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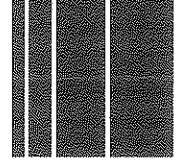

Report on Final Design Subsurface and Foundation Investigation

Proposed Village at Ocean Gate Portland, Maine

for

The Village at Ocean Gate, LLC
c/o Atlas Investment Group, LLC
10 High Street
Boston, MA 02110

October 3, 2007

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Mr. Demetri Dasco, Managing Partner
The Village at Ocean Gate, LLC
c/o Atlas Investment Group, LLC
10 High Street
Boston, MA 02110

Report on Final Design Subsurface and Foundation Investigation
Proposed Village at Ocean Gate, Portland, Maine

Dear Demetri:

This report presents the results of our final design subsurface and foundation investigation for the proposed Village at Ocean Gate project in Portland, Maine. Foundation recommendations are limited to proposed Buildings 1 and 2 that make up Phase I only. Phase II, located along the northwestern side of Newbury Street, was not approved and the design is pending.

In summary, it is our opinion that the structure may be founded on spread and continuous footings bearing on undisturbed, naturally deposited soil in the northern portion of the building, or on soil improved by a process known as rammed aggregate piers at locations in the southern portion of the building. In addition, an earth-supported bituminous pavement may be used for the lowest level (parking level). Specific recommendations regarding foundation design and construction considerations are presented below.

Introduction

The Village at Ocean Gate will be developed on the site of the Village Café located at 112 Newbury Street. The area for proposed redevelopment is bound approximately by Middle Street, Hancock Street, Newbury Street and midway between Hancock and India Streets. The Village Café building will be removed to allow the new development. We understand that this Phase 1 of the development will include one level of underground parking between Newbury Street and Middle Street with two four-story, wood-framed residential buildings and courtyard above the parking. Existing ground surface elevations within the Phase I area vary from approximately El. 28 to El. 41.

The lowest level, with a plan area of approximately 42,000 square feet, will consist of retail and building entrances along Middle Street, with parking occupying the remainder of the lowest level. The retail areas will have floor levels at approximately El. 27. The garage entrance at Middle Street is at approximately El. 27 and ramps to approximately El. 33.3 at Newbury Street. Floor levels will be at or near existing grade near Middle Street, but will

require excavations of up to 11 feet below Newbury Street. Columns will have variable spacing, and column loads vary from approximately 76 kips in the retail area to 370 kips in the residential area. The residential buildings will have plan areas of approximately 16,800 square feet, with the lowest level at the courtyard at approximately El. 44.

Subsurface Explorations

Preliminary Borings

During the period June 20 to 21, 2005 Maine Test Borings, Inc. (MTB) of Brewer, Maine, drilled six borings, B1 to B6, at locations shown on Sheet 1, Subsurface Exploration Plan. MTB drilled the borings to depths below ground surface varying from 29.2 feet to 32.0 feet. Sebago Technics, Inc. monitored the borings and prepared the logs included in Appendix A. Table I summarizes the results of borings. MTB backfilled the borings with the excavated soil.

Borings were drilled using 2.5 inch inside diameter hollow stem augers. Soil samples were generally taken at 5-foot intervals. Standard Penetration Resistance (N) was measured at each sample interval in accordance with ASTM Test D1586. Undrained shear strength of the clay stratum was measured at various depths by field vane shear tests.

Sebago Technics determined the locations of borings by pacing from existing site features. We determined the ground surface elevations at borings by linear interpolation between the City of Portland ground surface contours at the plotted locations.

Final Design Borings

During the period December 21, 2005 to January 5, 2006, MTB drilled eight borings, B101 to B105 and B107 to B109, at locations shown on Sheet 1. MTB drilled the borings to depths below ground surface varying from 20.0 feet to 42.0 feet. Sebago Technics, Inc. monitored the borings and prepared the logs included in Appendix B. Table I summarizes the results of borings. MTB backfilled the borings with the excavated soil.

Boring B101 was drilled using 4-inch inside diameter flush joint casing. Two 3-inch diameter thin wall samples by stationary piston method were recovered. The remaining borings were drilled using 2.5-inch and 3.375-inch inside diameter hollow stem augers. Soil samples were generally taken at 5-foot intervals. Standard Penetration Resistance (N) was measured at each sample interval in accordance with ASTM Test D1586. Undrained shear strength of the clay stratum was measured at various depths by field vane shear tests. Groundwater observation wells were installed in completed borings B102 and B109.

Sebago Technics determined the locations of borings by pacing and taping from existing site features. We determined the ground surface elevations at borings by linear interpolation between the ground surface contours at the plotted locations.

The boring logs and related information depict the subsurface conditions and water levels encountered at the locations and during the times indicated on the logs. Subsurface conditions at other locations may differ from those encountered in the borings. The passage of time may result in a change in groundwater conditions at the exploration.

Subsurface Conditions

The borings encountered six principal soil units at the site overlying bedrock: fill, upper marine sand, marine silt, marine clay, lower marine sand and glacial till. Encountered thickness and generalized descriptions of the strata encountered are presented below in order of increasing depth below ground surface. Due to the complexity of the deposition process, strata thickness will vary and may be absent at specific locations.

Fill – Fill consists of loose to very dense, brown well-graded SAND with gravel (SW); to brown to black silty SAND (SM), with various amounts of brick, ash, wood and glass. Encountered thickness varies from 1.3 feet to 6.6 feet.

Sand – The upper layer of marine sand consists of very loose to dense, brown to gray silty SAND (SM); to poorly-graded SAND (SP). Encountered thickness varies from 1.3 feet to 17.8 feet.

Silt – Marine silt consists of soft to medium stiff, gray SILT (ML). Encountered thickness varies from 8.0 feet to 10.0 feet.

Clay – Marine clay consists of soft to stiff, gray brown to gray lean CLAY (CL) with frequent sand layers and seams. Undrained shear strength of the lower portion, measured by field vane shear tests, varied from 260 pounds per square feet (psf) to 820 psf. Undrained shear strength of the upper portion, based on correlations with N values, likely varies from 1,000 to 2,000 psf. Encountered thickness varies from 5.5 feet to 24.8 feet.

Sand – The lower marine sand consists of very loose to very dense, gray to brown silty SAND (SM); to poorly-graded SAND (SP) with occasional silt seams. Encountered thickness varies from 2.6 feet to 8.9 feet.

Glacial Till – Glacial till consists of loose to very dense, gray silty SAND with gravel (SM); to well-graded SAND (SW) with cobbles and boulders. Encountered thickness varies from 0.7 foot to greater than 35.5 feet.

Borings B2, B4, B6, B101, B105, B107 and B109 encountered refusal judged to be bedrock at depths below ground surface varying from 29.2 feet to 41.1 feet.

Water was observed in the borings at depths below ground surface varying from 7.0 feet to 18.5 feet. Water levels in the groundwater observation wells in B102 and B109 varied from 2.6 feet to 3.3 feet below ground surface (equivalent elevations varied from El. 25.9 to El. 34.2, respectively). Observations of water were made over a relatively short period of time and may not reflect the stabilized groundwater level. In addition, water levels at the site will vary with season, precipitation, temperature and construction activity in the area. Therefore, water levels during and following construction may vary from those observed in the borings.

Strength and Compressibility Characteristics of Clay Stratum

We estimated the stress history of the clay deposit by correlations with strength ratio, the ratio of shear strength to overburden stress, of similar clays in the area. The undrained shear strength of the lower portion of the clay stratum was measured in borings using field vane shear tests. Measured shear strength varies from 260 psf to 820 psf. The undrained shear strength of the upper portion of the deposit was estimated to be as high as 2,000 psf. The stress history of the deposit was estimated by comparing the strength ratio with correlations of strength ratio and stress history of clay from other projects with similar conditions. The estimated stress history is shown in Figure 1.

The stress history and appropriate compression ratios were estimated for the clay deposit as discussed above. The correlations indicate that the deposit is moderately overconsolidated; that is, the existing overburden stress is at least 1,000 psf less than the maximum previous stress in the lower portion of the clay and more than 3,000 psf near the top. The deposit likely became overconsolidated due to desiccation (drying) resulting from a lowering of the groundwater level at some time in the geologic past which created a stiff upper crust and also increased the effective overburden stress throughout the stratum.

The stress-strain or compressibility characteristics (settlement) of clays are highly dependent upon their stress history. If clay is stressed within the limits of the maximum previous stress, σ_{vm}, the strain (settlement) will be a function of the recompression ratio (RR) of the clay. If the applied stress exceeds the maximum previous stress, the strain will be proportional to the virgin compression ratio (CR). The compression ratio is typically 10 to 15 times the recompression ratio.

Recommendations for Foundation Design

Recommended Foundation Type and Design Criteria

The existing fill is not considered suitable for support of the building foundations. All fill within the limits of the building foundations should be excavated and replaced with non-woven geotextile fabric and ¾-inch crushed stone. We recommend that the building be supported on spread and continuous footings bearing on the undisturbed, naturally deposited soils that have been over excavated and protected by 3-inch thick lean concrete mud mats or non-woven geotextile fabric and a minimum 6-inch thickness of ¾-inch crushed stone. The geotextile fabric should have a minimum weight of 6 ounces per square yard. Columns and foundation walls in the southern portion of the building should be supported on soil that has been improved by rammed aggregate piers.

Footings should be proportioned for an allowable bearing stress in pounds per square foot (psf) equal to 3,000 psf. All footings should be a minimum of 2.0 feet wide.

Exterior footings should be founded at least 4.5 feet below the lowest adjacent ground surface exposed to freezing. Interior footings, if they are located in areas not subject to freezing temperatures, should be founded a minimum of 1.5 feet below the ground floor. If the garage is exposed to freezing temperatures, footings should be founded at least 4 feet below the ground floor.

In order to consider foundations bearing above the clay stratum, we estimated the settlement of the clay resulting from the increased stress from the building loads. We estimate that the total settlement of the northern half of the building will vary from 0.3 inch to 1.0 inch due to the compensation for the weight of the building with soil excavated for the parking level. We estimate that the total settlement in the southern half of the building will vary from 1.0 inch to 2.5 inches due to the lack of compensation for the building weight. We anticipate that settlement in the northern half of the building is acceptable. We recommend that the clay below the columns and wall footings in the southern half of the building, except in retail areas where foundation walls are limited to depth of freezing, be improved with the installation of rammed aggregate piers as installed by Geopier Foundation Company (Geopier). We recommend that Geopier design a system that will limit settlement below columns and walls to less than 1.0 inch. We anticipate that settlement of this magnitude is acceptable. However, JSN Associates, Inc. should determine final acceptability of settlement.

Ground Floor Slabs

We understand that most of the lowest level floor will consist of bituminous concrete for parking. The remainder of the lowest level will consist of retail, restaurant operations and building entrances, lobbies, elevators and stairs. We recommend that the lowest level floor slabs in these areas be designed as earth-supported slabs-on-grade, and the parking areas be designed as bituminous concrete pavement, bearing on a minimum of 12 inches of ¾-inch crushed stone underlain by non-woven geotextile fabric. The geotextile fabric should have a minimum weight of 6 ounces per square yard. Borings indicate that the existing fill consists primarily of well-graded sand to silty sand with various amounts of brick, ash, wood and glass. In our opinion, the existing fill is suitable for support of the slabs following proofrolling, as discussed below. Any soft or yielding areas encountered during proofrolling should be excavated and replaced with fabric and ¾-inch crushed stone. Normal dampproofing and vapor barriers should be provided below the slabs.

We recommend that the pavement for the lowest level parking consist of 3 inches of bituminous concrete, placed in two layers.

We recommend that a perimeter foundation drain and under slab drain system be constructed in the building. The perimeter foundation drain should be constructed on the outside of the foundation walls and an underslab drain below the basement slab. Drains should consist of 4-inch diameter perforated pipe surrounded by ¾-inch crushed stone and non-woven geotextile filter fabric. The invert of the foundation drains should be below the basement floor level and the underslab drain should include a loop around the perimeter of the slab and several cross-laterals to provide multiple paths for water flow. Gravity discharge and normal dampproofing and vapor barriers should be provided for the slabs and basement walls. A device to prevent backflow and a provision for pump discharge should be provide for gravity discharge in the event that water rises above the discharge invert level.

If gravity discharge is not available, discharge from the system may be accomplished by pumping. In order to provide for backup discharge, the system should be designed to pump from two sumps, one at each end of the lower ends of the basement, with standby pumps and emergency electric power available in the event of a power failure. We recommend that the discharge from each sump be designed for a flow of 50 gallons per minute.

Seismic Design Considerations

We recommend that the building be designed in accordance with the seismic requirements of the latest edition of the International Building Code; the site classification is Class D based on a calculation of the weighted average of overburden in the top 100 feet of the site; the site response coefficient F_a is 1.5 for a short period spectral response acceleration S_s of 0.375g; the site response coefficient F_v is 2.4 for the 1-second period spectral response acceleration S_1 of 0.10g. The subgrade soils are not considered liquefaction susceptible.

Lateral Foundation Loads

We recommend that lateral loads be resisted by bottom friction on footings and that a coefficient of friction equal to 0.35 be used for footings. If this does not provide sufficient lateral resistance, we will consider the problem in more detail to take into account other factors.

Lateral Soil Pressure

We recommend that basement walls which are restrained at the top and backfilled be designed to resist a lateral earth pressure calculated on the basis of an equivalent fluid unit weight of 55 pounds per cubic foot. This fluid unit weight assumes an at rest earth pressure coefficient of 0.45, a free-draining granular backfill, and an effective drainage system.

Backfill Materials

Structural fill used for backfill adjacent to walls should consist of sandy gravel to gravelly sand. It should be free of organic material, loam, trash, snow, ice, frozen soil and other objectionable material, and should conform to the following gradation:

Sieve Size	Percent Finer by Weight
3 inches	100
No. 4	30 to 90
No. 40	10 to 50
No. 200	0 to 8

Compacted structural fill should be placed in layers not exceeding eight inches in loose measure and compacted by self-propelled vibratory equipment at the approximate optimum moisture content to a dry density of at least 95 percent of the maximum dry density, as determined in accordance with ASTM Test Designation D1557. In confined areas, the loose layer thickness should be reduced to 6 inches and compaction performed by hand-guided vibratory equipment.

Compacted structural fill on the exterior of the foundation and basement walls should extend laterally a minimum of 2 feet from the wall. Backfill beyond this limit on the exterior of the building may consist of common fill. The top 12 inches of fill on the exterior of the building should consist of low permeability material or sidewalk pavement to minimize water infiltration next to the building. Grading should provide for runoff away from the building.

Common fill may consist of inorganic mineral soil that can be placed in layers and compacted. Common fill should be placed and spread in layers not exceeding 12 inches in thickness and compacted at the approximate optimum moisture content to a dry density of at least 92 percent of the maximum dry density, as determined in accordance with ASTM Test Designation D1557.

Construction Considerations

General

The primary purpose of this section of the report is to comment on items related to excavation, earthwork, and related geotechnical aspects of proposed construction. It is written primarily for the engineer having responsibility for preparation of plans and specifications. Since it identifies potential construction problems related to foundations and earthwork, it will also aid personnel who monitor the construction activity. Prospective contractors for this project must evaluate the construction problems on the basis of their own knowledge and experience in the Portland, Maine area, and on the basis of similar projects in other localities, taking into account their proposed construction methods, procedures, equipment and personnel.

Excavation, Lateral Support and Control of Water

Due to the proximity of city streets and adjacent buildings, foundation excavation for the lowest level floor, and foundations in approximately the northern half of the building, a temporary earth support system will be required. Excavations up to 11 feet below street level will be required along Newbury Street, portions of Hancock Street, and the west side of the project. Lateral support schemes that may be considered by the contractor include interlocking steel sheeting and soldier beams. We anticipate that internal lateral bracing may be required due to anticipated restrictions on installing external support such as tiebacks, which would extend below the city streets and adjacent buildings.

The temporary excavation support system must provide lateral support of the excavation, prevent damage to adjacent buildings, streets and utilities, and be designed to the following criteria:

1. Position the highest brace no deeper than 6 feet below existing exterior grade.
2. Excavation should not proceed more than 2 feet below any bracing level prior to the installation and loading of the brace.
3. Maintain the maximum cumulative horizontal movement at any point along the temporary excavation support walls to less than 1.5 inches.
4. Maintain the maximum vertical movement of any buildings to less than 0.75 inch; any utilities and streets to less than 1.0 inch.

If any of the above movement limits are exceeded, the contractor should immediately submit and implement a Movement Mitigation Plan. The plan may include additional vertical and horizontal supports or other measures. The contractor should demonstrate that the proposed measures can be implemented immediately, if required, to prevent damage to the adjacent buildings and streets and utilities.

Based on observed groundwater levels in borings and observation wells, dewatering will be required during excavation and foundation construction and until the permanent perimeter foundation and underslab drain system is operational. Based on the water levels observed, groundwater is likely perched in the fill and sand deposit above the clay stratum and is present in the sand and glacial till below the clay stratum. Perched groundwater may be as much as 3 to 4 feet above the lowest excavation level. Subsurface data indicate that excavation will be made in sand, silt and clay. In our opinion, dewatering and control of water from other sources can likely be controlled by sumps with open pumping. Dewatering must be done in a manner which will preserve the undisturbed bearing capacity of the bearing soils and permit construction "in-the-dry." Sumps and pumps should be installed with adequate filters to minimize loss of fine-grained soil.

We recommend that the contractor's proposed methods for making and dewatering the excavation be designed by a licensed professional engineer and the scheme submitted to the owner's engineer for review and comment prior to installation.

Subgrade Preparation

The subgrade soil is susceptible to disturbance from construction traffic. Equipment and personnel should not be permitted to travel across exposed footing bearing surfaces or exposed slab subgrades. As discussed above, we recommend that footing bearing surfaces be protected with 3-inch thick lean concrete mudmats or non-woven geotextile fabric and 6-inch thickness of ¾-inch crushed stone. Slab subgrade surfaces should be protected with non-woven geotextile fabric and 12-inch thickness of ¾-inch crushed stone. Any subgrade areas that are disturbed should be recompacted or excavated and replaced with crushed stone prior to placing mudmats or crushed stone. Subgrades should be protected against freezing temperatures if exposed during construction. Final excavation to subgrade should be performed using equipment with smooth-edge buckets.

Limitations of Recommendations

This report has been prepared for specific application to the subject project in accordance with generally accepted geotechnical engineering practices. In the event that any changes in the nature, design or location of the structure are planned, the conclusions and recommendations contained in this report should not be considered valid, unless the changes are reviewed and the conclusions of this report modified or verified in writing.

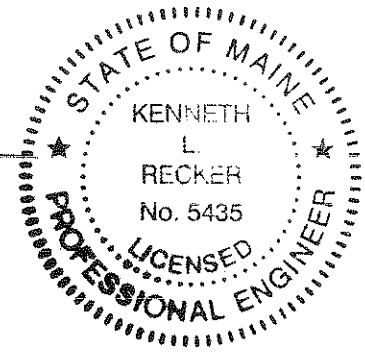
The recommendations presented herein are based in part upon the data obtained from the referenced borings. The nature and extent of variations from that disclosed by the borings may not become evident until construction. If variations then appear evident, it will be necessary to re-evaluate the recommendations of this report.

We request that we be provided the opportunity for a general review of final design and specifications in order to determine that our earthwork and foundation recommendations have been interpreted and implemented in the design and specifications as they were intended.

It has been a pleasure to work with you on this project. Please do not hesitate to contact us if you have any questions or need additional information.

Sincerely,

SEBAGO TECHNICS, INC.



Kenneth L. Recker, P.E.
Geotechnical Engineering Manager

KLR:KLR/JC
Enclosures:

Table I	- Summary of Borings
Sheet 1	- Subsurface Exploration Plan
Figure 1	- Stress History Phase I
Appendix A	- Logs of Preliminary Borings
Appendix B	- Logs of Final Design Borings

05109

TABLE I
SUMMARY OF BORINGS
PROPOSED VILLAGE AT OCEAN GATE
PORTLAND, MAINE

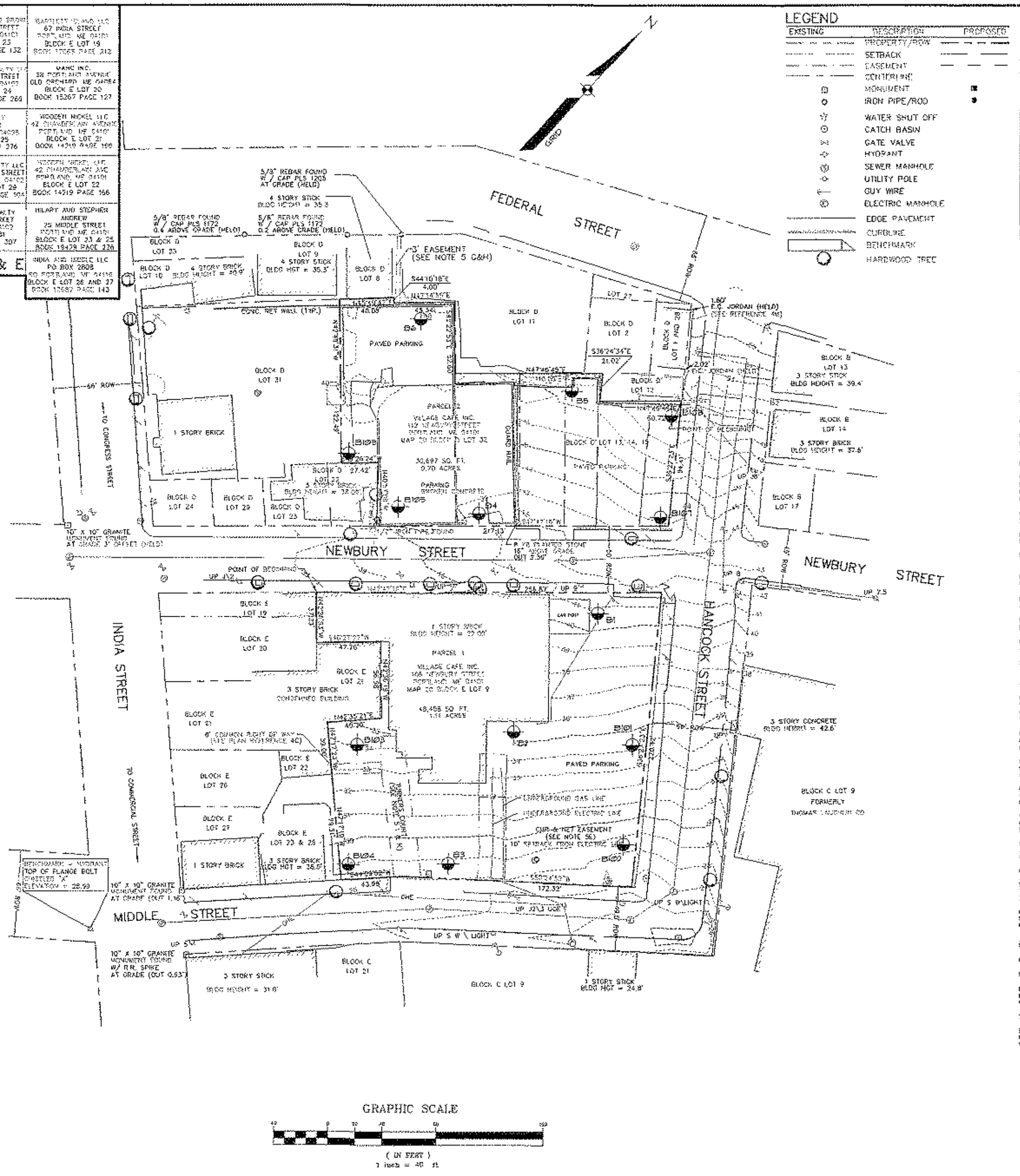
Boring Number	Depth (ft)	Approx. Gnd. Sur. El. (Ft)	Depth to Water (Ft)	Strata Thickness (Ft)						
					Marine Deposits				Glacial Till	Bedrock
				Fill	Upper Sand	Silt	Clay	Lower Sand		
B101	30.6	34.2	7.2	3.0	--	--	20.7	3.5	3.3	0.1*
B102	32.0	28.5	2.6	4.0	2.4	--	14.6	8.9	1.6*	--
B103	20.0	34.1	6.0	6.3	--	--	11.0	2.7*	--	--
B104	42.0	29.0	7.8	5.2	1.3	--	--	--	35.5*	--
B105	24.0	39.5	8.0	2.0	--	--	17.0	2.6	2.4	0.0*
B107	41.1	44.1	7.0	6.6	4.9	--	24.8	3.7	1.1	0.0*
B108	40.0	49.2		3.0	17.8	--	15.2	4.0*	--	--
B109	21.4	38.5	3.3	1.2	--	--	15.6	3.7	0.9	0.0*
B1	32.0	40.7	7.0	3.5	7.0	8.0	13.5*	--	--	--
B2	30.1	35.6	11.3	4.0	--	--	17.0	5.9	3.2	0.0*
B3	32.0	28.8	10.9	3.3	--	--	18.2	4.5	6.0*	--
B4	30.7	39.9	10.0	3.0	3.0	--	20.0	4.0	0.7	0.0*
B5	32.0	47.5	11.3	1.3	7.2	--	14.5	6.0	3.0*	--
B6	29.2	42.2	9.8	1.3	2.2	10.0	5.5	7.3	2.9	0.0*

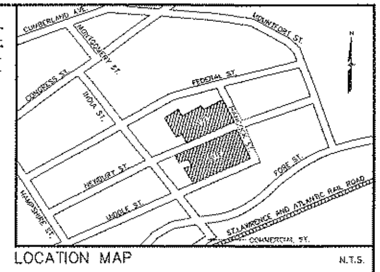
NOTES:

1. -- INDICATES STRATUM NOT ENCOUNTERED WITHIN DEPTH OF BORING.
2. * INDICATES DEPTH OF PENETRATION INTO STRATUM.

10/3/07,

[illegible] PO BOX 8119 PORTLAND, ME 04104 BLOCK B LOT 13 BOOK [illegible]	MOUKOI BROTHERS INC 47 INDIA STREET PORTLAND, ME 04101 BLOCK C LOT 27 BOOK [illegible] PAGE 103	FRANCIS JACKSON 138 HARTLEY STREET PORTLAND, ME 04103 BLOCK D LOT 10 BOOK 13590 PAGE 340	ALLISON AND DAVID BROWN 175 NEWBURY STREET PORTLAND, ME 04101 BLOCK D LOT 23 BOOK 15768 PAGE 132	[illegible] 67 INDIA STREET PORTLAND, ME 04101 BLOCK E LOT 19 BOOK [illegible] PAGE 213
[illegible] PROPERTIES LLC [illegible] FALMOUTH, ME 04105 BLOCK B LOT 14 BOOK [illegible] PAGE 140	TIM HARDY [illegible] STREET ST AUGUSTINE, FL 32080 BLOCK D LOT 1 AND 28 BOOK 12152 PAGE 50	49 FEDERAL STREET LLC [illegible] CHICAGO, IL 60640 BLOCK D LOT 11 BOOK 22369 PAGE 122	[illegible] REALTY LLC 312 ST. JOHN STREET PORTLAND, ME 04102 BLOCK D LOT 24 BOOK 14437 PAGE 266	MAHC INC. 38 PORTLAND AVENUE OLD ORCHARD, ME 04064 BLOCK E LOT 20 BOOK 15267 PAGE 127
HADLEY M. BROWN 97 NEWBURY STREET PORTLAND, ME 04101 BLOCK B LOT 17 BOOK 3934 PAGE 215	DAVID [illegible] 36 FEDERAL STREET #3 PORTLAND, ME 04101 BLOCK D LOT 2 BOOK 15975 PAGE 285	ROLAND SMALLEY JR. 73 WALL STREET PORTLAND, ME 04103 BLOCK D LOT 12 BOOK 12574 PAGE 217	BETTY POWSEY PO BOX 432 WESTBROOK, ME 04098 BLOCK D LOT 25 BOOK 5709 PAGE 276	WOODEN NICKEL LLC 42 CHAMBERLAIN AVENUE PORTLAND, ME 04101 BLOCK E LOT 21 BOOK 14219 PAGE 199
SHIPYARD BREWING CO LLC 86 NEWBURY STREET PORTLAND, ME 04101 BLOCK C LOT 9 BOOK 12206 PAGE 170	JONATHAN PRESTON PO BOX 1516 CHARLESTOWN, RI 02813 BLOCK D LOT 5 BOOK 18602 PAGE 237	VILLAGE CAFE INC. 108 NEWBURY STREET PORTLAND, ME 04101 BLOCK D LOT 13, 14, 15	[illegible] REALTY LLC 312 ST. JOHN STREET PORTLAND, ME 04102 BLOCK D LOT 29 BOOK 15585 PAGE 304	WOODEN NICKEL LLC 42 CHAMBERLAIN AVE PORTLAND, ME 04101 BLOCK E LOT 22 BOOK 14219 PAGE 156
MOUKOI BROTHERS INC 43 INDIA STREET PORTLAND, ME 04101 BLOCK C LOT 21 BOOK 13407 PAGE 126	KAC LLC 15 VISTA DRIVE SOUTH PORTLAND, ME 04106 BLOCK D LOT 8 BOOK 21982 PAGE 259	THOMAS AND MARCIA CABUSTA 123 NEWBURY STREET PORTLAND, ME 04101 BLOCK D LOT 32 BOOK 15415 PAGE 137	[illegible] REALTY 312 ST. JOHN STREET PORTLAND, ME 04102 BLOCK D LOT 31 BOOK 15585 PAGE 307	HILARY AND STEPHEN ANDREW 25 MIDDLE STREET PORTLAND, ME 04101 BLOCK E LOT 23 & 25 BOOK 19479 PAGE 226
ABUTTERS LIST: TAX MAP 20 BLOCKS B, C, D, & E				INDIA AND MIDDLE LLC PO BOX 2808 SO PORTLAND, ME 04116 BLOCK E LOT 26 AND 27 BOOK 12687 PAGE 143





LOCATION MAP

N.T.S.

GENERAL NOTES:

1) THE RECORD OWNER OF THE PROPERTY IS VILLAGE CAFE INC. DEEDS TO SUBJECT PARCELS ARE [illegible]

2) THE PROPERTY IS DEPICTED ON THE TOWN OF PORTLAND TAX MAP 20, BLOCK C LOT 9 AND BLOCK D, LOTS 13, 14, 15, AND 32.

3) THE BEARINGS SHOWN HEREON ARE BASED UPON GRID NORTH NAD 1983 MAINE WEST ZONE. CONTOURS AND ELEVATION SHOWN HEREON ARE BASED UPON NGVD 1929 VERTICAL DATUM.

4) PLAN REFERENCES:

A) PLAN TITLED "ALTA / ACSM TITLE SURVEY OF JORDAN'S MEATS PROPERTY, PREPARED FOR [illegible] BY SEBAGO TECHNICS, INC., DATED 4/29/05.

B) PLAN TITLED "PLAN OF LAND STANDARD BOUNDARY SURVEY ON FORE, INDIA, MIDDLE NEWBURY, HANCOCK, & MOUNTFORD STREETS, PORTLAND MAINE" PREPARED FOR AMERICAN HOIST & DERRICK CO., BY OWEN HASKELL, INC., DATED APRIL OF 1987.

C) ALTA/ACSM LAND TITLE SURVEY PREPARED FOR [illegible] REAL ESTATE HOLDINGS BY OWEN HASKELL INC., DATED NOV. [illegible]

D) PLAN TITLED "CITY OF PORTLAND, MAINE PUBLIC WORKS DEPARTMENT ENGINEERING SECTION – HAMPSHIRE STREET [illegible] BY THE CITY OF PORTLAND ENGINEERS OFFICE, DATED FEB 2002.

E) RIGHT OF WAY MAPS BY THE CITY OF PORTLAND: # 31, 52, 73, AND [illegible]

F) PLAN OF SEWER LINE – NEWBURY STREET, BUILT JUNE 1877, CITY PLAN #[illegible] PLAN OF SEWER LINE – MIDDLE STREET, RE-BUILT AUG. 1926, CITY PLAN #[illegible] PLAN OF SEWER LINE – INDIA, NEWBURY, FEDERAL, AND CONGRESS ST. RE-BUILT AUG. 1926, CITY PLAN #140/25.

G) PLAN OF PROPERTY IN PORTLAND, MAINE, PREPARED FOR THOMAS LAUGHLIN CO., DATED SEPT 2 1950; DRAWING #[illegible]

H) PLAN OF PORTLAND SEWER SYSTEM INFILTRATION – INFLOW ANALYSIS, PREPARED FOR PORTLAND WATER DISTRICT BY [illegible] ASSOCIATES, CITY DRAWING #V-4.

I) PORTLAND WATER DISTRICT, PORTLAND SHEET 9.

J) PLAN TITLED "PLAN OF LAND STANDARD BOUNDARY SURVEY ON FORE, INDIA, MIDDLE NEWBURY, HANCOCK, & MOUNTFORD STREETS, PORTLAND MAINE" PREPARED FOR SHIPYARD BREWING COMPANY, BY OWEN HASKELL, INC., DATED APRIL OF 1987, REVISED [illegible]

K) SITE PLAN "ADDITIONS TO: THE VILLAGE CAFE, PORTLAND, ME" BY JOHN H. LEASURE – ARCHITECT, INC., DATED JAN. 4, 1989.

L) PLAN TITLED "INDIA ST. REBUILDING SEWER – MIDDLE ST. TO CONGRESS ST." BY THE CITY OF PORTLAND DEPARTMENT OF PUBLIC WORKS, DRAWN ON [illegible], DRAWING #[illegible]

M) "PLAN OF PROPERTY IN PORTLAND, MAINE MADE FOR VILLAGE CAFE" DATED [illegible] BY [illegible] JORDAN.

5) SUBJECT TO:

A) RIGHTS AND EASEMENT GRANTED TO CENTRAL MAINE POWER COMPANY RECORDED IN THE CUMBERLAND COUNTY REGISTRY OF DEEDS IN BOOK 3456 PAGE 110.

B) RIGHTS AND EASEMENT GRANTED TO THOMAS LAUGHLIN COMPANY FOR NOISE, VIBRATION, SMOKE, SOOT, CINDERS, FUMES, GASES, AND VAPORS RECORDED IN THE CUMBERLAND COUNTY REGISTRY OF DEEDS IN THE FOLLOWING BOOKS AND PAGES: [illegible]

C) RIGHTS AND EASEMENT GRANTED BY AMERICO BEALE AND BRIGIDA DIBELLO TO CENTRAL MAINE POWER CO. AND NEW ENGLAND TELEPHONE CO. RECORDED IN THE CUMBERLAND COUNTY REGISTRY OF DEEDS IN BOOK 2806 PAGE 166.

D) RIGHTS AND EASEMENTS GRANTED BY AMERICO BEALE AND BRIGIDA DIBELLO TO CENTRAL MAINE POWER CO. AND NEW ENGLAND TELEPHONE CO. RECORDED IN THE CUMBERLAND COUNTY REGISTRY OF DEEDS IN BOOK 3027 PAGE 779.

E) RIGHTS AND EASEMENTS GRANTED TO THE VILLAGE CAFE, INC. TO CENTRAL MAINE POWER COMPANY AND NEW ENGLAND TELEPHONE COMPANY RECORDED IN THE CUMBERLAND COUNTY REGISTRY OF DEEDS IN BOOK 6293 PAGE 349.

F) RIGHTS AND EASEMENTS GRANTED BY WARWICK CORPORATION TO CENTRAL MAINE POWER COMPANY AND NEW ENGLAND TELEPHONE COMPANY RECORDED IN THE CUMBERLAND COUNTY REGISTRY OF DEEDS IN BOOK 3211 PAGE 557.

G) THREE FOOT WIDE RIGHT OF WAY LOCATED OFF THE SOUTHERLY SIDELINE OF FEDERAL STREET AS DESCRIBED IN DEED FROM [illegible] REALTY CO. TO WARWICK CORPORATION RECORDED IN THE CUMBERLAND COUNTY REGISTRY OF DEEDS IN BOOK 3142 PAGE 190.

H) 10 FOOT RIGHT OF WAY REFERENCED FROM BOOK 7279 PAGE [illegible], QUOTED AS READING "THE ABOVE DESCRIBED PREMISES MAY BE SUBJECT TO A RIGHT OF WAY ABOUT 10 FEET WIDE AND 42 FEET LONG LOCATED OFF THE SOUTHERLY SIDELINE OF FEDERAL STREET AS DESCRIBED IN A DEED TO WARWICK CORPORATION RECORDED IN THE CUMBERLAND COUNTY REGISTRY OF DEEDS BOOK 3142, PAGE 190." NOT SHOWN ON PLAN.

I) RIGHTS OF OTHERS TO PASS OVER PARCEL 1 TO AND FROM TURNER'S COURT AS DESCRIBED IN DEED FROM [illegible] TO VILLAGE CAFE, INC. RECORDED IN THE CUMBERLAND COUNTY REGISTRY OF DEEDS IN BOOK 4208 PAGE 175.

J) RIGHTS DESCRIBED IN DEED FROM ARNOLD M. [illegible] TO MARIA [illegible] NAVARRO RECORDED IN THE CUMBERLAND COUNTY REGISTRY OF DEEDS IN BOOK [illegible] PAGE 249.

K) LIMITS OF TURNERS COURT ARE BASED ON PLAN REFERENCE "M".

L) BUILDING AND OVERHANG EASEMENT FOR BLOCK D LOT 8 DESCRIBED IN BOOK 7575 PAGE 0074.

6) THERE IS NO 100 YEAR FLOOD PLAIN LOCATED ON THE SITE AND IS DESIGNATED AS ZONE C PER THE FLOOD INSURANCE RATE MAP FOR THE TOWN OF PORTLAND, MAINE, COMMUNITY PANEL NUMBER 230051 0014 B DATED JULY 17, 1986.

7) PUBLIC WATER AND SEWER IS PROVIDED TO SUBJECT PARCELS.

8) THIS PLAN IS BASED UPON A BOUNDARY SURVEY PERFORMED IN ACCORDANCE WITH THE STATE OF MAINE BOARD OF LICENSURE FOR PROFESSIONAL LAND SURVEYORS STANDARDS OF PRACTICE WITH THE EXCEPTION THAT NO NEW MONUMENT HAS BEEN SET AT THE ISSUANCE OF THIS PLAN.

GEOTECHNICAL LEGEND

B1 — NUMBER AND APPROXIMATE LOCATION OF BORINGS DRILLED BY MAINE TEST BORINGS INC. DURING JUNE 20 TO 21, 2005.

B101 — NUMBER AND APPROXIMATE LOCATION OF BORINGS DRILLED BY MAINE TEST BORINGS INC. DURING DECEMBER 21 AND JANUARY 3, 2006.

GEOTECHNICAL NOTES

1. BORINGS MONITORED BY SEBAGO TECHNICS, INC.
2. LOCATIONS OF BORINGS DETERMINED BY SEBAGO TECHNICS, INC. BY PACING AND TAPING FROM EXISTING SITE FEATURES.

REV.	BY:	DATE:	STATUS:
A	KR	2/21/06	GEOTECHNICAL BORINGS

THIS PLAN SHALL NOT BE MODIFIED WITHOUT WRITTEN PERMISSION FROM SEBAGO TECHNICS, INC. ANY ALTERATIONS, AUTHORIZED OR OTHERWISE, SHALL BE AT THE USER'S SOLE RISK AND WITHOUT LIABILITY TO SEBAGO TECHNICS, INC.

Sebago Technics
Engineering Expertise You Can Build On
One Chabot Street
Westbrook, Me 04098-1339
Tel (207) 856-0277

PROJECT NO.	FIELD BOOK	DESIGN	CHKD	DRAWN
03109	DIGITAL		DCS	DFB

SUBSURFACE EXPLORATIONS PLAN

OF: **VILLAGE CAFE INC.**
112 NEWBURY ST., 113 NEWBURY ST., 40 HANCOCK ST.
PORTLAND, MAINE
FOR: **GFI ACQUISITIONS I, LLC**
133 PEARL STREET, SUITE 400.
BOSTON, MA 02110

DATE	SCALE
7/12/05	1"=40'

SHEET 1 OF 1

05109

STRESS HISTORY
VILLAGE AT OCEAN GATE, PH I
PORTLAND, MAINE

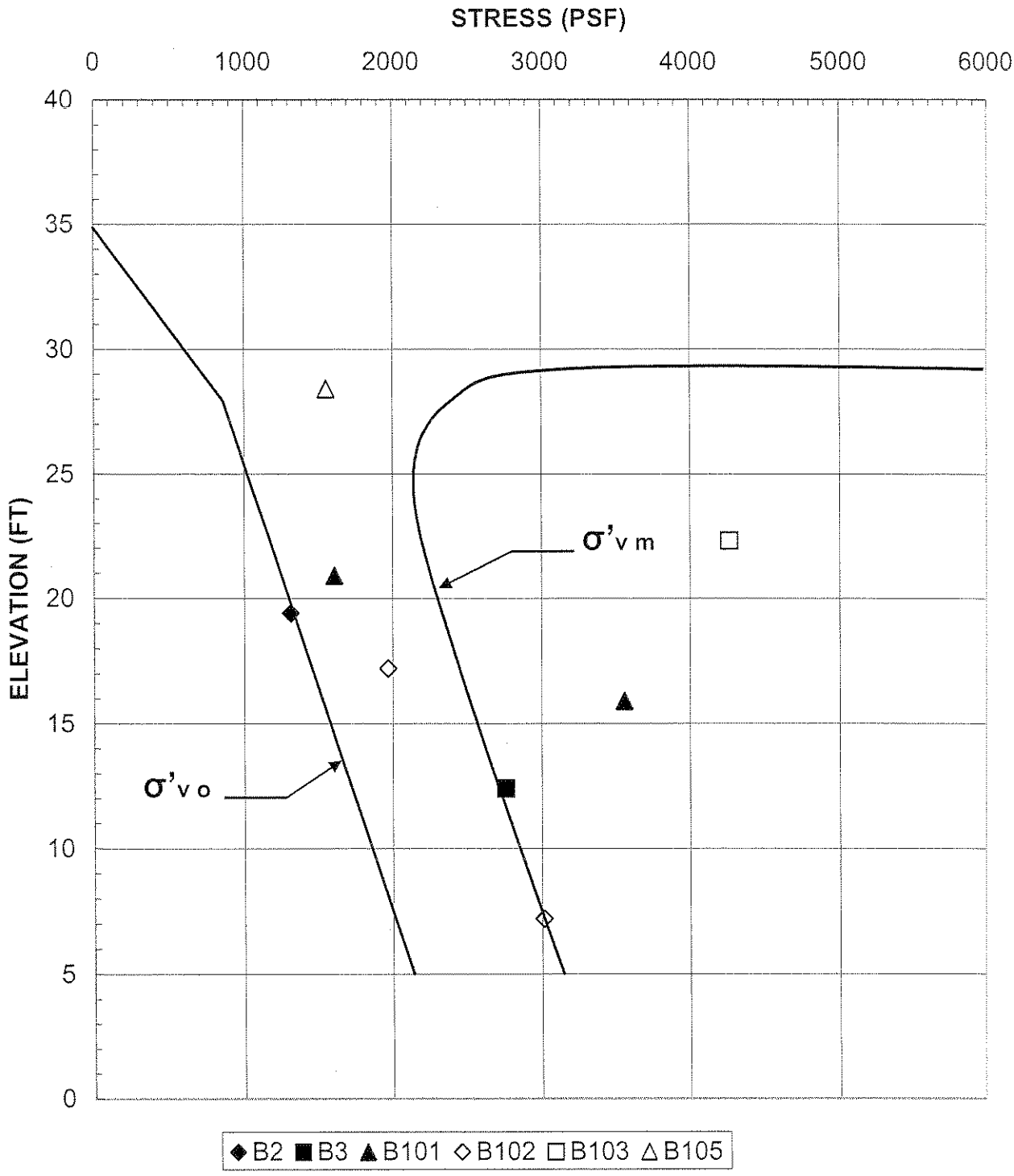


FIGURE 1

Appendix A

Logs of Preliminary Borings

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. **B1**

Page 1 of 2

PROJECT	VILLAGE CAFÉ REDEVELOPMENT	STI JOB NO.	05109
LOCATION	NEWBURY STREET, PORTLAND, MAINE	PROJECT MGR.	C. DIMATTEO
CLIENT	GFI ACQUISITIONS I, LLC	FIELD REP.	K. B. STEPHENSON
CONTRACTOR	MAINE TEST BORINGS, INC.	DATE STARTED	6/20/2005
DRILLER	B. ENOS	DATE FINISHED	6/20/2005

Elevation 40.7 ft. | Datum | Boring Location See Plan

Item	Casing	Sampler	Core Barrel
Type	HSA	SS	--
Inside Diameter (in.)	2.5	1.375	--
Hammer Weight (lb.)	--	140	
Hammer Fall (in.)	--	30	

Rig Make & Model: Mobil B53

- [x] Truck
- [ ] Tripod
- [ ] Cat-Head
- [ ] ATV
- [ ] Geoprobe
- [x] Winch
- [ ] Track
- [ ] Air Track
- [ ] Roller Bit
- [ ] Skid
- [ ] Cutting Head

Hammer Type: [x] Safety [ ] Doughnut [ ] Automatic

Drilling Mud: [ ] Bentonite [ ] Polymer [x] None

Casing Advance: Type Method Depth: HSA/SPIN/30.0

Drilling Notes:

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
0					0.2		-BITUMINOUS CONCRETE-										
	7	S1	0.3			SW	Medium dense, brown well-graded SAND with gravel (SW), mps =	10	10	30	25	20	5				
	18				1.0		1.3 in., dry -FILL-										
	12				1.2		Medium dense, red BRICK, dry -FILL-										
	13	10	2.3			SM	Medium dense, black silty SAND (SM), mps = 0.1 in., dry				5	80	15				
							-FILL-										
					3.5												
							Note: gray sandy silt in auger cuttings at 3.5 ft.										
5																	
	2	S2	5.0			SM	Very loose, gray silty SAND (SM), frequent silt seams, mps = 0.02					80	20				
	2						in., wet										
	1																
	1	15	7.0				-MARINE DEPOSITS-										
10																	
	1	S3	10.0		10.5	SW	Very loose, gray well-graded SAND (SW), mps = 0.2 in., wet			10	40	45	5				
	WOH					ML	Soft, gray SILT (ML), frequent sand partings, mps = 0.02 in.,					15	85		L	N	
	WOH						wet										
	WOH	16	12.0				-MARINE DEPOSITS-										
15																	
	1	S4	15.0			ML	Soft, gray SILT (ML), frequent sand seams, mps = 0.02 in.,					20	80		L	N	
	WOH						trace clay, wet										
	WOH																
	WOH	24	17.0														
							-MARINE DEPOSITS-										
					18.5												
20																	
	1	S5	20.0			CL	Soft, gray lean CLAY (CL), frequent sand partings to seams,					15	85	N	M	M	
	WOH						mps = 0.02 in., wet										
	WOH																
	1	24	22.0														
							-MARINE DEPOSITS-										
25																	
	WOR	S6	25.0			CL	Soft, gray lean CLAY (CL), frequent sand seams, mps = 0.2 in.,			5	10	15	75	N	M	M	
	WOR						wet										
	WOR																
	WOH	12	27.0														
							-MARINE DEPOSITS-										
30																	

Water Level Data

Date	Time	Elapsed Time (hr.)	Depth in feet to: Bottom of Casing	Bottom of Hole	Water
6/20/2005	1212		Caved	9.8	7.0

Sample ID

O	Open End Rod
T	Thin Wall Tube
U	Undisturbed Sample
S	Split Spoon Sample
G	Geoprobe
FV	Field Vane

Well Diagram

- Riser Pipe
- Screen
- Filter Sand
- Cuttings
- Grout
- Concrete
- Bentonite Seal

Summary

Overburden (Linear ft.)	32.0
Rock Cored (Linear ft.)	--
Number of Samples	7S

BORING NO. B1

Field Tests — Dilatancy: R - Rapid S - Slow N - None; Toughness: L - Low M - Medium H - High; Plasticity: N - Nonplastic L - Low M - Medium H - High; Dry Strength: N - None L - Low M - Medium H - High V - Very High

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. B1

Page 2 of 2

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
30	WOR	S7	30.0			CL	Soft, gray lean CLAY (CL), frequent fine sand seams, mps =				5	15	80	N	M	M	
	1						0.1 in., , , wet										
	WOH						-MARINE DEPOSITS-										
	1	24	32.0														
							Bottom of exploration at 32.0 ft. below ground surface										
							No refusal										
35																	
40																	
45																	
50																	
55																	
60																	
65																	
70																	

NOTES:

FILE NO. 05109

BORING NO. B1

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. **B2**

Page 1 of 2

PROJECT	VILLAGE CAFÉ REDEVELOPMENT	STI JOB NO.	05109
LOCATION	NEWBURY STREET, PORTLAND, MAINE	PROJECT MGR.	C. DIMATTEO
CLIENT	GFI ACQUISITIONS I, LLC	FIELD REP.	K. B. STEPHENSON
CONTRACTOR	MAINE TEST BORINGS, INC.	DATE STARTED	6/20/2005
DRILLER	B. ENOS	DATE FINISHED	6/20/2005

Elevation 35.6 ft. Datum — Boring Location See Plan

Item	Casing	Sampler	Core Barrel
Type	HSA	SS	--
Inside Diameter (in.)	2.5	1.375	--
Hammer Weight (lb.)	--	140	
Hammer Fall (in.)	--	30	

Rig Make & Model: Mobil B53

- [x] Truck [ ] Tripod [ ] Cat-Head
- [ ] ATV [ ] Geoprobe [x] Winch
- [ ] Track [ ] Air Track [ ] Roller Bit
- [ ] Skid [ ] [x] Cutting Head

Hammer Type: [x] Safety [ ] Doughnut [ ] Automatic

Drilling Mud: [ ] Bentonite [ ] Polymer [x] None

Casing Advance: Type Method Depth — HSA/SPIN/30.0

Drilling Notes: 2.0 x 7.0 in. Field Vane

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
0					0.2		-BITUMINOUS CONCRETE-										
	3	S1	0.3			SW	Loose, brown well-graded SAND (SW), mps = 0.2 in., dry			30	40	25	5				
	3				0.8		-FILL-										
	2					SM	Loose, dark brown silty SAND (SM), mps = 0.2 in., traces brick,										
	2	15	2.3				glass, ash, damp			20	15	50	15				
							-FILL-										
					4.0												
5	2	S2	5.0			CL	Stiff, gray-brown mottled lean CLAY (CL), damp						100	N	M	M	
	4																
	6																
	7	24	7.0				-MARINE DEPOSITS-										
					8.5												
10	WOH	S3	10.0			CL	Soft, gray lean CLAY (CL), occasional sand seams, mps = 0.02					5	95	N	M	M	
	WOH						in., wet										
	WOH																
	2	24	12.0				-MARINE DEPOSITS-										
15	WOR	FV1	15.0-15.6				FV1 from 15.0 to 15.6 ft. = 7/3 ft. lb., Su = 260 psf										
	WOH	S4	15.0			CL	Soft, gray CLAY (CL), occasional sand partings, mps = 0.02 in.,					5	95	N	M	M	
	WOH						wet										
	WOH	24	17.0				-MARINE DEPOSITS-										
20	WOR	S5	20.0			CL	Soft, gray lean CLAY (CL), frequent sand seams, mps = 0.02					10	90	N	M	M	
	WOR				21.0		in., wet										
	WOH					SM	Very loose, gray silty SAND (SM), frequent clay seams, mps = 0.02					80	20				
	WOH	24	22.0				in., wet										
							-MARINE DEPOSITS-										
25	WOR	S6	25.0			SP	Very loose, gray poorly-graded SAND (SP), occasional silt seams,					95	5				
	1						mps = 0.02 in, wet										
	WOH				26.9		-MARINE DEPOSITS-										
	5	10	27.0														
							-GLACIAL TILL DEPOSITS-										
30																	

Water Level Data

Date	Time	Elapsed Time (hr.)	Depth in feet to: Bottom of Casing	Bottom of Hole	Water
6/21/2005	0840		Caved	19.0	11.3

Sample ID

O	Open End Rod
T	Thin Wall Tube
U	Undisturbed Sample
S	Split Spoon Sample
G	Geoprobe
FV	Field Vane

Well Diagram

Riser Pipe, Screen, Filter Sand, Cuttings, Grout, Concrete, Bentonite Seal

Summary

Overburden (Linear ft.)	30.1
Rock Cored (Linear ft.)	--
Number of Samples	7S

BORING NO. B2

Field Tests

Dilatancy: R - Rapid S - Slow N - None
Toughness: L - Low M - Medium H - High
Plasticity: N - Nonplastic L - Low M - Medium H - High
Dry Strength: N - None L - Low M - Medium H - High V - Very High

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. B2

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
30	50/.1	S7	30.0			SM	Very dense, gray silty SAND (SM), mps = 1.0 in., wet	5		10	10	60	15				
		2	30.1		30.1		-GLACIAL TILL DEPOSITS-										
							Split spoon refusal at 30.1 ft. on probable bedrock										
							Bottom of exploration at 30.1 ft. below ground surface										
35																	
40																	
45																	
50																	
55																	
60																	
65																	
70																	

NOTES:

FILE NO. 05109

BORING NO. B2

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. B3

Page 1 of 2

PROJECT	VILLAGE CAFÉ REDEVELOPMENT	STI JOB NO.	05109
LOCATION	NEWBURY STREET, PORTLAND, MAINE	PROJECT MGR.	C. DIMATTEO
CLIENT	GFI ACQUISITIONS I, LLC	FIELD REP.	K. B. STEPHENSON
CONTRACTOR	MAINE TEST BORINGS, INC.	DATE STARTED	6/21/2005
DRILLER	B. ENOS	DATE FINISHED	6/21/2005

Elevation 28.8 ft. Datum — Boring Location See Plan

Item	Casing	Sampler	Core Barrel
Type	HSA	SS	--
Inside Diameter (in.)	2.5	1.375	--
Hammer Weight (lb.)	--	140	
Hammer Fall (in.)	--	30	

Rig Make & Model: Mobil B53

- [x] Truck
- [ ] ATV
- [ ] Track
- [ ] Skid
- [ ] Tripod
- [ ] Geoprobe
- [ ] Air Track
- [ ] Cat-Head
- [x] Winch
- [ ] Roller Bit
- [x] Cutting Head

Hammer Type: [x] Safety [ ] Doughnut [ ] Automatic

Drilling Mud: [ ] Bentonite [ ] Polymer [x] None

Casing Advance: Type Method Depth — HSA/SPIN/30.0

Drilling Notes: 2.0 x 7.0 Field Vane

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
0					0.2		-BITUMINOUS CONCRETE-										
	13	S1	0.3			SW	Dense, brown well-graded SAND with gravel (SW), mps = 1.3 in.,	10	5	30	30	20	5				
	18				0.7		dry -FILL-										
	15				1.0		Dense, red BRICK, dry -FILL-										
	12	14	2.3			SM	Dense, black silty SAND (SM), mps = 0.3 in., traces ash, brick, dry		5	20	20	40	15				
							-FILL-										
					3.3		Note: 7 in. cobble at approximately 1.0 ft. Brown silty sand in auger cuttings from 1.5 to 3.3 ft.										
5	6	S2	5.0			CL	Stiff, gray lean CLAY (CL), mps = 0.02 in., damp					5	95	N	M	M	
	5																
	5																
	7	16	7.0				-MARINE DEPOSITS-										
10	WOH	S3	10.0			CL	Medium stiff, gray lean CLAY (CL), frequent sand partings, mps = 0.02					5	95	N	M	M	
	WOH						in., wet										
	WOH																
	WOH	24	12.0				-MARINE DEPOSITS-										
15	WOR	FV1	15.0-15.6				FV1 from 15.0 to 15.6 ft. = 17/7 ft. lb., Su = 630 psf										
	WOH	S4	15.0			CL	Medium stiff, gray lean CLAY (CL), frequent sand partings to seams,					10	90	N	M	M	
	WOH						mps = 0.02 in., wet										
	WOH	24	17.0				-MARINE DEPOSITS-										
20	WOH	S5	20.0			CL	Medium stiff, gray lean CLAY (CL), frequent sand seams, three 0.25 in.					15	85	N	M	M	
	WOH						dropstones at 21.2 ft., wet										
	2				21.5												
	5	24	22.0			SP	Very loose, gray poorly-graded SAND (SP), mps = 0.02 in., wet					100					
							-MARINE DEPOSITS-										
25	WOH	S6	25.0			SM	Loose, gray silty SAND (SM), frequent silt seams, mps = 0.1 in.,				5	80	15				
	3				26.0		wet										
	3					SM	Loose, gray silty SAND with gravel (SM), mps = 1.3 in., wet	5	10	25	20	25	15				
	4	24	27.0				-GLACIAL TILL-										
30																	

Water Level Data

Date	Time	Elapsed Time (hr.)	Depth in feet to: Bottom of Casing	Bottom of Hole	Water
6/21/2005	1047		Caved	19.8	10.9

Sample ID

O	Open End Rod
T	Thin Wall Tube
U	Undisturbed Sample
S	Split Spoon Sample
G	Geoprobe
FV	Field Vane

Well Diagram

- Riser Pipe
- Screen
- Filter Sand
- Cuttings
- Grout
- Concrete
- Bentonite Seal

Summary

Overburden (Linear ft.)	32.0
Rock Cored (Linear ft.)	--
Number of Samples	7S

BORING NO. B3

Field Tests — Dilatancy: R - Rapid S - Slow N - None; Toughness: L - Low M - Medium H - High; Plasticity: N - Nonplastic L - Low M - Medium H - High; Dry Strength: N - None L - Low M - Medium H - High V - Very High

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. B3

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
30	5	S7	30.0			SM	Medium dense, gray silty SAND with gravel (SM), mps = 1.2 in.,	10	10	30	20	15	15				
	7						wet										
	5						-GLACIAL TILL DEPOSITS-										
	8	24	32.0														
							Bottom of exploration at 32.0 ft. below ground surface										
							No refusal										
35																	
40																	
45																	
50																	
55																	
60																	
65																	
70																	

NOTES:

FILE NO. 05109

BORING NO. B3

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. **B4**

Page 1 of 2

PROJECT	VILLAGE CAFÉ REDEVELOPMENT	STI JOB NO.	05109
LOCATION	NEWBURY STREET, PORTLAND, MAINE	PROJECT MGR.	C. DIMATTEO
CLIENT	GFI ACQUISITIONS I, LLC	FIELD REP.	K. B. STEPHENSON
CONTRACTOR	MAINE TEST BORINGS, INC.	DATE STARTED	6/20/2005
DRILLER	B. ENOS	DATE FINISHED	6/20/2005

Elevation 39.9 ft. Datum — Boring Location See Plan

Item	Casing	Sampler	Core Barrel
Type	HSA	SS	--
Inside Diameter (in.)	2.5	1.375	--
Hammer Weight (lb.)	--	140	
Hammer Fall (in.)	--	30	

Rig Make & Model: *Mobil B53*

☑ Truck ☐ Tripod ☐ Cat-Head
☐ ATV ☐ Geoprobe ☑ Winch
☐ Track ☐ Air Track ☐ Roller Bit
☐ Skid ☐ ☑ Cutting Head

Hammer Type: ☑ Safety ☐ Doughnut ☐ Automatic

Drilling Mud: ☐ Bentonite ☐ Polymer ☑ None

Casing Advance: Type Method Depth — HSA/SPIN/30.0

Drilling Notes: 2.0 x 7.0 in. Field Vane

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
0	9	S1	0.0		0.4	SM	Very dense, brown silty SAND with gravel (SM), mps = 1.2 in., dry	10	5	30	20	20	15				
	50/.4	8	0.9		0.6		-BITUMINOUS CONCRETE-										
						SM	Very dense, brown silty SAND with gravel (SM), mps = 1.0 in., damp	10	5	30	20	20	15				
							-FILL-										
					3.0		Note: brown silty sand in auger cuttings at 3.0 ft.										
5	2	S2	5.0			SM	Loose, brown silty SAND (SM), mps = 0.02 in., wet					85	15				
	2				6.0		-MARINE DEPOSITS-										
	3				6.6	CL	Medium stiff, gray-brown mottled lean CLAY (CL), mps = 0.02 in.,					5	95	N	M	M	
	3	20	7.0				damp										
						CL	Medium stiff, gray lean CLAY (CL), mps = 0.02 in., wet					5	95	N	M	M	
10	WOR	FV1	10.0-10.6				FV1 from 10.0 to 10.6 ft. = 15/7 ft. lb., Su = 560 psf										
	WOH	S3	10.0			CL	Medium stiff, gray lean CLAY (CL), occasional sand partings, frequent					5	95	N	M	M	
	WOH						black streaks, mps = 0.02 in., wet										
	2	24	12.0														
							-MARINE DEPOSITS-										
15	WOR	S4	15.0			CL	Medium stiff, gray lean CLAY (CL), frequent sand partings and black streaks,					5	95	N	M	M	
	WOH						mps = 0.02 in., wet										
	WOH																
	WOH	24	17.0														
							-MARINE DEPOSITS-										
20							Note: attempted field vane at 20.0 ft.- could not advance vane										
	2	S5	20.0		20.4	CL	Medium stiff, gray lean CLAY (CL), frequent sand partings and black					5	95	N	M	M	
	1						streaks, mps = 0.02 in., wet										
	1					CL	Medium stiff, gray lean CLAY (CL), frequent sand seams, mps = 0.02 in.,					15	85	N	M	M	
	1	24	22.0				wet										
							-MARINE DEPOSITS-										
					24.0												
25	WOR	FV2	25.0-25.6				FV2 from 25.0 to 25.6 ft.= 27/10 ft. lb., Su = 1,000 psf										
	WOH	S6	25.0			SM	Very loose, gray silty SAND (SM), frequent clay seams, mps = 0.02					80	20				
	WOH						in., wet										
	WOH	10	27.0														
							-MARINE DEPOSITS-										
30																	

Water Level Data

Date	Time	Elapsed Time (hr.)	Depth in feet to: Bottom of Casing	Bottom of Hole	Water
6/20/2005	1627		Caved	12.0	10.0

Sample ID

O	Open End Rod
T	Thin Wall Tube
U	Undisturbed Sample
S	Split Spoon Sample
G	Geoprobe
FV	Field Vane

Well Diagram

Riser Pipe, Screen, Filter Sand, Cuttings, Grout, Concrete, Bentonite Seal

Summary

Overburden (Linear ft.)	30.7
Rock Cored (Linear ft.)	--
Number of Samples	7S

BORING NO. B4

Field Tests — Dilatancy: R - Rapid S - Slow N - None; Toughness: L - Low M - Medium H - High; Plasticity: N - Nonplastic L - Low M - Medium H - High; Dry Strength: N - None L - Low M - Medium H - High V - Very High

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. B4

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
					30.0												
30	3	S7	30.0			SM	Very dense, gray silty SAND with gravel (SM), mps = 1.0 in., wet	5	10	20	10	40	15				
	50.2	8	30.7		30.7		-GLACIAL TILL DEPOSITS-										
							Split spoon refusal at 30.7 ft. on probable bedrock										
							Bottom of exploration at 30.7 ft. below ground surface										
35																	
40																	
45																	
50																	
55																	
60																	
65																	
70																	

NOTES:

FILE NO. 05109

BORING NO. B4

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. **B5**

Page 1 of 2

PROJECT	VILLAGE CAFÉ REDEVELOPMENT	STI JOB NO.	05109
LOCATION	NEWBURY STREET, PORTLAND, MAINE	PROJECT MGR.	C. DIMATTEO
CLIENT	GFI ACQUISITIONS I, LLC	FIELD REP.	K. B. STEPHENSON
CONTRACTOR	MAINE TEST BORINGS, INC.	DATE STARTED	6/20/2005
DRILLER	B. ENOS	DATE FINISHED	6/20/2005

Elevation 47.5 ft. Datum — Boring Location See Plan

Item	Casing	Sampler	Core Barrel
Type	HSA	SS	--
Inside Diameter (in.)	2.5	1.375	--
Hammer Weight (lb.)	--	140	
Hammer Fall (in.)	--	30	

Rig Make & Model: Mobil B53

- [x] Truck
- [ ] ATV
- [ ] Track
- [ ] Skid
- [ ] Tripod
- [ ] Geoprobe
- [ ] Air Track
- [ ] Cat-Head
- [x] Winch
- [ ] Roller Bit
- [x] Cutting Head

Hammer Type: [x] Safety [ ] Doughnut [ ] Automatic

Drilling Mud: [ ] Bentonite [ ] Polymer [x] None

Casing Advance: Type Method Depth — HSA/SPIN/30.0

Drilling Notes: 2.0 x 7.0 in. Field Vane

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
0					0.15		-BITUMINOUS CONCRETE-										
	3	S1	0.3			SW	Loose, brown well-graded SAND with gravel (SM), mps = 1.2 in., damp	5	10	20	20	45					
	3				1.3		-FILL-										
	5																
	8	14	2.3			SP	Loose, brown poorly-graded SAND (SP), mps = 0.02 in., rusty discolorations, damp					95	5				
							-MARINE DEPOSITS-										
5	4	S2	5.0		5.5	SP-SM	Very loose, brown poorly-graded SAND with silt (SP-SM), mps = 0.02 in., wet					90	10				
	1																
	1					SM	Very loose, gray silty SAND (SM), mps = 0.02 in., occasional silt seams, wet					85	15				
	2	15	7.0				-MARINE DEPOSITS-										
					8.5												
10	WOH	S3	10.0			CL	Medium stiff, gray lean CLAY (CL), occasional black streaks and sand seams, mps = 0.02 in., wet					10	90	N	M	M	
	WOH																
	WOH																
	1	24	12.0				-MARINE DEPOSITS-										
15	WOR	FV1	15.0-15.6				FV1 from 15.0 to 15.6 ft. = 20/10 ft. lb., Su = 740 psf										
	WOH	S4	15.0			CL	Medium stiff, gray lean CLAY (CL), frequent sand seams, mps = 0.02 in., wet					10	90	N	M	M	
	WOH																
	2	24	17.0				-MARINE DEPOSITS-										
20	WOH	S5	20.0			CL	Medium stiff, gray lean CLAY (CL), frequent sand seams, mps = 0.02 in., wet					15	85	N	M	M	
	WOH																
	WOH																
	1	24	22.0				-MARINE DEPOSITS-										
					23.0												
25	WOR	S6	25.0			SP	Very loose, gray poorly-graded SAND (SP), occasional silt seams, mps = 0.1 in., wet				5	85	10				
	1																
	1																
	1	18	27.0				-MARINE DEPOSITS-										
					29.0												
30							-GLACIAL TILL DEPOSITS-										

Water Level Data

Date	Time	Elapsed Time (hr.)	Depth in feet to: Bottom of Casing	Bottom of Hole	Water
6/20/2005	1820		Caved	17.0	11.3

Sample ID

O	Open End Rod
T	Thin Wall Tube
U	Undisturbed Sample
S	Split Spoon Sample
G	Geoprobe
FV	Field Vane

Well Diagram

Riser Pipe, Screen, Filter Sand, Cuttings, Grout, Concrete, Bentonite Seal

Summary

Overburden (Linear ft.)	32.0
Rock Cored (Linear ft.)	--
Number of Samples	7S
BORING NO.	B5

Field Tests — Dilatancy: R - Rapid S - Slow N - None; Toughness: L - Low M - Medium H - High; Plasticity: N - Nonplastic L - Low M - Medium H - High; Dry Strength: N - None L - Low M - Medium H - High V - Very High

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. B5

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
30	1	S7	30.0			SM	Very loose, gray silty SAND with gravel (SM), mps = 1.0 in., wet	5	10	20	20	30	15				
	1																
	WOH						-GLACIAL TILL DEPOSITS-										
	1	24	32.0														
							Bottom of exploration at 32.0 ft. below ground surface										
							No refusal										
35																	
40																	
45																	
50																	
55																	
60																	
65																	
70																	

NOTES:

FILE NO. 05109

BORING NO. B5

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil Identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. **B6**

Page 1 of 1

PROJECT	VILLAGE CAFÉ REDEVELOPMENT	STI JOB NO.	05109
LOCATION	NEWBURY STREET, PORTLAND, MAINE	PROJECT MGR.	C. DIMATTEO
CLIENT	GFI ACQUISITIONS I, LLC	FIELD REP.	K. B. STEPHENSON
CONTRACTOR	MAINE TEST BORINGS, INC.	DATE STARTED	6/20/2005
DRILLER	B. ENOS	DATE FINISHED	6/20/2005

Elevation 42.2 ft. Datum — Boring Location See Plan

Item	Casing	Sampler	Core Barrel
Type	HSA	SS	--
Inside Diameter (in.)	2.5	1.375	--
Hammer Weight (lb.)	--	140	
Hammer Fall (in.)	--	30	

Rig Make & Model: Mobil B53

☑ Truck ☐ Tripod ☐ Cat-Head
☐ ATV ☐ Geoprobe ☑ Winch
☐ Track ☐ Air Track ☐ Roller Bit
☐ Skid ☐ ☑ Cutting Head

Hammer Type: ☑ Safety ☐ Doughnut ☐ Automatic

Drilling Mud: ☐ Bentonite ☐ Polymer ☑ None

Casing Advance: Type Method Depth — HSA/SPIN/29.2

Drilling Notes: 2.0 x 7.0 in. Field Vane

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
0					0.25		-CONCRETE-										
	2	S1	0.5		0.8	SW	Loose, brown well-graded SAND (SM), mps = 0.2 in., wet -FILL-			20	40	40					
	5					SW-SM	Loose, gray to black well-graded SAND with silt and gravel (SW-SM),	10	10	20	20	30	10					
	4				1.3		mps = 1.3 in., wet -FILL-										
	5	10	2.5			SM	Loose, gray-brown silty SAND (SM), mps = 0.02 in., damp					85	15				
					3.5		-MARINE DEPOSITS-										
5	WOH	S2	5.0			ML	Soft, gray SILT (ML), occasional sand partings, mps = 0.02 in., wet					10	90		L	N	
	WOH																
	WOH																
	WOH	24	7.0				-MARINE DEPOSITS-										
10	WOH	FV1	10.0-10.6				FV1 from 10.0 to 10.6 ft. = 23/8 ft. lb., Su = 850 psf										
	WOH	S3	10.0			ML	Medium stiff, gray SILT (ML), frequent sand seams, mps = 0.02 in., wet					15	85		L	N	
	WOH																
	WOH	24	12.0				-MARINE DEPOSITS-										
					13.5												
15	WOH	S4	15.0			CL	Medium stiff, gray lean CLAY (CL), frequent sand layers, 0.75 in. drop-					40	60	N	M	M	
	3						stone at 16.5 ft., wet										
	WOH																
	3	24	17.0				-MARINE DEPOSITS-										
					19.0												
20	WOH	FV2	20.0-20.6				FV2 from 20.0 to 20.6 ft. = 30/10 ft. lb., Su = 1,110 psf										
	WOH	S5	20.0			SM	Very loose, gray silty SAND (SM), occasional clay seams, mps = 0.02 in.,					80	20				
	3				21.5		wet										
	10	24	22.0			SP	Very loose, brown poorly-graded SAND (SP), mps = 0.1 in., wet					5	95				
							-MARINE DEPOSITS-										
25	3	S6	25.0			SP	Very loose, gray-brown poorly-graded SAND (SP), mps = 0.1 in., wet					10	90				
	3						-MARINE DEPOSITS-										
	2				26.3												
	4	24	27.0			SM	Very loose, gray silty SAND (SM), mps = 1.2 in., wet	5	5	20	15	40	15				
					28.0		-GLACIAL TILL DEPOSITS-										
					29.2		-PROBABLE WEATHERED BEDROCK-										
30							HSA refusal on probable bedrock at 29.2 ft.										
							Bottom of exploration at 29.2 ft. below ground surface										

Water Level Data

Date	Time	Elapsed Time (hr.)	Depth in feet to: Bottom of Casing	Bottom of Hole	Water
6/20/2005	1419		Caved	19.0	9.8

Sample ID

O	Open End Rod
T	Thin Wall Tube
U	Undisturbed Sample
S	Split Spoon Sample
G	Geoprobe
FV	Field Vane

Well Diagram

Riser Pipe, Screen, Filter Sand, Cuttings, Grout, Concrete, Bentonite Seal

Summary

Overburden (Linear ft.)	28.0
Rock Cored (Linear ft.)	--
Number of Samples	6S

BORING NO. B6

Field Tests — Dilatancy: R - Rapid S - Slow N - None; Toughness: L - Low M - Medium H - High; Plasticity: N - Nonplastic L - Low M - Medium H - High; Dry Strength: N - None L - Low M - Medium H - High V - Very High

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

Appendix B

Logs of Final Design Borings

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. **B101**

PROJECT	FINAL DESIGN INVESTIGATION, REDEVELOPMENT OF VILLAGE CAFÉ	STI JOB NO.	05109
LOCATION	NEWBURY STREET, PORTLAND, MAINE	PROJECT MGR.	C. DIMATTEO
CLIENT	GPI ACQUISITIONS I, LLC	FIELD REP.	K. B. STEPHENSON
CONTRACTOR	MAINE TEST BORINGS, INC.	DATE STARTED	1/4/2006
DRILLER	R. IDANO	DATE FINISHED	1/5/2006

Elevation 34.2 ft. Datum NGVD 1929 Boring Location See Plan

Item	Casing	Sampler	Core Barrel
Type	NW	SS	--
Inside Diameter (in.)	4.0	1.375	--
Hammer Weight (lb.)	300	140	
Hammer Fall (in.)	16	30	

Rig Make & Model: Mobile B47

☐ Truck ☐ Tripod ☐ Cat-Head
☐ ATV ☐ Geoprobe ☑ Winch
☐ Track ☐ Air Track ☐ Roller Bit
☐ Skid ☑ Trailer ☑ Cutting Head

Hammer Type: ☐ Safety ☑ Doughnut ☐ Automatic

Drilling Mud: ☐ Bentonite ☐ Polymer ☑ None

Casing Advance: Type Method Depth — NW/DRIVEN/30.0

Drilling Notes: 2.0 x 7.0 Field Vane

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
0					0.2		-BITUMINOUS CONCRETE-										
							Note: augered to 1.1 ft.- probable boulder. Moved boring- augered to 1.0 ft.										
	14	S1	1.0			SM	Dense, dark brown silty SAND (SM), mps = 1.0 in., dry	5	5	30	30	15	15				
	18																
	20						-FILL-										
	12	7	3.0		3.0												
							Note: gray-brown clay in wash at 3.0 ft.										
							-MARINE DEPOSITS-										
					4.5												
5							Note: gray clay in wash at 4.5 ft.										
	13	NR	5.0				No recovery										
	7																
	8																
	5		7.0				-MARINE DEPOSITS-										
10																	
	HYDR.	U1	10.0			CL	Gray lean CLAY (CL)										
	PUSH																
							-MARINE DEPOSITS-										
		24	12.0														
	WOR	FV1	12.0-12.6				FV1 from 12.0 to 12.6 ft. = 9/1 ft. lb., Su = 330 psf										
	WOH	S2	12.0			CL	Soft, gray lean CLAY (CL), sand parting at 13.0 ft., mps = 0.02 in.,						100	N	M	M	
	WOH						occasional dark streaks, wet										
	WOH	24	14.0														
							-MARINE DEPOSITS-										
15																	
	HYDR.	U2	15.0			CL	Gray lean CLAY (CL)										
	PUSH																
							-MARINE DEPOSITS-										
		18	17.0														
	WOR	FV2	17.0-17.6				FV2 from 17.0 to 17.6 ft. = 21/5 ft. lb., Su = 780 psf										
	WOR	S3	17.0			CL	Medium stiff, gray lean CLAY (CL), occasional sand partings, 1 in. dropstone					5	95	N	M	M	
	1						at 17.2 ft., wet										
	WOH	24	19.0														
							-MARINE DEPOSITS-										
20																	
	WOR	S4	20.0			CL	Medium stiff, gray lean CLAY (CL), frequent sand partings to seams, mps =					15	85	N	M	M	
	1						0.02 in., wet										
	WOH																
	WOH	7	22.0														
							-MARINE DEPOSITS-										
					23.7												
25																	
	3	S5	25.0			SP	Medium dense, gray poorly-graded SAND (SP), mps = 0.02 in., wet					95	5				
	4																
	10						-MARINE DEPOSITS-										
	11	6	27.0														
					27.2												
							-GLACIAL TILL DEPOSITS-										
30																	

Water Level Data

Date	Time	Elapsed Time (hr.)	Depth in feet to: Bottom of Casing	Bottom of Hole	Water
1/5/2006	1200		30.0	30.5	8.5
1/5/2006	1223		--	25.8	7.2

Sample ID

O	Open End Rod
T	Thin Wall Tube
U	Undisturbed Sample
S	Split Spoon Sample
G	Geoprobe
FV	Field Vane

Well Diagram

Riser Pipe, Screen, Filter Sand, Cuttings, Grout, Concrete, Bentonite Seal

Summary

Overburden (Linear ft.)	30.5
Rock Cored (Linear ft.)	--
Number of Samples	6S, 2U

BORING NO. B101

Field Tests — Dilatancy: R - Rapid S - Slow N - None; Plasticity: N - Nonplastic L - Low M - Medium H - High; Toughness: L - Low M - Medium H - High; Dry Strength: N - None L - Low M - Medium H - High V - Very High

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. B101

Page 2 of 2

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
30	45	S6	30.0		30.5	SM	Dense, gray silty SAND (SM), mps = 0.2 in., wet			25	20	40	15				
	50/.1	2	30.6		30.6		-GLACIAL TILL DEPOSITS-										
							Very dense, dark gray weathered rock fragments-WEATHERED BEDROCK-										
							Split spoon refusal at 30.6 ft.										
							Bottom of exploration at 30.6 ft. below ground surface										
35																	
40																	
45																	
50																	
55																	
60																	
65																	
70																	

NOTES:

FILE NO. 05109

BORING NO. B101

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. **B102**

Page 1 of 2

PROJECT	FINAL DESIGN INVESTIGATION, REDEVELOPMENT OF VILLAGE CAFÉ	STI JOB NO.	05109
LOCATION	NEWBURY STREET, PORTLAND, MAINE	PROJECT MGR.	C. DIMATTEO
CLIENT	GFI ACQUISITIONS I, LLC	FIELD REP.	K. B. STEPHENSON
CONTRACTOR	MAINE TEST BORINGS, INC.	DATE STARTED	12/30/2005
DRILLER	M. PORTER/R. IDANO	DATE FINISHED	1/3/2006

Elevation 28.5 ft. Datum NGVD 1929 Boring Location See Plan

Item	Casing	Sampler	Core Barrel
Type	HSA	SS	--
Inside Diameter (in.)	3.375	1.375	--
Hammer Weight (lb.)		140	
Hammer Fall (in.)		30	

Rig Make & Model: Mobile B47

☐ Truck ☐ Tripod ☑ Cat-Head
☐ ATV ☐ Geoprobe ☑ Winch
☐ Track ☐ Air Track ☐ Roller Bit
☐ Skid ☑ Trailer ☑ Cutting Head

Hammer Type: ☐ Safety ☑ Doughnut ☑ Automatic

Drilling Mud: ☐ Bentonite ☐ Polymer ☑ None

Casing Advance: Type Method Depth — HSA/SPIN/30.0

Drilling Notes: 2.0 x 7.0 Field Vane

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
0					0.2		-BITUMINOUS CONCRETE-										
	11	S1	0.5			SW	Very dense, brown well-graded SAND (SW), mps = 0.75 in., damp	5	10	30	30	20	5				
	12				0.7		-FILL-										
	44	S2	2.0			SM	Very dense, gray-brown silty SAND (SM), ash, brick, mps = 1.3 in., wet	5	5	10	15	50	15				
							Note: probable cobbles from 2.0 to 4.0 ft.										
					4.0		-FILL-										
5	4	S2	5.0			SM	Loose, brown silty SAND (SM), frequent clay seams to layers, mps = 0.2 in., wet			5	5	65	25				
	4																
	6				6.4		-MARINE DEPOSITS-										
	8	S4	7.0			CL	Stiff, gray-brown lean CLAY (CL), wet						100	N	M	M	
							-MARINE DEPOSITS-										
10	WOR	FV1	10.0-10.6				FV1 from 10.0 to 10.6 ft. = 12/2 ft. lb., Su = 440 psf										
	WOH	S3	10.0			CL	Soft, gray lean CLAY (CL), frequent sand partings, occasional black streaks, mps = 0.02 in., wet					5	95	N	M	M	
	WOH																
	WOH	24	12.0														
							-MARINE DEPOSITS-										
15	WOR	S4	15.0			CL	Soft, gray lean CLAY (CL), frequent sand partings, mps = 0.02 in., wet					5	95	N	M	M	
	WOR																
	WOH						-MARINE DEPOSITS-										
	WOH	24	17.0														
					20.0												
20	WOR	FV2	20.0-20.6				FV2 from 20.0 to 20.6 ft. = 18/2 ft. lb., Su = 670 psf										
	WOR	S5	20.0			CL	Medium stiff, gray lean CLAY (CL), frequent sand seams to layers, mps = 0.02 in., wet					20	80	N	M	M	
	7				21.5												
	6	24	22.0			SP	Loose, gray poorly-graded SAND (SP), mps = 0.02 in., wet					95	5				
							-MARINE DEPOSITS-										
25	1	S6	25.0			SP	Very loose, gray poorly-graded SAND (SP), mps = 0.02 in., occasional silt laminae, wet					95	5				
	WOH																
	3																
	7	24	27.0														
							-MARINE DEPOSITS-										
30																	

Water Level Data

Date	Time	Elapsed Time (hr.)	Depth in feet to: Bottom of Casing	Bottom of Hole	Water
1/3/2006	1115		30.0	26.0	18.5
1/19/2006			Well	20.0	2.6

Sample ID

O	Open End Rod
T	Thin Wall Tube
U	Undisturbed Sample
S	Split Spoon Sample
G	Geoprobe
FV	Field Vane

Well Diagram

Riser Pipe, Screen, Filter Sand, Cuttings, Grout, Concrete, Bentonite Seal

Summary

Overburden (Linear ft.)	32.0
Rock Cored (Linear ft.)	--
Number of Samples	7S

BORING NO. B102

Field Tests — Dilatancy: R - Rapid S - Slow N - None; Plasticity: N - Nonplastic L - Low M - Medium H - High
Toughness: L - Low M - Medium H - High; Dry Strength: N - None L - Low M - Medium H - High V - Very High

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. B102

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (In.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
30	14	S7	30.0		30.4	SP	Medium dense, gray poorly-graded SAND (SP), mps = 0.02 in., wet					100					
	5						-MARINE DEPOSITS-										
	14					SM	Medium dense, gray silty SAND with gravel (SM), mps = 1.2 in., wet	5	10	10	10	50	15				
	31	24	32.0				-GLACIAL TILL DEPOSITS-										
							Bottom of exploration at 32.0 ft. below ground surface No refusal										
							Note: Installed 1.0 in. PVC observation well at 20.0 ft.										
35																	
40																	
45																	
50																	
55																	
60																	
65																	
70																	

NOTES:

FILE NO. 05109

BORING NO. B102

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. **B103**

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PROJECT	FINAL DESIGN INVESTIGATION, REDEVELOPMENT OF VILLAGE CAFÉ	STI JOB NO.	05109
LOCATION	NEWBURY STREET, PORTLAND, MAINE	PROJECT MGR.	C. DIMATTEO
CLIENT	GFI ACQUISITIONS I, LLC	FIELD REP.	K. B. STEPHENSON
CONTRACTOR	MAINE TEST BORINGS, INC.	DATE STARTED	12/30/2005
DRILLER	M. PORTER	DATE FINISHED	12/30/2005

Elevation 34.1 ft. Datum NGVD 1929 Boring Location See Plan

Item	Casing	Sampler	Core Barrel
Type	HSA	SS	--
Inside Diameter (in.)	3.375	1.375	--
Hammer Weight (lb.)	--	140	
Hammer Fall (in.)	--	30	

Rig Make & Model: Mobile B47

☐ Truck ☐ Tripod ☐ Cat-Head
☐ ATV ☐ Geoprobe ☑ Winch
☐ Track ☐ Air Track ☐ Roller Bit
☐ Skid ☑ Trailer ☑ Cutting Head

Hammer Type: ☐ Safety ☑ Doughnut ☐ Automatic

Drilling Mud: ☐ Bentonite ☐ Polymer ☑ None

Casing Advance: Type Method Depth: HSA/SPIN/20.0

Drilling Notes: 2.0 x 7.0 in. Field Vane

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
0					0.1		-BITUMINOUS CONCRETE-										
	10	S1	0.5			SM	Dense, gray-brown silty SAND (SM), ash, brick, mps = 0.75 in.,		5	10	20	50	15				
	15						dry										
	16				2.3		-FILL-										
	15	14	2.5			SW	Dense, brown well-graded SAND (SW), mps = 0.5 in., dry		5	30	40	25					
					3.0		-FILL-										
									5	15	20	40	20				
5																	
	2	S2	5.5			SM	Medium dense, dark brown silty SAND (SM), ash, mps = 0.3 in., wet		5	15	20	40	20				
	5																
	8				6.3		-FILL-										
	12	24	7.5			CL	Stiff, gray lean CLAY (CL), frequent sand partings, mps = 0.02 in.,					15	85	N	M	M	
							wet										
							-MARINE DEPOSITS-										
10																	
	WOH	FV1	10.5-11.1				FV1 from 10.5 to 11.1 ft. = 22/3 ft. lb., Su = 820 psf										
	WOH	S3	10.5			CL	Medium stiff, gray lean CLAY (CL), frequent sand seams, mps =					10	90	N	M	M	
	WOH						0.02 in., wet										
	1	24	12.5				-MARINE DEPOSITS-										
15																	
	WOR	S4	15.5			CL	Medium stiff, gray lean CLAY (CL), frequent sand layers, mps = 0.02 in.,					20	80	N	M	M	
	4						wet										
	1				17.3		-MARINE DEPOSITS-										
	10	24	17.5			SP	Loose, gray poorly-graded SAND (SP), mps = 0.02 in., wet					95	5				
							-MARINE DEPOSITS-										
							Advanced HSA to 20.0 ft. Running sand conditions in augers										
20																	
							Bottom of exploration at 20.0 ft. below ground surface										
							No refusal										
25																	
30																	

Water Level Data

Date	Time	Elapsed Time (hr.)	Depth in feet to: Bottom of Casing	Bottom of Hole	Water
12/30/2005	0949		--	17.0	6.0

Sample ID

O	Open End Rod
T	Thin Wall Tube
U	Undisturbed Sample
S	Split Spoon Sample
G	Geoprobe
FV	Field Vane

Well Diagram

Riser Pipe, Screen, Filter Sand, Cuttings, Grout, Concrete, Bentonite Seal

Summary

Overburden (Linear ft.)	20.0
Rock Cored (Linear ft.)	--
Number of Samples	48

BORING NO. B103

Field Tests — Dilatancy: R - Rapid S - Slow N - None
Toughness: L - Low M - Medium H - High
Plasticity: N - Nonplastic L - Low M - Medium H - High
Dry Strength: N - None L - Low M - Medium H - High V - Very High

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. **B104**

Page 1 of 2

PROJECT	FINAL DESIGN INVESTIGATION, REDEVELOPMENT OF VILLAGE CAFÉ	STI JOB NO.	05109
LOCATION	NEWBURY STREET, PORTLAND, MAINE	PROJECT MGR.	C. DIMATTEO
CLIENT	GFI ACQUISITIONS I, LLC	FIELD REP.	K. B. STEPHENSON
CONTRACTOR	MAINE TEST BORINGS, INC.	DATE STARTED	12/29/2005
DRILLER	M. PORTER	DATE FINISHED	12/29/2005

Elevation 29.0 ft. Datum NGVD 1929 Boring Location See Plan

Item	Casing	Sampler	Core Barrel
Type	HSA	SS	--
Inside Diameter (in.)	3.375	1.375	--
Hammer Weight (lb.)	--	140	
Hammer Fall (in.)	--	30	

Rig Make & Model: Mobile B47

☐ Truck ☐ Tripod ☐ Cat-Head
☐ ATV ☐ Geoprobe ☑ Winch
☐ Track ☐ Air Track ☐ Roller Bit
☐ Skid ☑ Trailer ☑ Cutting Head

Hammer Type: ☐ Safety ☑ Doughnut ☐ Automatic

Drilling Mud: ☐ Bentonite ☐ Polymer ☑ None

Casing Advance: Type Method Depth — HSA/SPIN/40.0

Drilling Notes:

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (In.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test: Dilatancy	Toughness	Plasticity	Strength
0					0.2		-BITUMINOUS CONCRETE-										
	5	S1	0.5			SW-SM	Very dense, brown well-graded SAND with silt and gravel (SW-SM), mps = 1.3 in., wet	10	10	30	30	10	10				
	12						-FILL-										
	75/.4	7	1.9				Note: brick, 3 to 6 in. rock fragments from 1.9 to 5.0 ft.										
5							-FILL-										
	2	S2	5.0		5.2	SW-SM	Loose, brown well-graded SAND with silt and gravel (SW-SM), wet	10	10	30	30	10	10				
	1				6.0	SM	Loose, brown silty SAND (SM), frequent silt seams, wet-MARINE DEPOSITS					70	30				
	6				6.5	SM	Loose, gray silty SAND (SM), one 1 in. gravel piece, wet	5			10	65	20				
	9	15	7.0		7.0	SW-SM	Loose, brown well-graded SAND with silt and gravel (SW-SM), mps = 1.0 in., wet	10	10	30	30	10	10				
							-GLACIAL TILL DEPOSITS-										
10	1	S3	10.0			SM	Medium dense, gray-brown silty SAND (SM), one in. gravel piece, wet	50		10	10	15	15				
	2																
	9						-GLACIAL TILL DEPOSITS-										
	16	1	12.0														
15	1	S4	15.0			SM	Very loose, gray silty SAND with gravel (SM), mps = 1.3 in., wet	10	5	10	30	30	15				
	1																
	2																
	1	24	17.0				-GLACIAL TILL DEPOSITS-										
20	WOR	S5	20.0			SM	Very loose, gray silty SAND (SM), mps = 0.2 in., wet			10	45	30	15				
	WOH																
	4						Note: probably pushed gravel										
	3	2	22.0				-GLACIAL TILL DEPOSITS-										
25	8	NR	25.0				No recovery										
	13																
	15																
	8		27.0				-GLACIAL TILL DEPOSITS-										
30																	

Water Level Data

Date	Time	Elapsed Time (hr.)	Depth in feet to: Bottom of Casing	Bottom of Hole	Water
12/29/2005	1515		--	9.4	7.8

Sample ID

O	Open End Rod
T	Thin Wall Tube
U	Undisturbed Sample
S	Split Spoon Sample
G	Geoprobe
FV	Field Vane

Well Diagram

Riser Pipe, Screen, Filter Sand, Cuttings, Grout, Concrete, Bentonite Seal

Summary

Overburden (Linear ft.)	42.0
Rock Cored (Linear ft.)	--
Number of Samples	8S

BORING NO. B104

Field Tests — Dilatancy: R - Rapid S - Slow N - None; Toughness: L - Low M - Medium H - High; Plasticity: N - Nonplastic L - Low M - Medium H - High; Dry Strength: N - None L - Low M - Medium H - High V - Very High

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. B104

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
30	15	S6	30.0			SM	Dense, gray silty SAND with gravel (SM), mps = 1.3 in., slightly bonded,	15	10	30	20	10	15				
	23						wet										
	27																
	33	12	32.0				-GLACIAL TILL DEPOSITS-										
35	18	S7	35.0			SM	Very dense, gray silty SAND with gravel (SM), mps = 1.3 in., slightly	15	10	30	20	10	15				
	30						bonded, wet										
	36				36.9												
	27	9	37.0				Very dense, dark gray rock fragments, dry										
							Note: probable cobble										
					38.0		-GLACIAL TILL DEPOSITS-										
40	86	S8	40.0			SW	Very dense, gray well-graded SAND (SW), mps = 0.2 in., wet			10	40	45	5				
	53																
	76						-GLACIAL TILL DEPOSITS-										
	87	12	42.0														
							Bottom of exploration at 42.0 ft. below ground surface										
							No refusal										
45																	
50																	
55																	
60																	
65																	
70																	

NOTES:

FILE NO. 05109

BORING NO. B104

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. **B105**

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PROJECT	FINAL DESIGN INVESTIGATION, REDEVELOPMENT OF VILLAGE CAFÉ	STI JOB NO.	05109
LOCATION	NEWBURY STREET, PORTLAND, MAINE	PROJECT MGR.	C. DIMATTEO
CLIENT	GFI ACQUISITIONS I, LLC	FIELD REP.	K. B. STEPHENSON
CONTRACTOR	MAINE TEST BORINGS, INC.	DATE STARTED	1/4/2006
DRILLER	R. IDANO	DATE FINISHED	1/4/2006

Elevation 39.5 ft. Datum NGVD 1929 Boring Location See Plan

Item	Casing	Sampler	Core Barrel
Type	HSA	SS	--
Inside Diameter (in.)	2.5	1.375	--
Hammer Weight (lb.)	--	140	
Hammer Fall (in.)	--	30	

Rig Make & Model: Mobile B47

☐ Truck ☐ Tripod ☐ Cat-Head
☐ ATV ☐ Geoprobe ☑ Winch
☐ Track ☐ Air Track ☐ Roller Bit
☐ Skid ☑ Trailer ☑ Cutting Head

Hammer Type: ☐ Safety ☑ Doughnut ☐ Automatic

Drilling Mud: ☐ Bentonite ☐ Polymer ☑ None

Casing Advance: Type Method Depth: HSA/SPIN/24.0

Drilling Notes: 2.0 x 7.0 in. Field Vane

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
0							Advanced SSA through frost to 1.0 ft.										
	9	S1	1.0			SM	Medium dense, brown silty SAND (SM), mps = 0.5 in., dry		10	30	25	20	15				
	7				2.0		-FILL-										
	7					CL	Stiff, gray-brown mottled lean CLAY (CL), damp						100	N	M	M	
	13	16	3.0														
							-MARINE DEPOSITS-										
5																	
	5	S2	5.0		5.8	CL	Stiff, gray-brown mottled lean CLAY (CL), damp						100	N	M	M	
	7					CL	Stiff, gray lean CLAY (CL), occasional sand partings and dark streaks,					5	95	N	M	M	
	7						mps = 0.02 in., wet										
	7	24	7.0														
							-MARINE DEPOSITS-										
10																	
	WOH	FV1	10.0-10.6				FV1 from 10.0 to 10.6 ft. = 9/4 ft. lb., Su = 330 psf										
	WOH	S3	10.0			CL	Soft, gray lean CLAY (CL), occasional sand partings and dark					5	95	N	M	M	
	WOH						streaks, mps = 0.02 in., wet										
	WOH	24	12.0														
							-MARINE DEPOSITS-										
15																	
	1	S4	15.0			CL	Soft, gray lean CLAY (CL), occasional sand partings and dark					5	95	N	M	M	
	1				15.6		streaks, mps = 0.02 in., wet										
	1					CL	Soft, gray lean CLAY (CL), frequent sand seams to layers, mps =					20	80	N	M	M	
	2	24	17.0				0.02 in., wet										
							-MARINE DEPOSITS-										
					19.0												
20																	
	1	S5	20.0			SP	Loose, brown poorly-graded SAND (SP), mps = 0.1 in., clay seam				15	80	5				
	3						at 21.2 ft., wet										
	2				21.6		-MARINE DEPOSITS-										
	1	24	22.0			SM	Loose, gray silty SAND (SM), mps = 0.5 in., wet		10	30	25	20	15				
							-GLACIAL TILL DEPOSITS-										
					24.0												
25							HSA refusal at 24.0 ft.										
							Bottom of exploration at 24.0 ft. below ground surface										
30																	

Water Level Data

Date	Time	Elapsed Time (hr.)	Depth in feet to: Bottom of Casing	Bottom of Hole	Water
1/4/2006	1025		--	19.0	8.0

Sample ID

O	Open End Rod
T	Thin Wall Tube
U	Undisturbed Sample
S	Split Spoon Sample
G	Geoprobe
FV	Field Vane

Well Diagram

Riser Pipe, Screen, Filter Sand, Cuttings, Grout, Concrete, Bentonite Seal

Summary

Overburden (Linear ft.)	24.0
Rock Cored (Linear ft.)	--
Number of Samples	5S

BORING NO. B105

Field Tests — Dilatancy: R - Rapid S - Slow N - None
Toughness: L - Low M - Medium H - High
Plasticity: N - Nonplastic L - Low M - Medium H - High
Dry Strength: N - None L - Low M - Medium H - High V - Very High

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. **B107**

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PROJECT	FINAL DESIGN INVESTIGATION, REDEVELOPMENT OF VILLAGE CAFÉ	STI JOB NO.	05109
LOCATION	NEWBURY STREET, PORTLAND, MAINE	PROJECT MGR.	C. DIMATTEO
CLIENT	GFI ACQUISITIONS I, LLC	FIELD REP.	K. B. STEPHENSON
CONTRACTOR	MAINE TEST BORINGS, INC.	DATE STARTED	12/21/2005
DRILLER	R. IDANO	DATE FINISHED	12/22/2005

Elevation 44.1 ft. Datum NGVD 1929 Boring Location See Plan

Item	Casing	Sampler	Core Barrel
Type	HSA	SS	--
Inside Diameter (in.)	3.375	1.375	--
Hammer Weight (lb.)	--	140	
Hammer Fall (in.)	--	30	

Rig Make & Model: Mobile B47

☐ Truck ☐ Tripod ☐ Cat-Head
☐ ATV ☐ Geoprobe ☑ Winch
☐ Track ☐ Air Track ☐ Roller Bit
☐ Skid ☑ Trailer ☑ Cutting Head

Hammer Type: ☐ Safety ☑ Doughnut ☐ Automatic

Drilling Mud: ☐ Bentonite ☐ Polymer ☑ None

Casing Advance: Type Method Depth — HSA/SPIN/40.0

Drilling Notes: 2.0 x 7.0 in. Field Vane

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
0					0.2		-BITUMINOUS CONCRETE-										
	36	S1	0.5			SW-SM	Very dense, brown well-graded SAND with silt and gravel (SW-SM), mps =		20	30	10	30	10				
	29						0.3 in., dry										
	50/.1	8	1.6				-FILL-										
							Note: unable to auger through obstruction at 1.6 ft. Moved boring 4.0 ft.										
5																	
	50	S2	5.0			SW-SM	Medium dense, brown well-graded SAND with silt and gravel (SW-SM),		20	30	10	30	10				
	12						brick, wood, glass, mps = 0.3 in., dry										
	9				6.6		-FILL-										
	8	12	7.0			SP	Medium dense, gray-brown poorly-graded SAND (SP), wet					100					
							-MARINE DEPOSITS-										
10					10.0												
	WOR	S3	10.0			SP	Very loose, gray-brown poorly-graded SAND (SP), frequent clay seams, mps =					85	15				
	WOR						0.02 in., wet										
	WOH				11.5												
	1	20	12.0			CL	Very soft, gray lean CLAY (CL), frequent sand seams, mps = 0.02 in., wet					15	85	N	M	M	
							-MARINE DEPOSITS-										
15																	
	WOR	FV1	15.0-15.6				FV1 from 15.0 to 15.6 ft. = 10/1 ft. lb., Su = 370 psf										
	WOR	S4	15.0			CL	Soft, gray lean CLAY (CL), wet						100	N	M	M	
	WOR				16.6												
	WOR	24	17.0			CL	Soft, gray lean CLAY with sand (CL), mps = 0.02 in., wet					20	80	N	M	M	
							-MARINE DEPOSITS-										
20																	
	WOR	S5	20.0			CL	Soft, gray lean CLAY (CL), occasional black streaks and fine sand partings,					5	95	N	M	M	
	WOR						mps = 0.02 in., wet										
	WOR																
	WOH	24	22.0														
							-MARINE DEPOSITS-										
25																	
	WOR	FV2	25.0-25.6				FV2 from 25.0 to 25.6 ft. = 11/2 ft. lb., Su = 410 psf										
	WOR	S6	25.0			CL	Soft, gray lean CLAY (CL), occasional fine sand partings, mps = 0.02 in.,					5	95	N	M	M	
	WOH						wet										
	1	24	27.0														
							-MARINE DEPOSITS-										
30																	

Water Level Data

Date	Time	Elapsed Time (hr.)	Depth in feet to: Bottom of Casing	Bottom of Hole	Water
12/22/2005	1120		40.0	41.1	16.3
12/22/2005	1145		--	8.5	7.0

Sample ID

O	Open End Rod
T	Thin Wall Tube
U	Undisturbed Sample
S	Split Spoon Sample
G	Geoprobe
FV	Field Vane

Well Diagram

Riser Pipe, Screen, Filter Sand, Cuttings, Grout, Concrete, Bentonite Seal

Summary

Overburden (Linear ft.)	41.1
Rock Cored (Linear ft.)	--
Number of Samples	9S
BORING NO.	B107

Field Tests

Dilatancy: R - Rapid S - Slow N - None
Toughness: L - Low M - Medium H - High
Plasticity: N - Nonplastic L - Low M - Medium H - High
Dry Strength: N - None L - Low M - Medium H - High V - Very High

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. B107

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
30	WOR	S7	30.0		30.4	CL	Soft, gray lean CLAY (CL), occasional fine sand partings, mps = 0.02 in., wet					5	95	N	M	M	
	WOH					CL	Soft, gray lean CLAY (CL), frequent sand seams to layers, mps = 0.02 in., we					20	80	N	M	M	
	WOH																
	4	24	32.0				-MARINE DEPOSITS-										
35	WOR	S8	35.0			CL	Stiff, gray lean CLAY (CL), frequent sand seams to layers, mps = 0.02 in., we					20	80	N	M	M	
	2						-MARINE DEPOSITS-										
	10				36.3												
	15	24	37.0			SP	Medium dense, gray poorly-graded SAND (SP), mps = 0.02 in., wet					100					
							-MARINE DEPOSITS-										
40					40.0												
	34	S9	40.0			SW	Very dense, gray well-graded SAND (SW), mps = 0.2 in., wet				10	40	50				
	63						-MARINE DEPOSITS-										
	50/.1	24	41.1		41.3												
							Split spoon refusal at 41.1 ft. on probable bedrock										
							Bottom of exploration at 41.1 ft. below ground surface										
45																	
50																	
55																	
60																	
65																	
70																	

NOTES:

FILE NO. 05109

BORING NO. B107

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. **B108**

Page 1 of 2

PROJECT	FINAL DESIGN INVESTIGATION, REDEVELOPMENT OF VILLAGE CAFÉ	STI JOB NO.	05109
LOCATION	NEWBURY STREET, PORTLAND, MAINE	PROJECT MGR.	C. DIMATTEO
CLIENT	GFI ACQUISITIONS I, LLC	FIELD REP.	K. B. STEPHENSON
CONTRACTOR	MAINE TEST BORINGS, INC.	DATE STARTED	12/22/2005
DRILLER	R. IDANO	DATE FINISHED	12/22/2005

Elevation 49.2 ft. Datum NGVD 1929 Boring Location See Plan

Item	Casing	Sampler	Core Barrel
Type	HSA	SS	--
Inside Diameter (in.)	3.375	1.375	--
Hammer Weight (lb.)	--	140	
Hammer Fall (in.)	--	30	

Rig Make & Model: Mobile B47

☐ Truck ☐ Tripod ☐ Cat-Head
☐ ATV ☐ Geoprobe ☑ Winch
☐ Track ☐ Air Track ☐ Roller Bit
☐ Skid ☑ Trailer ☑ Cutting Head

Hammer Type: ☐ Safety ☑ Doughnut ☐ Automatic

Drilling Mud: ☐ Bentonite ☐ Polymer ☑ None

Casing Advance: Type Method Depth — HSA/SPIN/40.0

Drilling Notes: 2.0 x 7.0 in. Field Vane

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
0					0.2		-BITUMINOUS CONCRETE-										
	13	S1	0.5			SM	Dense, brown to black silty SAND (SM), brick, ash, mps = 0.5 in., dry		5	10	10	60	15				
	11																
	21						-FILL-										
	36	9	2.5														
					3.0												
5	9	S2	5.0			SP	Dense, brown poorly-graded SAND (SP), occasional coarse to medium sand			5	5	85	5				
	13						seams, silt seams from 6.8 to 7.0 ft., mps = 0.2 in., wet										
	19																
	17	16	7.0				-MARINE DEPOSITS-										
					8.5												
10	2	S3	10.0			SM	Very loose, gray-brown mottled silty SAND (SM), trace coarse sand, one					85	15				
	1						1.3 in. gravel piece, wet										
	2																
	4	18	12.0				-MARINE DEPOSITS-										
15	1	S4	15.0			SM	Loose, brown silty SAND (SM), frequent silt seams, mps = 0.5 in., wet		5	10	15	50	20				
	2				16.0												
	2					SM	Loose, gray silty SAND (SM), frequent clay seams, mps = 0.1 in., wet				5	70	25				
	3	24	17.0				-MARINE DEPOSITS-										
20	WOR	S5	20.0			SM	Very loose, gray silty SAND (SM), frequent clay seams, mps = 0.1 in., wet				5	70	25				
	WOH				20.8		-MARINE DEPOSITS-										
	1					CL	Medium stiff, gray lean CLAY (CL), wet										
	3	24	22.0				-MARINE DEPOSITS-										
25	WOH	FV1	25.0-25.6				FV1 from 25.0 to 25.6 ft. = 15/2 ft. lb., Su = 560 psf										
	WOH	S6	25.0			CL	Medium stiff, gray lean CLAY (CL), occasional sand partings, mps = 0.02					5	95	N	M	M	
	1						in., wet										
	2	24	27.0				-MARINE DEPOSITS-										
30																	

Water Level Data

Date	Time	Elapsed Time (hr.)	Depth in feet to: Bottom of Casing	Bottom of Hole	Water

Sample ID

O	Open End Rod
T	Thin Wall Tube
U	Undisturbed Sample
S	Split Spoon Sample
G	Geoprobe
FV	Field Vane

Well Diagram

Riser Pipe, Screen, Filter Sand, Cuttings, Grout, Concrete, Bentonite Seal

Summary

Overburden (Linear ft.)	40.0
Rock Cored (Linear ft.)	--
Number of Samples	85

BORING NO. B108

Field Tests — Dilatancy: R - Rapid S - Slow N - None
Toughness: L - Low M - Medium H - High
Plasticity: N - Nonplastic L - Low M - Medium H - High
Dry Strength: N - None L - Low M - Medium H - High V - Very High

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. B108

Page 2 of 2

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
30	WOR	S7	30.0			CL	Medium stiff, gray lean CLAY (CL), frequent sand partings to seams, mps = 0.02 in., wet					15	85	N	M	M	
	WOR																
	3																
	4	24	32.0				-MARINE DEPOSITS-										
35	WOR	S8	35.0			CL	Medium stiff, gray lean CLAY (CL), frequent sand layers, mps = 0.02 in., wet					40	60	N	M	M	
	2				36.0												
	10					SP-SM	Medium dense, gray poorly-graded SAND with silt (SP-SM), mps = 0.1 in., wet				10	80	10				
	15	24	37.0				-MARINE DEPOSITS-										
40							Note: advanced HSA to 40.0 ft. Running sand conditions in augers										
							Bottom of exploration at 40.0 ft. below ground surface										
							No refusal										
45																	
50																	
55																	
60																	
65																	
70																	

NOTES:

FILE NO. 05109

BORING NO. B108

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. **B109**

Page 1 of 1

PROJECT	FINAL DESIGN INVESTIGATION, REDEVELOPMENT OF VILLAGE CAFÉ	STI JOB NO.	05109
LOCATION	NEWBURY STREET, PORTLAND, MAINE	PROJECT MGR.	C. DIMATTEO
CLIENT	GFI ACQUISITIONS I, LLC	FIELD REP.	K. B. STEPHENSON
CONTRACTOR	MAINE TEST BORINGS, INC.	DATE STARTED	12/23/2005
DRILLER	R. IDANO	DATE FINISHED	12/23/2005

Elevation 38.5 ft. Datum NGVD 1929 Boring Location See Plan

Item	Casing	Sampler	Core Barrel
Type	HSA	SS	--
Inside Diameter (in.)	3.375	1.375	--
Hammer Weight (lb.)	--	140	
Hammer Fall (in.)	--	30	

Rig Make & Model: Mobile B47

☐ Truck ☐ Tripod ☐ Cat-Head
☐ ATV ☐ Geoprobe ☑ Winch
☐ Track ☐ Air Track ☐ Roller Bit
☐ Skid ☑ Trailer ☑ Cutting Head

Hammer Type: ☐ Safety ☑ Doughnut ☐ Automatic

Drilling Mud: ☐ Bentonite ☐ Polymer ☑ None

Casing Advance: Type Method Depth — HSA/SPIN/20.0

Drilling Notes: 2.0 x 7.0 in. Field Vane

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
0							Note: advance HSA through ice/frost to 1.0 ft.										
	36	S1	1.0		1.2	SM	Dense, brown silty SAND (SM), mps = 0.5 in., damp -FILL-		5	5	5	75	15				
	19					CL	Very stiff, gray lean CLAY (CL), frequent sand partings, mps = 0.02 in.,					15	85	N	M	M	
	11						damp										
	16	18	3.0				-MARINE DEPOSITS-										
5	8	S2	5.0			CL	Very stiff, gray lean CLAY (CL), frequent sand partings, mps = 0.02 in.,					10	90	N	M	M	
	9						damp										
	10																
	8	24	7.0				-MARINE DEPOSITS-										
10	WOH	S3	10.0			CL	Soft, gray lean CLAY (CL), one 0.5 in. concretion, wet					10	90	N	M	M	
	WOH																
	WOH																
	1	24	12.0				-MARINE DEPOSITS-										
					14.4												
15	WOR	FV1	15.0-15.6				FV1 from 15.0 to 15.6 ft. = 10/1 ft. lb., Su = 370 psf										
	WOH	S4	15.0			CL	Soft, gray lean CLAY (CL), frequent sand layers, mps = 0.02 in., wet					25	75	N	M	M	
	4				16.8												
	10	24	17.0			SP	Medium dense, gray poorly-graded SAND (SP), mps = 0.02 in., wet					100					
							-MARINE DEPOSITS-										
20	8	S5	20.0		20.5	SP	Very dense, gray to brown poorly-graded SAND (SP), mps = 0.02 in., wet					100					
	13					SM	Very dense, gray silty SAND with gravel (SM), mps = 0.75 in., wet	5	10	15	15	40	15				
	50/.4	24	21.4		21.4		-GLACIAL TILL DEPOSITS-										
							Split spoon refusal at 21.4 ft.										
							Bottom of exploration at 21.4 ft. below ground surface										
							Note: Installed 1.0 in PVC observation well at 20.0 ft.										
25																	
30																	

Water Level Data

Date	Time	Elapsed Time (hr.)	Depth in feet to: Bottom of Casing	Bottom of Hole	Water
1/19/2006			Well	20.0	3.3

Sample ID

O	Open End Rod
T	Thin Wall Tube
U	Undisturbed Sample
S	Split Spoon Sample
G	Geoprobe
FV	Field Vane

Well Diagram

Riser Pipe, Screen, Filter Sand, Cuttings, Grout, Concrete, Bentonite Seal

Summary

Overburden (Linear ft.)	21.4
Rock Cored (Linear ft.)	--
Number of Samples	5S
BORING NO.	B109

Field Tests — Dilatancy: R - Rapid S - Slow N - None
Toughness: L - Low M - Medium H - High
Plasticity: N - Nonplastic L - Low M - Medium H - High
Dry Strength: N - None L - Low M - Medium H - High V - Very High

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.