Form # P 04

DISPLAY THIS CARD ON PRINCIPAL FRONTAGE OF WORK

CITY OF PORTLAND

BUILDING INSPECTION

PERMIT

PERMIT ISSUED
FEB 7 2006
CITY OF PORTLAND

Please Read Application And Notes, If Any, Attached

Permit Number: 060226

This is to certify that PORMAN ONE 2004 LP /C & B Construction Mgmt & B ers,

has permission to FOUNDATION ONLY-Wh Foods M ket

AT 160 FOX ST 024 D001001

provided that the person or persons rm or tion a epting this permit shall comply with all of the provisions of the Statutes of ine and of the O nances of the City of Portland regulating the construction, maintenance and e of buildings and uctures, and of the application on file in this department.

Apply to Public Works for street line and grade if nature of work requires such information.

Notification of inspection must be given and when permission procured before this building or part thereof is lined or otherwise closed-in 24 HOUR NOTICE REQUIRED.

A certificate of occupancy must be procured by owner before this building or part thereof is occupied.

OTHER REQUIRED APPROVALS

Fire Dept. Greg Cass 1-16-06

Health Dept. _____

Appeal Board _____

Other _____
Department Name

Director - Building & Inspection Services

PENALTY FOR REMOVING THIS CARD

Permit No:	Issue Date:	CBL:
06-0026	PERMIT ISSUED FEB - 7 2006 CITY OF PORTLAND	024 D001001

Location of Construction:	Owner Name:	Owner Address:	Phone:
160 FOX ST	PORMAN ONE 2004 LP	600 CONGRESS AVE STE 400	
Business Name:	**Contractor Name:**	**Contractor Address:**	**Phone**
	CM & B Construction Mgmt & Buil	o Kilboan Lane Lynnfield	7812469400
Lessee/Buyer's Name	**Phone:**	**Permit Type:**	**Zone:**
		Foundation Only/Commercial	B-5

Past Use:	Proposed Use:	Permit Fee:	Cost of Work:	CEO District:
Commercial CBL 024 D001 & 024 D002	Commercial/ FOUNDATION ONLY-Whole Foods Market Connected w/ Permit #06-0027			1

FIRE DEPT: ☑ Approved ☐ Denied
See Conditions
Signature: Lorrie Cross

INSPECTION:
Use Group: Type: FOUNDATION ONLY
Signature: 2/7/06

Proposed Project D
FOUNDATION ONLY-Whole Foods Market

Action: ☐ Approved ☐ Approved w/Conditions ☐ Denied
Signature: Date:

Permit Taken By:	Date Applied For:
ldobson	01/06/2006

Zoning Approval

Special Zone or Reviews	Zoning Appeal	Historic Preservation
☐ Shoreland N/A	☐ Variance	☑ Not in District or Landmar
☐ Wetland	☐ Miscellaneous	☐ Does Not Require Review
☐ Flood Zone Panel 13 Zone C	☐ Conditional Use	☐ Requires Review
☐ Subdivision	☐ Interpretation	☐ Approved
☒ Site Plan #2004-0225	☐ Approved	☐ Approved w/Conditions
Maj ☒ Minor ☐ MM ☐	☐ Denied	☐ Denied
Date: 1/19/06	Date: 4	Date:

CERTIFICATION

I hereby certify that I am the owner of record of the named property, or that the proposed work is authorized by the owner of record and that I have been authorized by the owner to make this application as his authorized agent and I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in the application is issued, I certify that the code official's authorized representative shall have the authority to enter all areas covered by such permit at any reasonable hour to enforce the provision of the code(s) applicable to such permit.

SIGNATURE OF APPLICANT ADDRESS DATE PHONE

RESPONSIBLE PERSON IN CHARGE OF WORK, TITLE DATE PHONE

General Building Permit Application

If you or the property owner owes real estate **or** personal property taxes or user charges on **any** property within the City, payment arrangements must be made before permits of any kind are accepted.

Location/Address of Construction: FRANKLIN ST. ARTERIAL, FOX & SOMERSET ST., PEARL ST.		
Total Square Footage of Proposed Structure: 46,225 SF	Square Footage of Lot: 4.58A = 199,505 SF	
Tax Assessor's Chart, Block & Lot — Chart# 24 Block# D Lot# 1+2	Owner: WHOLE FOODS MARKET PROPERTIES 550 BOWIE ST. AUSTIN, TX 78703	Telephone: (781) 831-0337
Lessee/Buyer's Name (If Applicable)	Applicant name, address & telephone: WHOLE FOODS MARKET 125 CAMBRIDGE PARK DR. CAMBRIDGE, MA 02140 ATTN: ROBERT DONAHUE (781) 831-0337	Cost Of Work: $ 100,000 Fee: $ 921 C of O Fee: $
Current Specific use: STORAGE / BUSINESS If vacant, what was the previous use? STORAGE Proposed Specific use: MERCANTILE		
Project description: CONSTRUCTION OF PROPOSED GROCERY STORE - FOUNDATIONS ONLY.		
Contractor's name, address & telephone: CONSTRUCTION MANAGEMENT AND BUILDERS, INC. Who should we contact when the permit is ready: JEFFREY A. ANDERSON Mailing address: 6 KIMBALL LANE LYNNFIELD, MA 01940	Phone: (781) 246-9400	

Please submit all of the information outlined in the Commercial Application Checklist.
Failure to **do** so will result in the automatic denial of your permit.

In order to be sure the City fully understands the full scope of the project, the Planning and Development Department may request additional information prior to the issuance of a permit. For further information visit us on-line at www.portlandmaine.gov, stop by the Building Inspections office, room 315 City Hall or call 874-8703.

DEPT. OF BUILDING INSPECTION
CITY OF PORTLAND, ME
JAN - 6 2006
RECEIVED

I hereby certify that I am the Owner of record of the named property, or that the owner of record authorizes the proposed work and that I have been authorized by the owner to make this application as his/her authorized agent. I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in this application is issued, I certify that the Code Official's authorized representative shall have the authority to enter all areas covered by this permit at any reasonable hour to enforce the provisions of the codes applicable to this permit.

Signature of applicant: _____ **Date:** JAN. 5, 2006

This is not a permit; **you** may not commence **ANY** work until the permit is issued.

City of Portland, Maine - Building or Use Permit

389 Congress Street, 04101 Tel: (207) 874-8703, **Fax:** (207) 874-8716

Permit No:	Date Applied For:	CBL:
06-0026	01/06/2006	024 D001001

Location of Construction:	Owner Name:	Owner Address:	Phone:
160 FOX ST	PORMAN ONE 2004 LP	600 CONGRESS AVE STE 400	
Business Name:	**Contractor Name:**	**Contractor Address:**	**Phone**
	CM & B Construction Mgmt & Buil	6 Kimball Lane Lynnfield	(781) 246-9400
Lessee/Buyer's Name	**Phone:**	**Permit Type:**	
		Foundation Only/Commercial	

Proposed Use:	Proposed Project Description:
Commercial/ FOUNDATION ONLY-Whole Foods Market Connected w/ Permit #06-0027	FOUNDATION ONLY-Whole Foods Market

Dept: Fire **Status:** Approved with Conditions **Reviewer:** Cptn Greg Cass **Approval Date:** 01/16/2006

Note: **Ok to Issue:** ☑

1) Project requires State Fire Marshals approval.

2) All building construction top comply with NFPA 101

3) Fire Alarm system to comply with NFPA 72

4) Sprinkler system to comply with NFPA 13

Dept: Fire **Status:** Pending **Reviewer:** Cptn Greg Cass **Approval Date:**

Note: **Ok to Issue:** ☐

1) A fire protection plan needs to be submitted.
Showing All access and egress.
All fire hydrants within 1500 feet.
A Life safety plan will be required building permit approval

Dept: DRC **Status:** Approved with Conditions **Reviewer:** Rick Knowland **Approval Date:** 01/08/2005

Note: **Ok to Issue:** ☑

1) 1. See Planning Division conditions of approval.

Dept: Planning **Status:** Approved with Conditions **Reviewer:** Rick Knowland **Approval Date:** 02/08/2005

Note: **Ok to Issue:** ☑

Location of Construction:	Owner Name:	Owner Address:	Phone:
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Business Name:	**Contractor Name:**	**Contractor Address:**	**Phone**
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Lessee/Buyer's Name	**Phone:**	**Permit Type:**	
		Foundation Only/Commercial	

2.That all exterior signs shall be reviewed and approved by Planning Staff.

3. That a sewer capacity letter substantiating the ability to serve this project shall be obtained from Public Works.

4. That the design plans for all roadway and interesection improvements including the median barrier at the right-in right-out entrance on Somerset Street and the on-site raised island barrier at the Somerset Street driveway shall be reviewed and approved by City Staff.

5. Developer shall contribute $50,000 toward the cost of Marginal Way improvements and construct a median barrier at the right-in right-out entrance on Somerset Street.

6. Site plan shall be revised for planning staff review and approval reflecting a sidewalk along Franklin Street (within the public street right-of-way) that has a straight alignment rather than the curvilinear alignment shown on the plan. Brick sidewalks shall be installed along Franklin Street unless the City's sidewalk policy is changed to allow a different sidewalk material.

7. That a drainage maintenance agreement in the name of the new developer shall be submitted for City staff review and approval.

8. Prior to ordering and installation of the Holophane street light fixtures, Developer shall contact the the Planning Office (Rick Knowland, tel. No. 874-8725) to confirm the appropriate color of the light fixture and pole.

9. Trees within the public sidewalk along Somerset Street and Pearl Street shall be be planted in granite framed planters as proposed on the approved plan rather than tree grates.

10. That the public sidewalk along Fox/Somerset Street abutting the outside cafe shall be a minimum seven (7) feet wide at its closest point to the cafe unless otherwise approved in writing by the Planning Division to a lesser width.

11. Unless otherwise approved in writing by the City, the proposed building shall not be located within the City of Portland sewer easement adjacent to Franklin Street. In addition the proposed transformer shall not be located in the City street right-of-way nor the City sewer easemernt.

12. Applicant shall address all review comments of the Engineering Review Consultant which shall be submitted to Stephen Bushey for review and approval.

13. The footprint area of the phase 2 retail/office building shall be reserved for that purpose and shall not be used for parking.

14. An easement shall be reserved to the City for the storm drain that runs through the old rail easement on the site. Said easement shall be submitted for review and approval by Public Works. Contact Eric Labelle.

Comments:

1/6/2006-ldobson: Per Mike permit taken without funds. Lannie

General Building Permit Application

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Proposed Specific use: MERCANTILE

Project description:
CONSTRUCTION OF PROPOSED GROCERY STORE - FOUNDATIONS ONLY.

Contractor's name, address & telephone: CONSTRUCTION MANAGEMENT AND BUILDERS, INC.
Who should we contact when the permit is ready: JEFFREY A. ANDERSON
Mailing address: 6 KIMBALL LANE, LYNNFIELD, MA 01940
Phone: (781) 246-9400

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Signature of applicant: _____ Date: JAN. 5, 2006

This is not a permit; you may not commence ANY work until the permit is issued.

Applicant: PORTAN ONE 2004 LP Date: 1/19/06

Address: 160 Fox ST C-B-L: 024-D-001-2

CHECK-LIST AGAINST ZONING ORDINANCE

Date - Existing Bldgs on site to Be Demoed → see permit #06-0027

Zone Location - B-5 #06-0026

Interior or corner lot -

Proposed Use/Work - Foundation only for Wholefoods grocery Store

Sewage Disposal - City

Lot Street Frontage - No Min req

Front Yard - }
Rear Yard - } NONE req
Side Yard - }

Separate permits required for the separate Retail/office bldg

Projections -

Width of Lot - N/A

Height - 65' max - 40' Scaled

Lot Area - None req - No min

Lot Coverage/ Impervious Surface - 100% Allowed

Area per Family - N/A

Off-street Parking - None required per zone

Loading Bays - ≈ 46,000 ft retail - 2 required 3 loading Bays Shown

Site Plan - major # 2004-0225

Shoreland Zoning/ Stream Protection - N/A

Flood Plains - Panel 13 Zone C

Note: Project Approved prior to changes in Zoning requiring Parking to be 35' from St. lines — (went into effect 03/05

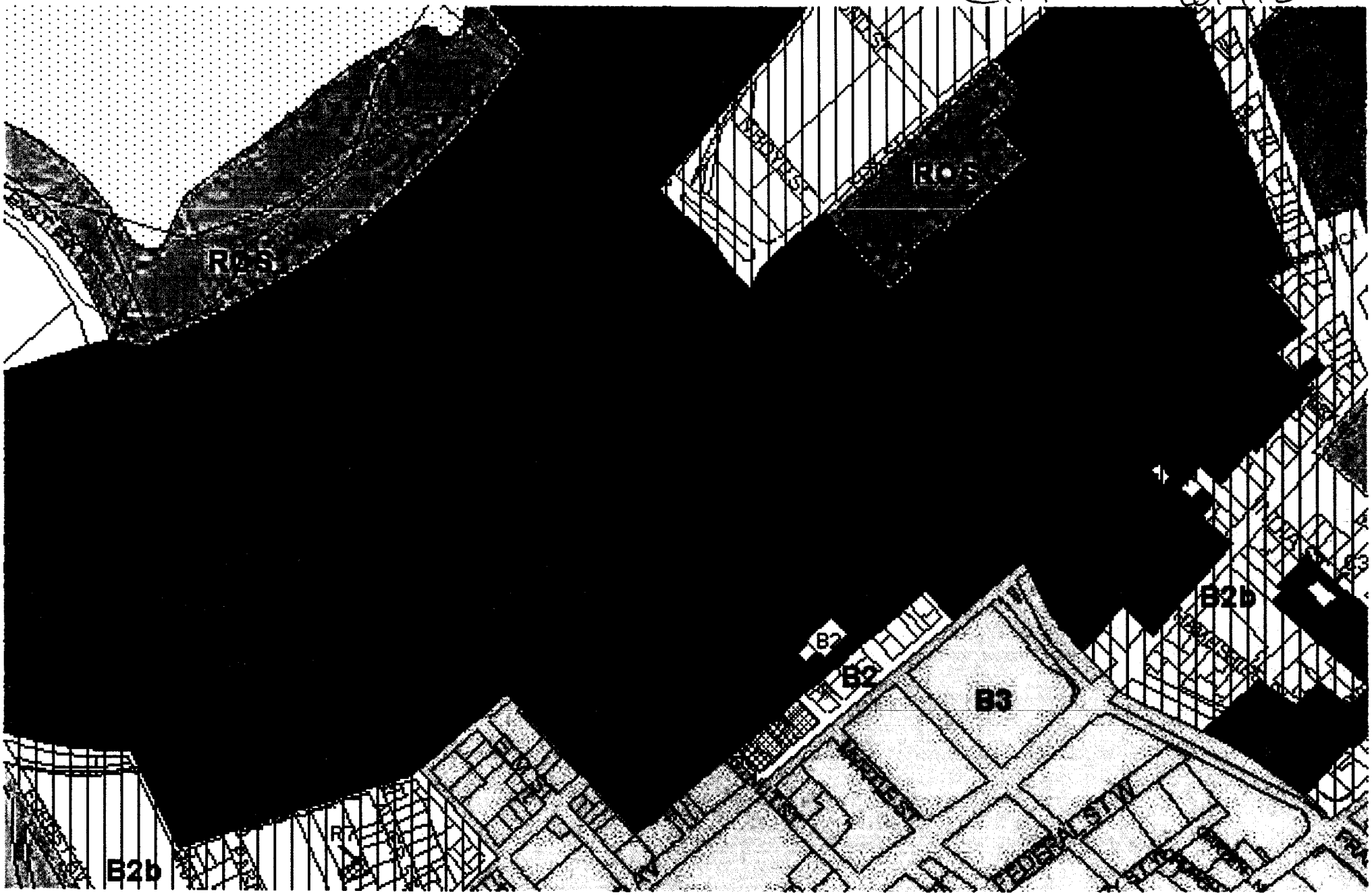


CHART #24 BLOCK D LOT 1+2
R-5
R-5
B2b
B2
B2
B3
B2b
B2b
WARREN ST
FEDERAL ST

Sebago Technics

Report on Subsurface and Foundation Investigation

Proposed Somerset Marketplace Portland, Maine

for

BL Companies
355 Research Parkway
Meriden, CT 06450

October 10, 2005

One Chabot Street, P.O. Box 1339, Westbrook, Maine 04098-1339 Ph. 207-856-0277 Fax 856-2206

Sebago Technics

sebagotechnics.com
One Chabot Street
P.O. Box 1339
Westbrook, Maine
04098-1339
Ph. 207-856-0277
Fax 856-2206

October 11, 2005
04187

Mr. Allan Rice
BL Companies
355 Research Parkway
Meriden. CT 06450

Report on Subsurface and Foundation Investigation
Proposed Somerset Marketplace, Portland, Maine

Dear Mr. Rice:

This report presents the results of our subsurface and foundation investigation for the proposed Somerset Marketplace in Portland, Maine.

In summary, it is our opinion that the proposed buildings may be supported on spread footings baring on improved fill or on compacted structural fill placed after excavation and removal of unsuitable soils. In addition, slabs-on-grade may be used for the lowest floor levels. Specific recommendations regarding foundation design and construction considerations are presented below.

Introduction

The approximately 4.5-acre parcel is located at the southwest corner of the intersection of Franklin Arterial with Fox/Somerset Street. The parcel is presently occupied by two buildings having plan areas of approximately 54,000 square feet for the east building and 59,000 square feet for the west building. We were not able to determine the foundation types for the existing buildings, but anticipate that they are supported on spread footings with slabs-on-grade . The remainder of the parcel is paved with bituminous concrete. site grades vary from approximately El. 8.0 to 10.0, and the lowest ground floors of the buildings are at El. 13.75. An abandoned railroad spur is located between the buildings.

Proposed Construction

The proposed new structures will be two- story buildings having plan areas of approximately 46,000 square feet and 8,000 square feet, respectively. The larger building will overlie approximately the northern half of the 54,000 square foot building and extend approximately 60 feet beyond the north side of the existing building and 20 feet beyond the east side. The lowest floor level is at El. 11.5. The portions of the new building beyond the limits of the existing structure will have a raise-in-grade of approximately 2.0 feet. The smaller building

will overlie a small portion of the north side of the existing 59,000 square foot building. The lowest floor is at El. 100.0. The portions of the new building beyond the limits of the existing structure will have a raise-in-grade of approximately 1.0 foot. We anticipate that the buildings will be steel framed with masonry and brick exterior walls. Site development will include access roads and paved parking.

Subsurface Explorations

2004 Borings

On May 26 and 27, 2004, Maine Test Borings, Inc. (MTB) drilled four borings, B1 to B4, at the four corners of the proposed larger building as shown on Sheet 1, Subsurface Exploration Plan. MTB drilled the borings to depths below ground surface varying from 32.0 feet to 82.0 feet. Sebago Technics, Inc. (Sebago Technics) monitored the test borings and prepared the logs included in Appendix A. Table I summarizes the results of test borings. MTB backfilled the borings with the drilled soil and placed a bituminous patch at the surface of B1 and B2.

Tho borings were drilled using 2.4-inch I.D. hollow stem augers. Field vane shear tests were conducted at various depths in the clay stratum. Soil samples were typically recovered at nominal 5 foot intervals. Standard Penetration Tests (SPT) were conducted in accordance with ASTM procedures.

Sebago Technics determined the locations of test borings by taping from the existing structure. We determined the ground surface elevations at test borings by survey methods.

2005 Borings

During September 26 to 28, 2005, MTB drilled four borings, B101 to B104, at locations shown on Sheet 1. MTB drilled the borings to depths below ground surface varying from 62.0 feet to 72.0 feet. Sebago Technics monitored the borings and prepared the logs included in Appendix B. MTB backfilled the borings with the drilled soil and placed a bituminous patch at the surface of all borings.

The borings were drilled using 2.5-inch I.D. hollow stem augers. Field vane shear tests were conducted at various depths in the clay stratum. Soil samples were typically recovered at nominal 5 foot intervals. Standard Penetration Tests (SPT) were conducted in accordance with ASTM procedures.

Sebago Technics determined the locations and ground surface elevations of borings by survey methods,

The test boring logs and related information depict subsurface conditions and water levels only at their specific locations at the time indicated on the logs. Soil conditions at other locations may differ from conditions at these locations. Also, the passage of time may result in a change in groundwater conditions at exploration locations.

Subsurface Conditions

The test borings encountered four principal soil units: fill, harbor bottom deposits, marine deposits, and glacial till deposits. Encountered thickness and generalized descriptions of the soil units are presented below in order of increasing depth below ground surface. Due to the complexity of the deposition process, strata thickness will vary and may be absent at specific locations.

Fill - Fill consists of loose to dense, brown to gray brown, well-graded SAND with gravel (SW); to silty SAND (SM) with various amounts of gravel, ash and brick. Encountered thickness varied from 6.0 feet to 11.5 feet.

Harbor Bottom Deposits - The harbor bottom deposits consist of loose to medium dense, gray silty SAND (SM); to soft, gray sandy SILT (ML) with shells. Encountered thickness varied from 2.8 feet to 9.0 feet.

Marine Deposit - The marine deposit consists of sand and clay. There is a marine sand sub-layer within the clay stratum. The upper portion of the clay stratum consists of soft to stiff, gray lean *CLAY* (CL), trace fine sand and frequent black streaks. Encountered thickness varied from 9.3 feet to 36.0feet. The upper marine sand sub-layer consists of very loose to medium dense, gray silty SAND (SM); to dense, gray well-graded SAND with gravel (SW). Encountered thickness varied from 2.1 feet to 12.0 feet. The lower portion of the clay stratum consists of soft to medium stiff, gray lean CLAY (CL). Field vane shear strength of the clay portions of the marine deposit varied from 220 pounds per square foot (psf) to 1,860 psf. A lower sand sub-layer, consisting of very loose to medium dense, gray silty SAND with gravel (SM) was *encountered* below the clay in borings B1 and *B3*. Encountered thickness varied from 2.8 feet to *5.0* feet.

Glacial Till Deposit - The glacial till consists of loose to medium dense, gray silty SAND with gravel (SM), with cobbles and boulders. Borings penetrated up to 12.5 feet into the glacial till.

Groundwater was encountered in the borings *at* depths below ground surface varying from 6.8 feet to *27.0* feet. Observations of water were made over a relatively short period of time and may not reflect the stabilized groundwater level. In addition, water levels will vary with precipitation, season, temperature and construction activity in the area. Therefore, water levels during and following construction will vary from those measured in the borings.

Engineering Properties of the Marine Clay

The stress history of the clay deposit, as developed from correlations with shear strength of similar clays in the area, is summarized on Figure 1. The undrained shear strength of *the* clay stratum was determined by field wane shear tests in the borings. Measured undrained shear strength varied from 220 psf to 1,860 psf. The stress history of the deposit was estimated by comparing *the* measured undrained shear strength with correlations for strength and stress history of clay from other projects with similar conditions

The stress-strain or compressibility characteristics (settlement) of clays are highly dependent upon their stress history. If clay is stressed within the limits of the maximum previous stress, σ_m, the strain (settlement) will be a function of the recompression ratio (RR) of the clay. If the applied stress exceeds the maximum previous stress, the strain will be proportional to the virgin compression ratio (CR). The compression ratio is typically 10 to 15 times the recompression ratio.

The stress history and appropriate compression ratios were estimated for the clay deposit as discussed above. The correlations indicate that the deposit is overconsolidated; that is, the existing overburden stress is less than the maximum previous stress. The deposit likely became overconsolidated due to desiccation (drying) resulting from a lowering of the groundwater level at some time in the geologic past which also increased the effective overburden stress throughout the stratum. A highly overconsolidated, surficial "crust" is present at the surface of the deposit. It is our opinion that the moderate overconsolidation below the crust is due to stress increases caused by the long term lowering of the groundwater level.

Recommendations for Foundation Design

Recommended Foundation Type and Design Criteria

The bituminous concrete, existing building foundations, and fill in its present condition are not considered suitable for support of the buildings. The borings indicate that the fill consists primarily of well-graded SAND with gravel and silt and silty SAND. In our opinion, the fill will provide adequate support for the foundations provided the fill is compacted by Intensive Surface Compaction (ISC) as described below Therefore, it is our opinion that the buildings may be supported on the improved fill or on compacted structural fill placed after removal of any unsuitable materials encountered (organics and disturbed soil'). All previous construction (buildings, foundations and underground utilities) should be removed from within the limits of the buildings prior to ISC.

We recommend that, for uniformity, the footings be proportioned €or an allowable bearing stress in pounds per square feet (psf), equal to 1,000 multiplied by the least lateral dimension of the footing in feel, up to a maximum of 3,000 psf. All footings should be at least 1.5 feet wide,

Exterior footings should be founded at least 4.5 feet below the lowest adjacent ground surface exposed to freezing. Interior footings should be founded a minimum of 1.5 feet below the ground floor slab.

Compacted structural fill supporting footings should extend laterally from the footings to at least the limits defined by 1 horizontal to 1 vertical lines sloped outward and downward from points located at least 2 feet horizontally beyond the bottom edges of the footings.

In order to consider foundations bearing above the clay stratum, we estimated the settlement of the clay resulting from the increased stress from the raise-in-grade beyond the existing buildings and building loads. We estimate that the total settlement will be on the order of 1.5 inches, with differential settlement on the order of 0.75 inch in 40 feet. We anticipate that settlement of this magnitude is acceptable. However, final acceptability of settlement should be determined by the structural engineer.

Intensive surface compaction should be performed using a minimum 30,000 pound vibratory roller operating at 30 cycles per second (Hz) and a forward speed of 1 to 2 feet per second. Compaction should consist of 10 coverages of the vibratory roller. The direction of each two successive coverages should be rotated perpendicular to the previous two coverages. Following intensive surface compaction, a minimum of two coverages of the roller should be applied without vibration to recompact the upper portion of the fill. Fill containing debris and wood and organics should be removed and replaced with structural fill prior to surface compaction. Any soft or unsuitable areas encountered should be excavated and replaced with compacted structural fill. After intensive surface compaction, the site can be refilled to slab subgrade.

Ground Floor Slab

We recommend that the lowest level floor slabs be designed as earth-supported slabs-on-grade baring on a minimum 6-inch thickness of compacted structural fill. All previous construction and fill containing debris should be removed from within the building limits prior to placing fill. All fill placed below the floor slabs for raises-in-grade should consist of compacted structural fill. Normal dampproofing and vapor barrier should be provided for the lowest level slab and walls.

Seismic Design Considerations

We recommend that the buildings are designed in accordance with the seismic requirements of the latest edition of the International Building Code. The site classification is Class E, the site response coefficient F_a is 2.1 for the short period spectral response acceleration S_s of 0.375g; the site response coefficient F_v is 3.5 for the 1-second period spectral response acceleration S_1 of 0.10g.

Lateral Foundation Loads

We recommend that lateral loads be resisted by bottom friction on footings and that a coefficient of friction equal to 0.35 be used for footings. If this does not provide sufficient lateral resistance, we will consider the problem in more detail to take into account other factors.

Backfill Materials

Structural fill used below foundations and floor slabs and for backfill adjacent to walls should consist of sandy gravel to gravelly sand. It should be free of organic material, loam, trash, snow, ice, Frozen soil and other objectionable material, and should conform to the following gradation:

Sieve Size	Percent Finer by Weight
6 inches	100
No. 4	30 to 90
No. 40	10 to 50
No. 200	0 to 8

Compacted structural fill should be placed in layers not exceeding eight inches in loose measure and compacted by self-propelled vibratory equipment at the approximate optimum moisture content to a dry density of at least 95 percent of the maximum dry density, as determined in accordance with ASTM Test Designation D1557. In confined areas, the maximum particle size should be reduced to 3 inches and the loose layer thickness should be reduced to 6 inches and compaction performed by hand-guided vibratory equipment.

Compacted structural fill on the outside of the foundation should extend laterally a minimum of 2 feet from the wall. Backfill beyond this limit may consist of common fill. The top 12 inches of fill on the exterior of the building should consist of low permeability material or bituminous concrete pavement to minimize water infiltration next to the building. Grading should provide for *runoff* away from the building.

Common fill may consist of inorganic mineral *soil* that can be placed in layers and compacted. Common fill should be placed and spread in layers *not* exceeding 12 inches in thickness and compacted with a minimum of two systematic passes of the equipment placing the fill.

Pavement Section

We recommend the following pavement sections:

Standard-Duty and Heavy-Duty Flexible (Asphalt) Pavement:

Standard Duty Pavement
3 inches bituminous concrete placed in two layers
4 inches gravel base course
12 inches sand or gravel subbase course

Heavy-Duty Pavement
4 inches bituminous concrete placed in two layers
4 inches gravel base course
12 inches sand or gravel subbase course

Heavy-Dun Rigid Pavement (Concrete)

6 inches Portland Cement Concrete
4 inches gravel base course
12 inches sand or gravel subbase course

Base and subbase course materials should conform to the following gradations:

Base Course

Screened or Crushed Gravel (Maine DOT Standard Specification, Highways and Bridges; Section 703.06a, Type A)

Sieve Size	Percent Finer by Weight
2 inches	100
1/2 inch	45 to 70
1/4 inch	30 to 55
No. 40	0 to 20
No. 200	0 to 5

Subbase Course

Sand or Gravel (Maine DOT: Section 703.06b, Type D)

Sieve Size	Percent Finer by Weight
4 inches	100
1/4 inch	25 to 70
No. 40	0 to 30
No. 200	0 to 7

(Note: Type D aggregate should be modified to a maximum 4 *in.* size.)

All fill containing debris should be removed from within the limits of pavement. Subbase course material should be placed in maximum 8-inch thick loose lifts and compacted at approximately optimum moisture content to a *dry* density of at least *95* percent of maximum dry density, as determined in accordance with ASTM Test Designation D1557. Base course material should be placed in one lift and compacted with a minimum of two coverages with self-propelled vibratory compaction equipment. Existing foundations below the parking area should be removed *to* at least 2.5 feet below the pavement.

Construction Considerations

General

The primary purpose of this section of the report is *to* comment on items related to excavation, earthwork and related geotechnical aspects of proposed construction, It is written primarily for the engineer having responsibility for preparation *of* plans and specifications. Since it identifies potential construction problems related to foundations and earthwork, it will also aid personnel who monitor the construction activity, Prospective contractors for this project must evaluate the construction problems on the basis of their own knowledge and experience in the Portland, Maine area, and on the basis of similar projects in other localities, taking into account their proposed construction methods, procedures, equipment and personnel.

Excavation, Lateral Support and Control of Water

We anticipate that foundation excavation can be accomplished with sloped open excavation through the overburden soils provided safe side slopes can be maintained. Some sloughing and raveling should be anticipated in temporary slopes. Existing foundations within the limits of proposed foundations and floor slabs should be completely removed and the excavation to bearing level backfilled with compacted structural fill or crushed stone, as appropriate. Existing foundations below the parking area should be removed to at least 2.5 feet below the pavement. Temporary excavations should be made in accordance with all OSHA and other applicable regulatory agency requirements.

We anticipate that groundwater may be encountered at proposed subgrade level or bearing level of footings. If encountered, open pumping from sumps can likely control groundwater. In general, the contractor should control groundwater and water from runoff and other sources by methods which prevent disturbance of bearing surfaces or adjacent soils and allow construction in-the-dry.

Subgrade Preparation

The subgrade soil is susceptible to disturbance from construction traffic. Equipment and personnel should not be permitted to travel across exposed footing bearing surfaces or exposed slab subgrades. Any subgrade areas that are disturbed should be recompacted or excavated and replaced with compacted structural fill prior to placing of concrete. Subgrades should be protected against freezing temperatures if exposed during construction. Final excavation to subgrade should be performed using equipment with smooth-edge buckets.

Construction Monitoring

The Foundation recommendations contained herein are based on the known and predictable behavior of a properly engineered and constructed foundation. Monitoring of the foundation construction is required to enable the geotechnical engineer to keep in contact with procedures and techniques used in construction. Therefore, we recommend that a person qualified by training and experience be present to provide monitoring at the site during Intensive Surface Compaction, preparation of foundation bearing surfaces, and placement of compacted structural fill.

Limitations of Recommendations

This report has been prepared *for* specific application to the subject project in accordance with generally accepted geotechnical engineering practices, In the event that any changes in the nature, design or location of the buildings are planned, the conclusions and recommendations contained in this report should not be considered valid, unless the changes are reviewed and the conclusions of this report modified or verified in writing.

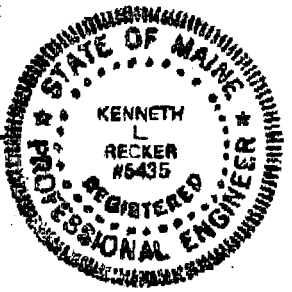
The recommendations presented herein are based in part on the data obtained from the referenced test borings. The nature and extent of variations between the explorations may not become evident until construction. If variations then appear evident, it will be necessary to re-evaluate the recommendations of this report.

We request that we be provided the opportunity for a general review of final design and specifications in order to determine that our earthwork and foundation recommendations have been interpreted and implemented in the design and specifications as they were intended.

It has been a pleasure to work with you on this project. Please do not hesitate to contact us if you have any questions or need additional information.

Sincerely,

SEBAGO TECHNICS, INC



Kenneth L. Recker, P.E.
Geotechnical Engineering Manager

KLR:klr/jc
Enclosures:

Table I	- Summary of Borings
Sheet 1	- Subsurface Exploration Plan
Figure 1	- Stress History
Appendix A	- Logs of 2004 Borings
Appendix B	- Logs of 2005 Borings

TABLE I
SUMMARY OF BORINGS
PROPOSED SOMERSET MARKETPLACE
FRANKLIN ARTERIAL AND FOX STREET
PORTLAND, MAINE

Boring Number	Depth (Ft)	Ground Surface El. (Ft)	Depth to Water (Ft)	Strata Thickness (Ft)						
				Fill	Harbor Bottom	Marine Clay	Marine Sand	Marine Clay	Marine Sand	Glacial Till
B101	72.0	9.1	6.9	10.5	3.5	14.0	7.5	25.9	--	10.6*
B102	67.0	9.0	7.0	10.2	2.8	36.0	1.5	13.5	--	3.0*
B103	67.0	9.0	9.5	7.5	5.5	45.0	--	--	--	9.0*
B104	62.0	9.0	7.5	8.0	7.1	17.9	2.1	23.9	--	3.0*
B1	62.0	8.2	NE	8.0	8.2	9.3	5.0	19.7	2.8	9.0*
B2	32.0	8.8	27.0	6.0	9.0	13.0	4.0*	--	--	--
B3	62.0	8.0	6.8	9.0	6.0	10.0	3.0	27.0	5.0	2.0*
B4	82.0	8.3	NE	11.5	4.0	13.5	12.0	28.5	--	12.5*

NOTES:

1. ELEVATIONS REFERENCED TO NATIONAL GEODETIC VERTICAL DATUM (NGVD) 1929.
2. NE INDICATES GROUNDWATER NOT OBSERVED WITHIN DEPTH OF BORING.
3. -- INDICATED STRATUM NOT ENCOUNTERED WITHIN DEPTH OF BORING.
4. * INDICATES DEPTH OF PENETRATION INTO STRATUM.

WHOLE FOODS
MARKET

PROPOSED GROCERY STORE
FRANKLIN ST. ARTERIAL,
FOX/SOMERSET ST., PEARL ST.
PORTLAND, CUMBERLAND COUNTY, MAINE

DEMOLITION AND FOUNDATION PERMIT SET

ISSUE DATE: 16 DECEMBER 2005

TENANT

WHOLE FOODS MARKET
125 CAMBRIDGEPARK DRIVE
CAMBRIDGE, MA 02140
(617) 492-5500

ARCHITECT / ENGINEER

BL Companies
355 Research Parkway
Meriden, CT 06450
(203) 630-1406

DESIGN ARCHITECT

Bottino Grund Architects, LLP
1412 West 6th Street
Austin, TX 78703
(512) 322-0055

CIVIL ENGINEER

Sebago Technics
One Chabot Street
Westbrook, Maine 04098
(207) 856-0277

REFRIGERATION ENGINEER

Vision Engineering
27 Baron Road
Franklin, MA. 02038
(508) 528-3110

SYMBOLS

Symbol	Description
X/X	WALL/BUILDING SECTION
X/X	DETAIL
XXX	COLUMN LINE NUMBER
X	INTERIOR ELEVATION
RMNAME XXX	ROOM NUMBER
X	WINDOW NUMBER
XXX	DOOR NUMBER
X	PARTITION TYPE
XXX / 0'-0"	ELEVATION
XX-X	FINISH MATERIAL
XX-X	EQUIPMENT TAG

MATERIALS

- CONCRETE BLOCK
- METAL
- GYPSUM DRYWALL
- RIGID INSULATION
- CONCRETE
- BATT INSULATION
- SAWN LUMBER
- BLOCKING
- SEALANT AND BACKER ROD
- BRICK
- PLYWOOD
- FINISHED WOOD
- LAMINATED VENEER LUMBER

ABBREVIATIONS

THE FOLLOWING ABBREVIATIONS ARE USED TYPICALLY ON THE CONSTRUCTION DOCUMENTS IF ABBREVIATIONS ARE USED THAT ARE NOT ON THIS LIST, IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO CONFIRM THEIR DEFINITION PRIOR TO SIGNING THE CONTRACT.

AFF	ABOVE FINISH FLOOR
AP	ACCESS PANEL
ACT	ACOUSTICAL CEILING TILE
ADJ	ADJUST, ADJUSTABLE
AC	AIR CONDITIONING
AHU	AIR HANDLING UNIT
AL,ALUM	ALUMINUM
APPROX	APPROXIMATE
ARCH	ARCHITECT(URAL)
	AT
BLKG	BLOCKING
BD	BOARD
BLDG	BUILDING
CAB	CABINET
CG	CORNER GUARD
CPT	CARPET
CLG	CEILING
CL	CLEAR, CENTERLINE
CT	CERAMIC TILE
CLOS	CLOSET
CJ	CONTROL JOINT
COL	COLUMN
CONC	CONCRETE
CMU	CONCRETE MASONRY UNIT
CONST	CONSTRUCTION
CONT	CONTINUOUS
CO	CLEAN OUT
CORR	CORRUGATED
DBL	DOUBLE
DTL	DETAIL
DIAG	DIAGONAL
DIAM,D	DIAMETER
DIM	DIMENSION
DN	DOWN
DWG	DRAWING
DF	DRINKING FOUNTAIN
DS	DOWN SPOUT
(E)	EXISTING
E	EAST
EA	EACH
EIFS	EXTERIOR INSULATING FINISH SYSTEM
ELEC	ELECTRICAL
ELEV,EL	ELEVATION, ELEVATOR
EMER	EMERGENCY
EQ	EQUIVALENT
EQUIP	EQUIPMENT
EXH	EXHAUST
EXIST	EXISTING
EJ	EXPANSION JOINT
EXP	EXPANSION
EXT	EXTERIOR
FD	FLOOR DRAIN
FT	FEET
FF	FINISH FLOOR
FIN	FINISH
FE	FIRE EXTINGUISHER
FEC	FIRE EXTINGUISHER CABINET
FHC	FIRE HOSE CABINET
FP	FIREPROOF
FL	FLOOR, FLUSH
FLUOR	FLUORESCENT
FTG	FOOTING
FDN	FOUNDATION
GA	GAUGE
GALV	GALVANIZED
GC	GENERAL CONTRACTOR
GFRC	GLASS FIBER REINFORCED CONCRETE
GL	GLASS
GWB,GYP	GYPSUM WALLBOARD
HC	HANDICAPPED
HDW	HARDWARE
HDWD	HARDWOOD
HTG	HEATING
HVAC	HEATING, VENTILATING & AIR CONDITIONING
HT	HEIGHT
HM	HOLLOW METAL
HOR	HORIZONTAL
IN	INCHES
INCL	INCLUDE, INCLUDING
ID	INSIDE DIAMETER
INS	INSULATION
INT	INTERIOR
INV	INVERT
JC	JANITORS CLOSET
JT	JOINT
JB	JUNCTION BOX
KO	KNOCK OUT
LBL	LABEL
LAM	LAMINATE
LAV	LAVATORY
LB	POUND
LH	LEFT HAND
LTG	LIGHTING
MH	MANHOLE
MAX	MAXIMUM
MECH	MECHANICAL
MET	METAL
MIN	MINIMUM
MISC	MISCELLANEOUS
MO	MASONRY OPENING
NOM	NOMINAL
N	NORTH
NIC	NOT IN CONTRACT
NTS	NOT TO SCALE
OC	ON-CENTER
OPNG	OPENING
OPP	OPPOSITE
OD	OUTSIDE DIAMETER
OA	OVERALL
OH	OVERHEAD
PT	PAINT, PRESSURE TREATED
PR	PAIR
PERF	PERFORATED
PLAS	PLASTIC, PLASTER
PLAM	PLASTIC LAMINATE
PWD	PLYWOOD
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
QT	QUARRY TILE
RAD	RADIATOR, RADIATION
R	RADIUS, RISER
RCP	REFLECTED CEILING PLAN
REQD	REQUIRED
S	SOUTH
SCHED	SCHEDULE
SD	STORM DRAIN
SLNT	SEALANT
SHT	SHEET
SIM	SIMILAR
SF	SQUARE FEET
SI	SQUARE INCHES
SQ	SQUARE
SS	STAINLESS STEEL
SSF	STAINLESS STEEL FRAME
TEL	TELEPHONE
TEMP	TEMPORARY
T&G	TONGUE & GROOVE
TOC	TOP OF CONCRETE
TOF	TOP OF FOOTING
TOW	TOP OF WALK
TYP	TYPICAL
UON	UNLESS OTHERWISE NOTED
UL	UNDERWRITERS' LABORATORIES, INC.
UTIL	UTILITY
VB	VAPOR BARRIER, VINYL BASE
VWC	VINYL WALL COVERING
VENT	VENTILATION
VERT	VERTICAL
VCT	VINYL COMPOSITION TILE
W	WEST
WC	WALL COVERING
WSCT	WAINSCOT
WT	WEIGHT
WIN	WINDOW
W/	WITH
W/O	WITHOUT
WD	WOOD

DRAWING LIST

CIVIL

- C-1 EXISTING CONDITIONS PLAN
- C-2 SITE DEMOLITION PLAN
- C-3 SITE PLAN
- C-4 GRADING AND UTILITY PLAN
- C-5 EROSION AND SEDIMENTATION CONTROL PLAN
- C-6 LANDSCAPE PLAN
- C-7 DETAILS
- C-8 DETAILS

ARCHITECTURAL

- A0.00 COVER SHEET
- A0.01 GENERAL INFORMATION
- A0.02 CODE ANALYSIS AND EGRESS PLANS
- A1.01 FIRST FLOOR PLAN
- A1.02 MEZZANINE FLOOR PLAN
- A1.11 FIRST FLOOR PLAN - EQUIPMENT
- A1.21 MEZZANINE FLOOR PLAN - EQUIPMENT
- A2.01 EXTERIOR ELEVATIONS
- A3.01 BUILDING SECTIONS
- A3.02 BUILDING SECTIONS

STRUCTURAL

- S1.00 COLUMN SCHEDULE AND GENERAL NOTES
- S1.01 FOUNDATION PLAN
- S2.01 STRUCTURAL DETAILS 1
- S2.02 STRUCTURAL DETAILS 2

BL Companies
ARCHITECTURE
ENGINEERING
PLANNING
LANDSCAPE ARCHITECTURE
LAND SURVEYING
ENVIRONMENTAL SCIENCE
ANALYTICAL SERVICES

WHOLE FOODS

PROPOSED GROCERY STORE
FRANKLIN ST. ARTERIAL, FOX/SOMERSET ST., PEARL ST.
PORTLAND, CUMBERLAND COUNTY, MAINE

REFERENCE DRAWING ONLY
NOT FOR CONSTRUCTION

ARCHITECT/ENGINEER SEAL

Designed	D.B.
Drawn	J.P.
Checked	A.R.
Approved	D.B.
Scale	AS NOTED
Project No.	[illegible]
Date	12-16-05
CAD File	[illegible]

Title
GENERAL INFORMATION

Sheet No.
A0.01

BUILDING CODE

APPLICABLE CODES AND REGULATIONS

THE FOLLOWING CODES AND REGULATIONS ARE CURRENTLY ADOPTED BY THE STATE OF MAINE AT THE TIME OF THE BUILDING PERMIT FOR THIS FACILITY:

- MAINE MODEL BUILDING CODE
- 2003 INTERNATIONAL BUILDING CODE
- 2003 UNIFORM PLUMBING CODE/MAINE INTERNAL PLUMBING CODE
- 2002 NATIONAL ELECTRICAL CODE
- 2003 INTERNATIONAL ENERGY CONSERVATION CODE
- NFPA 1, 2003 UNIFORM FIRE CODE
- ASHRAE STANDARD 90.1 - 2001

USE GROUP CLASSIFICATION — M MERCANTILE OCCUPANCY, SECTION 309.1

CONSTRUCTION CLASSIFICATION — IIB NON-COMBUSTIBLE, SECTION 602.2

SEISMIC DESIGN DATA: — SEISMIC USE GROUP: I; SEISMIC PERFORMANCE CATEGORY: D

SEE STRUCTURAL DRAWINGS FOR ADDITIONAL SEISMIC DESIGN DATA

FIRE SUPPRESSION SYSTEM — BUILDING SHALL BE FULLY SPRINKLERED, SECTION 903.2.6 & 903.3.1.1

BUILDING HEIGHT

ALLOWABLE:
- TABLE 503 — 4 STORIES (55 FEET)
- SECTION 504.2 — 1 STORY / 20 FT. ALLOWABLE INCREASE FOR SPRINKLERS

ACTUAL:
- 1 STORY - 45'-8"
- MEZZANINE LEVEL NOT A STORY PER 502.1 and 505.1
- MEZZANINE AREA = 9,804 S.F.
- MAIN LEVEL AREA = 46,256 S.F.
- 9,804 / 46,256 = 21.2%
- MEZZANINE = LESS THAN 33% OF THE MAIN FLOOR AREA

BUILDING AREA

ALLOWABLE:
- TABLE 503 — 12,500 S.F.
- SECTION 507.2 — AREA NOT LIMITED; 1 STORY GROUP M BUILDING WITH AUTOMATIC SPRINKLER SYSTEM PROVIDED THROUGHOUT IN ACCORDANCE WITH 903.3.1.1

ACTUAL:
- MAIN FLOOR = 46,256 S.F.
- MEZZANINE = 9,804 S.F.
- TOTAL AREA = 56,060 S.F.

MEZZANINES

AREA LIMITATION (505.2)
- ALLOWABLE: 15,418 SF (1/3 AREA OF ROOM THAT IT SERVES)
- PROPOSED: 9,804 SF

EGRESS (505.3)
- TWO MEANS OF EGRESS REQUIRED, TWO PROVIDED.

OPENNESS (505.4) (NOT REQUIRED)
- EXCEPTION 2 - A MEZZANINE HAVING TWO OR MORE MEANS OF EGRESS IS NOT REQUIRED TO BE OPEN TO THE ROOM IN WHICH ITS LOCATED, IF AT LEAST ONE OF THE MEANS OF EGRESS PROVIDES DIRECT ACCESS TO AN EXIT FROM THE MEZZANINE LEVEL.

BUILDING SQUARE FOOTAGE CALCULATIONS

FIRST FLOOR	AREA
SALES AREA	31,281 S.F.
SEATING AREA	1,547 S.F.
KITCHEN AREA	1,680 S.F.
PREP/STORAGE AREAS	11,748 S.F.
TOTAL - FIRST FLOOR	46,256 S.F.

MEZZANINE	AREA
OFFICE AREAS	4,117 S.F.
PREP/STORAGE AREAS	5,681 S.F.
TOTAL - FIRST FLOOR	9,798 S.F.

OCCUPANT LOAD (RELATED TO EXIT CAPACITY) TABLE 1004.1.2

FIRST FLOOR	AREA	MULTIPLIER	OCCUPANTS
SALES AREA	31,281 S.F.	30 GROSS	1,042
SEATING AREA	1,547 S.F.	15 NET	145 SEATS (ACTUAL)
KITCHEN AREA	1,680 S.F.	200 GROSS	9
PREP/STORAGE AREAS	11,764 S.F.	300 GROSS	39
SUBTOTAL			1,235

MEZZANINE	AREA	MULTIPLIER	OCCUPANTS
OFFICE AREAS	4,117 S.F.	100 GROSS	42
PREP/STORAGE AREAS	5,681 S.F.	300 GROSS	19
SUBTOTAL			61
TOTAL BUILDING OCCUPANCY			1,296

CAPACITY OF EGRESS COMPONENTS TABLE 1005.1

- STAIRWAYS: .2 INCHES / OCCUPANT (WITH SPRINKLER SYSTEM)
- OTHER COMPONENTS: .15 INCHES / OCCUPANT (WITH SPRINKLER SYSTEM)

LOCATION	OCCUPANCY	WIDTH PER OCCUPANT	WIDTH REQUIRED	WIDTH PROVIDED
MEZZANINE	61	.20"	12.2"	88"
FIRST FLOOR	1,235	.15"	185.3"	552"
BUILDING TOTAL	1,296	.15"	194.4"	636"

NUMBER OF EXITS

MINIMUM # OF EXITS (TABLE 1019.1)

LOCATION	OCCUPANCY	EXITS REQUIRED	EXITS PROVIDED
MEZZANINE	61	2	3 - LOCATIONS
FIRST FLOOR	1,235	4	4 - LOCATIONS
BUILDING TOTAL	1,296	4	4 - LOCATIONS

MEANS OF EGRESS MISCELLANEOUS

GENERAL LIMITATIONS (1003.0)	SEE DRAWINGS
MEANS OF EGRESS ILLUMINATION (1006.0)	SEE DRAWINGS
EXIT SIGNS (1011.0)	SEE DRAWINGS
EXIT ACCESS (1013.0)	SEE DRAWINGS
COMMON PATH OF TRAVEL (1013.3)	75-FT MAX. ALLOWED; 70-FT MAX. PROVIDED
EXIT ACCESS ARRANGEMENT (1014.2.1) DISTANCE OF AT LEAST 1/3 OF OVERALL DIAGONAL OF AREA SERVED	105-FT MIN. REQUIRED; 165-FT MIN. PROVIDED
EXIT ACCESS TRAVEL DISTANCE (1015.0)	250-FT MAX. (W/SPRINKLERS); 245-FT MAX. PROVIDED
CORRIDOR FIRE RATING (TABLE 1016.1)	0 HR. (W/SPRINKLERS)
DEAD END CORRIDORS (1016.3)	20-FT MAX. ALLOWED; 10-FT MAX. PROVIDED
VERTICAL ENCLOSURE FIRE RATING (1019.0)	1 HR. (LESS THAN 4 STORIES)

BUILDING CODE

RATING OF CONSTRUCTION ELEMENTS TABLE 601 (TYPE IIB CONSTRUCTION)

THE FOLLOWING LIST OF STRUCTURAL ELEMENTS AND THEIR RESPECTIVE FIRE RATINGS TAKES PRECEDENCE OVER ALL DETAILS IN THE DRAWINGS AND ALL REQUIREMENTS OF THE SPECIFICATIONS. IF A CONFLICT EXISTS BETWEEN THIS LIST, THE DRAWINGS AND/OR THE SPECIFICATIONS, THE CONTRACTOR SHALL CALL IT TO THE ATTENTION OF THE ARCHITECT. IF THE CONTRACTOR HAS ANY QUESTIONS REGARDING ANY RATINGS, THEY SHALL REQUEST A CLARIFICATION BEFORE BEGINNING THE WORK.

STRUCTURAL ELEMENT	FIRE RATING
STRUCTURAL FRAME: INCLUDING COLUMNS, GIRDERS, TRUSSES	0 HR
BEARING WALLS: EXTERIOR	0 HR
EXTERIOR (BASED ON FIRE SEPARATION DISTANCE - TABLE 602)	0 HR
INTERIOR	0 HR
NON-BEARING WALLS AND PARTITIONS: EXTERIOR (BASED ON FIRE SEPARATION DISTANCE - TABLE 602)	0 HR
INTERIOR	0 HR
FLOOR CONSTRUCTION: INCLUDING SUPPORTING BEAMS AND JOISTS	0 HR
ROOF CONSTRUCTION: INCLUDING SUPPORTING BEAMS AND JOISTS	0 HR

FIRE RESISTANCE RATINGS MISCELLANEOUS

ELEVATOR SHAFT ENCLOSURE (707.4)	1 HR.
ELEVATOR MACHINE ROOM (3006.4)	1 HR.
STAIR ENCLOSURES (1019.1)	1 HR.

PLUMBING FIXTURES UNIFORM PLUMBING CODE TABLE 4-1

WATER CLOSETS:

GENDER	POPULATION	REQUIRED	ACTUAL
SEATING (RESTAURANT) - 145 SEATS			
FEMALE	73	2	3
MALE	73	2 + 1 URINAL	2 + 1 URINAL
OFFICES (EMPLOYEES - PUBLIC BUILDING) - 61 OCCUPANTS			
FEMALE	31	3	3
MALE	31	2 + 1 URINALS	2 + 1 URINAL

LAVATORIES:

GENDER	POPULATION	REQUIRED	ACTUAL
SEATING (RESTAURANT) - 145 SEATS			
FEMALE	73	1	2
MALE	73	1	2
OFFICES (EMPLOYEES - PUBLIC BUILDING) - 61 OCCUPANTS			
FEMALE	31	1	2
MALE	31	1	2

DRINKING FOUNTAINS:

	POPULATION	REQUIRED	ACTUAL
SEATING (RESTAURANT) - 161 SEATS	145	0	2
OFFICES (EMPLOYEES - PUBLIC BUILDING) - 61 OCCUPANTS	61	0	0

ZONING INFORMATION

SIGNAGE (14-369-20)

BUILDING SIGNS

NUMBER OF BUILDING SIGNS PERMITTED PER LOT:	1 PER FACADE PLUS 1
NUMBER OF BUILDING SIGNS PROVIDED:	1 PER FACADE (4 TOTAL)
MAXIMUM SIZE OF BUILDING SIGNS:	2 S.F. PER LINEAR FOOT OF BUILDING FACADE

NORTH FACADE:
288 LINEAR FEET x 2 = 576 S.F. MAXIMUM BUILDING SIGN
BUILDING SIGN PROVIDED = 175 S.F.

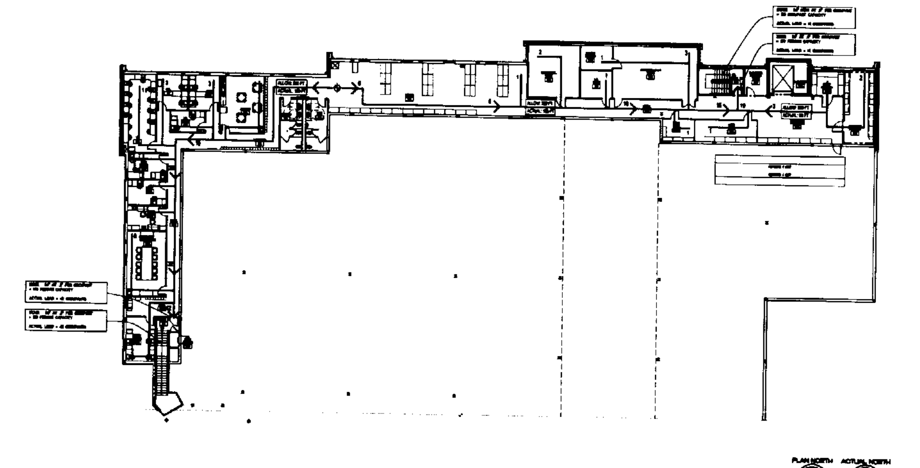
SOUTH FACADE:
288 LINEAR FEET x 2 = 576 S.F. MAXIMUM BUILDING SIGN
BUILDING SIGN PROVIDED = 175 S.F.

EAST FACADE:
196 LINEAR FEET x 2 = 392 S.F. MAXIMUM BUILDING SIGN
BUILDING SIGN PROVIDED = 175 S.F.

WEST FACADE:
196 LINEAR FEET x 2 = 392 S.F. MAXIMUM BUILDING SIGN
BUILDING SIGN PROVIDED = 175 S.F.

FREESTANDING SIGNS

NUMBER OF SIGNS PERMITTED PER ABUTTING STREET:	1 SIGN
NUMBER OF FREESTANDING SIGNS PROVIDED:	2 SIGNS
MAXIMUM SIZE OF FREESTANDING SIGNS:	16 S.F. MAXIMUM; 6-FEET HIGH MAXIMUM

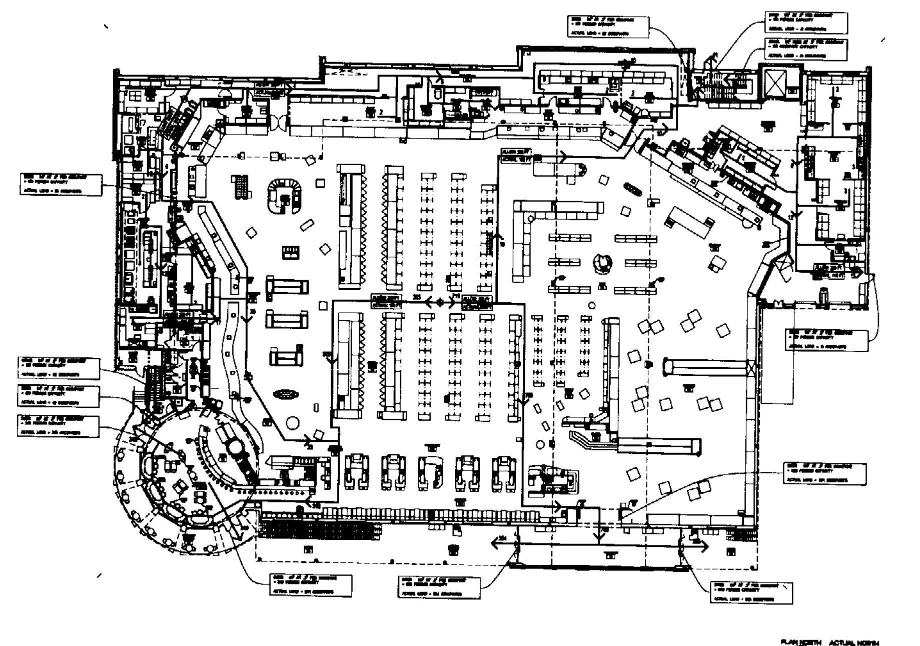


2 MEZZANINE EGRESS PLAN

FIRE RATED PARTITION LEGEND

1 HR. RATED PARTITION

SEE SHEET A0-01 AND A0-03 FOR ADDITIONAL FIRE PROTECTION INFORMATION



1 FIRST FLOOR EGRESS PLAN

BL Companies
ARCHITECTURE
ENGINEERING
PLANNING
LANDSCAPE ARCHITECTURE
LAND SURVEYING
ENVIRONMENTAL SCIENCES
ANALYTICAL SERVICES

WHOLE FOODS

PROPOSED GROCERY STORE
FRANKLIN ST. ARTERIAL, FOX/SOMERSET ST., PEARL ST.
PORTLAND, CUMBERLAND COUNTY, MAINE

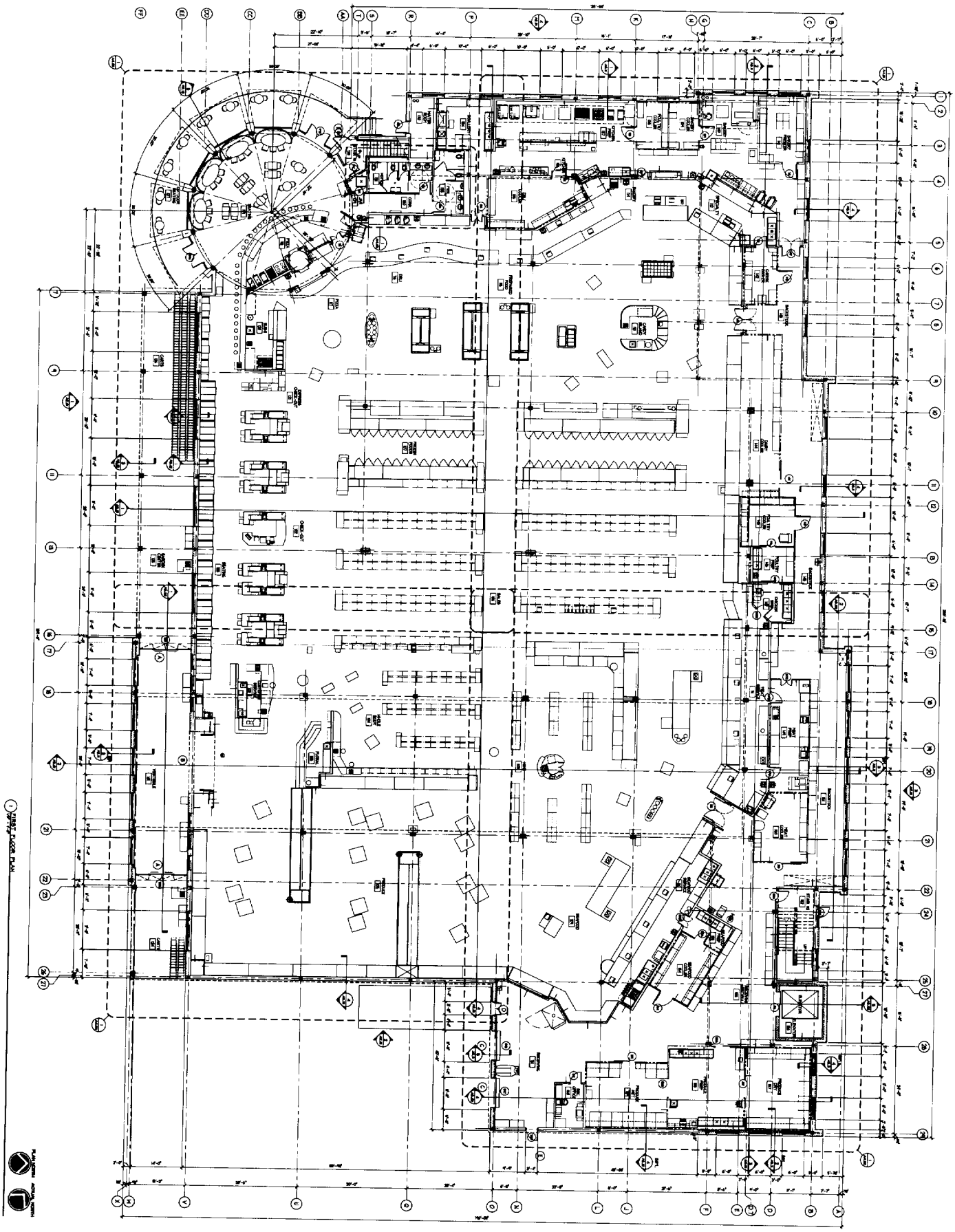
REFERENCE DRAWING ONLY
NOT FOR CONSTRUCTION

ARCHITECT/ENGINEER SEAL

Designed	D.B.
Drawn	J.P.
Checked	A.R.
Approved	B.B.
Scale	AS NOTED
Project No.	04C0829
Date	12-16-05
CAD File	A04C082901

Title
CODE ANALYSIS AND EGRESS PLANS

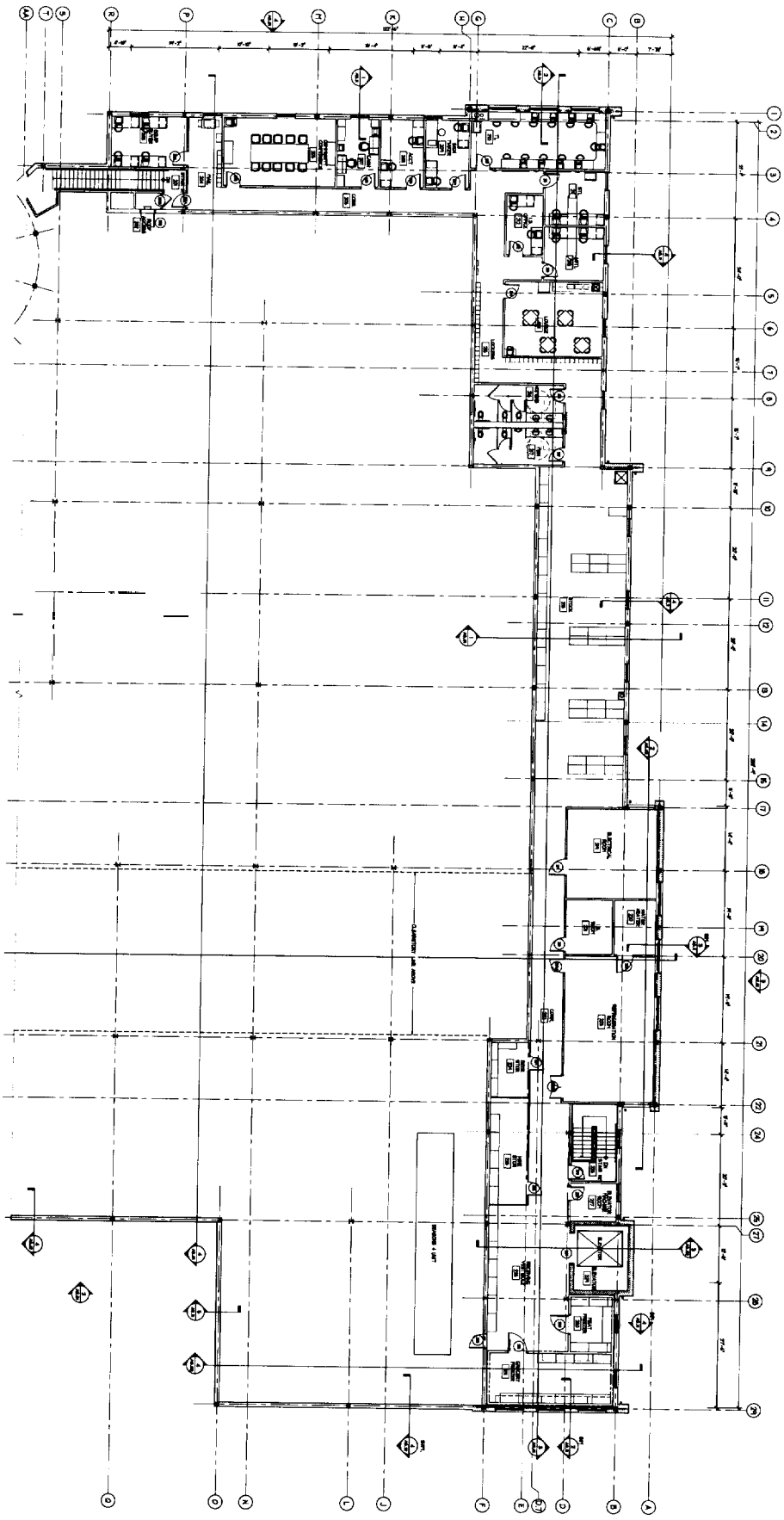
Sheet No.
A0.02



REFERENCE DRAWING ONLY
NOT FOR CONSTRUCTION

PROPOSED GROCERY STORE
FRANKLIN ST. ARTERIAL, FOX/SOMERSET ST., PEARL ST.
PORTLAND, CUMBERLAND COUNTY, MAINE

A1.01



1 MEZZANINE FLOOR PLAN

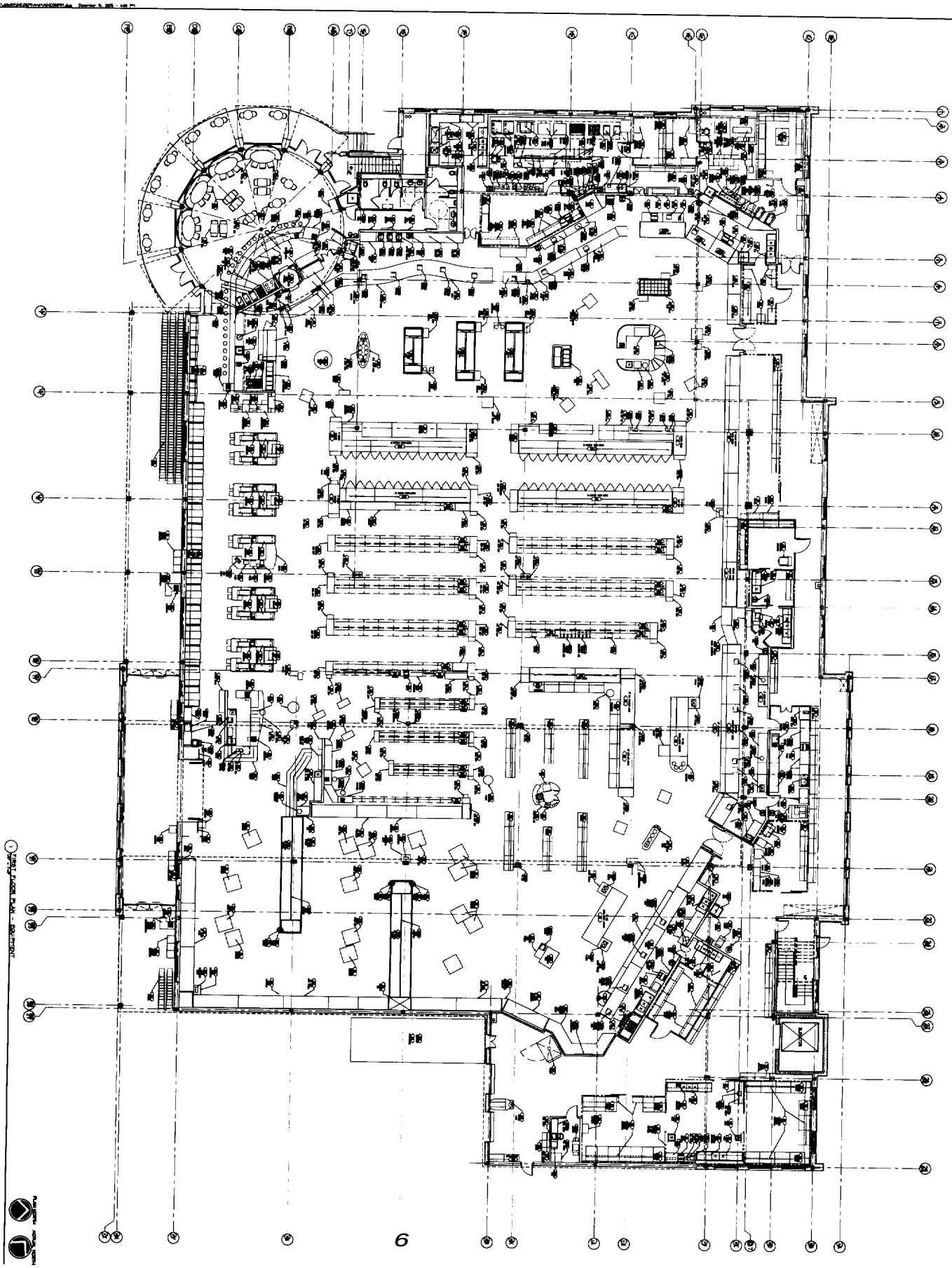
REFERENCE DRAWING ONLY
NOT FOR CONSTRUCTION

PROPOSED GROCERY STORE
FRANKLIN ST. ARTERIAL, FOX/SOMERSET ST., PEARL ST.
PORTLAND, CUMBERLAND COUNTY, MAINE

WHOLE FOODS

BL Companies

A1.02

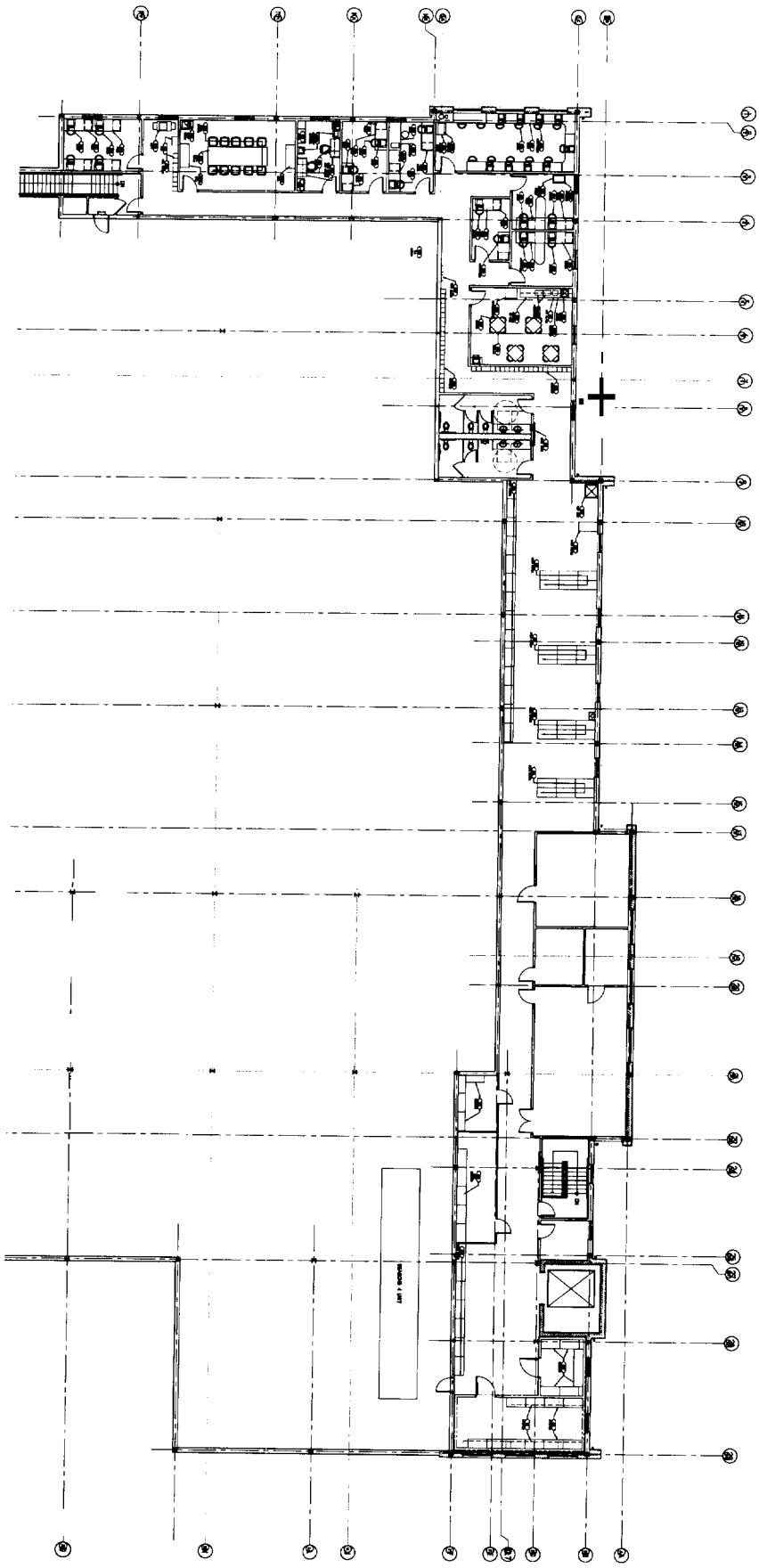


1 FIRST FLOOR PLAN - EQUIPMENT

REFERENCE DRAWING ONLY
NOT FOR CONSTRUCTION

PROPOSED GROCERY STORE
FRANKLIN ST. ARTERIAL, FOX/SOMERSET ST., PEARL ST.
PORTLAND, CUMBERLAND COUNTY, MAINE

A1.11

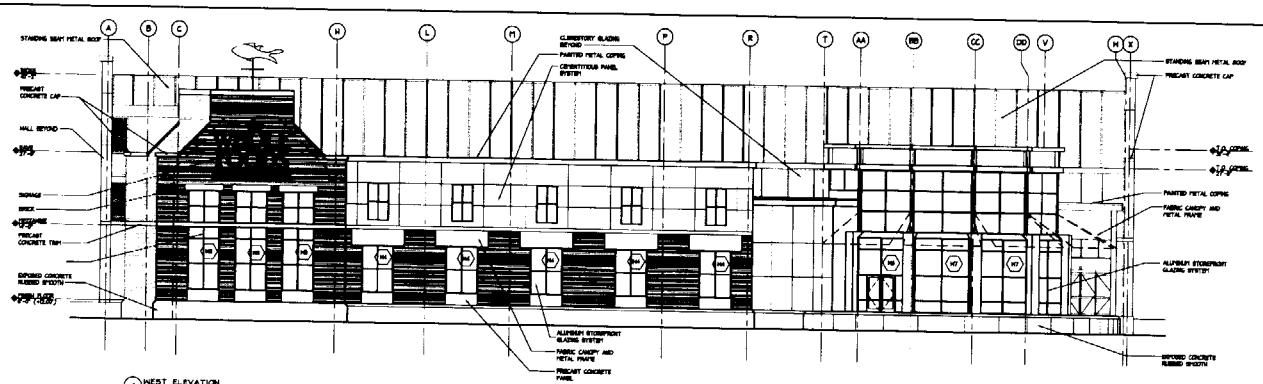


MEZZANINE FLOOR PLAN - EQUIPMENT

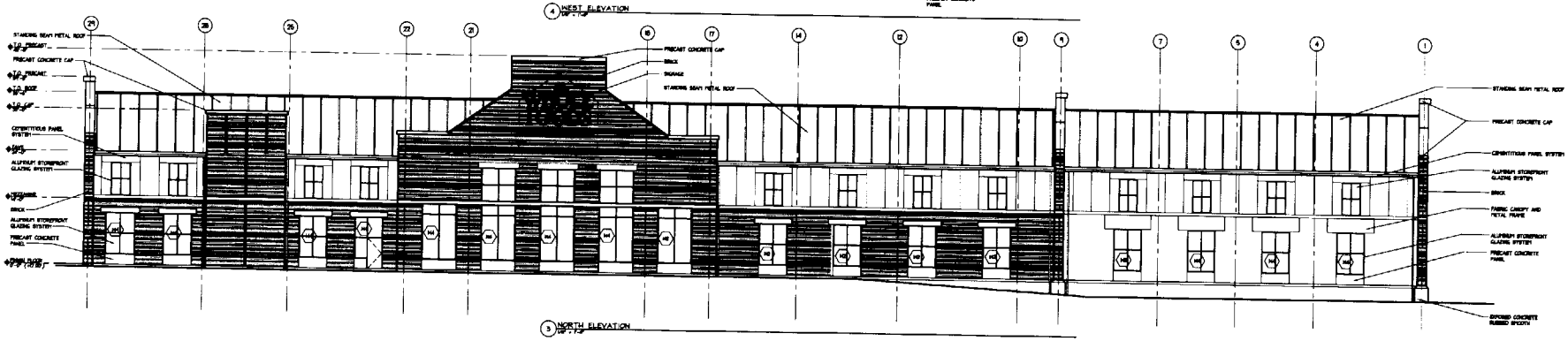
REFERENCE DRAWING ONLY
NOT FOR CONSTRUCTION

PROPOSED GROCERY STORE
FRANKLIN ST. ARTERIAL, FOX/SOMERSET ST., PEARL ST.
PORTLAND, CUMBERLAND COUNTY, MAINE

A121



4 WEST ELEVATION



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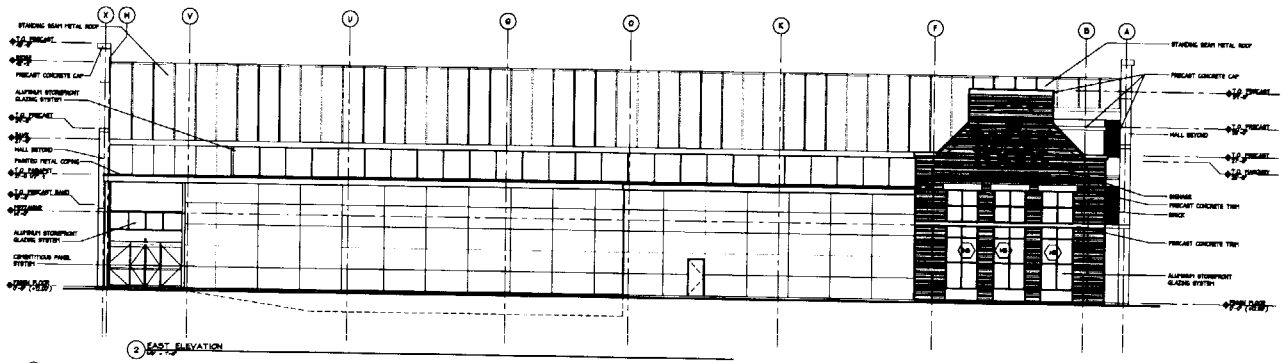
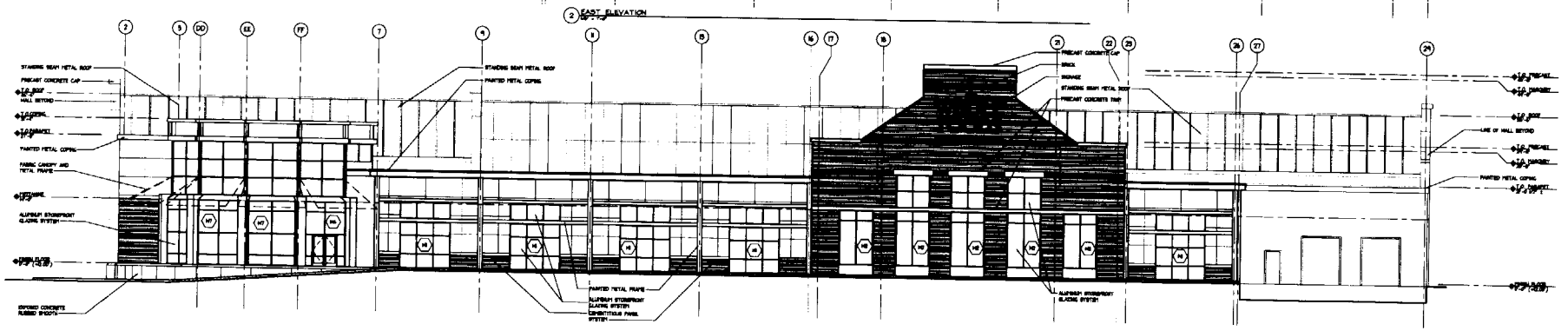


2 EAST ELEVATION



1 SOUTH ELEVATION

BL Companies

ARCHITECTURE
INTERIORS
PLANNING
LANDSCAPE ARCHITECTURE
LAND SURVEYING
ENVIRONMENTAL SCIENCES
ANALYTICAL SERVICES

100 Foreside Parkway
Portland, ME 04101
(207) 828-1101
(207) 828-1915 Fax

WHOLE FOODS

PROPOSED GROCERY STORE
FRANKLIN ST. ARTERIAL, FOX/SOMERSET ST., PEARL ST.
PORTLAND, CUMBERLAND COUNTY, MAINE

REFERENCE DRAWING ONLY
NOT FOR CONSTRUCTION

ARCHITECT/ENGINEER SEAL

Designed
Drawn
Checked
Approved
Date
Project No. 48.10702
Scale 1/8"=1'-0"
Date 12-14-05
CAD File

EXTERIOR ELEVATIONS

Sheet No.

A2.01

A3.01

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PROPOSED GROCERY STORE
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A3.02

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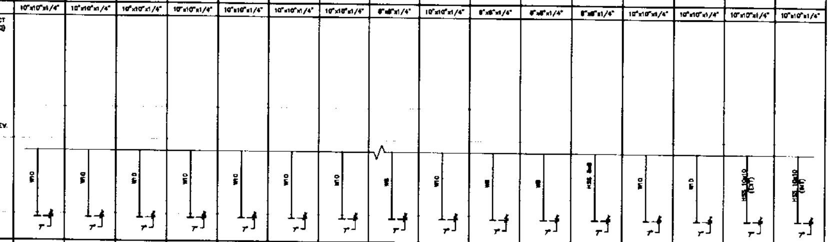
COLUMN SCHEDULE

COLUMN LOCATION	A17, A18, A19, A21, A22, B29, F29, Q27	B9, B10, B12, B14, B17, C1, C4, C5, C7, C9, D27	B22	B25, B26, K2, M2, P2, V9, V11, V13, V16, V18, V21	D24, D.7-21, E10, E11, E13, E15, E18, E20, F21, F24, F27, F29, G4, G8, G8, Q9, J18, J21, K4, L29	D26	H1, R2	L29, Q27, T3, U27	M4, N8, N10, N13, N18, N21, P4, Q18, Q21, S8, S10, S13, U18, U21	D28	V7, V27	W7, W9, W11, W13, W16, W23, W26	X-17, X-22	X-18, X-21	AA (2 COLS), BB, CC, DD, EE, FF	BB, CC, DD, EE, FF
CAP PLATE	10"x10"x1/4"	10"x10"x1/4"	10"x10"x1/4"	10"x10"x1/4"	10"x10"x1/4"	10"x10"x1/4"	10"x10"x1/4"	8"x8"x1/4"	10"x10"x1/4"	8"x8"x1/4"	8"x8"x1/4"	8"x8"x1/4"	10"x10"x1/4"	10"x10"x1/4"	10"x10"x1/4"	10"x10"x1/4"
(T.O.S. WITH RESPECT TO DATUM @ COL. Q) HIGH ROOF ELEV. (+38'-3") MAIN ROOF ELEV. (+30'-0") MEZZANINE LEVEL ELEV. (+14'-0") DATUM ELEV. (+0'-0") (DIMENSION TO B/BASE PL. TYP.)	W10	W10	W10	W10	W10	W10	W10	W8	W10	W8	W8	HSS 8x8	W10	W10	HSS 10x10 (EXT)	HSS 10x10 (INT)
BASE PLATE (W x L x t)	12"x18"x1"	12"x18"x1"	12"x18"x1"	12"x18"x1"	18"x18"x1"	16"x16"x1"	14"x14"x1"	12"x18"x1"	18"x18"x1"	12"x14"x1"	12"x14"x1"	14"x14"x1"	12"x18"x1"	12"x18"x1"	16"x16"x1"	16"x16"x1"
ANCHOR BOLTS (F1554)	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
BASE PLATE DETAIL	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
PILASTER DETAIL	P2	P1	P3	P4	-	P1	P1	P5	-	P6	P7	P1	P1	P2	P1	P1



GENERAL NOTES

DESIGNED IN ACCORDANCE WITH THE MAINE MODEL BUILDING CODE AND THE 2003 INTERNATIONAL BUILDING CODE.

IF ANY FIELD CONDITIONS PRECLUDE COMPLIANCE WITH THESE DRAWINGS AND/OR CONDITIONS SPECIFIED, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ENGINEER AND SHALL NOT PROCEED WITH THE AFFECTED WORK.

TYPICAL DETAILS AND NOTES SHOWN HEREIN SHALL APPLY UNLESS SPECIFICALLY SHOWN OR NOTED OTHERWISE. CONSTRUCTION DETAILS NOT FULLY SHOWN OR NOTED SHALL BE SIMILAR TO DETAILS SHOWN FOR SIMILAR CONDITIONS.

ALL UNDERGROUND UTILITY LOCATIONS SHALL BE VERIFIED PRIOR TO STARTING EXCAVATION WORK.

WORK AREAS SHALL BE MARKED, FENCED, AND OTHERWISE SECURED SO AS TO PROVIDE PROPER PROTECTION FOR THE PUBLIC, AND AS REQUIRED BY THE BUILDING INSPECTOR.

IT SHALL BE THE CONTRACTORS SOLE RESPONSIBILITY TO DESIGN AND PROVIDE ADEQUATE SHORING, BRACING AND FORMWORK, ETC., AS REQUIRED FOR THE PROTECTION OF LIFE AND PROPERTY DURING CONSTRUCTION. THIS INCLUDES PROVIDING TEMPORARY BRACING, SHORING, GUYS OR TIE-DOWNS. THESE TEMPORARY SUPPORTS SHALL REMAIN IN PLACE UNTIL ALL STRUCTURAL COMPONENTS ARE STABLE AND COMPLETED.

ALL FOOTINGS HAVE BEEN DESIGNED FOR AN ALLOWABLE NET SOIL BEARING PRESSURE OF 3,000 PSF, PER GEOTECHNICAL REPORT DATED OCTOBER 10, 2005 BY SEBAGO TECHNICS. ALL FOOTINGS SHALL BEAR ON ADEQUATE UNDISTURBED SOILS OR COMPACTED CONTROLLED FILL AS RECOMMENDED IN THE GEOTECHNICAL REPORT.

THE BOTTOM OF ALL EXTERIOR FOOTING ELEVATIONS SHALL BE A MINIMUM OF 4'-6" BELOW FINISHED EXTERIOR GRADE. COMPACT BOTTOM OF FOOTING SUBGRADE WITH PLATE COMPACTOR. REMOVE ALL DEBRIS FROM BOTTOM OF FOOTING PRIOR TO PLACING CONCRETE. DO NOT PLACE CONCRETE ON FROZEN SOIL, ICE, MUD, OR IN STANDING WATER. ALL FOUNDATION SUBGRADES SHALL BE INSPECTED AND APPROVED UNDER THE SUPERVISION OF A REGISTERED PROFESSIONAL ENGINEER PRIOR TO BEING CONCRETED.

FLOOR LIVE LOADS:
- MEZZANINE (OFFICE) = 100 PSF
- MEZZANINE (MECHANICAL) = 250 PSF

ROOF LOADS:
- LIVE LOAD = 20 PSF
- SNOW LOAD
  - Pg = 80 PSF
  - Pf (SLOPE < 5°) = 38 PSF.
  - Ps (5°< SLOPE < 30°) = 31.8 PSF.
  - SNOW EXPOSURE CATEGORY, Ce = 1.0
  - THERMAL FACTOR, Ct = 1.0
  - SNOW LOAD IMPORTANCE FACTOR, Is = 1.0
  - (LOADS INCREASED FOR DRIFT, SLIDING, ETC.)

WIND LOADS:
- BASIC WIND SPEED = 100 MPH
- WIND IMPORTANCE FACTOR, Iw = 1.0
- EXPOSURE CATEGORY = B
- INTERNAL PRESSURE COEFFICIENT, Gcpi = -0.18
- DESIGN WIND PRESSURE ON COMPONENTS AND CLADDING, p = XX.X PSF.
- SEE DRAWING S2.0X FOR UPLIFT LOADS

SEISMIC DESIGN CRITERIA:
- SEISMIC IMPORTANCE FACTOR, Ie = 1.0
- SEISMIC USE GROUP, I
- SPECTRAL RESPONSE ACCELERATIONS: Ss = 38.3, S1 = 8.7
- SITE CLASS E
- SPECTRAL RESPONSE COEFFICIENTS: Sds = 0.630, Sd1 = 0.227
- SEISMIC DESIGN CATEGORY, D
- BASIC SEISMIC FORCE RESISTING SYSTEM: INTERMEDIATE MOMENT FRAMES
- SEISMIC RESPONSE COEFFICIENT, Cs = 0.12
- DESIGN BASE SHEAR, V = 0.12W
- RESPONSE MODIFICATION FACTOR, R = 4.5
- ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE ANALYSIS

ALL COMPACTED CONTROLLED FILL BEING PLACED MUST BE TESTED BY AN APPROVED TESTING LABORATORY DURING THE FILL OPERATION. SLABS AND FOOTINGS SHALL NOT BE PLACED UNTIL FILL TEST REPORTS HAVE BEEN CHECKED AND APPROVED BY THE ENGINEER. PRIOR TO PLACING CONCRETE, EXISTING SOIL CONDITIONS SHALL BE INSPECTED AND VERIFIED BY THE GEOTECHNICAL ENGINEER. ALL UNSUITABLE SOIL SHALL BE REMEDIED AS DIRECTED BY THE GEOTECHNICAL ENGINEER. BUILDING PAD PREPARATION SHALL BE IN ACCORDANCE WITH THE SITE DRAWINGS AND SPECIFICATIONS.

ALL SPECIAL INSPECTION REPORTS PERFORMED IN ACCORDANCE WITH THE STATEMENT OF SPECIAL INSPECTIONS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW IN A TIMELY MANNER. THE FINAL REPORT OF SPECIAL INSPECTIONS SHALL NOT BE ISSUED UNTIL ALL SPECIAL INSPECTION REPORTS HAVE BEEN RECEIVED AND APPROVED BY THE ENGINEER.

STRUCTURAL STEEL NOTES

STEEL SHALL CONFORM TO THE "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS, ALLOWABLE STRESS DESIGN AND PLASTIC DESIGN", LATEST EDITION, OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION.

ALL STRUCTURAL STEEL SHALL CONFORM TO ASTM A992, EXCEPT AS MODIFIED BELOW:
- ALL MISCELLANEOUS STRUCTURAL STEEL SHALL CONFORM TO ASTM A36.
- ALL HOLLOW STRUCTURAL SECTIONS SHALL CONFORM TO ASTM A500, GRADE B.
- ALL ANCHOR RODS SHALL CONFORM TO ASTM F1554, GRADE 36.

ALL WELDS SHALL USE E70XX ELECTRODES AND CONFORM TO THE "STRUCTURAL WELDING CODE (STEEL)", AWS D1.1, LATEST EDITION.

ALL BOLTED CONNECTIONS SHALL BE BEARING TYPE, UTILIZING 3/4" DIA. ASTM A325-N TENSION CONTROL BOLTS, UNLESS SHOWN OTHERWISE.

ALL BOLTS, NUTS, AND WASHERS SHALL BE IN ACCORDANCE WITH THE LATEST SPECIFICATIONS APPROVED BY THE RESEARCH COUNCIL ON RIVETED AND BOLTED STRUCTURAL JOINTS. USE DIRECT TENSION INDICATORS, CONFORMING TO ASTM F959, ON ALL A325 BOLTS.

ALL STRUCTURAL STEEL SHALL BE SHOP PRIMED (ONE COAT OF 2 MIL. DRY FILM) AND SHOP PAINTED (ONE COAT OF 1.5 MIL. DRY FILM). SHOP PRIMER AND PAINT TO BE FABRICATOR'S STANDARD COLOR.

OPENINGS THROUGH BEAMS AND COLUMNS SHALL NOT BE PERMITTED UNLESS APPROVED BY THE STRUCTURAL ENGINEER.

FIELD FLAME CUTTING OF ANY STRUCTURAL MEMBERS IS NOT PERMITTED.

EXPOSED TOPS OF ALL STEEL HOLLOW STRUCTURAL SECTION COLUMNS SHALL BE COVERED WITH A SHOP WELDED CAP PLATE AND SHALL BE WATERTIGHT.

ALL STEEL SHALL BE FABRICATED, ERECTED, AND TESTED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES"

STRUCTURAL STEEL SHOP DRAWINGS, PREPARED BY THE STEEL SUBCONTRACTORS, SHOWING COMPLETE DIMENSIONS, DETAILS, SIZES, AND GRADES OF STEEL MEMBERS AND CONNECTIONS, TYPE AND NUMBER OF WELDS AND BOLTS, SHALL BE SUBMITTED FOR APPROVAL PRIOR TO THE FABRICATION OF THE STEEL COMPONENTS. THE STRUCTURAL STEEL SUBCONTRACTOR SHALL BE RESPONSIBLE FOR CHECKING, VERIFICATION AND COORDINATION OF DIMENSIONS AND DETAILS WITH THE STRUCTURAL AND OTHER PORTIONS OF THE CONTRACT DRAWINGS. THE STEEL SUBCONTRACTOR SHALL NOT REPRODUCE ANY PORTION OF THE STRUCTURAL CONTRACT DRAWINGS FOR UTILIZATION AS SHOP DRAWINGS, UNLESS APPROVAL OF THE STRUCTURAL ENGINEER IS OBTAINED.

THE STEEL FABRICATOR SHALL BE AISC CERTIFIED, OR A MEMBER OF THE STRUCTURAL STEEL FABRICATORS OF NEW ENGLAND.

ALL STRUCTURAL STEEL ERECTION AND SHORING SHALL BE INSPECTED BY AN INDEPENDENT TESTING AGENCY RETAINED BY THE OWNER FOR THE FOLLOWING ITEMS:
- VISUALLY INSPECT ALL CONNECTIONS, INCLUDING, BUT NOT LIMITED TO, BOLTS, WELDS, METAL DECK WELDS TO SUPPORT STRUCTURE, AND METAL DECK SIDE LAPS.
- INSPECT STEEL ERECTION FOR ORIENTATION AND PLUMBNESS.

ALL STEEL CONNECTIONS SHALL BE DESIGNED BY THE STEEL FABRICATOR TO RESIST MINIMUM THE REACTIONS SHOWN THUS XX ON THESE DRAWINGS.

ALL STEEL JOISTS SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE STANDARD SPECIFICATIONS OF THE STEEL JOIST INSTITUTE.

PROVIDE JOIST BRIDGING OF AMOUNT AND TYPE AT LOCATIONS AS REQUIRED BY THE JOIST MANUFACTURER.

ALL BAR JOISTS SHALL BE DESIGNED FOR A NET WIND UPLIFT PRESSURE AS SHOWN ON THE DRAWINGS OVER THEIR TRIBUTARY AREAS.

STEEL DECK NOTES

STEEL ROOF DECK SHALL CONFORM TO AISI SPECIFICATION FOR THE DESIGN OF LIGHT GAGE, COLD-FORMED STRUCTURAL STEEL MEMBERS AND THE STEEL DECK INSTITUTE'S DESIGN REQUIREMENTS. COMPOSITE STEEL FLOOR DECK SHALL CONFORM TO ASTM A-611, GRADE C AND D, OR FROM A-653-94 STRUCTURAL QUALITY GRADE 33 OR HIGHER.

STEEL ROOF DECK, UNLESS SPECIFIED OTHERWISE, SHALL BE 1 1/2" DEEP, 20 GAGE MINIMUM, TYPE "B" BY VULCRAFT OR APPROVED EQUAL. STEEL DECK SHALL BE GALVANIZED G60.

STEEL ROOF DECK SHALL BE CONTINUOUS OVER A MINIMUM OF THREE SPANS, AND SHALL BE FASTENED TO THE SUPPORTS AT THE UNIT ENDS AND AT INTERMEDIATE SUPPORTS AS RECOMMENDED BY THE STEEL DECK MANUFACTURER. MINIMUM ATTACHMENT REQUIREMENTS SHALL BE 5/8" DIA. PUDDLE WELDS AT 12" o.c. AT EVERY SUPPORT AND 6" o.c. AT DECK PERIMETER WITH #10 TEK SCREWS AT 12" o.c. AT ALL SIDE LAPS. SEE DECK ATTACHMENT DETAIL FOR FURTHER CLARIFICATION.

STEEL FLOOR DECK, UNLESS SPECIFIED OTHERWISE, SHALL BE 1 1/2" DEEP, 20 GAGE MINIMUM, TYPE "VL" BY VULCRAFT OR APPROVE EQUAL. STEEL DECK SHALL BE GALVANIZED G60.

STEEL FLOOR DECK SHALL BE CONTINUOUS OVER A MINIMUM OF THREE SPANS, AND SHALL BE FASTENED TO THE SUPPORTS AT THE UNIT ENDS AND AT INTERMEDIATE SUPPORTS AS INDICATED IN FASTENING SCHEDULES. SEE FRAMING PLANS FOR FURTHER INFORMATION.

ALL ACCESSORIES TO THE STEEL ROOF DECK, INCLUDING EDGE ANGLES, POUR STOPS, COLUMN CLOSURES, END CLOSURES, COVER PLATES AND GIRDER FILLERS, SHALL BE IN ACCORDANCE WITH THE STEEL DECK INSTITUTE.

COLD FORMED METAL FRAMING NOTES

ALL LIGHTWEIGHT STEEL PRODUCTS, DETAILING, FABRICATION AND INSTALLATION SHALL MEET THE REQUIREMENTS OF AISI "SPECIFICATIONS FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS", 1986 EDITION WITH 1989 ADDENDUM.

STEEL GRADES:
- 12, 14, & 16 GAGE COMPONENTS Fy (MIN) = 50 ksi (ASTM A-446 GRADED)
- 18 GAGE COMPONENTS Fy (MIN) = 33 ksi (ASTM A-446 GRADE B)
- 20 GAGE COMPONENTS Fy (MIN) = 33 ksi (ASTM A-446 GRADE A)

ALL COMPONENTS SHALL BE ZINC COATED (G60 GALVANIZED) IN ACCORDANCE WITH ASTM A-525.

FLAME CUTTING OF MEMBERS OR HOLES IS NOT PERMITTED.

ALL FIELD CONNECTIONS SHALL CONFORM TO DETAILS SHOWN ON FABRICATION PLANS.

ALL STUDS, HARDWARE, ACCESSORIES, AND FASTENERS SHALL BE GALVANIZED.

ALL WELDING SHALL BE IN ACCORDANCE WITH AWS D1.3.

CONCRETE NOTES

ALL CONCRETE WORK SHALL CONFORM TO ACI "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE", ACI-318-02, AND "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES", ACI-315-LATEST EDITION.

ALL CONCRETE FORMWORK SHALL CONFORM WITH ACI "RECOMMENDED PRACTICES FOR CONCRETE FORMWORK", ACI-347.

ALL STRUCTURAL CONCRETE SHALL CONFORM WITH ACI "SPECIFICATIONS FOR STRUCTURAL CONCRETE BUILDINGS", ACI-301.

AIR ENTRAIN ALL EXTERIOR CONCRETE.

ALL CONCRETE FOR EXTERIOR FOUNDATION WALLS SHALL BE NORMAL WEIGHT CONCRETE WITH A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS OF 4,000 PSI. ALL OTHER CONCRETE SHALL BE NORMAL WEIGHT CONCRETE WITH A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS OF 3,000 PSI.

ALL CONCRETE SHALL BE CONTINUOUSLY MOIST CURED FOR SEVEN (7) DAYS MINIMUM. CONCRETE SHALL BE KEPT MOIST BY FOG SPRAYING, CURING PAPER AND/OR ACCEPTABLE CURING COMPOUNDS.

CONTRACTOR SHALL SUBMIT MIX DESIGNS FOR APPROVAL. MIX DESIGN SHALL INDICATE 28-DAY STRENGTHS, CEMENT CONTENT, AIR CONTENT, WATER-CEMENT RATIO, AMOUNT OF FINE AND COARSE AGGREGATES, AND ADMIXTURES.
- MAX. WATER-CEMENT RATIO = 0.53 FOR 3000 PSI; 0.45 FOR 4000 PSI
- MAX. SLUMP-
  - 4" FOR SLABS
  - 5" FOR ALL OTHER CONCRETE
- MAX. AGGREGATE SIZE-
  - FOOTINGS & FOUNDATIONS - 3/4" TO 1 1/2"
  - SLAB-ON-GRADE - 3/4" TO 1"
  - CONCRETE FILL - 1/2"

CEMENT SHALL BE PORTLAND CEMENT, TYPE I OR II, CONFORMING TO ASTM C-150. AGGREGATES SHALL CONFORM TO ASTM C-33. AIR ENTRAINING ADMIXTURES SHALL CONFORM TO ASTM C-260. NON-SHRINK GROUT SHALL CONFORM TO ASTM C-109. EXPANSION JOINTS SHALL BE 1/2" THICK ASPHALT IMPREGNATED FIBERBOARD JOINT MATERIAL, CONFORMING TO ASTM D-1751. CURING COMPOUND SHALL BE CLEAR, CONFORMING TO ASTM C-309.

COLD WEATHER CONCRETE WORK SHALL CONFORM TO ACI-306.

HOT WEATHER CONCRETE WORK SHALL CONFORM TO ACI-305.

ALL FLOOR SLABS SHALL BE CONSTRUCTED TO HAVE A MINIMUM FLATNESS OF Ff=20 AND A MINIMUM LEVELNESS OF Fl=17.

THE MAXIMUM LENGTH BETWEEN CONSTRUCTION JOINTS OF WALLS AND FOOTINGS SHALL NOT EXCEED 40 FEET.

STRUCTURAL STEEL WILL DEFLECT UNDER WEIGHT OF CONCRETE. POUR FLOOR SLABS LEVEL REGARDLESS OF STEEL DEFLECTION.

ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615 GRADE 60. WELDED WIRE MESH SHALL CONFORM TO ASTM A185 AND ASTM A82.

WELDED WIRE MESH SHALL HAVE MINIMUM END AND SIDE LAPS OF 1'-0".

CONCRETE ACCESSORIES SHALL BE ADEQUATE TO MAINTAIN REINFORCING ACCURATELY IN PLACE AND BE NON-CORROSIVE, NON-STAINING TYPE.

LAP ALL BAR REINFORCING PER ACI-318. STAGGER SPLICES IN WALLS AND SLABS AND ALL HORIZONTAL REINFORCEMENT.

CONCRETE SHALL NOT BE DROPPED THROUGH REINFORCING STEEL (AS IN WALLS AND COLUMNS) SO AS TO CAUSE SEGREGATION OF AGGREGATES. USE HOPPERS, CHUTES OR TRUNKS OF VARYING LENGTHS SO THAT THE FREE UNCONFINED FALL OF CONCRETE SHALL NOT EXCEED FIVE (5) FEET.

MINIMUM REINFORCEMENT COVER, UNLESS NOTED OTHERWISE:
- CONCRETE CAST AGAINST EARTH: 3"
- CONCRETE EXPOSED TO EARTH OR WEATHER:
  - No. 6 TO No. 18 BARS - 2"
  - No. 5 AND SMALLER - 1 1/2"
- CONCRETE NOT EXPOSED TO EARTH OR WEATHER:
  - No. 11 AND SMALLER - 3/4"

ALL CONCRETE WORK, REINFORCING PLACEMENT, AND FORMWORK SHALL BE INSPECTED BY AN INDEPENDENT TESTING AGENCY RETAINED BY THE OWNER FOR THE FOLLOWING ITEMS:
- INSPECT BOTTOM OF FOOTING SOIL CONDITIONS.
- INSPECT FORMWORK AND PLACEMENT OF CONCRETE.
- SAMPLING FRESH CONCRETE PER ASTM C-172, MODIFIED FOR SLUMP BY ASTM C-94.
- SLUMP PER ASTM C-143. ONE TEST AT POINT OF DISCHARGE FOR EACH DAYS POUR FOR EACH TYPE OF CONCRETE.
- AIR CONTENT PER ASTM C-173 (VOLUMETRIC) OR ASTM C-231 (PRESSURE). ONE TEST FOR EACH DAYS POUR FOR EACH TYPE OF CONCRETE.
- COMPRESSION TEST CYLINDERS PER ASTM C-31 (4 CYLINDERS)
- CONCRETE TEMPERATURE - TEST EACH TIME A SET OF CYLINDERS IS CAST.
- COMPRESSION STRENGTH TEST PER ASTM C-39. ONE SET FOR EACH DAYS POUR PLUS ADDITIONAL SETS FOR EACH 50 cy OVER AND ABOVE THE FIRST 25 cy. ONE CYLINDER TESTED AT 7 DAYS, TWO CYLINDERS TESTED AT 28 DAYS, AND ONE CYLINDER RETAINED FOR LATER TESTING IF REQUIRED.

CONTROLLED FILL NOTES

CONTROLLED FILL SHALL BE INERT, GRANULAR FILL FREE FROM ORGANIC MATERIAL, WITH LESS THAN 20% PASSING A No. 200 SIEVE. STONES SHALL NOT BE GREATER THAN 6".

PLACE FILL IN LAYERS NOT TO EXCEED 12". COMPACT WITH PLATE TAMPER OR JUMPING JACK TO A MINIMUM OF 95% OF MAXIMUM DRY UNIT WEIGHT AS DETERMINED BY THE MODIFIED PROCTOR TEST, ASTM D1557. PLACE FILL AT OPTIMUM MOISTURE CONTENT.

BL Companies

WHOLE FOODS

PROPOSED GROCERY STORE
FRANKLIN ST. ARTERIAL, FOX/SOMERSET ST, PEARL ST.
PORTLAND, CUMBERLAND COUNTY, MAINE

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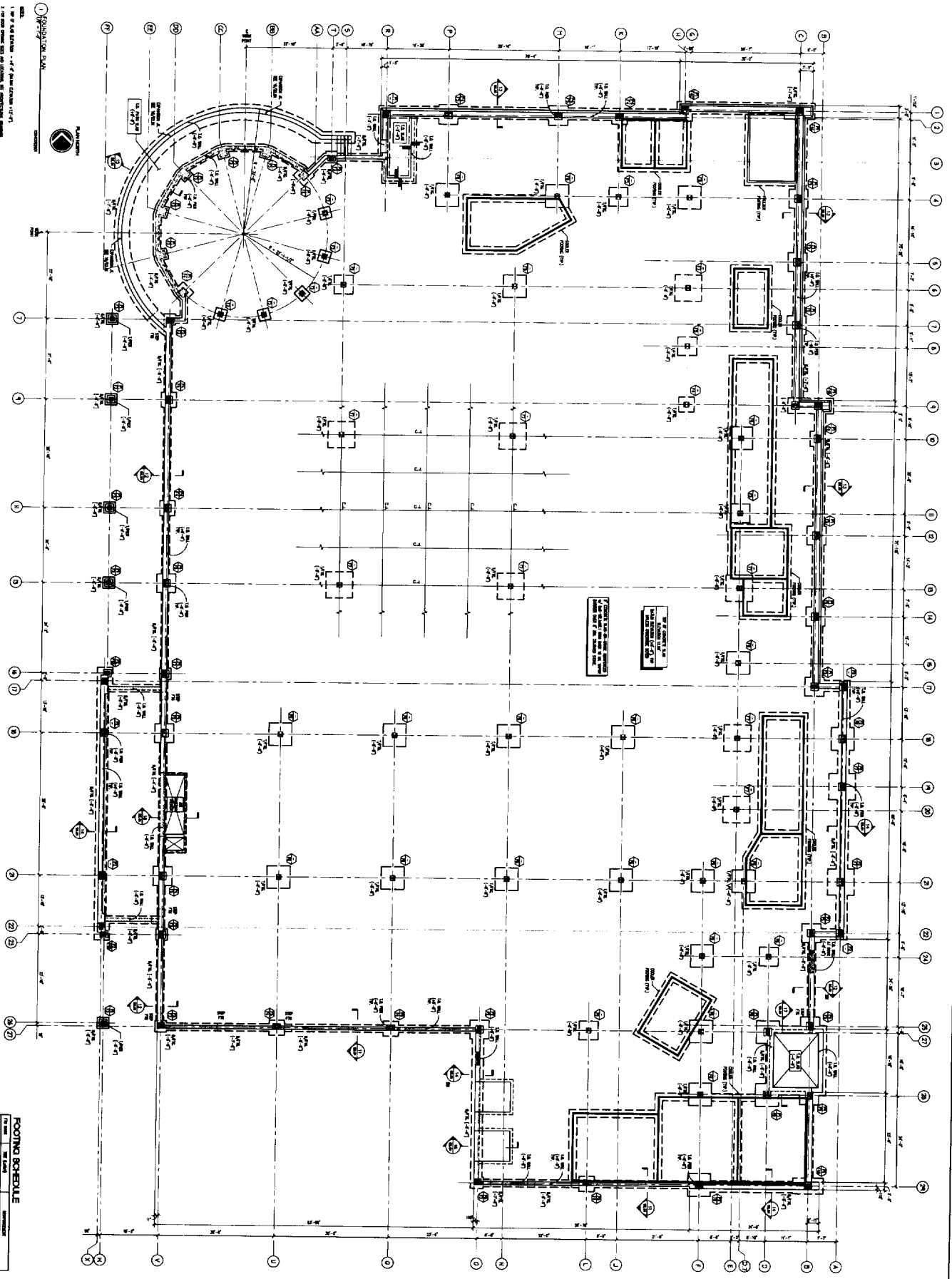
COLUMN SCHEDULE AND GENERAL NOTES

S1.00



PIER SCHEDULE

FOOTING SCHEDULE

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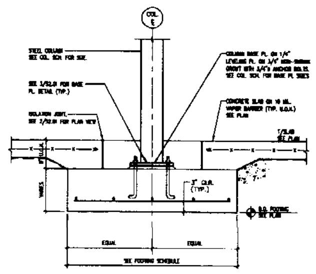
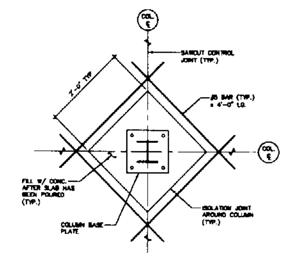
PROPOSED GROCERY STORE
FRANKLIN ST. ARTERIAL, FOX/SOMERSET ST., PEARL ST.
PORTLAND, CUMBERLAND COUNTY, MAINE

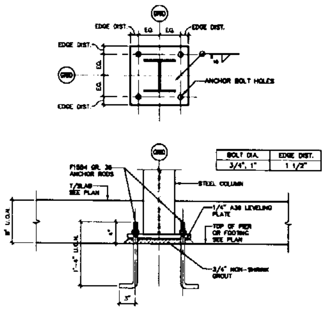
S1.01



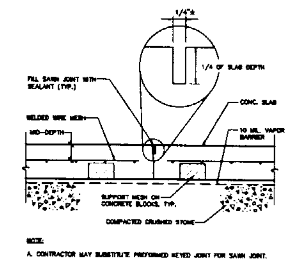
1 S2.01 **TYP. INTERIOR SPREAD FOOTING**
SCALE: N.T.S. WBCB-301



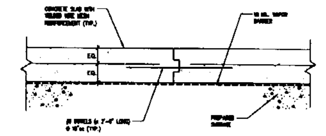
2 S2.01 **TYPICAL COLUMN ISOLATION JOINT DETAIL**
SCALE: N.T.S. WBCB-302



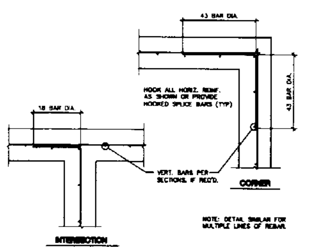
3 S2.01 **TYPICAL BASE PLATE DETAILS**
SCALE: N.T.S. WBCB-303



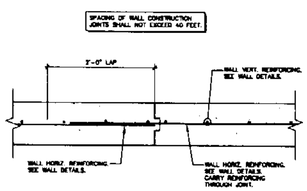
4 S2.01 **SLAB CRACK CONTROL JOINT DETAIL**
SCALE: N.T.S. WBCB-304



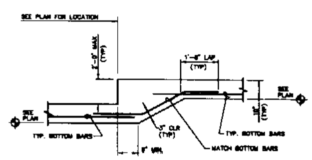
5 S2.01 **SLAB CONSTRUCTION JOINT DETAIL**
SCALE: N.T.S. WBCB-305



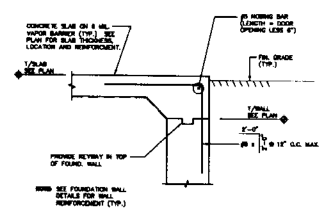
6 S2.01 **FOUNDATION WALL INTERSECTION AND CORNER REINFORCEMENT DETAIL**
SCALE: N.T.S. WBCB-306



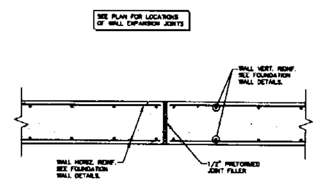
7 S2.01 **CONCRETE WALL CONSTRUCTION JOINT DETAIL**
SCALE: N.T.S. WBCB-307



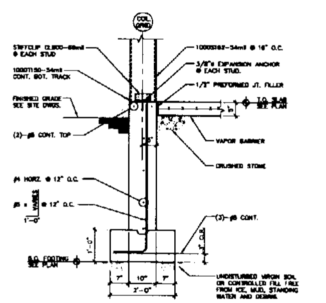
8 S2.01 **TYPICAL STEP FOOTING DETAIL**
SCALE: N.T.S. WBCB-316



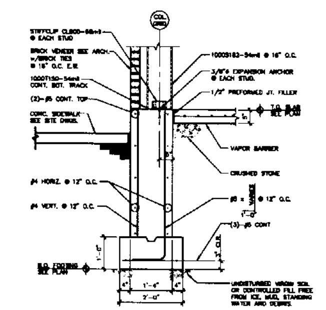
9 S2.01 **TYPICAL SLAB AT DOOR DETAIL**
SCALE: N.T.S. WBCB-317



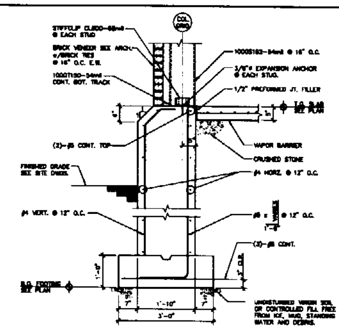
10 S2.01 **TYPICAL CONCRETE WALL EXPANSION JOINT DETAIL**
SCALE: N.T.S. WBCB-319



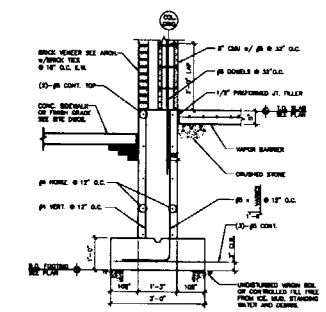
11 S2.01 **FOUNDATION WALL DETAIL**
SCALE: 3/4" = 1'-0" WBCB-309



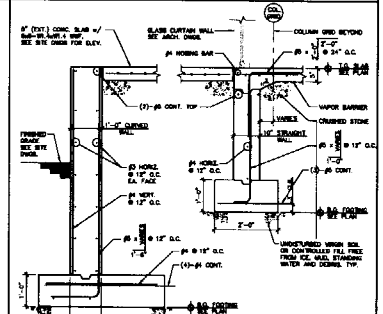
12 S2.01 **FOUNDATION WALL DETAIL**
SCALE: 3/4" = 1'-0" WBCB-310



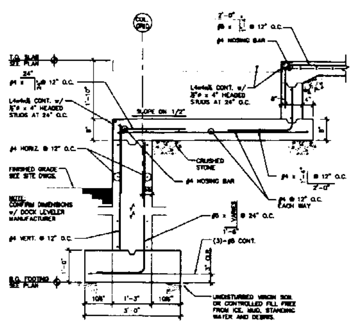
13 S2.01 **FOUNDATION WALL DETAIL**
SCALE: 3/4" = 1'-0" WBCB-311



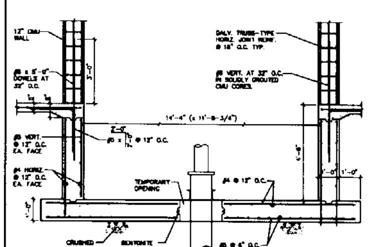
14 S2.01 **FOUNDATION WALL DETAIL**
SCALE: 3/4" = 1'-0" WBCB-312



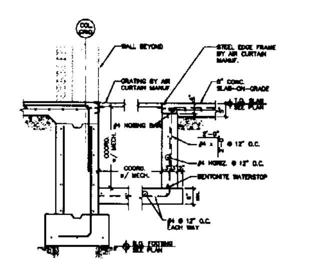
15 S2.01 **FOUNDATION WALL DETAIL**
SCALE: 3/4" = 1'-0" WBCB-313



16 S2.01 **FOUNDATION DETAIL AT LEVELING DOCK**
SCALE: 3/4" = 1'-0" WBCB-314



17 S2.01 **ELEVATOR PIT DETAIL**
SCALE: 1/2" = 1'-0" WBCB-315



18 S2.01 **AIR CURTAIN TRENCH DETAIL**
SCALE: 3/4" = 1'-0" WBCB-318

BL Companies

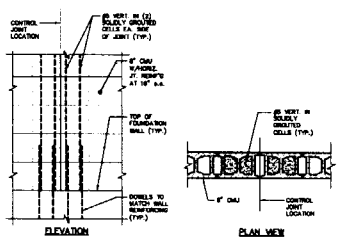
WHOLE FOODS

PROPOSED GROCERY STORE
FRANKLIN ST. ARTERIAL, FOX/SOMERSET ST., PEARL ST.
PORTLAND, CUMBERLAND COUNTY, MAINE

FOR PERMIT/PRICING ONLY
NOT FOR CONSTRUCTION

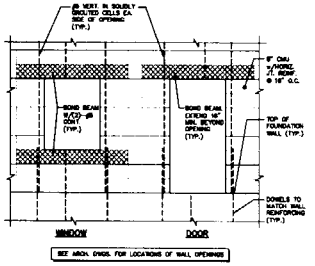
Title: STRUCTURAL DETAILS

Sheet No. **S2.01**



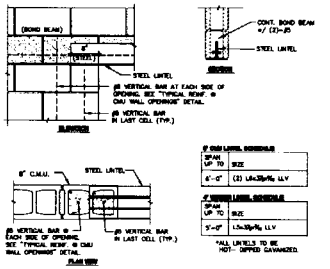
3 / S2.02 CMU CONTROL JOINT REINFORCING

SCALE: N.T.S. WSMF-303



4 / S2.02 CMU WALL OPENING REINFORCING

SCALE: N.T.S. WSMF-304



5 / S2.02 STEEL LINTEL DETAIL

SCALE: N.T.S. WSMF-305

BL Companies

WHOLE FOODS

PROPOSED GROCERY STORE
FRANKLIN ST. ARTERIAL, FOX/SOMERSET ST., PEARL ST.
PORTLAND, CUMBERLAND COUNTY, MAINE

FOR PERMIT/PRICING ONLY
NOT FOR CONSTRUCTION

STRUCTURAL DETAILS

S2.02

BL Companies

Transmittal

DATE: January 11,2006

ATTENTION: Lannie Dodson

COMPANY: City of Portland Inspections Division

ADDRESS: Portland City Hall
389 Congress Street
Portland, ME 04101

JAN 11 2006

DISTRIBUTION:

FROM: Alan H. Rice, R.A., NCARB

PROJECT NAME: Whole Foods – Portland, ME PROJECT NUMBER: 04C0829

SUBJECT: Permit Certificates

RECEIVED JAN 11 2006

WE ARE SENDING YOU:

☒ Attached ☐ Under separate cover via the following items:

☐ Shop Drawings ☐ Prints ☐ Plans ☐ Samples ☐ specifications

☐ Copyofletter ☐ Changeorder ☒ Other: As Noted Below

COPIES	DATE	NO.	DESCRIPTION
1	1/9/06	`	Statement of Special Inspections

THESE ARE TRANSMITTED as checked below:

☐ For approval	☐ No exceptions taken	☐ Resubmit	☐ For approval
☒ For your use	☐ Furnish as corrected	☐ Submit	☐ For distribution
☐ As requested	☐ Revise and resubmit	☐ Return	☐ Corrected pints
☐ For review & comment	☐ Rejected	☐ Prints returned after loan to us	
☐ For bids due , 200	☐ Resubmitfor record	☐	

REMARKS:
The attached Statement of Special Inspections is forwarded to you for the Demolition and Foundation Permit Applications for the Whole Foods Market project at Franklin St., Fox/Somerset St. and Pearl St. Please advise if additional information is required to complete this application.

Signature: Alan H. Rice, R. A., NCARB / Project Manager

ARCHITECTURE
ENGINEERING
PLANNING
LANDSCAPE ARCHITECTURE
LAND SURVEYING
ENVIRONMENTAL SCIENCES

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Meriden. Connecticut
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800 301 3077 T
203.630.1406 T
203.630.2615 F

blcompanies com

Statement of Special Inspections

Project: Whole Foods Market Proposed Grocery Store

Location: Franklin Street, Fox/Somerset Street, and Pearl Street

Owner: Whole Foods Market

Owner's Address: 125 Cambridge Park Drive
Cambridge, MA

Architect of Record: BL Companies

Structural Engineer of Record: BL Companies

This Statement of Special Inspections *is* submitted as a condition for permit issuance in accordance with the Special Inspection requirements of the International Building Code, 2003. It includes a Schedule of Special Inspection Services applicable to this project as well as the name of the Special Inspector and the identity *of* other approved agencies intended to be retained for conducting these inspections.

The Special Inspector shall keep records of all inspections and shall furnish inspection reports to the Building Official, Structural Engineer and Architect *of* Record. Discovered discrepancies shall be brought to the immediate attention of the Contractor for correction. If such discrepancies are not corrected, the discrepancies shall be brought to the attention of the Building Official, Structural Engineer and Architect of Record. The Special Inspection program does not relieve the Contractor of his or her responsibilities.

Interim reports shall be submitted to the Building Official, Owner, Structural Engineer and Architect of Record.

A Final *Report of* Special *Inspections* documenting completion of all required Special Inspections and correction of any discrepancies noted in the inspections shall be submitted prior to issuance of a Certificate of Use and Occupancy.

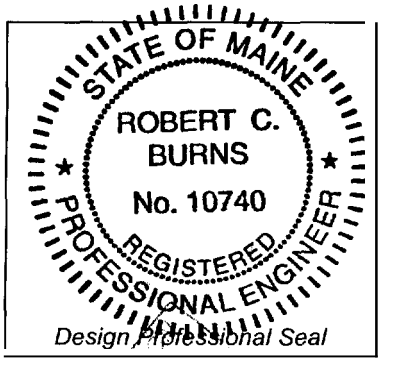
Job site safety and means and methods of construction are solely the responsibility of the Contractor.

Interim Report Frequency: Weekly or ☐ per attached schedule.

Prepared by:

Robert Burns, PE
(type or print name)

1/9/2005
Signature Date



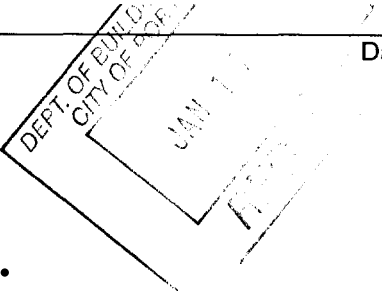
Design Professional Seal

Owner's Authorization:

1/20/06
Signature Date

Building Official's Acceptance:

Signature Date



Schedule of Special Inspection Services

The following sheets comprise the required schedule of special inspections for this project. The construction divisions which require special inspections for this project are as follows:

- ☒ Soils and Foundations
- [XI Cast-in-Place Concrete
- ☐ Precast Concrete
- ☒ Masonry
- ☒ Structural Steel
- ☒ Cold-Formed Steel Framing
- ☐ Spray Fire Resistant Material
- ☐ Wood Construction
- ☒ Exterior Insulation and Finish System
- ☐ Special Cases

Inspection Agents	Firm	Address
1. Special Inspector	*T.B.D.*	
2. Testing Laboratory	*T.B.D.*	
3. Other		
4. Other		

The inspection and testing agent shall be engaged by the Owner or the Owner's Agent, and not by the Contractor or Subcontractor whose work is to be inspected or tested. Any conflict of interest must be disclosed to the Building Official, prior to commencing work.

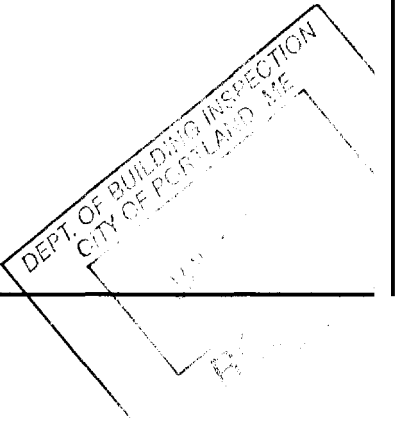
The credentials of all Inspectors and testing technicians shall be provided if requested.

It is recommended that the person administering the Special Inspections program be a Professional Engineer experienced in the design of buildings.

Key for Minimum Qualifications of Inspection Agents (where indicated on Schedules)	
PE	Professional Engineer
EIT	Engineering in Training
ACI	American Concrete Institute Certified Concrete Field Testing Technician
AWS	American Welding Society Certified Welding Inspector
ASNT	American Society of Non-Destructive Testing - Level II or III

Qualifications of inspection agents may be indicated on the Schedule in instances where the Structural Engineer deems such requirements are appropriate.

Item	Agent No. (Qualif.)	Scope
1. Shallow Foundations	1 (PE) 2 (EIT)	Review Field Reports Visual confirmation of soil quality below footings.
2. Controlled Structural Fill *1705.7*	1 (PE) 2 (EIT)	Review field reports. Verify compaction operations for raise-in-grade fill within building perimeter. Perform compaction tests for lifts beneath slab-on-grade and footings on fill.
3. Deep Foundations *1705.8; 1705.9; 1816.13*		
4. Other		



Item	Agent No. (Qualif.)	Scope
1. Mix Design[1] *1705.4.1*	1 (PE)	Review submittals for all structural concrete.
2. Material Certification[1] *1705.4.1*	1 (PE)	Review material certificates for all structural concrete, including admixtures.
3. Reinforcement Installation[2] *1705.4.2*	1 (PE) 2 (EIT)	Review field reports Inspect placement of reinforcement for footings, foundations, slab-on-grade.
4. Post-Tensioning Operations[3] *1705.4.5*		Not applicable
5. Batching Plant[4] *1705.4.4*		Not applicable
6. Formwork Geometry *CT 1705.4.3*	1 (PE) 2 (EIT)	Review field reports Verify during pre-placement inspections for footings and foundations
7. Concrete Placement[1] *1705.4.4*	1 (PE) 2 (ACI)	Review concreting operations with contractor. Review field reports. Verify proper placement techniques.
8. Evaluation of Concrete Strength[1] *1705.4.4*	1 (PE) 2 (ACI)	Review test results. Test all structural concrete per specification.
9. Curing and Protection[1] *1705.4.4*	1 (PE) 2 (ACI)	Review curing operations with contractor. Review field reports. Verify proper curing methods and times are used.
10. Other		Not Applicable

	Agent No. (Qualif.)	Scope
1. Material Certification *1705.5*	1 (PE)	Review all submittals required by Specification.
2. Mixing of Mortar and Grout[9] *1705.5*	1 (PE) 2 (EIT)	Review field reports. Review field operations per Specification.
3. Installation of Masonry" *1705.5*	1 (PE) 2 (EIT)	Review field reports Inspect per Specifications for proper installation methods and procedures.
4. Reinforcement Installation" *I705.5*	1 (PE) 2 (EIT)	Review field reports Inspect for proper placement, ensure sizes and spacing are per plans.
5. Grouting Operations[12] *1705.5*	1 (PE) 2 (EIT)	Review field reports Verify operations and procedures are per specifications.
6. Weather Protection[13] *1705.5*	1 (PE) 2 (EIT)	Review field reports Ensure adequate protection is being provided as per Specifications.
7. Evaluation of Masonry Strength[14] *I705.5*	1 (PE) 2 (EIT)	Review field reports Provide testing as per Specifications.
8. Anchors and Ties[15] *I705.5*	1 (PE) 2 (EIT)	Review field reports. Inspect for proper placement, ensure types and spacing are per plans.
9. Other		Not applicable

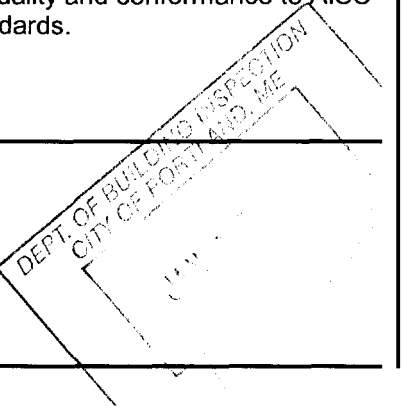
Structural Steel

Item	Agent No. (Qualif.)	scope
1. Fabricator Certification/ Quality Control Procedures *1705.2;1705.3.1*	1 (PE)	Verify fabricator's quality control program, if not AISC certified.
2. Material Certification[16, 17, 18, 19,] *1705.3.2*	1 (PE) 2 (EIT)	Review all submittals required by specifications. Field review markings.
3. Open Web Steel Joists	1 (PE) 2 (EIT)	Review manufacturer's submittals. Review field reports. Field review bridging and support attachments.
4. Bolting[L"] *1705.3.3.1*	1 (PE) 2 (EIT)	Review field reports. All bolts to be visually evaluated. Test per Specifications when visual discrepancies are observed.
5. Welding[L'] *1705.3.3.2*	1 (PE) 2 (AWS)	Review welding certificates. Review field reports. All welds to be visually evaluated. Test per specifications when visual discrepancies are observed. Ultrasonically test all full-penetration welds.
6. Shear Connectors[LL]	1 (PE) 2 (EIT)	Review field reports. Field test shear connectors using ring test or bend test.
7. Structural Details *1705.3.3.3*	1 (PE) 2 (EIT)	Review field reports. Field review all framing for quality and conformance to AISC erection and fabrication standards.
8. Metal Deck	1 (PE) 2 (EIT)	Review manufacturers submittals. Review field reports. Field review deck placement, visually inspect deck attachments.
9. Other		Not applicable.

Cold-Formed Steel Framing

Project: Whole Foods Market – Portland, ME

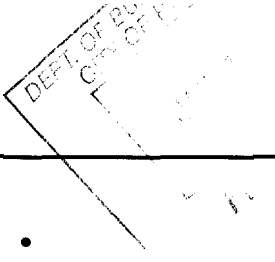
Item	Agent No. (Qualif.)	Scope
1. Member Sizes	1 (PE) 2 (EIT)	Review field reports. Verify that sizes and spacing of joists, studs, posts, or strapping correspond to drawings.
2. Material Thickness	1 (PE) 2 (EIT)	Review field reports. Verify that gauge (thickness) *of* joists, studs, posts, or strapping correspond to drawings.
3. Material Properties	1 (PE) 2 (EIT)	Review manufacturers product data to ensure conformance with ASTM standards cited in Specifications. Field verify that correct ASTM designations are being used.
4. Mechanical Connections	1 (PE) 2 (EIT)	Review field reports. Field verify size, type, and number of screws at critical connections.
5. Welding		Not applicable.
6. Framing Details	1 (PE) 2 (EIT)	Review field reports Field review all framing for quality and conformance to AISC erection and fabrication standards.
7. Other		



Exterior Insulation & Finish Systems (EIFS)

Project: Whole Foods Market – Portland, ME

Item	Agent No. (Qualif.)	Scope
1. Material Submittals	1 (PE)	Review all submittals.
2. Condition of Substrate *1705.13*	1 (PE)	Review field reports.
	2 (EIT)	Verify substrate conforms with drawings. Verify substrate complies with tolerances per specifications.
3. Application of Foam Plastic Board *1705.13*	1 (PE)	Review Field reports.
	2 (EIT)	Verify application procedures conform with drawings. Verify materials conform with specifications.
4. Application of Coatings *1705.13*	1 (PE)	Review field reports.
	2 (EIT)	Verify materials and application procedures conform with specifications.
5. Application of Mesh *1705.13*	1 (PE)	Review field reports
	2 (EIT)	Verify application procedures comply with specifications. Inspect mesh installation at corners and high impact areas. Verify back-wrapping at all insulation start/stop points.
6. Ambient Condition and Curing *1705.13*	1 (PE)	Review field reports.
	2 (EIT)	Verify conditions are appropriate for application. Verify surfaces are not damp or frozen. Insure curing procedures conform with specifications. Verify finishes are not installed in direct sunlight.
7. Flashing and Joint Details *1705.13*	1 (PE)	Review field reports
	2 (EIT)	Verify isolation/ expansion/ aesthetic joints conform with drawings. Verify flashing is installed according to drawings.
8. Sealants/Caulks *1705.13*	1 (PE)	Review field reports
	2 (EIT)	Insure materials conform with specifications. Inspect sealant installation, verify backer –rod installation.
9. Other		Not applicable



References

1. ACI 318-02, *Building Code Requirements for Structural Concrete,* Chapter 3.
2. ACI 318-02, *Building Code Requirements for Structural Concrete,* § 7.4, 7.5, 7.6 and 7.7.
3. ACI 318-02, *Building Code Requirements for Structural Concrete,* § 18.18.
4. ACI 318-02, *Building Code Requirements for Structural Concrete,* Chapter 4 and § 5.2, 5.3, 5.4 and 5.8.
5. ACI 318-02, *Building Code Requirements for Structural Concrete,* § 5.9 and 5.10.
6. ACI 318-02, *Building Code Requirements for Structural Concrete,* § 5.6
7. ACI 318-02, *Building Code Requirements for Structural Concrete,* § 5.11, 5.12 and 5.13.
8. ACI 530.1 / ASCE 6 / TMS 602 – 02, *Specifications for Masonry Structures,* § 2.3.
9. ACI 530.1 / ASCE 6 / TMS 602 – 02, *Specifications for Masonry Structures,* § 2.6.
10. ACI 530.1 / ASCE 6 / TMS 602 – 02, *Specifications for Masonry Structures,* § 3.2.
11. ACI 530 / ASCE 5 / TMS 402 – 02, *Building Code Requirements for Masonry Structures,* Chapter 8.
12. ACI 530.1 / ASCE 6 / TMS 602 – 02, *Specifications for Masonry Structures,* § 3.5.
13. ACI 530.1 / ASCE 6 / TMS 602 – 02, *Specifications for Masonry Structures,* § 1.8.
14. ACI 530.1 / ASCE 6 / TMS 602 – 02, *Specifications for Masonry Structures,* § 1.4.
15. ACI 530 / ASCE 5 / TMS 402 – 02, *Building Code Requirements for Masonry Structures,* § 4.2 and 5.14.
16. AISC ASD – 89, *Specification for Structural Steel Buildings – Allowable Stress Design and Plastic Design,* § A3.4 and A3.6.
17. AISC LRFD – 99, *Load and Resistance Factor Design Specification for Structural Steel Buildings,* § A3.3 and A3.5.
18. ASTM A6 – 01**b,** *Specification for General Requirements for Rolled Steel Plates, Shapes, Sheet Piling, and Bars for Structural Use.*
19. ASTM A568 – 01, *Specification for Steel Sheet, Carbon and High-Strength, Low-Alloy, Hot-Rolled and Cold Rolled, General Requirements For.*
20. RCSC - 85 (88), *Specification for Structural Joints Using A325 or A490 Bolts,* § 9.
21. AWS D1.1 – 00, *Structural Welding Code – Steel,* § 6.
22. AWS D1.1 – 00, *Structural Welding Code – Steel,* § 7.8.