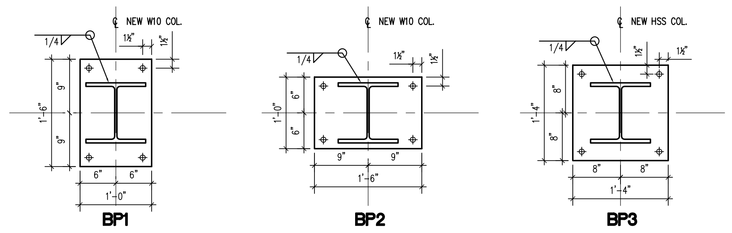
COLUMN SCHEDULE

COLUMN LOCATION	A17, A19, A22, B9, B25, C1	A18	A21, B28, C7	B10, B12	B14, B17, B22, B29, C4, C5, F29	C9	D24, D28, D.7–21, E15, F21, G8	D27	E10, E13	E11, G9, P4	E20, F28, G4, G6	E18	F24, F27	H1, R2	J18, J21, U18, U21, V18, V21	K2, M2	K4, M4	L27	L29, Q27, U27	N6, S6	N10, N13, S10, S13	N18, N21, Q18, Q21	O27, V16, V23	O29, V7, V27	P2	P.6–3.7	T3	V9, V11	V13	W7, W9.1, W10.8, W13, W14.9, W24.1, W26	X–18, X–21	AA (2 COLS), BB, CC, DD, EE, FF	BB, CC, DD, EE, FF
CAP PLATE	–	–	–	–	–	–	–	12"x10"x3/4"	–	–	–	–	–	–	–	–	–	–	–	16"x10"x3/4"	16"x12"x3/4"	–	–	–	–	10"x8"x3/4"	–	–	–	–	–	–	–
(T.O.S. WITH RESPECT TO DATUM @ COL. Q)																																	
HIGH ROOF ELEV. (VARIES – SEE PLAN)																																	
MAIN ROOF ELEV. (VARIES – SEE PLAN)																																	
MEZZANINE LEVEL ELEV. (+14'–0")																																	
DATUM ELEV. (+0'–0") (DIMENSION TO B/BASE PL. TYP.)	W10x33 7"	W10x54 7"	W10x33 7"	W10x45 7"	W10x33 7"	W10x39 7"	W10x33 7"	W10x33 7"	W10x45 7"	W10x39 7"	W10x33 7"	W10x54 7"	W10x45 7"	W10x33 7"	W10x39 7"	W10x33 7"	W10x33 7"	W10x39 7"	W8x28 7"	W10x39 7"	W10x49 7"	W10x68 7"	W10x33 7"	W8x28 7"	W10x39 7"	W8x28 7"	W8x28 7"	W10x45 7"	W10x39 7"	W8x28 7"	W10x33 7"	W8x40 (EXT) 7"	W8x40 (INT) 7"
BASE PLATE (W x L x t)	12"x18"x3/4"	18"x12"x1–1/4"	16"x12"x1"	12"x18"x1"	12"x18"x1"	12"x18"x1"	16"x16"x1"	12"x18"x3/4"	16"x16"x1–1/4"	16"x16"x1–1/4"	16"x16"x1–1/4"	16"x16"x1–1/4"	16"x16"x1–1/4"	14"x14"x1"	16"x16"x1"	12"x18"x1"	16"x16"x1"	16"x16"x1"	12"x16"x3/4"	16"x16"x1"	16"x16"x1"	16"x16"x1"	12"x18"x3/4"	12"x14"x3/4"	18"x12"x1–1/4"	12"x16"x3/4"	12"x16"x3/4"	12"x18"x1–1/4"	18"x12"x1–1/4"	14"x14"x3/4"	12"x18"x3/4"	16"x16"x3/4"	14"x14"x3/4"
ANCHOR BOLTS (F1554)	(4)–3/4"ø	(4)–3/4"ø	(4)–3/4"ø	(4)–3/4"ø	(4)–3/4"ø	(4)–3/4"ø	(4)–3/4"ø	(4)–3/4"ø	(4)–3/4"ø	(4)–3/4"ø	(4)–3/4"ø	(4)–3/4"ø	(4)–3/4"ø	(3)–3/4"ø	(4)–3/4"ø	(4)–3/4"ø	(4)–3/4"ø	(4)–3/4"ø	(4)–3/4"ø	(4)–3/4"ø	(4)–3/4"ø	(4)–3/4"ø	(4)–3/4"ø	(3)–3/4"ø	(4)–3/4"ø	(4)–3/4"ø	(4)–3/4"ø	(4)–3/4"ø	(4)–3/4"ø	(4)–3/4"ø	(4)–3/4"ø	(4)–3/4"ø	(4)–3/4"ø
BASE PLATE DETAIL	BP1	BP2	BP4	BP1	BP1	BP1	BP3	BP1	BP3	BP3	BP3	BP3	BP3	BP5	BP3	BP1	BP3	BP3	BP6	BP3	BP3	BP3	BP1	BP7	BP2	BP6	BP6	BP1	BP2	BP8	BP1	BP8	BP8



WSSB–001

GENERAL NOTES

DESIGNED IN ACCORDANCE WITH THE MAINE MODEL BUILDING CODE AND THE 2003 INTERNATIONAL BUILDING CODE.

IF ANY FIELD CONDITIONS PRECLUDE COMPLIANCE WITH THESE DRAWINGS AND/OR CONDITIONS SPECIFIED, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ENGINEER AND SHALL NOT PROCEED WITH THE AFFECTED WORK.

TYPICAL DETAILS AND NOTES SHOWN HEREIN SHALL APPLY UNLESS SPECIFICALLY SHOWN OR NOTED OTHERWISE. CONSTRUCTION DETAILS NOT FULLY SHOWN OR NOTED SHALL BE SIMILAR TO DETAILS SHOWN FOR SIMILAR CONDITIONS.

ALL UNDERGROUND UTILITY LOCATIONS SHALL BE VERIFIED PRIOR TO STARTING EXCAVATION WORK.

WORK AREAS SHALL BE MARKED, FENCED, AND OTHERWISE SECURED SO AS TO PROVIDE PROPER PROTECTION FOR THE PUBLIC, AND AS REQUIRED BY THE BUILDING INSPECTOR.

IT SHALL BE THE CONTRACTORS SOLE RESPONSIBILITY TO DESIGN AND PROVIDE ADEQUATE SHORING, BRACING AND FORMWORK, ETC., AS REQUIRED FOR THE PROTECTION OF LIFE AND PROPERTY DURING CONSTRUCTION. THIS INCLUDES PROVIDING TEMPORARY BRACING, SHORING, GUYS OR TIE–DOWNS. THESE TEMPORARY SUPPORTS SHALL REMAIN IN PLACE UNTIL ALL STRUCTURAL COMPONENTS ARE STABLE AND COMPLETED.

ALL FOOTINGS HAVE BEEN DESIGNED FOR AN ALLOWABLE NET SOIL BEARING PRESSURE OF 1,000 PSF MULTIPLIED BY THE LEAST LATERAL DIMENSION OF THE FOOTING IN FEET, UP TO A MAXIMUM OF 3,000 PSF, PER GEOTECHNICAL REPORT DATED OCTOBER 10, 2005 BY SEBAGO TECHNICS. ALL FOOTINGS SHALL BEAR ON ADEQUATE UNDISTURBED SOILS OR CONTROLLED FILL COMPACTED BY INTENSIVE SURFACE COMPACTION (ISC) AS RECOMMENDED IN THE GEOTECHNICAL REPORT.

THE BOTTOM OF ALL EXTERIOR FOOTING ELEVATIONS SHALL BE A MINIMUM OF 4'–6" BELOW FINISHED EXTERIOR GRADE. COMPACT BOTTOM OF FOOTING SUBGRADE WITH PLATE COMPACTOR. REMOVE ALL DEBRIS FROM BOTTOM OF FOOTING PRIOR TO PLACING CONCRETE. DO NOT PLACE CONCRETE ON FROZEN SOIL, ICE, MUD, OR IN STANDING WATER. ALL FOUNDATION SUBGRADES SHALL BE INSPECTED AND APPROVED UNDER THE SUPERVISION OF A REGISTERED PROFESSIONAL ENGINEER PRIOR TO BEING CONCRETED.

FLOOR LIVE LOADS:
MEZZANINE (OFFICE) = 100 PSF
MEZZANINE (MECHANICAL)= 250 PSF

ROOF LOADS:
LIVE LOAD = 35 PSF
SNOW LOAD
Pg = 50 PSF
Pf (SLOPE < 5°) = 35 PSF.
Ps (5°< SLOPE < 35°) = 31.5 PSF.
SNOW EXPOSURE CATEGORY, Ce = 1.0
THERMAL FACTOR, Ct = 1.0
SNOW LOAD IMPORTANCE FACTOR, Is = 1.0
(LOADS INCREASED FOR DRIFT, SLIDING, ETC.)

WIND LOADS:
BASIC WIND SPEED = 100 MPH
WIND IMPORTANCE FACTOR, Iw = 1.0
EXPOSURE CATEGORY = B
INTERNAL PRESSURE COEFFICIENT, Gcpi = –0.18

SEISMIC DESIGN CRITERIA:
SEISMIC IMPORTANCE FACTOR, Ie = 1.0
SEISMIC USE GROUP, I
SPECTRAL RESPONSE ACCELERATIONS: Ss = 36.3
S1 = 9.7
SITE CLASS E
SPECTRAL RESPONSE COEFFICIENTS: Sds = 0.520
Sd1 = 0.227
SEISMIC DESIGN CATEGORY, D
BASIC SEISMIC FORCE RESISTING SYSTEM: INTERMEDIATE MOMENT FRAMES
SEISMIC RESPONSE COEFFICIENT, Cs = 0.12
DESIGN BASE SHEAR, V = 0.12W
RESPONSE MODIFICATION FACTOR, R = 4.5
ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE ANALYSIS

ALL COMPACTED CONTROLLED FILL BEING PLACED MUST BE TESTED BY AN APPROVED TESTING LABORATORY DURING THE FILL OPERATION. SLABS AND FOOTINGS SHALL NOT BE PLACED UNTIL FILL TEST REPORTS HAVE BEEN CHECKED AND APPROVED BY THE ENGINEER. PRIOR TO PLACING CONCRETE, EXISTING SOIL CONDITIONS SHALL BE INSPECTED AND VERIFIED BY THE GEOTECHNICAL ENGINEER. ALL UNSUITABLE SOIL SHALL BE REMEDIED AS DIRECTED BY THE GEOTECHNICAL ENGINEER. BUILDING PAD PREPARATION SHALL BE IN ACCORDANCE WITH THE SITE DRAWINGS AND SPECIFICATIONS.

ALL SPECIAL INSPECTION REPORTS PERFORMED IN ACCORDANCE WITH THE STATEMENT OF SPECIAL INSPECTIONS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW IN A TIMELY MANNER. THE FINAL REPORT OF SPECIAL INSPECTIONS SHALL NOT BE ISSUED UNTIL ALL SPECIAL INSPECTION REPORTS HAVE BEEN RECEIVED AND APPROVED BY THE ENGINEER.

MASONRY NOTES

NEW MASONRY WALLS ARE DESIGNED IN ACCORDANCE WITH ACI 530 WITH APPENDIX A.3 SPECIAL PROVISIONS FOR SEISMIC DESIGN.

CONSTRUCTION SHALL CONFORM TO ACI 530.1 – LATEST EDITION, "SPECIFICATIONS FOR MASONRY STRUCTURES". ALL MASONRY CONSTRUCTION SHALL BE INSPECTED AND TESTED TO ASSURE CONFORMANCE WITH THE DETAILS SHOWN ON THESE PLANS.

MASONRY UNITS SHALL CONFROM TO ASTM C90 WITH COMPRESSIVE STRENGTH OF 1900 PSI, LIGHT WEIGHT BLOCK.

MASONRY MORTAR SHALL BE TYPE S, CONFORMING TO ASTM C270.

ALL BAR REINFORCING SHALL CONFORM TO ASTM A615, GRADE 60.

ALL BAR REINFORCING SHALL BE GROUTED SOLID FULL HEIGHT USING LOW LIFT METHOD.

ALL BOND BEAMS SHALL BE GROUTED SOLID.

GROUT SHALL CONFORM TO ASTM C94 OR C476.

PROVIDE NO. 9 WIRE STANDARD TRUSS–TYPE HORIZONTAL REINFORCEMENT AT 8" o.c. TYPICAL, UNLESS OTHERWISE NOTED.

PROVIDE GALVANIZED TRIANGLE TIES AT 16" o.c. AT ALL COLUMNS IN MASONRY WALLS.

PROVIDE #5 VERTICAL, SOLIDLY GROUTED AT ALL DOOR, WINDOW, AND CONSTRUCTION JOINTS. SEE DETAIL 3/S2.02 AND 4/S2.02.

STRUCTURAL STEEL NOTES

STEEL SHALL CONFORM TO THE "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS, ALLOWABLE STRESS DESIGN AND PLASTIC DESIGN", LATEST EDITION, OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION.

ALL STRUCTURAL STEEL SHALL CONFORM TO ASTM A992, EXCEPT AS MODIFIED BELOW:
ALL MISCELLANEOUS STRUCTURAL STEEL SHALL CONFORM TO ASTM A36.
ALL HOLLOW STRUCTURAL SECTIONS SHALL CONFORM TO ASTM A500, GRADE B.
ALL ANCHOR RODS SHALL CONFORM TO ASTM F1554, GRADE 55.

ALL WELDS SHALL USE E70XX ELECTRODES AND CONFORM TO THE "STRUCTURAL WELDING CODE (STEEL)", AWS D1.1, LATEST EDITION.

ALL BOLTED CONNECTIONS SHALL BE BEARING TYPE, UTILIZING 3/4" DIA. ASTM A325–N TENSION CONTROL BOLTS, UNLESS SHOWN OTHERWISE.

ALL BOLTS, NUTS, AND WASHERS SHALL BE IN ACCORDANCE WITH THE LATEST SPECIFICATIONS APPROVED BY THE RESEARCH COUNCIL ON RIVETED AND BOLTED STRUCTURAL JOINTS. USE DIRECT TENSION INDICATORS, CONFORMING TO ASTM F959, ON ALL A325 BOLTS.

ALL STRUCTURAL STEEL SHALL BE SHOP PRIMED (ONE COAT OF 2 MIL. DRY FILM) AND SHOP PAINTED (ONE COAT OF 1.5 MIL. DRY FILM). SHOP PRIMER AND PAINT TO BE FABRICATOR'S STANDARD COLOR.

OPENINGS THROUGH BEAMS AND COLUMNS SHALL NOT BE PERMITTED UNLESS APPROVED BY THE STRUCTURAL ENGINEER.

FIELD FLAME CUTTING OF ANY STRUCTURAL MEMBERS IS NOT PERMITTED.

EXPOSED TOPS OF ALL STEEL HOLLOW STRUCTURAL SECTION COLUMNS SHALL BE COVERED WITH A SHOP WELDED CAP PLATE AND SHALL BE WATERTIGHT.

ALL STEEL SHALL BE FABRICATED, ERECTED, AND TESTED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES"

STRUCTURAL STEEL SHOP DRAWINGS, PREPARED BY THE STEEL SUBCONTRACTORS, SHOWING COMPLETE DIMENSIONS, DETAILS, SIZES, AND GRADES OF STEEL MEMBERS AND CONNECTIONS, TYPE AND NUMBER OF WELDS AND BOLTS, SHALL BE SUBMITTED FOR APPROVAL PRIOR TO THE FABRICATION OF THE STEEL COMPONENTS. THE STRUCTURAL STEEL SUBCONTRACTOR SHALL BE RESPONSIBLE FOR CHECKING, VERIFICATION AND COORDINATION OF DIMENSIONS AND DETAILS WITH THE STRUCTURAL AND OTHER PORTIONS OF THE CONTRACT DRAWINGS. THE STEEL SUBCONTRACTOR SHALL NOT REPRODUCE ANY PORTION OF THE STRUCTURAL CONTRACT DRAWINGS FOR UTILIZATION AS SHOP DRAWINGS, UNLESS APPROVAL OF THE STRUCTURAL ENGINEER IS OBTAINED.

THE STEEL FABRICATOR SHALL BE AISC CERTIFIED, OR A MEMBER OF THE STRUCTURAL STEEL FABRICATORS OF NEW ENGLAND.

ALL STRUCTURAL STEEL ERECTION AND SHORING SHALL BE INSPECTED BY AN INDEPENDENT TESTING AGENCY RETAINED BY THE OWNER FOR THE FOLLOWING ITEMS:
VISUALLY INSPECT ALL CONNECTIONS, INCLUDING, BUT NOT LIMITED TO, BOLTS, WELDS, METAL DECK WELDS TO SUPPORT STRUCTURE, AND METAL DECK SIDE LAPS.
INSPECT STEEL ERECTION FOR ORIENTATION AND PLUMBNESS.

ALL STEEL CONNECTIONS SHALL BE DESIGNED BY THE STEEL FABRICATOR TO RESIST MINIMUM THE REACTIONS SHOWN THUS XXk ON THESE DRAWINGS.

ALL STEEL JOISTS SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE STANDARD SPECIFICATIONS OF THE STEEL JOIST INSTITUTE.

PROVIDE JOIST BRIDGING OF AMOUNT AND TYPE AT LOCATIONS AS REQUIRED BY THE JOIST MANUFACTURER.

ALL BAR JOISTS SHALL BE DESIGNED FOR A NET WIND UPLIFT PRESSURE AS SHOWN ON THE DRAWINGS OVER THEIR TRIBUTARY AREAS.

STEEL DECK NOTES

STEEL ROOF DECK SHALL CONFORM TO AISI SPECIFICATION FOR THE DESIGN OF LIGHT GAGE, COLD–FORMED STRUCTURAL STEEL MEMBERS AND THE STEEL DECK INSTITUTE'S DESIGN REQUIREMENTS. COMPOSITE STEEL FLOOR DECK SHALL CONFORM TO ASTM A–611, GRADE C AND D, OR FROM A–653–94 STRUCTURAL QUALITY GRADE 33 OR HIGHER.

STEEL ROOF DECK, UNLESS SPECIFIED OTHERWISE, SHALL BE 1 1/2" DEEP, 20 GAGE MINIMUM, TYPE "B" BY VULCRAFT OR APPROVED EQUAL. STEEL DECK SHALL BE PRIMERED.

STEEL ROOF DECK SHALL BE CONTINUOUS OVER A MINIMUM OF THREE SPANS, AND SHALL BE FASTENED TO THE SUPPORTS AT THE UNIT ENDS AND AT INTERMEDIATE SUPPORTS AS RECOMMENDED BY THE STEEL DECK MANUFACTURER. MINIMUM ATTACHMENT REQUIREMENTS SHALL BE 5/8" DIA. PUDDLE WELDS AT 12" o.c. AT EVERY SUPPORT AND 6" o.c. AT DECK PERIMETER WITH #10 TEK SCREWS AT 12" o.c. AT ALL SIDE LAPS. SEE DECK ATTACHMENT DETAIL FOR FURTHER CLARIFICATION.

STEEL FLOOR DECK, UNLESS SPECIFIED OTHERWISE, SHALL BE 1 1/2" DEEP, 20 GAGE MINIMUM, TYPE "VL" BY VULCRAFT OR APPROVE EQUAL. STEEL DECK SHALL BE PRIMERED.

STEEL FLOOR DECK SHALL BE CONTINUOUS OVER A MINIMUM OF THREE SPANS, AND SHALL BE FASTENED TO THE SUPPORTS AT THE UNIT ENDS AND AT INTERMEDIATE SUPPORTS AS INDICATED IN FASTENING SCHEDULES. SEE FRAMING PLANS FOR FURTHER INFORMATION.

ALL ACCESSORIES TO THE STEEL FLOOR AND ROOF DECK, INCLUDING EDGE ANGLES, POUR STOPS, COLUMN CLOSURES, END CLOSURES, COVER PLATES AND GIRDER FILLERS, SHALL BE IN ACCORDANCE WITH THE STEEL DECK INSTITUTE.

COLD FORMED METAL FRAMING NOTES

ALL LIGHTWEIGHT STEEL PRODUCTS, DETAILING, FABRICATION AND INSTALLATION SHALL MEET THE REQUIREMENTS OF AISI "SPECIFICATIONS FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS", 1986 EDITION WITH 1989 ADDENDUM.

STEEL GRADES:
12, 14, & 16 GAGE COMPONENTS Fy (MIN) = 50 ksi (ASTM A–446 GRADED)
18 GAGE COMPONENTS Fy (MIN) = 33 ksi (ASTM A–446 GRADE B)
20 GAGE COMPONENTS Fy (MIN) = 33 ksi (ASTM A–446 GRADE A)

ALL COMPONENTS SHALL BE ZINC COATED (G60 GALVANIZED) IN ACCORDANCE WITH ASTM A–525.

FLAME CUTTING OF MEMBERS OR HOLES IS NOT PERMITTED.

ALL FIELD CONNECTIONS SHALL CONFORM TO DETAILS SHOWN ON FABRICATION PLANS.

ALL STUDS, HARDWARE, ACCESSORIES, AND FASTENERS SHALL BE GALVANIZED.

ALL WELDING SHALL BE IN ACCORDANCE WITH AWS D1.3.

CONCRETE NOTES

ALL CONCRETE WORK SHALL CONFORM TO ACI "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE", ACI–318–02, AND "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES", ACI–315–LATEST EDITION.

ALL CONCRETE FORMWORK SHALL CONFORM WITH ACI "RECOMMENDED PRACTICES FOR CONCRETE FORMWORK", ACI–347.

ALL STRUCTURAL CONCRETE SHALL CONFORM WITH ACI "SPECIFICATIONS FOR STRUCTURAL CONCRETE BUILDINGS", ACI–301.

AIR ENTRAIN ALL EXTERIOR CONCRETE.

ALL CONCRETE FOR EXTERIOR FOUNDATION WALLS SHALL BE NORMAL WEIGHT CONCRETE WITH A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS OF 4,000 PSI. ALL OTHER CONCRETE SHALL BE NORMAL WEIGHT CONCRETE WITH A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS OF 3,000 PSI.

ALL CONCRETE SHALL BE CONTINUOUSLY MOIST CURED FOR SEVEN (7) DAYS MINIMUM. CONCRETE SHALL BE KEPT MOIST BY FOG SPRAYING, CURING PAPER AND/OR ACCEPTABLE CURING COMPOUNDS.

CONTRACTOR SHALL SUBMIT MIX DESIGNS FOR APPROVAL. MIX DESIGN SHALL INDICATE 28–DAY STRENGTHS, CEMENT CONTENT, AIR CONTENT, WATER–CEMENT RATIO, AMOUNT OF FINE AND COARSE AGGREGATES, AND ADMIXTURES.
MAX. WATER–CEMENT RATIO = 0.53 FOR 3000 PSI
0.45 FOR 4000 PSI
MAX. SLUMP–
4" FOR SLABS
5" FOR ALL OTHER CONCRETE
MAX. AGGREGATE SIZE–
FOOTINGS & FOUNDATIONS – 3/4" TO 1 1/2"
SLAB–ON–GRADE – 3/4" TO 1"
CONCRETE FILL – 1/2"

CEMENT SHALL BE PORTLAND CEMENT, TYPE I OR II, CONFORMING TO ASTM C–150. AGGREGATES SHALL CONFORM TO ASTM C–33. AIR ENTRAINING ADMIXTURES SHALL CONFORM TO ASTM C–260. NON–SHRINK GROUT SHALL CONFORM TO ASTM C–109. EXPANSION JOINTS SHALL BE 1/2" THICK ASPHALT IMPREGNATED FIBERBOARD JOINT MATERIAL CONFORMING TO ASTM D–1751. CURING COMPOUND SHALL BE CLEAR, CONFORMING TO ASTM C–309.

COLD WEATHER CONCRETE WORK SHALL CONFORM TO ACI–306.

HOT WEATHER CONCRETE WORK SHALL CONFORM TO ACI–305.

ALL FLOOR SLABS SHALL BE CONSTRUCTED TO HAVE A MINIMUM FLATNESS OF Ff=20 AND A MINIMUM LEVELNESS OF Fl=17.

THE MAXIMUM LENGTH BETWEEN CONSTRUCTION JOINTS OF WALLS AND FOOTINGS SHALL NOT EXCEED 40 FEET.

STRUCTURAL STEEL WILL DEFLECT UNDER WEIGHT OF CONCRETE. POUR FLOOR SLABS LEVEL REGARDLESS OF STEEL DEFLECTION.

ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615 GRADE 60. WELDED WIRE MESH SHALL CONFORM TO ASTM A185 AND ASTM A82.

WELDED WIRE MESH SHALL HAVE MINIMUM END AND SIDE LAPS OF 1'–0".

CONCRETE ACCESSORIES SHALL BE ADEQUATE TO MAINTAIN REINFORCING ACCURATELY IN PLACE AND BE NON–CORROSIVE, NON–STAINING TYPE.

LAP ALL BAR REINFORCING PER ACI–318. STAGGER SPLICES IN WALLS AND SLABS AND ALL HORIZONTAL REINFORCEMENT.

CONCRETE SHALL NOT BE DROPPED THROUGH REINFORCING STEEL (AS IN WALLS AND COLUMNS) SO AS TO CAUSE SEGREGATION OF AGGREGATES. USE HOPPERS, CHUTES OR TRUNKS OF VARYING LENGTHS SO THAT THE FREE UNCONFINED FALL OF CONCRETE SHALL NOT EXCEED FIVE (5) FEET.

MINIMUM REINFORCEMENT COVER, UNLESS NOTED OTHERWISE:
–CONCRETE CAST AGAINST EARTH: 3"
–CONCRETE EXPOSED TO EARTH OR WEATHER:
No. 6 TO No. 18 BARS – 2"
No. 5 AND SMALLER – 1 1/2"
–CONCRETE NOT EXPOSED TO EARTH OR WEATHER:
No. 11 AND SMALLER – 3/4"

ALL CONCRETE WORK, REINFORCING PLACEMENT, AND FORMWORK SHALL BE INSPECTED BY AN INDEPENDENT TESTING AGENCY RETAINED BY THE OWNER FOR THE FOLLOWING ITEMS:
INSPECT BOTTOM OF FOOTING SOIL CONDITIONS.
INSPECT FORMWORK AND PLACEMENT OF CONCRETE.
SAMPLING FRESH CONCRETE PER ASTM C–172, MODIFIED FOR SLUMP BY ASTM C–94. SLUMP PER ASTM C–143. ONE TEST AT POINT OF DISCHARGE FOR EACH DAYS POUR FOR EACH TYPE OF CONCRETE.
AIR CONTENT PER ASTM C–173 (VOLUMETRIC) OR ASTM C–231 (PRESSURE). ONE TEST FOR EACH DAYS POUR FOR EACH TYPE OF CONCRETE.
COMPRESSION TEST CYLINDERS PER ASTM C–31 (4 CYLINDERS)
CONCRETE TEMPERATURE – TEST EACH TIME A SET OF CYLINDERS IS CAST.
COMPRESSION STRENGTH TEST PER ASTM C–39. ONE SET FOR EACH DAYS POUR PLUS ADDITIONAL SETS FOR EACH 50 cy OVER AND ABOVE THE FIRST 25 cy. ONE CYLINDER TESTED AT 7 DAYS, TWO CYLINDERS TESTED AT 28 DAYS, AND ONE CYLINDER RETAINED FOR LATER TESTING IF REQUIRED.

CONTROLLED FILL NOTES

CONTROLLED FILL SHALL BE INERT, GRANULAR FILL FREE FROM ORGANIC MATERIAL, WITH LESS THAN 20% PASSING A No. 200 SIEVE. STONES SHALL NOT BE GREATER THAN 6".

PLACE FILL IN LAYERS NOT TO EXCEED 12". COMPACT WITH PLATE TAMPER OR JUMPING JACK TO A MINIMUM OF 95% OF MAXIMUM DRY UNIT WEIGHT AS DETERMINED BY THE MODIFIED PROCTOR TEST, ASTM D1557. PLACE FILL AT OPTIMUM MOISTURE CONTENT.

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PORTLAND, CUMBERLAND COUNTY, MAINE

No. Date Desc:

REVISIONS
1 02-24-06 ISSUED FOR CONST.
2 04-14-06 FOUNDATION SET
3 04-21-06 STEEL PACKAGE ADDENDUM
4 05-12-06 RESUBMIT FOR SITE PLAN

ARCHITECT/ENGINEER SEAL

Designed	E.A.B.
Drawn	E.A.B.
Checked	C.J.A.
Approved	B.B.
Scale	AS NOTED
Project No.	0402829
Date	12–16–05
CAD File	XS04082901

Title
COLUMN SCHEDULE AND GENERAL NOTES

Sheet No.
S1.00

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THESE DRAWINGS SHALL NOT BE UTILIZED BY ANY PERSON, FIRM OR CORPORATION WITHOUT THE SPECIFIC WRITTEN PERMISSION OF BL COMPANIES