THE TEMPORARY BRACING/SHORING **GENERAL NOTES** GROUND SNOW LOAD, Pg = 50 PSFSNOW EXPOSURE FACTOR, Ce = 1.0 SNOW LOAD IMPORTANCE FACTOR, I = 1.0FLAT ROOF SNOW LOAD, Pf = 35 PSF + DRIFT BASIC WIND SPEED = 100 MPH IMPORTANCE FACTOR, lw = 1.0MAIN WINDFORCE-RESISTING SYSTEM (INCLUDES WINDWARD + LEEWARD) = 20 PSF SOIL SITE CLASSIFICATION = DDESIGN SPECTRAL RESPONSE ACCELERATION: RESPONSE MODIFICATION COEFFICIENT.....R = 1.5 OCCUPANCY IMPORTANCE FACTOR...... = 1.0 $...V_r = Cs * W = 0.06 * W$ (W = SEISMIC WEIGHT)

SCALE: NTS

SCALE: NTS

ALL CONCRETE WORK, INCLUDING MATERIAL SELECTION, ADMIXTURES, MIXING, AND PLACEMENT OF CONCRETE SHALL BE IN CONFORMANCE WITH APPLICABLE BUILDING CODES. IN ADDITION, REFERENCE THE FOLLOWING CONCRETE STANDARDS AND SPECIFICATIONS:

ACI 318 AMERICAN CONCRETE INSTITUTE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE ACI 301 AMERICAN CONCRETE INSTITUTE SPECIFICATIONS FOR STRUCTURAL CONCRETE

ACI 305 STANDARD SPECIFICATION FOR HOT WEATHER CONCRETING ACI 306 STANDARD SPECIFICATION FOR COLD WEATHER CONCRETING

TEACHER GOLONE I TANNIMETERS AND TOLLOWS.											
	LOCATION	MAX W/C RATIO	ťc	AIR-ENTRAINMENT							
	INT. CONC./WALLS/SLABS	.52	3,000 PSI	2% ± 1½%							
	FOUNDATIONS, FOOTINGS, & FOUNDATION WALLS	.52	3,000 PSI	5–7%							
	INT. SLAB-ON-GRADE	.47	4,000 PSI	2% ± 1½%							
	EXT. SLAB-ON-GRADE	.45	4,000 PSI	6% ± 1½%							

WHERE: W/C = WATER TO CEMENT RATIO ANDf'c = COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS

MAXIMUM AGGREGATE SIZE SHALL BE $\frac{3}{4}$ ", IN CONFORMANCE WITH ASTM C33. USE PORTLAND CEMENT TYPE II, IN CONFORMANCE WITH ASTM 150. AIR ENTRAINING ADMIXTURES SHALL CONFORM TO ASTM C 260. ADMIXTURES SHALL CONFORM TO "SPECIFICATION FOR CHEMICAL ADMIXTURES FOR CONCRETE" ASTM C 494. FLY ASH USED AS ADMIXTURES SHALL CONFORM TO ASTM C 618. CALCIUM CHLORIDE OR ADMIXTURES CONTAINING CALCIUM CHLORIDE IS NOT PERMITTED.

MAXIMUM SLUMP AFTER THE ADDITION OF A WATER-REDUCING ADMIXTURE IS 8 INCHES.

CONCRETE EXPOSED TO FREEZING AND THAWING, INCLUDING FOUNDATIONS, FOOTINGS, FOUNDATION WALLS, AND EXTERIOR WALKWAYS SHALL BE AIR ENTRAINED WITH AIR CONTENT BETWEEN 5% AND 6%. CONTRACTOR SHALL NOT PLACE CONCRETE ON FROZEN GROUND OR IN WATER. ADEQUATE EQUIPMENT SHALL BE PROVIDED FOR HEATING CONCRETE MATERIALS AND PROTECTING CONCRETE DURING NEAR-FREEZING OR FREEZING WEATHER. REFERENCE ACI 306, AS NOTED ABOVE, FOR RECOMMENDATIONS FOR COLD WEATHER CONCRETING.

CONTRACTOR SHALL SUBMIT PROPOSED CONCRETE MIX DESIGN AND LABORATORY TESTS OF FABRICATED CYLINDERS VERIFYING CONCRETE STRENGTH OR PERFORMANCE HISTORY OF MIX TO ENGINEER FOR ACCEPTANCE PRIOR TO PLACEMENT OF CONCRETE. CONCRETE USED ON SITE SHALL BE FIELD TESTED IN ACCORDANCE WITH AND IN THE PRESENCE OF AN APPROVED TESTING AGENCY. FIELD TESTING INFORMATION SHALL INDICATE SLUMP, AIR CONTENT, AND TEMPERATURE. COMPRESSION TEST 1 CYLINDER AT 7 DAYS AND 2 AT 28 DAYS. HOLD AN ADDITIONAL CYLINDER FOR A 56 DAY BREAK, IF NECESSARY. PROVIDE A SET OF 4 CYLINDERS FOR EACH PLACEMENT AND PER 50 CUBIC YARDS OF CONCRETE PLACED. THE

CONSTRUCTION JOINTS IN WALLS SHALL BE PERMITTED AS DETAILED ON THE STRUCTURAL DRAWINGS. SURFACES OF CONCRETE CONSTRUCTION JOINTS SHALL BE CLEANED AND LAITANCE REMOVED. IMMEDIATELY BEFORE NEW CONCRETE IS PLACED, CONSTRUCTION JOINTS SHALL BE WETTED AND STANDING WATER

WHERE ELECTRICAL CONDUIT/ RADIANT HEATING TUBES RUN IN THE SLAB, THEY SHALL BE LOCATED AT MID-DEPTH OF THE SLAB. ALUMINUM CONDUIT AND SLEEVES ARE NOT PERMITTED.

REMOVED. VERTICAL CONSTRUCTION JOINTS IN WALLS SHALL NOT EXCEED A SPACING OF 40 FEET.

ANCHOR BOLTS SHALL CONFORM TO ASTM F1554. ANCHOR BOLTS SHALL HAVE HEAVY HEX NUTS AND LOCK WASHERS.

CONCRETE NOTES

USE DEFORMED BILLET-STEEL REINFORCING BARS, GRADE 60, IN CONFORMANCE WITH ASTM A615. REINFORCEMENT SHALL BE ACCURATELY PLACED AND SUPPORTED PRIOR TO CONCRETE PLACEMENT. AND SHALL BE SECURED AGAINST DISPLACEMENT.

THE CONTRACTOR SHALL SUBMIT REINFORCING SHOP DRAWINGS TO THE ENGINEER FOR REVIEW AND ACCEPTANCE PRIOR TO COMMENCING FABRICATION. REINFORCEMENT SHALL BE DETAILED IN ACCORDANCE WITH ACI "MANUAL OF STANDARD PRACTICE FOR DETAILING OF REINFORCED CONCRETE STRUCTURES". SHOP DRAWINGS SHALL SHOW REINFORCING STEEL PLACEMENT DETAILS AND SECTIONS.

MINIMUM CONCRETE COVER FOR REINFORCEMENT		
CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	3 INCHES	
CONCRETE EXPOSED TO EARTH OR WEATHER	2 INCHES	
CONCRETE NOT EXPOSED TO EARTH OR WEATHER IN SLABS AND WALLS (FOR PRIMARY REINFORCEMENT, TIES, AND STIRRUPS)	1½ INCHES	
CONCRETE NOT EXPOSED TO EARTH OF WEATHER IN COLUMNS AND BEAMS	1½ INCHES	
	1,72 11101120	

CONTINUOUS REINFORCEMENT SHALL BE TENSION LAP SPLICED PER LAP SPLICE LENGTH TABLE, U.N.O..

LAP SPL	LAP SPLICE LENGTH TABLE									
BAR SIZE	#3	#4	# 5	#6	# 7	#8	#9			
MIN LAP SPLICE (INCHES)	18	24	30	36	48	64	81			

REINFORCEMENT HOOKS SHALL CONFORM TO STANDARD HOOKS ACCORDING TO ACI 318. WELDING OF REINFORCEMENT IS NOT PERMITTED, U.N.O.

CONCRETE REINFORCING NOTES

SUBGRADE PREPARATION AND DETERMINATION (INCLUDING ALLOWABLE BEARING PRESSURE, STRUCTURAL FILL GRADATION REQUIREMENTS. COMPACTION REQUIREMENTS AND POST-CONSTRUCTION SETTLEMENT ANALYSIS) BENEATH FOOTINGS AND SLABS-ON-GRADE AND BEHIND FOUNDATION WALLS SHALL BE PROVIDED BY A GEOTECHNICAL ENGINEER. ALL FILL USED TO SUPPORT FOUNDATIONS AND SLABS-ON-GRADE SHALL CONSIST OF A WELL-GRADED, GRANULAR MATERIAL PER THE RECOMMENDATIONS OF THE GEOTECHNICAL ENGINEER. STRUCTURAL SLABS SHALL BE CONSTRUCTED ON A MINIMUM 12" THICK LAYER OF STRUCTURAL FILL SOIL WITH PROPERTIES PER THE GEOTECHNICAL ENGINEER.

PRESUMED ALLOWABLE SOIL BEARING PRESSURE USED IN DESIGN = 2,000 PSF. BEARING CAPACITIES SHALL BE VERIFIED BY GEOTECHNICAL ENGINEER. MINIMUM FROST DEPTH COVER = 4'-0" FOR EXTERIOR FOOTINGS BELOW FINAL EXTERIOR GRADE. CONTRACTOR SHALL NOTIFY ENGINEER OF ANY DISCREPANCIES.

FOUNDATIONS SHALL BEAR ON UNDISTURBED NATIVE SOIL, UNLESS NOTED OTHERWISE. BEARING ELEVATIONS SHALL BE LOWERED WHERE SUITABLE SOILS ARE NOT ENCOUNTERED. WHERE OVEREXCAVATION HAS OCCURRED, CONTRACTOR MAY PLACE LEAN CONCRETE ON TOP OF NATIVE SOIL. THE CONTRACTOR SHALL NOTIFY THE GEOTECHNICAL AND STRUCTURAL ENGINEER IF ANY UNSUITABLE SOILS ARE ENCOUNTERED PRIOR TO PLACING

FOUNDATION WALLS SHALL BE BACKFILLED SIMULTANEOUSLY ON BOTH SIDES OF THE WALL. FOUNDATION WALLS AND SLAB-ON-GRADES SHALL REACH THEIR FULL 28 DAY COMPRESSIVE STRENGTH PRIOR TO BACKFILLING. THE CONTRACTOR SHALL PROVIDE TEMPORARY SHORING/BRACING FOR WALLS WHEN BACKFILL IS PLACED PRIOR TO CONCRETE ACHIEVING ITS FULL 28 DAY STRENGTH. BACKFILL FOR FOUNDATION WALLS IS BASED ON DRAINED CONDITIONS. SEE ARCHITECTURAL, CIVIL, AND MECHANICAL DRAWINGS FOR FOUNDATION DRAINAGE SYSTEM.

PROTECT FOUNDATIONS FROM FROST AND KEEP BOTTOM OF TRENCH DRY DURING CONSTRUCTION. IF GROUNDWATER IS ENCOUNTERED NEAR OR ABOVE THE BASE OF THE FOOTINGS, EXCAVATIONS SHALL BE DEWATERED DURING CONSTRUCTION. SURFACE WATER SHALL BE DIVERTED AWAY FROM EXCAVATIONS.

CONTRACTOR SHALL BE RESPONSIBLE FOR THE SHORING AND BRACING OF EXISTING STRUCTURES DURING EXCAVATION, BACKFILLING, AND CONSTRUCTION. CONTRACTOR SHALL SLOPE EXCAVATIONS TO ACHIEVE SOIL

FOUNDATION NOTES SCALE: NTS

MASONRY CONSTRUCTION AND MATERIALS SHALL CONFORM TO ALL REQUIREMENTS OF BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (ACI 530/ASCE5/TMS 402) AND SPECIFICATION FOR MASONRY STRUCTURES (ACI 530.1/ASCE6/TMS 602).

MASONRY: A. CONCRETE UNITS

1. UNITS: ASTM C90 GRADE N, TYPE 1 WITH MINIMUM COMPRESSIVE STRENGTH = 1,900 PSI

2. MORTAR: ASTM C270, TYPE S 3. ASSEMBLY/PRISM MINIMUM COMPRESSIVE STRENGTH I'm = 1,500 PSI

4. ALL UNITS SHALL BE LAID IN RUNNING BOND, UNLESS NOTED OTHERWISE

5. PLACE MASONRY WHILE MORTAR IS SOFT AND PLASTIC 6. CONCRETE MASONRY UNITS (CMU) SHALL BE STANDARD UNITS WITH NOMINAL FACE DIMENSIONS

LENGTH = 16 INCHES. HEIGHT = 8 INCHES. WIDTH AS INDICATED

B. VERTICAL REINFORCEMENT

1. ASTM A615, GRADE 60

2. UNLESS NOTED OTHERWISE, PROVIDE #5 BAR @ 32" o.c. 3. PROVIDE ADDITIONAL VERTICAL REINFORCEMENT AT EACH SIDE OF CONTROL JOINTS, AT

INTERSECTION OF EXTERIOR WALLS, AND AT EACH SIDE OF ALL MASONRY OPENINGS.

4. VERTICAL REINFORCEMENT SHALL BE CONTINUOUS FOR FULL HEIGHT OF WALL

C. GROUT

1. ASTM C476 WITH MINIMUM COMPRESSIVE STRENGTH = 2000 PSI

2. FULLY GROUT ALL CELLS WITH REINFORCEMENT AND ANCHORS 3. CONSOLIDATE AND RECONSOLIDATE GROUT ASSEMBLED WALLS WITH VIBRATOR

HORIZONTAL JOINT REINFORCEMENT

MASONRY NOTES

SCALE: NTS

1. ASTM 951, HOT DIPPED GALVANIZED OR STAINLESS STEEL

2. 2xW2.1 (8 GA) LADDER REINFORCEMENT SPACED AT 16" o.c. VERTICAL

3. BEGIN HORIZONTAL REINFORCEMENT AT TOP OF FIRST COURSE AND LAP 6 INCHES, MIN. 4. MINIMUM COVER: 1/2 INCHES WHEN EXPOSED TO EARTH OR WEATHER, 1/2 INCH OTHERWISE

SECURE ALL CMU WALL SUPPORTED FIXTURES, EQUIPMENT, ETC. TO CMU WALL PER STRUCTURAL DRAWINGS AND

MANUFACTURER'S RECOMMENDATIONS. DO NOT USE EXPANSION ANCHORS.

ALL STRUCTURAL STEEL WORK SHALL CONFORM TO:

AMERICAN INSTITUTE OF STEEL CONSTRUCTION, MANUAL OF STEEL CONSTRUCTION, NINTH EDITION CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES

STRUCTURAL STEEL MEMBERS SHALL BE IN CONFORMANCE WITH THE FOLLOWING: ALL STEEL, U.N.O. ASTM A992, GRADE 50

ASTM A36, Fy=36 KSI ANGLES, PLATES STRUCTURAL TUBING ASTM A500, GRADE B, Fy=46 KSI STEEL PIPE ASTM A53, TYPE E OR S, GRADE B, Fy=35 KSI

SHOP DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW PRIOR TO COMMENCING FABRICATION.

SHOP DRAWINGS SUBMITTALS SHALL INCLUDE:

1. CERTIFIED MILL TEST REPORTS OF STRUCTURAL STEEL (INCLUDING NAMES AND LOCATIONS OF MILLS

2. CERTIFIED MILL TEST REPORTS OF BOLTS, NUTS AND WASHERS (INCLUDING NAMES AND LOCATIONS OF MILLS

AND SHOPS). 3. STRUCTURAL STEEL FABRICATION AND ERECTION DRAWINGS WHICH INCLUDE BOLTED CONNECTIONS (SHOP AND

FIELD) AND WELDED CONNECTIONS (SHOP AND FIELD) DEPICTING AWS WELDING SYMBOLS. 4. METAL DECK SHOP DRAWINGS DEPICTING SHEAR STUD LAYOUT ON BEAMS AND GIRDERS.

OWNER SHALL RETAIN A QUALIFIED TESTING AGENCY TO PERFORM AND VERIFY THE FOLLOWING:

1. VISUAL INSPECTION OF ALL WELDS. 2. ULTRASONIC TESTING, IN ACCORDANCE WITH ASTM E-164, ON 100% OF ALL FIELD FULL PENETRATION WELDS.

3. PROVIDE RANDOM VERIFICATION VIA ULTRASONIC TESTING OF SHOP FULL PENETRATION WELDS.

4. FIELD BOLTED CONNECTIONS, INCLUDING VERIFICATION OF BOLT GRADES.

5. SHEAR STUD QUANTITY, PROPER INSTALLATION, SIZE, AND SPACING. SHEAR STUDS SHALL CONFORM TO AWS D1.1.

BOLTED CONNECTIONS:

SCALE: NTS

SCALE: NTS

1. FIELD CONNECTIONS SHALL UTILIZE MINIMUM 34" DIAMETER A325 HIGH STRENGTH BOLTS, U.N.O.. BOLTED CONNECTION SHALL BE SLIP CRITICAL (SC) AT ALL MOMENT FRAMES, BRACED FRAMES, AND AT ADDITIONAL LOCATIONS INDICATED IN THE DRAWINGS. SLIP CRITICAL CONNECTIONS SHALL UTILIZE LOAD INDICATOR WASHERS OR TENSION CONTROL BOLTS. BOLT HOLES SHALL BE STANDARD SIZE, U.N.O..

JOINTS USING ASTM A325 BOLTS. 3. ANCHOR BOLTS SHALL CONFORM TO ASTM F1554, GRADE 36, STANDARD HEX HEAD FURNISHED WITH HEAVY

2. HIGH STRENGTH BOLTS SHALL BE INSTALLED AND TIGHTENED PER AISC SPECIFICATION FOR STRUCTURAL

HEX NUTS AND LOCK WASHERS. 4. CONTRACTOR SHALL DESIGN CONNECTIONS NOT ALREADY DETAILED ON STRUCTURAL DRAWINGS. DESIGN SHALL BE STAMPED BY A LICENSED STRUCTURAL ENGINEER AND SUBMITTED PRIOR TO COMMENCING

FABRICATION.

1. WELDING SHALL CONFORM TO AWS D1.1. USE LOW-HYDROGEN SMAW ELECTRODES WITH MINIMUM TENSILE STRENGTH OF 70 KSI.

STRUCTURAL STEEL SHALL RECEIVE THE FOLLOWING PROTECTIVE COATINGS:

1. DO NOT PAINT SURFACES TO RECEIVE METAL DECK AND/ OR SHEAR CONNECTORS FASTENED BY WELDING, CONTACT SURFACES OF HIGH STRENGTH BOLTED CONNECTIONS, FINISHED BEARING SURFACES, AND SURFACES TO BE WELDED IN THE FIELD. IF REQUIRED, PROTECT THESE SURFACES BY RUST-INHIBITING COATING THAT CAN BE REMOVED EASILY PRIOR TO ERECTION.

2. UNEXPOSED STRUCTURAL STEEL SHALL BE CLEANED IN ACCORDANCE WITH SSPC-SP3 AND PAINTED WITH PRIMER PAINT, TNEMEC 10-99, OR EQUIVALENT, U.N.O..

3. EXPOSED STRUCTURAL STEEL TO RECEIVE ZINC-RICH EPOXY PAINT SHALL BE FIRST CLEANED IN ACCORDANCE WITH SSPC-SP6 , COMMERCIAL BLAST CLEANING. USE TNEMEC ZIN-RICH EPOXY PAINT, OR

EQUIVALENT. APPLY FINISH COAT PER ARCHITECT. 4. EXPOSED STRUCTURAL STEEL TO BE HOT-DIPPED GALVANIZED SHALL BE IN ACCORDANCE WITH ASTM A123.

SHEAR CONNECTOR STUDS:

1. SHEAR CONNECTOR STUDS SHALL BE NELSON, OR EQUIVALENT, 3/4" DIAMETER, U.N.O.. WELD STUDS PER STUD MANUFACTURER'S RECOMMENDATIONS THROUGH METAL DECKING. STUD LENGTH SHALL BE 1" BELOW TOP OF CONCRETE SLAB-ON-DECK.

2. SHEAR STUDS, WHERE REQUIRED, ARE INDICATED ON THE DRAWINGS AS [XX], WHERE XX IS THE NUMBER OF STUDS EQUALLY SPACED BETWEEN SUPPORTS ON A BEAM OR GIRDER.

STRUCTURAL STEEL NOTES

LEGEND

SCALE: NTS

ANCHOR BOLT

BOTTOM OF FOOTING

STRUCTURAL STEEL CHANNEL

CAST-IN-PLACE CONCRETE

CONCRETE MASONRY UNIT

COMPLETE PENETRATION WELD

CONSTRUCTION JOINT

ADDITIONAL

ARCHITECT

BUILDING

BEAM

BOTTOM

BEARING

BETWEEN

CANTILEVER

CENTERLINE

CLEAR

COLUMN

CONCRETE

CONNECTION

CONTINUOUS

CONTRACTOR

CUBIC YARD

DIAMETER

DIMENSION

DRAWING

EXISTING

EACH FACE

ELEVATION

EQUIPMENT

EACH SIDE

EACH WAY

EXPANSION

FOUNDATION

FINISH FLOOR

FIELD VERIFY

GALVANIZED

HORIZONTAL

INSIDE FACE

INFORMATION

HEIGHT

JOINT

HOLLOW STRUCTURAL SHAPE

KIP (1 KIP = 1.000 LBS)KIPS PER SQUARE INCH

FLANGE

FLOOR

FOOTING

FOOT

FOOTING DESIGNATION

EXTERIOR

EQUAL

DISCONTINUOUS

CONTROL JOINT

ADDL

CIP

CLR

CMU

CONC

CONN

CONT

CONTR

DIA

DISCONT

EL, ELEV

EQUIP

FTG

GALV

INFO

HOR, HORIZ

DWG

B/FTG, BOF

DOUBLE ANGLE

LINEAR FOOT

MAXIMUM

MINIMUM

MECHANICAL

NEAR FACE

NEAR SIDE

ON CENTER

OPENING

OPPOSITE

OUTSIDE FACE

PIER DESIGNATION

PREFABRICATED

REINFORCING STEEL

REQUIRED

SECTION

SIMILAR

SPACING

STANDARD

STIFFENER

STRAIGHT

STRUCTURAL

TOP AND BOTTOM

TOP OF CONCRETE

TOP OF FOOTING

TEMPERATURE

TOP OF SHELF

TOP OF SLAB

TOP OF STEEL

TOP OF WALL

TYPICAL

VERTICAL VERIFY IN FIELD

WITHOUT

WEIGHT

WORK POINT

WELDED WIRE FABRIC

STRUCTURAL TUBING

UNLESS NOTED OTHERWISE

STRUCTURAL STEEL WIDE FLANGE

STEEL

ROOF DRAIN

SLIP CRITICAL

SHEATHING

SLAB-ON-GRADE

SPECIFICATIONS

STAINLESS STEEL

PARTIAL PENETRATION WELD

POUNDS PER SQUARE FOOT

POUNDS PER SQUARE INCH

NOT TO SCALE

NUMBER

MANUFACTURER

MISCELLANEOUS

LONG LEG HORIZONTAL

LONG LEG VERTICAL

POUND

LLV

MAX

MECH

MFR

MISC

NTS

OPNG

PREFAB

PSI

REINF

SECT

SOG

SPAC

STD

STIFF

STL

STR

T&B

STRUCT

TOC, T/CONC

T/FTG, TOF

T/SHELF

T/SLAB

T/STL

T/WALL

UNO

VER, VERT

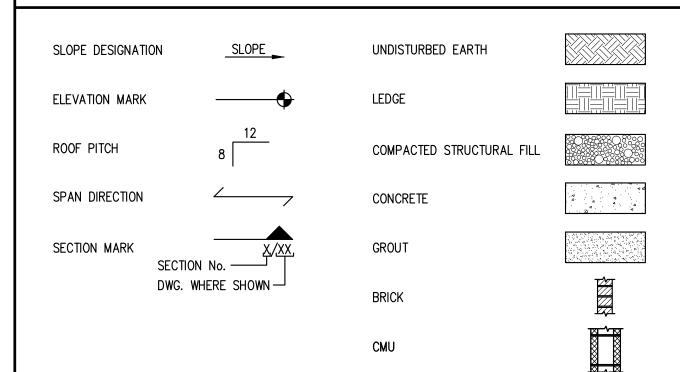
SPECS

SHEATH

REQ, REQD

OPP

ABBREVIATIONS SCALE: NTS

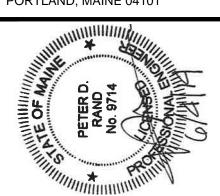


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SHEET TITLE:

SCALE: NTS-

STRUCTURAL **NOTES**

DESIGNED: PDR DRAWN: PDR 5-21-14 PROJECT NUMBER: 14-030