GENERAL NOTES

1. THE NOTES ON THESE DRAWINGS ARE NOT INTENDED TO REPLACE SPECIFICATIONS. SEE SPECIFICATIONS FOR REQUIREMENTS IN ADDITION TO GENERAL NOTES. INCONSISTENCIES BETWEEN THESE DRAWINGS AND THE SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT PRIOR TO PROCEEDING WITH THE AFFECTED PORTION OF THE WORK.

2. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH JOB SPECIFICATIONS AND ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND SITE DRAWINGS. CONSULT THESE DRAWINGS FOR LOCATIONS AND DIMENSIONS OF OPENINGS, CHASES, INSERTS, REGLETS, SLEEVES, DEPRESSIONS, AND OTHER DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS.

3. ALL DIMENSIONS, EXISTING CONDITIONS, AND AS-BUILT CONDITIONS MUST BE VERIFIED IN THE FIELD. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK.

4. THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE ONLY AFTER THE STRUCTURAL WORK CONTAINED IN THE S- DRAWINGS IS COMPLETED. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO ENSURE THE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS OR TIEDOWNS. SUCH MATERIAL SHALL REMAIN THE PROPERTY OF THE CONTRACTOR AFTER COMPLETION OF THE PROJECT.

5. SECTIONS AND DETAILS SHOWN ON ANY STRUCTURAL DRAWINGS SHALL BE CONSIDERED TYPICAL FOR SIMILAR CONDITIONS AS DETERMINED BY THE STRUCTURAL ENGINEER. THE STRUCTURAL ENGINEER RESERVES THE RIGHT TO INTERPRET DETAILS TO ADDRESS OTHER PROJECT CONDITIONS.

6. THE CONTRACTOR SHALL SUBMIT COMPLETE SHOP DRAWINGS FOR ALL PARTS OF THE WORK, INCLUDING DESCRIPTION OF SHORING, AND CONSTRUCTION METHODS AND SEQUENCING WHERE APPLICABLE. NO PERFORMANCE OF THE WORK INCLUDING, BUT NOT LIMITED TO, DEMOLITION OF EXISTING STRUCTURE, OR FABRICATION OR ERECTION OF NEW STRUCTURAL ELEMENTS, SHALL COMMENCE WITHOUT REVIEW OF THE SHOP DRAWINGS BY THE ARCHITECT AND ENGINEER. SUBMIT ONE COPY AND ONE SEPIA. COPY WILL BE REVIEWED AND SEPIA WILL BE RETURNED. FOR SHOP DRAWINGS AND SUBMITTALS REQUIRED, REFERENCE THE PROJECT SPECIFICATIONS.

7. ALL APPLICABLE FEDERAL, STATE, AND MUNICIPAL REGULATIONS SHALL BE FOLLOWED, INCLUDING THE FEDERAL DEPARTMENT OF LABOR OCCUPATIONAL SAFETY AND HEALTH ACT.

8. IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE (2003 EDITION, SECTION 1704.1), A STATEMENT OF SPECIAL INSPECTIONS IS REQUIRED AS A CONDITION FOR PERMIT ISSUANCE BY THE LOCAL CODE OFFICIAL. THIS STATEMENT SHALL INCLUDE A COMPLETE LIST OF MATERIALS AND WORK REQUIRING SPECIAL INSPECTIONS, THE INSPECTIONS TO BE PERFORMED AND A LIST OF THE INDIVIDUALS, APPROVED AGENCIES AND FIRMS INTENDED TO BE RETAINED FOR CONDUCTING SUCH INSPECTIONS.

9. REFERENCE THE PROJECT SPECIFICATIONS FOR ALL TESTING REQUIREMENTS.

DESIGN LOADS

1. BUILDING CODE:
   INTERNATIONAL BUILDING CODE, 2003 EDITION (SEISMIC DESIGN PER 2006 EDITION)
   ASCE 7-02 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES. (ASCE 7-05 FOR SEISMIC DESIGN)

2. DESIGN FLOOR LIVE LOADS:
   OFFICES: 50 PSF + 20 PSF PARTITION ALLOWANCE
   CORRIDORS ABOVE FIRST FLOOR: 80 PSF
   STAIRS: 100 PSF
   MEETING ROOMS: 100 PSF
   STORAGE: 125 PSF

3. DESIGN ROOF SNOW LOAD:
   GROUND SNOW LOAD (Pg): 60 PSF
   SNOW EXPOSURE FACTOR (Ce): 1.0
   SNOW LOAD IMPORTANCE FACTOR (Is): 1.0
   SNOW LOAD THERMAL FACTOR (Ct): 1.1
   FLAT ROOF SNOW LOAD (PF): 46.2 PSF + DRIFT

5. DESIGN SEISMIC LOADS (FOR SELF SUPPORTING MEZZANINES):
   EQUIVALENT LATERAL FORCE PROCEDURE
   SEISMIC USE GROUP: I
   SEISMIC IMPORTANCE FACTOR (Ie):1.0
   MAPPED SPECTRAL RESPONSE ACCELERATIONS:
   Ss: 0.370
   S1: 0.100
   SEISMIC SITE CLASS: D
   SPECTRAL RESPONSE COEFFICIENTS:
   Sds: 0.371
   Sd1: 0.160
   SEISMIC DESIGN CATEGORY: C
   BASIC STRUCTURAL SYSTEM: BUILDING FRAME SYSTEM
   MOMENT RESISTING FRAME SYSTEM
   BASIC SEISMIC FORCE RESISTING SYSTEM:
   ORDINARY MOMENT STEEL MOMENT FRAMES
   RESPONSE MODIFICATION FACTOR (R):, X: 3.0
   Y: 3.0
   SEISMIC RESPONSE COEFFICIENT (Cs), X: 0.124
   Y: 0.124

FOUNDATION NOTES (SOIL SUPPORTED)

1. FOUNDATION DESIGN IS BASED ON SHALLOW SPREAD FOOTINGS BEARING ON SUITABLE UNDISTURBED NATIVE SOILS AND/OR NEW COMPACTED STRUCTURAL FILL EXTENDING TO UNDISTURBED NATIVE SOIL.

2. PRESUMPTIVE BEARING CAPACITY 3,000 PSF.

3. EXTEND BOTTOM OF EXTERIOR FOOTINGS AT LEAST 4.5 FEET BELOW THE FINAL EXTERIOR GRADE FOR PROTECTION AGAINST FROST.

4. NO FILL FOR BUILDING SUPPORT SHALL BE PLACED UNTIL SUBGRADES HAVE BEEN OBSERVED AND APPROVED BY THE GEOTECHNICAL ENGINEER.

5. SOILS EXPOSED AT THE BASE OF ALL SATISFACTORY FOUNDATION EXCAVATIONS SHOULD BE PROTECTED AGAINST ANY DETRIMENTAL CHANGE IN CONDITION, SUCH AS DISTURBANCE FROM RAIN OR FROST. SURFACE RUNOFF SHOULD BE DRAINED AWAY FROM THE EXCAVATIONS AND NOT BE ALLOWED TO POND. FOUNDATION EXCAVATIONS AND SHOULD BE ADEQUATELY PROTECTED FROM RAINFALL OR FREEZING CONDITIONS. GROUNDWATER SHOULD BE ANTICIPATED FOR EXCAVATIONS AND APPROPRIATE DEWATERING MEASURES SHALL BE EMPLOYED.

6. EXCAVATIONS FOR BUILDING CONSTRUCTION SHALL BE IN ACCORDANCE WITH OSHA REQUIREMENTS. BRACED EXCAVATIONS SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF MAINE. DO NOT UNDERMINE EXISTING FOUNDATIONS OF ANY ADJACENT STRUCTURES. REFER TO THE GEOTECHNICAL REPORT FOR ADDITIONAL AND/OR MORE SPECIFIC REQUIREMENTS.

CONCRETE NOTES

1. CONCRETE WORK SHALL CONFORM TO "ACI MANUAL OF CONCRETE PRACTICE", LATEST EDITION. THIS PUBLICATION IS AVAILABLE THROUGH THE AMERICAN CONCRETE INSTITUTE (248) 848-3800.

2. ALL CONCRETE SHALL HAVE A 28-DAY COMPRESSIVE STRENGTH OF 3,000 PSI, U.N.O. EXTERIOR SLAB-ON-GRADE SHALL HAVE A 28-DAY COMPRESSIVE STRENGTH OF 4,500 PSI. ADDITIONAL CONCRETE MIX PERFORMANCE DATA INCLUDING AIR CONTENT, WATER-CEMENT RATIO, AIR CONTENT, AGGREGATE SIZE, SLUMP, ETC. HAS BEEN INCLUDED IN THE PROJECT SPECIFICATIONS. SEE THE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.

3. CONCRETE SHALL NOT BE PLACED IN WATER OR ON FROZEN GROUND.

4. PROVIDE PVC SLEEVES WHERE PIPES PASS THROUGH EXTERIOR CONCRETE, OR SLABS.

5. REINFORCING BARS SHALL CONFORM TO ASTM A615 GRADE 60 DEFORMED BARS AND SHALL BE DETAILED, FABRICATED AND PLACED IN ACCORDANCE WITH ACI 315, LATEST EDITION.

6. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185 AND BE PROVIDED IN FLAT SHEETS.

7. FIBER REINFORCEMENT SHALL BE TYPE III SYNTHETIC VIRGIN HOMOPOLYMER POLYPROPYLENE FIBERS CONFORMING TO ASTM C1116.

8. MINIMUM CONCRETE PROTECTIVE COVERING FOR REINFORCEMENT, UNLESS NOTED OTHERWISE, SHALL BE AS FOLLOWS:

   A) SURFACES CAST AGAINST AND PERMANENTLY IN CONTACT WITH EARTH, 3.0"
   B) FORMED SURFACES IN CONTACT WITH EARTH OR EXPOSED TO WEATHER
   #5 BARS, 5/8" DIAMETER WIRE, AND SMALLER, 1.5"
   #6 THROUGH #11 BARS, 2.0"
   C) SURFACES NOT IN CONTACT WITH EARTH OR EXPOSED TO WEATHER
   WALLS, SLABS, JOISTS #11 BARS AND SMALLER, 1.0"
   BEAMS, GIRDERS, AND COLUMNS; ALL REINFORCEMENT, 1.5"

9. REINFORCEMENT SHALL BE CONTINUOUS AROUND CORNERS AND AT INTERSECTIONS. PROVIDE LAPPED BARS AT NECESSARY SPLICES OR HOOKED BARS AT DISCONTINUOUS ENDS. PROVIDE TENSION LAP SPLICES PER THE SCHEDULE THIS DRAWING, FOR ALL REINFORCING UNLESS OTHERWISE SHOWN ON PLAN.

10. WELDING OF REINFORCEMENT IS NOT PERMITTED.

11. FOR ALL OPENINGS IN CONCRETE WALLS AND SLABS, PROVIDE SUPPLEMENTAL REINFORCING AROUND OPENING AS SHOWN ON THE CONTRACT DOCUMENTS TYPICAL DETAILS. NO PENETRATIONS SHALL BE MADE THROUGH FOOTINGS WITHOUT WRITTEN PERMISSION FROM ENGINEER.

12. CONSTRUCTION JOINTS SHOWN ON DRAWINGS ARE MANDATORY. OMISSIONS, ADDITIONS, OR CHANGES SHALL NOT BE MADE EXCEPT WITH THE SUBMITTAL OF A WRITTEN REQUEST TOGETHER WITH DRAWINGS OF THE PROPOSED JOINT LOCATIONS FOR APPROVAL OF THE STRUCTURAL ENGINEER. WHERE CONSTRUCTION JOINTS ARE NOT SHOWN, OR WHEN ALTERNATE LOCATIONS ARE PROPOSED, DRAWINGS SHOWING LOCATION OF CONSTRUCTION AND CONTROL JOINTS AND CONCRETE PLACING SEQUENCE SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW PRIOR TO PREPARATION OF THE REINFORCEMENT SHOP DRAWINGS. CONCRETE SHALL BE PLACED WITHOUT HORIZONTAL CONSTRUCTION JOINTS EXCEPT WHERE SHOWN OR NOTED. VERTICAL CONSTRUCTION JOINTS AND STOPS IN CONCRETE BEAMS/GRADE BEAMS SHALL BE MADE AT MIDSPAN OR AT POINTS OF MINIMUM SHEAR, UNLESS NOTED OTHERWISE.

13. SPACING OF CONSTRUCTION JOINTS, UNLESS NOTED OTHERWISE SHALL BE AS FOLLOWS:
   A) SLABS ON GRADE SEE FOUNDATION PLAN

** EXCEED ONLY WHERE INTERMEDIATE CONTRACTION JOINTS ARE PROVIDED. MINIMUM OF 72 HOURS SHALL ELAPSE BETWEEN ADJACENT CONCRETE PLACEMENTS.

14. ANCHOR RODS SHALL BE HEADED RODS CONFORMING TO ASTM F1554, GRADE 36 KSI WELDABLE STEEL, UNLESS NOTED OTHERWISE ON DRAWINGS.

15. ALL GROUT BENEATH BASE PLATES & BEARING PLATES SHALL BE "5-STAR" 5000-PSI NON-SHRINK GROUT BY U.S. GROUT CORP.

16. SLAB THICKNESSES INDICATED ON THE DRAWINGS ARE MINIMUMS. PROVIDE SUFFICIENT CONCRETE TO ACCOUNT FOR STRUCTURE DEFLECTION, SUBGRADE FLUCTUATIONS, AND TO OBTAIN THE SPECIFIED SLAB ELEVATION AT THE FLATNESS AND LEVELNESS INDICATED.

17. INSTALLATION OF REINFORCEMENT SHALL BE COMPLETED AT LEAST 24 HOURS PRIOR TO THE SCHEDULED CONCRETE PLACEMENT. NOTIFY ARCHITECT AND STRUCTURAL ENGINEER OF COMPLETION AT LEAST 24 HOURS PRIOR TO THE SCHEDULED COMPLETION OF THE INSTALLATION OF REINFORCEMENT.

18. ALL ITEMS TO BE EMBEDDED INTO CONCRETE SHALL BE INSTALLED PRIOR TO PLACEMENT OF CONCRETE. PROVIDE ADDITIONAL REINFORCEMENT AND/OR TEMPLATES AS REQUIRED TO ENSURE THE CORRECT POSITION OF EMBEDMENTS. "WET SETTING" OF EMBEDMENTS INTO CONCRETE IS PROHIBITED.

STRUCTURAL STEEL NOTES

1. STRUCTURAL STEEL FABRICATION, ERECTION, AND CONNECTION DESIGN SHALL CONFORM TO AISC "SPECIFICATION FOR THE DESIGN FABRICATION, AND ERECTION OF STRUCTURAL STEEL" 9TH EDITION, AND THE "CODE OF STANDARD PRACTICE, LATEST EDITION.

2. STRUCTURAL STEEL: STEEL PLATES, SHAPES, AND BARS, CONFORM TO ASTM A36 UNLESS NOTED OTHERWISE (U.N.O.). STRUCTURAL STEEL SHAPES DESIGNATED ON THE DRAWINGS FOR WIDE-FLANGE SECTIONS: ASTM A992 (ASTM A572 GRADE 50 WITH SPECIAL REQUIREMENTS PER AISC TECHNICAL BULLETIN #3 DATED MARCH, 1997)

3. STRUCTURAL TUBING: CONFORM TO ASTM A500 GRADE B 46 KSI.

4. FIELD CONNECTIONS SHALL BE BOLTED USING ASTM A325N HIGH STRENGTH BOLTS (U.N.O.) EXCEPT WHERE SLIP CRITICAL CONNECTIONS ARE REQUIRED AND NOTED BY A325 (SC) ON THE DRAWINGS. PROVIDE SLIP CRITICAL (SC) CONNECTIONS AT ALL MOMENT CONNECTIONS, BRACED FRAMES, RELIEVING ANGLES AND AS OTHERWISE NOTED. USE A490 BOLTS WHERE INDICATED.

5. WHERE WELDING IS INDICATED, ALL WELDING SHALL CONFORM TO AWS D1.1-LATEST EDITION. ELECTRODES SHALL BE CONFORM TO AWS A5.1 E70XX SERIES WITH PROPER ROD TO PRODUCE OPTIMUM WELD (LOW HYDROGEN).

6. SEE CONCRETE NOTES AND DRAWINGS FOR ANCHOR BOLT INFORMATION, TYP.

7. PROVIDE 3/8" MINIMUM STIFFENER PLATES EACH SIDE OF BEAM WEB AT BEAMS FRAMING OVER COLUMNS AND AT BEAMS SUPPORTING COLUMNS ABOVE.

8. PROVIDE 1/4" THICK LEVELING PLATE UNDER ALL COLUMN BASE PLATES UNLESS OTHERWISE NOTED. LEVELING PLATES SHALL BE SET AND GROUTED PRIOR TO ERECTING COLUMNS.

9. PROVIDE ALL MISCELLANEOUS ANGLES, PLATES, ANCHORS, BOLTS, ETC., SHOWN ON ARCHITECTURAL DRAWINGS FOR SUPPORT OF BLOCKING, PARAPETS, FINISHES, ETC. COORDINATE WITH MISCELLANEOUS METAL FABRICATOR TO ENSURE COMPLETE COVERAGE OF ALL ITEMS.

10. PROVIDE L 4 x 4 x 1/4 SLAB SUPPORT ANGLE AS REQUIRED AT COLUMNS WHERE STRUCTURAL MEMBERS DO NOT FRAME IN AT ALL FOUR SIDES.

11. ALL INTERIOR VISUALLY EXPOSED STRUCTURAL STEEL TO RECEIVE (1) COAT OF SHOP PRIMER. COORD COLOR AND TOP COATS WITH ARCH.

METAL DECK

1. THE METAL ROOF AND FLOOR DECK SHALL BE FORMED OF STEEL SHEETS CONFORMING TO ASTM STANDARD A611.

2. FLOOR AND ROOF DECK SHALL BE AS NOTED ON THE DRAWINGS (OR EQUIVALENT).

3. FOR DECK ATTACHMENTS, PENETRATIONS AND ACCESSORIES, REFER TO SPECIFICATIONS.

COLD FORMED FRAMING NOTES

1. PRODUCTS AND INSTALLATION SHALL MEET THE REQUIREMENTS OF AISI SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS, 2001 EDITION (REF IBC03 PG 580), AWS SPECIFICATION FOR WELDING SHEET STEEL IN STRUCTURES, D1.3, ASTM 653 STANDARD SPECIFICATION FOR SHEET STEEL, ZINC (GALVANIZED) OR ZINC-IRON ALLOY-COATED (GALVANIZED) BY THE HOT DIP PROCESS AND ASTMC-955 STANDARD SPECIFICATION FOR LOAD BEARING (TRANSVERSE AND AXIAL STEEL STUDS, RUNNER (TRACK) AND BRACING AND BRIDGING, FOR SCREW APPLICATION OF GYPSUM BOARD AND METAL PLASTER BASES.

2. FRAMING MATERIALS SHALL BE AS INDICATED ON THE DRAWINGS AS MANUFACTURED BY DIETRICH INDUSTRIES, INC. 500 GRANT ST., SUITE 2226, PITTSBURGH, PA. 15219, (412) 281-2805. APPROVED EQUALS WILL BE CONSIDERED.

3. ALL GALVANIZED STUDS, JOISTS, TRACK, BRIDGING AND ACCESSORIES SHALL BE FORMED FROM STEEL HAVING A G-60 COATING MEETING ASTM C 955.

4. WALL BRIDGING AND SOLID BLOCKING SHALL BE PROVIDED TO BRACE STUDS AGAINST ROTATION. INSTALL WALL BRIDGING AND BLOCKING PER DETAILS THIS DWG.

5. SCREWS SHALL BE SELF DRILLING, SELF TAPPING, ZINC COATED AND NOT LESS THAN #10.

6. SCREW PENETRATION THROUGH JOINED MATERIALS SHALL NOT BE LESS THAN THREE EXPOSED SCREW THREADS

7. PROTECTIVE COATINGS ON SCREW FASTENERS SHALL BE COMPATIBLE WITH LIGHT GAGE MATERIAL BEING JOINED

8. CONTRACTOR SHALL REFER TO INSTALLATION INSTRUCTIONS PUBLISHED BY THE SCREW MANUFACTURER AND ASTM C954 FOR MINIMUM SPACING AND EDGE DISTANCE REQUIREMENTS AND TORQUE REQUIREMENTS.

9. POWDER ACTUATED FASTENERS INTO STEEL SHALL BE HILTI X-U KNUCKLED SHANK FASTENERS.

10. POWDER ACTUATED FASTENERS DESIGNATED 0.157" DIAMETER INTO CONCRETE SHALL BE HILTI X-U PINS AND SHALL NOT BE INSTALLED UNTIL FULL COMPRESSIVE STRENGTH IS OBTAINED. PROVIDE 1" MIN EMBEDMENT.

11. CONTRACTOR SHALL REFER TO INSTRUCTIONS PUBLISHED BY THE P.A.F. MANUFACTURER FOR MINIMUM SPACING, EDGE DISTANCE AND CONCRETE EMBEDMENT AND ADDITIONAL INSTALLATION REQUIREMENT.

12. CUTTING OF COLD FORMED STEEL FRAMING SHALL BE BY SAW, SHEAR OR PLASMA CUTTING EQUIPMENT. OXYACETYLENE TORCH CUTTING IS NOT PERMITTED.

13. TEMPORARY BRACING SHALL BE PROVIDED AND REMAIN IN PLACE UNTIL WORK IS PERMANENTLY STABILIZED.

14. TOP TRACKS SHALL BE CONTINUOUS. WHERE SPLICING OF TRACK IS NECESSARY BETWEEN STUD SPACING, A PIECE OF STUD SHALL BE PLACED BETWEEN ADJACENT TRACKS AND FASTENED BY WELDS OR SCREWS TO EACH SIDE OF THE TRACK, EACH END, UNO.

15. SPLICING OF FRAMING COMPONENTS, OTHER THAN TRACK, IS NOT PERMITTED.

16. A SEALANT SHALL BE APPLIED TO CONCRETE OR MASONRY SURFACES PRIOR TO ANCHORING TRACKS.

17. PROVIDE HORIZ STRAP BRIDGING FOR ALL WALLS. HORIZ BRIDGING SHALL BE CONT 20GA x 1 1/2" (MIN) WIDE STEEL STRAPS ON EA FACE OF STUD, LOCATED AT MAX 4'-0" ON CENTER FOR THE FULL HEIGHT OF THE WALL. PROVIDE TRACK SOLID BLOCKING AT 10'-0" ON CENTER ALONG THE WALL AT EA LINE OF BRIDGING. PROVIDE AN ADDITIONAL LINE OF BRIDGING A MAX OF 12" BELOW ALL SLIP TRACK CONNECTIONS. ALTERNATELY, BRIDGING CHANNELS AND BRIDGING CLIPS MAY BE USED.

18. SLIDE CLIPS SHALL BE SUPERSTUD DC1500 DEFLECTION CLIPS.

TIMBER NOTES

1. ALL TIMBER FRAMING SHALL BE IN ACCORDANCE WITH THE AITC TIMBER CONSTRUCTION MANUAL - LATEST EDITION, AND THE AF & PA NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION (NDS) 2001 EDITION.

2. INDIVIDUAL TIMBER FRAMING MEMBERS SHALL BE VISUALLY GRADED. MINIMUM GRADE NO1/NO2 SPRUCE-PINE-FIR KILN DRIED TO 19% MAXIMUM MOISTURE CONTENT UNLESS OTHERWISE INDICATED ON THE DRAWINGS.

3. ALL ROOF AND WALL SHEATHING SHALL BE APA PERFORMANCE-RATED. SHEATHING SHALL BE NAILED TO THE FRAMING AS FOLLOWS, U.N.O.:

   A. ROOFS: 8d NAILS AT 6" O.C. AT SUPPORTED PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS.
   B. WALLS: 8d NAILS AT 6" O.C. AT SUPPORTED PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS.

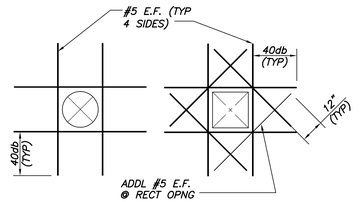
4. FASTENING NOT SPECIFIED SHALL CONFORM WITH IBC TABLE 2304.9.1. NAIL FASTENERS SHALL MEET THE REQUIREMENTS OF ASTM F1667. UNLESS NOTED OTHERWISE, NAILS REFERENCED ON DRAWINGS ARE TO BE COMMON NAILS WITH DIMENSIONS AS FOLLOWS.

   6d: 2 LONG BY 0.113 DIAMETER SHANK WITH 0.266 DIAMETER HEAD
   8d: 2 1/2" LONG BY 0.131 DIAMETER SHANK WITH 0.281 DIAMETER HEAD
   10d: 3 LONG BY 0.148 DIAMETER SHANK WITH 0.312 DIAMETER HEAD
   12d: 3 1/4" LONG BY 0.148 DIAMETER SHANK WITH 0.312 DIAMETER HEAD
   16d: 3 1/2" LONG BY 0.162 DIAMETER SHANK WITH 0.344 DIAMETER HEAD
   20d: 4 LONG BY 0.192 DIAMETER SHANK WITH 0.406 DIAMETER HEAD
   30d: 4 1/2" LONG BY 0.207 DIAMETER SHANK WITH 0.438 DIAMETER HEAD

5. ALL TIMBER CONNECTION HARDWARE (JOIST HANGERS, POST BASES, SHEARWALL HOLDOWNS, ETC) SHALL BE AS INDICATED ON THE DRAWINGS AND MANUFACTURED BY SIMPSON STRONG-TIE. ALL CONNECTION HARDWARE SHALL BE HOT-DIPPED GALVANIZED G-90 (U.N.O.). CONNECTION HARDWARE USED IN CONJUNCTION WITH PRESERVATIVE TREATMENT SHALL BE GALVANIZED G185 (ZMAX.) USE FASTENERS & HANGERS OF SAME MATERIAL & COATING. REFER TO MANUFACTURER'S LITERATURE FOR PROPER HANDLING AND INSTALLATION GUIDELINES.

REBAR LAP SPLICE TABLE

BAR SIZE	LAP LENGTH		
	3,000 PS1	3,500 PS1	4,000 PS1
#3	30"	28"	24"
#4	36"	34"	32"
#5	48"	45"	42"
#6	56"	52"	48"
#7	81"	78"	72"
#8	93"	88"	80"



TYP OPENING IN WALL OR SLAB DETAIL
N.T.S.

STATE OF MAINE
PAUL B. BECKER
NO. 8554
LICENSED PROFESSIONAL ENGINEER

POWERPAY OFFICE FIT UP

Winton Scott Architects
5 Milk Street
Portland, ME 04101

Wright Ryan Construction,Inc.
10 Danforth Street
Portland, ME 04101

Becker Structural Engineers, Inc.
75 York Street
Portland, Maine 04101

Mechanical Systems Engineers
10 Royal River Center
Yarmouth, Maine 04096

Bartlett Design
942 Washington Ave.
Bath, Maine 04530

Fore Solutions
386 Fore Street
Portland, Maine 04101

GENERAL NOTES

S1.0

Scale: AS NOTED | March 20, 2009