

GENERAL

1. ALL WORK SHALL CONFORM TO THE REQUIREMENTS OF ALL APPLICABLE STATE AND LOCAL CODES, INCLUDING BUT NOT LIMITED TO:
2003 INTERNATIONAL BUILDING CODE
ANSI/ASCE 7-02
ACI 318-02 BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE
ACI 301 SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS
AISC STEEL CONSTRUCTION MANUAL
ANSI/AFPA NDS-01

ANY DISCREPANCIES BETWEEN THE ABOVE LISTED CODES AND THE CONSTRUCTION DOCUMENTS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER FOR CLARIFICATION BEFORE PROCEEDING WITH AFFECTED WORK.

2. ALL WORK SHALL BE PERFORMED BY PERSONS QUALIFIED IN THEIR TRADE AND LICENSED TO PRACTICE SUCH TRADE IN THE STATE IN WHICH THE PROJECT IS LOCATED.
3. THESE DRAWINGS SHALL BE USED IN CONJUNCTION WITH ANY ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS IN ADDITION TO SPECIFICATIONS AND ANY SHOP DRAWINGS PROVIDED BY SUBCONTRACTORS AND SUPPLIERS.
4. ALL DIMENSIONS, ELEVATIONS, AND CONDITIONS SHALL BE VERIFIED IN THE FIELD BY THE GENERAL CONTRACTOR (G.C.) AND ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER FOR CLARIFICATION BEFORE PROCEEDING WITH THE AFFECTED PART OF WORK.
5. UNLESS OTHERWISE NOTED, DETAILS, SECTIONS, AND NOTES SHOWN ON THESE DRAWINGS SHALL BE CONSIDERED TYPICAL FOR ALL SIMILAR DETAILS.
6. THESE DRAWINGS DO NOT SHOW SIZE, LOCATION, OR TYPE OF OPENINGS IN THE FOUNDATION SYSTEM FOR ELECTRICAL, PLUMBING, OR MECHANICAL EQUIPMENT. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR LOCATING OF THESE ITEMS.
7. ALL SHOP DRAWINGS PROVIDED BY OTHERS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW PRIOR TO THE FABRICATION OF MATERIAL OR THE PURCHASE OF NON-RETURNABLE STOCK. QUANTITY AND DIMENSIONAL REVIEW IS THE CONTRACTOR'S RESPONSIBILITY.
8. FOUNDATION DRAINS ARE REQUIRED AS SHOWN ON DRAWINGS AND/OR GEOTECHNICAL REPORT.

9. ANY AND ALL TEMPORARY BRACING OR SHORING WHICH IS NEEDED TO HOLD THE STRUCTURE IN A SAFE AND STABLE POSITION UNTIL THE BUILDING IS COMPLETE, IS SOLELY THE RESPONSIBILITY OF THE CONTRACTOR. CONSULT INDEPENDENT ENGINEER IF DESIGN ASSISTANCE OR REVIEW IS NEEDED.
10. THE BUILDING PERMIT APPLICANT (e.g. OWNER, CONTRACTOR) MUST PROVIDE SPECIAL INSPECTIONS PER THE REQUIREMENTS OF CHAPTER 17 OF THE 2003 INTERNATIONAL BUILDING CODE AND FURNISH INSPECTION REPORTS TO THE CODE OFFICIAL AND TO THE ENGINEER OF RECORD. THE TESTING/INSPECTION AGENCY(S) MUST BE APPROVED BY THE ENGINEER OF RECORD. A SCHEDULE OF SPECIAL INSPECTIONS IS LISTED ON SHEET SP.

11. THE ENGINEER, AT HIS OPTION, MAY PROVIDE THE CONTRACTOR WITH ELECTRONIC FILES FOR HIS/HER CONVENIENCE AND USE IN THE PREPARATION OF SHOP DRAWINGS. DATA CONTAINED ON THESE ELECTRONIC FILES ARE THE ENGINEER'S INSTRUMENT OF SERVICE AND MAY NOT BE ELECTRONICALLY COPIED FOR REUSE AS SHOP DRAWINGS. FURTHERMORE, THESE ELECTRONIC FILES ARE NOT CONSTRUCTION DOCUMENTS AND THEREBY, THE CONTRACTOR IS NOT RELIEVED OF HIS/HER DUTY TO FULLY COMPLY WITH THE CONTRACT DOCUMENTS, INCLUDING, WITHOUT LIMITATION, THE NEED TO CHECK, CONFIRM AND COORDINATE ALL DIMENSIONS AND DETAILS, TAKE FIELD MEASUREMENTS, VERIFY FIELD CONDITIONS AND COORDINATE THE CONTRACTOR'S WORK WITH THAT OF OTHER CONTRACTORS FOR THE PROJECT. ADDITIONALLY, THE CONTRACTOR MAY NOT MANUALLY ALTER THE HARD COPIES OF THE CONSTRUCTION DOCUMENTS AND REUSE THEM AS SHOP DRAWINGS.

DESIGN LOADS

1. THE STRUCTURE IS DESIGNED IN ACCORDANCE WITH IBC 2003 TO CARRY ALL THE DEAD LOADS OF THE VARIOUS STRUCTURAL, ARCHITECTURAL, MECHANICAL AND OTHER SYSTEMS LISTED TO THE ENGINEER AT THE TIME OF THE ISSUANCE OF THESE DRAWINGS. THE STRUCTURE IS DESIGNED TO CARRY THE FOLLOWING LIVE LOADS:

RESIDENTIAL 40 PSF LIVE (1ST, 2ND, 3RD, AND 4TH FLOORS)
COURTYARD 100 PSF LIVE/SNOW
GARAGE 40 PSF LIVE/3000 LBS. POINT LOAD

BASIC GROUND SNOW LOAD 60 PSF
 $C_e = 1.0$
 $C_t = 1.1$
 $I_s = 1.0$

ROOF TRUSSES SEE WOOD TRUSSES ON SHEET SN.1
WIND SPEED = 100 MPH
EXPOSURE "C"

$I_w = 1.0$
SEISMIC
SITE CLASS "D"
 $I_e = 1.0$
 $S_D = 0.25$
 $S_I = 0.07$
SEISMIC PERFORMANCE CATEGORY "B"

SOIL BEARING

1. ALL FOOTINGS SHALL BE CARRIED DOWN TO REST ON UNDISTURBED SOIL OR SHALL BEAR ON STRUCTURAL FILL COMPACTED IN 12" LAYERS TO 95% COMPACTION OR ON RAMMED EARTH PIERS (SEE GEOTECHNICAL FOUNDATIONS). THE UNDERLYING SOILS AND THE STRUCTURAL FILL SHALL HAVE A MINIMUM SAFE LOAD BEARING CAPACITY OF 3000 PSF.
2. REMOVE ALL EXISTING TOPSOIL, PAVEMENT, ORGANIC MATERIALS, OR OTHER SOIL THAT APPEAR TO BE UNSUITABLE PRIOR TO PREPARING THE FOOTING GRADE.
3. IF ANY ADVERSE SOIL CONDITIONS ARE ENCOUNTERED WHICH EXTEND BELOW FOOTING LEVEL, SUCH AS THOSE LISTED ABOVE AND NOTED IN THE GEOTECHNICAL REPORT, THE GENERAL CONTRACTOR SHALL CONTACT THE ENGINEER IMMEDIATELY FOR DETERMINATION OF HOW TO REMEDY THE CONDITION BEFORE CONTINUATION OF THE WORK.
4. NO FOOTINGS SHALL BE PLACED IN WATER OR ON FROZEN GROUND. ALL EXTERIOR CONSTRUCTION SHALL BE CARRIED DOWN TO A MINIMUM OF FOUR FEET AND SIX INCHES (4'-6") BELOW FINISHED, ADJACENT EXTERIOR GRADE.
5. REFER TO GEOTECHNICAL REPORT BY SEBAGO TECHNICS, INC. FOR ALL INFORMATION REGARDING EXCAVATION, BACKFILL, STRUCTURAL FILL, SUBGRADE PREPARATION, ETC. IF ANY CONTRADICTING INFORMATION IS FOUND BETWEEN GEOTECHNICAL REPORT AND STRUCTURAL DRAWINGS, GEOTECHNICAL REPORT SHALL GOVERN.

CAST-IN-PLACE-CONCRETE

1. ALL WORK SHALL CONFORM TO "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" (ACI 318-99) AND "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" (ACI 301)
2. INTERIOR SLABS ON GRADE TO BE OF THICKNESS SHOWN ON DRAWINGS WITH WELDED WIRE MESH. SEE DETAILS FOR SIZE AND LOCATION.
3. PROVIDE 10-MIL REINFORCED POLYETHYLENE MOISTURE VAPOR RETARDER DIRECTLY BELOW ALL INTERIOR SLABS ON GRADE. OVERLAP SEAMS MINIMUM 6" AND TAPE AS REQUIRED TO MAINTAIN POSITION.
4. ALL FOOTINGS ARE TO REST ON UNDISTURBED SOIL OR CLEAN GRANULAR FILL COMPACTED IN LAYERS OF 12" OR LESS TO 95% COMPACTION.
5. MINIMUM CONCRETE PROTECTION FOR REINFORCING STEEL SHALL BE AS FOLLOWS:
CONCRETE CAST AGAINST EARTH: 3 INCHES
FORMED CONCRETE EXPOSED TO EARTH OR WEATHER:
1-1/2 INCHES FOR #5 BARS AND SMALLER
2 INCHES FOR #6 BARS AND GREATER
6. CALCIUM CHLORIDE IS PROHIBITED IN ANY CONCRETE MIX.
7. CONCRETE SHALL BE ADEQUATELY PROTECTED FROM HOT OR COLD WEATHER AS REQUIRED BY ACI PUBLICATIONS 305 AND 306, RESPECTIVELY.
8. ALL CONCRETE FOR WALLS, FOOTINGS, AND INTERIOR SLABS SHALL ATTAIN A MINIMUM ULTIMATE COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS (U.N.O.). CYLINDERS SHALL BE TAKEN AND TESTED IN ACCORDANCE WITH ACI RECOMMENDATIONS. EXTERIOR SLABS SHALL ATTAIN A MINIMUM ULTIMATE COMPRESSIVE STRENGTH OF 4000 PSI AT 28 DAYS.
9. SLAB CONTROL JOINTS, WHERE SHOWN, SHALL BE SAW CUT AND SHALL BE CUT IMMEDIATELY AFTER FINISHING. JOINTS SHALL BE AT MINIMUM 1/4 OF THE THICKNESS OF THE SLAB.
10. WALL CONTROL JOINTS SHALL BE PLACED AS SHOWN ON DRAWINGS OR AT A MAXIMUM OF 40 FEET ON CENTER.
11. BACKFILL BOTH SIDES OF THE FOUNDATION WALL SIMULTANEOUSLY TO THE MAXIMUM HEIGHT POSSIBLE.
12. ALL CONCRETE SHALL BE CURED BY AN APPROVED METHOD AS PRESCRIBED BY ACI.
13. MAXIMUM WATER TO CEMENT RATIO SHALL BE 0.5 FOR 3000 PSI CONCRETE AND 0.45 FOR 4000 PSI CONCRETE MIXES WITH MID-RANGE WATER REDUCERS (MRWR) USED. W/C RATIO FOR 3000 PSI CONCRETE IN FOOTINGS MAY BE 0.53 WITHOUT THE USE OF MID-RANGE WATER REDUCERS. MINIMUM CEMENT QUANTITIES SHALL BE 517 LB./YD FOR 3000 PSI CONCRETE AND 611 LB./YD FOR 4000 PSI CONCRETE.
14. MAXIMUM CONCRETE SLUMP SHALL BE 4 INCHES WITHOUT MRWR AND 6 INCHES WITH MRWR. MRWR MUST BE USED IN ALL CONCRETE EXCEPT FOOTINGS.

REINFORCING STEEL

1. ALL REINFORCING, EXCEPT AS NOTED, SHALL BE DEFORMED BARS CONFORMING TO ASTM A615 GRADE 60.
2. WELDED WIRE FABRIC REINFORCEMENT SHALL CONFORM TO THE REQUIREMENTS OF ASTM A185. USE FLAT SHEETS ONLY.
3. ALL REINFORCING SHALL BE DETAILED IN ACCORDANCE WITH THE LATEST ACI DETAILING MANUAL.
4. WHERE CONTINUOUS BARS ARE CALLED FOR, INDICATED, REQUIRED, THEY SHALL RUN CONTINUOUSLY AROUND CORNERS, LAPPED AT NECESSARY SPLICES, SPLICES STAGGERED AND HOOKED AT DISCONTINUOUS ENDS. LAP LENGTHS SHALL BE AS SHOWN OR NOTED ON THE DRAWINGS. IF LAP/SPLICE LENGTHS ARE NOT INDICATED FOLLOW ACI STANDARDS.

SLAB-ON-GRADE CONTROL JOINTS

1. CONTROL JOINTS IN CONCRETE SLABS ARE GENERALLY SPACED IN A MANNER TO CONTROL CRACK LOCATIONS OCCURRING DUE TO CURING SHRINKAGE AND THERMAL MOVEMENT OF CONCRETE. WELDED WIRE FABRIC DOES NOT INHIBIT CRACKING, BUT HOLDS CONCRETE TIGHTLY TOGETHER AFTER CRACKING HAS OCCURRED. IN ORDER TO BETTER CONTROL RANDOM CRACKING OF CONCRETE THE FOLLOWING MEASURES ARE RECOMMENDED:
A) SUPPLY A WELL CONTRACTED AND CONSISTENT SUBGRADE.
B) LIMIT WATER VOLUME IN CONCRETE USING A STIFFER MIX.
C) SUPPLY ADEQUATE CURING MEASURES. WET CURE OR USE CURING SEALERS.
D) LIMIT JOINT SPACING TO 2 TIMES SLAB THICKNESS IN FEET.
2. SLAB CURLING IS ALSO A PROBLEM WHICH HAS BECOME MORE PREVALENT WITH MODERN CONCRETE MIXES WHICH HAVE HIGHER STRENGTHS. THE FOLLOWING MEASURES IN ADDITION TO THOSE STATED ABOVE ARE RECOMMENDED TO LIMIT CURLING OF CONCRETE SLABS-ON-GRADE:
A) CURE THE SLAB PROPERLY.
B) USE HIGHER QUANTITY OF COARSE AGGREGATES IN THE MIX.
C) USE A LOWER AMOUNT OF CEMENT.

RAMMED AGGREGATE PIERS

1. RAMMED AGGREGATE PIERS AND GROUTED RAMMED AGGREGATE PIERS SHALL BE PROVIDED BENEATH COLUMN FOOTINGS AND WALL FOOTINGS AS RECOMMENDED BY THE GEOTECHNICAL ENGINEER AND APPROVED BY THE STRUCTURAL ENGINEER.
2. THE AGGREGATE PIER INSTALLER SHALL RETAIN A PROFESSIONAL ENGINEER, LICENSED IN THE STATE OF MAINE, TO PROVIDE STAMPED SHOP DRAWINGS FOR THE STRUCTURAL ENGINEER'S REVIEW.
3. THE SHOP DRAWINGS SHALL INDICATE FOOTING LOCATIONS WHERE THE FOOTING DESIGNED FOR SPREAD-FOOTING BEARING CONDITIONS IS REQUIRED TO BE INCREASED OR RECONFIGURED TO ACCOMMODATE THE AGGREGATE PIER LAYOUT AND GEOMETRY. G.C. NOTE THAT SOME FOOTINGS MAY INCREASE IN SIZE TO ACCOMMODATE PIER LAYOUT.
4. SEE DETAIL 3/50.6 FOR RAMMED AGGREGATE PIER LAYOUT PLAN.

STRUCTURAL STEEL

1. STRUCTURAL STEEL WORK SHALL CONFORM TO ALL REQUIREMENTS OF THE 2003 INTERNATIONAL BUILDING CODE.
2. STRUCTURAL STEEL WORK SHALL CONFORM TO "SPECIFICATIONS FOR DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS (AISC CURRENT EDITION)", "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS (AISC CURRENT EDITION)", AND "STRUCTURAL WELDING CODE (AWS D1.1)".
3. STRUCTURAL STEEL SHALL BE NEW STEEL CONFORMING TO THE FOLLOWING:
A) ROLLED SHAPES AND PLATES - ASTM A36 (EXCEPT AS NOTED BELOW)
B) WIDE FLANGE SHAPES - ASTM A572, 50 KSI
C) STRUCTURAL TUBES - ASTM A500, GRADE B
D) ANCHOR RODS - ASTM F1554 GRADE 36 (HEADED BOLTS)
4. ALL BOLTED CONNECTIONS SHALL USE NEW BOLTS. SLIP-CRITICAL BOLTS ARE PROHIBITED FROM ALL CONNECTIONS. ALL BOLTS SHALL BE INSTALLED AS BEARING TO A "SNUG-TIGHTENED" CONDITION, UNLESS NOTED OTHERWISE ON THE DRAWINGS. ALL BOLTED CONNECTIONS SHALL BE DESIGNED, FABRICATED, AND INSTALLED IN COMPLIANCE WITH RCSC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS", DATED JUNE 23, 2000.
5. SINGLE ANGLE SHEAR CONNECTIONS SHALL NOT BE USED.
6. VOIDS BENEATH COLUMN BASE PLATES SHALL BE DRY PACKED WITH NON-SHRINK CONSTRUCTION GROUT BEFORE APPLICATION OF LOADS.
7. WELDED CONNECTIONS SHALL BE MADE BY AWS QUALIFIED WELDERS USING FILLER MATERIAL CONFORMING TO E70XX.
8. PROVIDE TEMPORARY ERECTION BRACING TO HOLD STRUCTURAL STEEL FRAMING SECURELY IN PLACE. MAINTAIN BRACING UNTIL FLOOR DECK AND PERMANENT LATERAL BRACING ARE FULLY INSTALLED. BRACING REQUIREMENTS ARE NOT PROVIDED BY THE E.O.R.
9. STRUCTURAL STEEL SHALL BE TRUE AND PLUMB BEFORE CONNECTIONS ARE FINALLY BOLTED OR WELDED.
10. ALL BOLTS AND FIELD WELDING MUST BE COMPLETED PRIOR TO RELEASING HOISTING CABLES.
11. FIELD CUTTING OF STRUCTURAL STEEL OR ANY MODIFICATIONS SHALL NOT BE MADE WITHOUT APPROVAL BY ENGINEER.

12. ALL CONNECTIONS SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER RETAINED BY THE FABRICATOR. SHOP DRAWINGS AND STAMPED CALCULATIONS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW AND APPROVAL PRIOR TO FABRICATION. FABRICATOR'S ENGINEER SHALL BE LICENSED IN ME AND CARRY PROFESSIONAL LIABILITY INSURANCE WITH A MINIMUM PER INCIDENT AND ANNUAL COVERAGE OF \$1,000,000.

13. ALL STRUCTURAL STEEL SHALL RECEIVE ONE (1) SHOP COAT OF RUST INHIBITIVE PRIMER OR TEMPORARY COATING APPROPRIATE FOR INTUDESCENT PAINT. ALL STEEL EXPOSED TO WEATHER OR OUTSIDE THE MAIN ENVELOPE OF THE BUILDING SHALL BE HOT-DIP GALVANIZED. DO NOT PAINT TOP FLANGE WHERE SHEAR STUDS WILL BE WELDED FOR COMPOSITE CONSTRUCTION.

14. THE STEEL FABRICATOR SHALL BE AISC CERTIFIED, OR BE ABLE TO DEMONSTRATE TO THE ENGINEER'S SATISFACTION THAT ALL AISC PROCEDURES FOR FABRICATION, QUALITY CONTROL, AND RECORD KEEPING ARE STRICTLY ADHERED TO. THE ENGINEER SHALL DETERMINE IF FABRICATOR QUALIFICATIONS ARE ACCEPTABLE.

15. SHOP DRAWINGS SHALL BE PREPARED BY FABRICATOR. PHOTO COPIES OF STRUCTURAL DRAWINGS ARE NOT ACCEPTABLE.

16. THE TESTING AGENCY (TO BE APPROVED BY JSN ASSOCIATES, INC.) MUST PERFORM A VISUAL INSPECTION OF ALL FIELD WELDS. ADDITIONALLY, ALL FIELD FILLET WELDS GREATER THAN 5/16" AND MULTIPASS WELDS AND PARTIAL PENETRATION WELDS MUST BE SPOT TESTED AT A RATE OF ONE TEST PER MEMBER USING THE MAGNETIC PARTICLE METHOD. ONE HUNDRED PERCENT (100%) OF ALL FIELD AND SHOP FULL PENETRATION WELDS MUST BE TESTED USING THE ULTRASONIC METHOD.

STEEL DECKS

1. STEEL FLOOR DECK SHALL BE GALVANIZED 3" LOCK FLOOR, 18 GAGE MINIMUM COMPOSITE DECK AS MANUFACTURED BY 'USD' OR APPROVED EQUAL.
2. FLOOR DECKS SHALL SPAN OVER THREE (3) OR MORE SUPPORTS.
3. SHEET STEEL FOR GALVANIZED COMPOSITE FLOOR DECKS MUST CONFORM TO ASTM A653-94 "STRUCTURAL QUALITY", GRADE 33 OR HIGHER. GALVANIZING MUST CONFORM TO ASTM A924-94 WITH A MINIMUM COATING CLASS OF G60 AS DEFINED IN A653-94.

FOUNDATION UNDERPINNING

1. FOUNDATION UNDERPINNING WILL BE REQUIRED DURING THE COURSE OF CONSTRUCTION. C.P./CONTRACTOR SHOULD CONSULT INDEPENDENT ENGINEER FOR RECOMMENDATIONS AND DESIGN. FINAL DESIGN SHOULD BE SUBMITTED TO E.O.R. FOR REVIEW PRIOR TO PROCEEDING WITH AFFECTED AREA OF WORK.

STEEL STAIRS

1. STEEL STAIRS TO BE DESIGNED BY STEEL STAIR FABRICATOR TO CONFORM TO STRUCTURAL AND DIMENSIONAL REQUIREMENTS OF CONTRACT DRAWINGS AND ALL APPLICABLE CODE REQUIREMENTS.
2. STEEL STAIR FABRICATOR SHALL PROVIDE SHOP DRAWINGS FOR REVIEW AND APPROVAL PRIOR TO START OF FABRICATION.

MASONRY VENEER LINTEL SCHEDULE

ROUGH OPENING	LINTEL SIZE
UP TO 3'-0"	L3-1/2 x 3-1/2 x 5/16
3'-1" TO 4'-6"	L4 x 3-1/2 x 5/16
4'-6" TO 6'-0"	L5 x 3-1/2 x 5/16
6'-0" TO 8'-0"	L6 x 3-1/2 x 5/16
8'-0" TO 11'-0"	L7 x 4 x 3/8

NOTES:

1. ALL LINTELS SHALL BE HOT DIPPED GALVANIZED.
2. PROVIDE 8" OF BEARING EACH END OF ALL LINTELS.
3. UNEQUAL LEG LINTELS SHALL BE INSTALLED WITH LONG LEG VERTICAL.
4. LINTELS SHOWN ARE FOR 4" VENEER THICKNESS ONLY.

CONCRETE MASONRY UNIT CONSTRUCTION

1. CONCRETE MASONRY UNIT (CMU) CONSTRUCTION SHALL CONFORM TO "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" (ACI 530-99 / ASCE 5-99 / TMS 402-99).
2. REINFORCED MASONRY SHALL CONSIST OF MASONRY UNITS, MORTAR BETWEEN UNITS, GROUT IN CELLS, LINTELS, BOND BEAMS, HORIZONTAL JOINT REINFORCING, AND STEEL REINFORCING IN VERTICAL CELLS, BOND BEAMS AND LINTELS.
3. CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90 AND SHALL HAVE A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 1900 PSI. CERTIFICATION OF UNIT STRENGTH SHALL BE PROVIDED BY MANUFACTURER.
4. GROUT SHALL BE CONCRETE WITH A MINIMUM 28-DAY COMPRESSIVE STRENGTH (F_c) OF 2000 PSI, WITH A MAXIMUM COARSE AGGREGATE SIZE OF 3/8", SLUMP AT POINT OF PLACEMENT OF 8 TO 11 INCHES, AND DESIGNED FOR PUMPING. GROUT SHALL CONFORM TO ASTM C476 "SPECIFICATION FOR MORTAR AND GROUT FOR MASONRY".
5. THE MINIMUM COMPRESSIVE STRENGTH OF CMU CONSTRUCTION (F_m) SHALL BE 1500 PSI AND SHALL BE DETERMINED USING THE UNIT STRENGTH METHOD PER ACI 530-99/ASCE 5-99/TMS 402-99 SECTION 1.4.
6. MORTAR FOR REINFORCED MASONRY SHALL MEET THE APPLICABLE REQUIREMENTS OF ASTM SPECIFICATION C270, TYPE S.
7. GROUT AND MORTAR SHALL BE KEPT ENTIRELY SEPARATE, AND SHALL NOT BE USED INTERCHANGEABLY.
8. PROVIDE LADDER-MESH HORIZONTAL JOINT REINFORCEMENT AT 16" ON CENTER (EVERY OTHER COURSE), CONFORMING TO ANSI/ASTM A82, WITH 9-GAGE SIZE RODS AND CROSS TIES. JOINT REINFORCEMENT SHALL BE CONTINUOUS WITH SECTIONS LAPPED 6" MINIMUM, EXCEPT AT CONTROL JOINTS WHERE JOINT REINFORCING SHALL TERMINATE. JOINT REINFORCEMENT IN EXTERIOR WALLS AND INTERIOR WALLS EXPOSED TO MOISTURE SHALL BE HOT-DIP GALVANIZED AFTER FABRICATION (ASTM A641-98). JOINT REINFORCEMENT IN ALL OTHER WALLS SHALL BE MILL GALVANIZED (ASTM A153-98).
9. TYPICAL VERTICAL REINFORCING SHALL BE #5 BARS AT 32" ON CENTER, UNLESS NOTED OTHERWISE ON PLANS. VERTICAL REINFORCING SHALL BE PLACED AT EACH JAMB OF EACH WALL OPENING AND AT EACH CORNER AND WALL INTERSECTION.
10. VERTICAL REINFORCING SHALL BE CONTINUOUS AND SHALL LAP A MINIMUM OF 48 BAR DIAMETERS. (30" FOR #5 BARS). BARS SHALL BE SUPPORTED BY WIRE POSITIONERS AS REQUIRED TO MAINTAIN PROPER POSITION IN CELL.
11. CELLS ARE TO BE GROUTED USING LOW-LIFT GROUTING PROCEDURES. CELLS SHALL BE FILLED TO DEPTH 4" AND RODDED OR VIBRATED PERMITTED TO REST FOR A PERIOD OF 30-60 MINUTES, AN ADDITIONAL 4" DEPTH FILLED, AND AGAIN RODDED OR VIBRATED. THIRD VIBRATING SHALL EXTEND AT LEAST 12" INTO PREVIOUSLY GROUTED LAYER. GROUT SHALL BE PUMPED TO PLACE. GROUT LEVEL AT EACH LIFT SHALL STOP MIN 1/2" BELOW TOP OF CMU TO FORM A KEYWAY.
12. IF HIGH-LIFT GROUTING IS DESIRED, THE CONTRACTOR MUST SUBMIT A WRITTEN PROPOSED PROCEDURE COMPLYING WITH ACI CODE TO THE ENGINEER FOR REVIEW AND APPROVAL.
13. MORTAR PLASTICITY SHALL BE MAINTAINED BY RE-TEMPERING AS REQUIRED UP TO 2-1/2 HOURS AFTER ORIGINAL MIXING. MORTAR REQUIRING RE-TEMPERING AFTER THAT PERIOD SHALL BE DISCARDED.
14. GROUT SHALL NOT BE RE-TEMPERED, BUT SHALL BE DISCARDED IMMEDIATELY IF PLASTICITY IS LOST BEFORE GROUT IS PLACED IN WALL. GROUT SHALL BE USED WITHIN 1-1/2 HOURS OF INITIAL MIXING.
15. COLD OR HOT WEATHER MASONRY CONSTRUCTION SHALL CONFORM TO THE ACI 530-99/ASCE 5-99/TMS 402-99 SECTION 1.8 AND ACI 305 AND 306, RESPECTIVELY.
16. METAL LATH SHALL BE USED UNDER BOND BEAMS TO CONFINE GROUT FROM HOLLOW CORES.
17. PROCEDURES OF NCM-TEK #3-3A SHALL BE FOLLOWED FOR ALL REINFORCED MASONRY CONCRETE CONSTRUCTION.
18. LAY ALL CONCRETE MASONRY UNITS IN RUNNING BOND, UNLESS NOTED OTHERWISE.
19. INSPECTION OF MASONRY CONSTRUCTION SHALL BE PERFORMED AS REQUIRED BY IBC 2003 CHAPTER 17. SEE SHEET SN2.

20. ALL STRUCTURAL MEMBERS SHALL BE DESIGNED IN ACCORDANCE WITH AMERICAN IRON AND STEEL INSTITUTE (AISI) "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS", 1996 EDITION.

21. ALL STUDS, JOISTS AND ACCESSORIES SHALL BE MANUFACTURED PER ASTM C955. ALL STUDS, JOISTS AND ACCESSORIES SHALL BE GALVANIZED TO HAVE A MINIMUM G-60 COATING IN CONFORMANCE WITH ASTM C665. STUDS, JOISTS AND ACCESSORIES, 16 GAGE OR HEAVIER SHALL BE FORMED FROM SHEET STEEL CONFORMING TO ASTM A653, Fy=50 KSI. THOSE 18 GAGE OR LIGHTER SHALL BE FORMED OF SHEET STEEL CONFORMING TO ASTM A653, Fy=33 KSI.

22. REFER TO MARINOWARE TECHNICAL PUBLICATION "STUD-RITE LIGHTWEIGHT STEEL FRAMING SYSTEM" FOR TECHNICAL INFORMATION, RECOMMENDATIONS, DETAILS, SUGGESTED SPECIFICATIONS, ERECTION AND BRACING.

23. ALL LIGHT GAGE STUD AND TRACK COMPONENTS SHALL BE CLEARLY IDENTIFIED WITH STANDARD INDUSTRY MARKINGS OR COLOR CODING.

24. ALL CURTAIN WALL STUDS SHALL BE FASTENED TO THE BOTTOM TRACK WITH A NO. 6 SCREW TO EACH FLANGE. AT TOP, USE 2 INCH DEFLECTION TRACK WITH A 3/4" GAP BETWEEN TRACK AND TOP OF STUD. FASTEN STUD TO TRACK WITH A NO. 6 SCREW ON ONE SIDE ONLY. INSTALL ONE ROW OF HORIZONTAL BRIDGING WITHIN TWO FEET OF THE TOP OF THE WALL.

25. CURTAIN WALL STUDS SHALL BE 6" WIDE AND HAVE A 1-5/8" FLANGE WITH A RETURN. CURTAIN WALL STUDS SHALL BE MIN. 18 GAGE AND SPACED TO A MAXIMUM OF 24 INCHES ON CENTER. BOTTOM TRACK SHALL BE MIN. 18 GAGE AND DEFLECTION TRACK SHALL BE MIN. 16 GAGE.

26. STAMPED LIGHT GAGE SHOP DRAWINGS AND CALCULATIONS SHALL BE PROVIDED BY LIGHT GAGE CONTRACTOR. CONTRACTOR'S ENGINEER SHALL BE LICENSED IN THE STATE OF MAINE AND CARRY PROFESSIONAL LIABILITY INSURANCE WITH A MIN. PER INCIDENT AND ANNUAL COVERAGE OF \$500,000. SHOP DRAWINGS SHALL DETAIL WINDOW AND DOOR HEADERS, SILL AND POSTS, AND CONNECTIONS. SHOP DRAWINGS SHALL INCLUDE A KEY PLAN, FRAMING ELEVATIONS, AND SECTIONS.

27. DEFLECTION CRITERIA SHALL BE L/600 BEHIND BRICK VENEER AREAS AND L/240 ELSEWHERE.

28. ALL STUD WALLS SHALL HAVE HORIZONTAL BRIDGING AND CROSS BRACING INSTALLED AS SHOWN ON DRAWINGS. ALL BRACING SHALL BE INSTALLED BEFORE APPLICATION OF LOADS.

29. BEARING WALL STUDS SHALL BE INSTALLED SEATED SQUARELY AGAINST THE WEB (WITHIN 1/16") OF THE TOP AND BOTTOM TRACK TO ASSURE TRANSFER OF AXIAL LOAD. STUDS SHALL BE PLUMBED, ALIGNED, AND SECURED TO THE CONTINUOUS RUNNER TRACKS AT EACH END AND EACH SIDE BEFORE THE INSTALLATION OF COMPONENTS WHICH INDUCE AXIAL LOAD.

30. CONTRACTOR'S LIGHT GAGE ENGINEER SHALL BE REQUIRED TO PERFORM SITE VISITS AS NECESSARY TO INSURE THAT LIGHT GAGE WORK CONFORMS WITH THEIR DESIGN.

WOOD FRAMING

1. ALL ROUGH FRAMING SHALL BE SPRUCE-PINE-FIR, NO. 2 OR BETTER, UNLESS OTHERWISE NOTED OR SHOWN ON THE DRAWINGS.
2. ALL TWO (2) INCH NOMINAL LUMBER TO BE SEASONED TO 19% MAXIMUM MOISTURE CONTENT.
3. ALL LUMBER AND PLYWOOD SHALL BE GRADE-STAMPED BY THE APPROPRIATE MANUFACTURER'S ASSOCIATION FOR THE APPROPRIATE USE.
4. ALL WOOD IN CONTACT WITH CONCRETE, MASONRY, OR EARTH SHALL BE PRESSURE TREATED SOUTHERN PINE.
5. ALL WOOD FRAMING SHALL BE BUILT PLUMB, LEVEL, SQUARE, AND TRUE WITH ADEQUATE BRACING AND CONNECTION HARDWARE TO ENSURE A RIGID STRUCTURE.
6. ROUGH CONNECTIONS SHALL BE ACCURATELY CUT AND TIGHTLY FITTED AS NECESSITATED BY THE CONDITIONS ENCOUNTERED TO PROVIDE FULL BEARING WITHOUT USE OF SHIMS.
7. ALL WOOD FRAMED FLOORS SHALL BE SHEATHED WITH 3/4" TONGUE AND GROOVE ADVANTECH APA RATED EXPOSURE I PLYWOOD, GLUED AND NAILED, UNLESS OTHERWISE SHOWN OR NOTED.
8. ALL SHEATHING SHALL BE LAID WITH LONG DIMENSIONS PERPENDICULAR TO SUPPORTS. STAGGER ALL JOINTS. PROVIDE BLOCKING AS REQUIRED TO PROVIDE FASTENING AT THE FULL PERIMETER OF ALL PANELS OF WALL SHEATHING. ADJACENT SHEATHING PANELS SHALL BE NAILED TO A COMMON STUD, BLOCKING, OR PLATE.
9. ALL SHEATHING SHALL BE FASTENED AT ALL PANEL EDGES AND IN FIELD OF PANEL AS NOTED ON SPECIFIC SHEAR WALLS AND DIAPHRAGMS. NOTE THAT ALL PANEL EDGES SHALL BE BLOCKED. SEE SHEAR WALL SCHEDULE.
10. ALL INTERIOR DOOR HEADERS SHALL CONSIST OF TWO 2X8'S WITH ONE LAYER OF 1/2" PLYWOOD SPACER, UNLESS OTHERWISE NOTED OR SHOWN ON THE DRAWINGS. ALL EXTERIOR WINDOW AND DOOR HEADERS THREE (3) FEET WIDE TO SIX (6) FEET WIDE SHALL BE THREE 2X10'S WITH TWO LAYERS OF 1/2" PLYWOOD, U.N.O.
11. ALL HEADERS OVER 5'-9" IN LENGTH SHALL BEAR ON DOUBLE STUD POSTS AS A MINIMUM, UNLESS OTHERWISE NOTED ON THE DRAWINGS.
12. SIMPSON CONSTRUCTION HARDWARE (OR APPROVED EQUAL) SHALL BE FASTENED ACCORDING TO THE MANUFACTURER'S SPECIFICATIONS AND NAILING SCHEDULE. THE GENERAL CONTRACTOR MUST BE FAMILIAR WITH, AND HAVE THE APPROPRIATE PRODUCT CATALOGS ON SITE.
A. ALL SPECIFIED FASTENERS MUST BE INSTALLED ACCORDING TO THE INSTRUCTIONS IN THE SIMPSON CATALOG. INCORRECT FASTENER QUANTITY, SIZE, TYPE, MATERIAL, OR FINISH MAY CAUSE THE CONNECTION TO FAIL.
B. 16D FASTENERS ARE COMMON NAILS (8 GAGE X 3-1/2") AND CANNOT BE REPLACED WITH 16D SINKERS (9 GAGE X 3-1/4") UNLESS OTHERWISE SPECIFIED.
C. BOLT HOLES SHALL BE A MINIMUM OF 1/32" AND A MAXIMUM OF 1/16" LARGER THAN THE BOLT DIAMETER (PER THE 1997 NDS, SECTION 8.1.2.1.).
D. INSTALL ALL SPECIFIED FASTENERS BEFORE LOADING THE CONNECTION.
E. PNEUMATIC NAILERS MAY BE USED TO INSTALL CONNECTORS, PROVIDED THE CORRECT QUANTITY AND TYPE OF NAILS ARE PROPERLY INSTALLED IN THE NAIL HOLES. TOOLS WITH NAIL HOLE-LOCATING MECHANISMS SHOULD BE USED. FOLLOW THE MANUFACTURER'S INSTRUCTIONS AND USE THE APPROPRIATE SAFETY EQUIPMENT.
F. JOIST SHALL BEAR COMPLETELY ON THE CONNECTOR SEAT AND THE GAP BETWEEN THE JOIST AND THE HEADER SHALL NOT EXCEED 1/8".

13. BEAMS NOTED AS "LVL" SHALL BE "MICROLLAM" AS MANUFACTURED BY TRUSJOIST (E=1,900,000 PSI, Fb=2600 PSI). MICROLLAM PRODUCTS SHALL BE PROPERLY STORED AND PROTECTED FROM WATER DAMAGE DURING CONSTRUCTION.

14. BEAMS NOTED AS "PSL" SHALL BE "PARALLAM" AS MANUFACTURED BY TRUSJOIST (E=2,000,000 PSI, Fb=2900 PSI). PARALLAM PRODUCTS SHALL BE PROPERLY STORED AND PROTECTED FROM WATER DAMAGE DURING CONSTRUCTION.

15. COLUMNS NOTED AS "PSL" SHALL BE "PARALLAM" AS MANUFACTURED BY TRUSJOIST (E=1,900,000 PSI, Fb=2600 PSI). PARALLAM PRODUCTS SHALL BE PROPERLY STORED AND PROTECTED FROM WATER DAMAGE DURING CONSTRUCTION.

16. UNLESS NOTED OTHERWISE, MINIMUM FASTENING OF WOOD MEMBERS SHALL CONFORM TO TABLE 2304.9.1 OF IBC 2003.

17. ALL PLYWOOD AND OSB SHEATHING SHALL BE APA RATED AND SHALL BE ADEQUATELY SPACED AT JOINTS (1/8" TYP) AS RECOMMENDED BY THE APA FOR EXPANSION.

18. ALL POSTS SHALL CONTINUE TO THE FOUNDATION. UNLESS OTHERWISE INDICATED, INSTALL SOLID BLOCKING WITHIN FLOOR PLENUM TO PROVIDE CONTINUITY OF LOAD PATH.

19. PRESSURE TREATED LUMBER SHALL BE WOLMANIZED L3 OUTDOOR WOOD TREATED WITH A NON-METALLIC CARBON BASED SOLUTION SUITABLE TO EXTERIOR EXPOSED SERVICE.

20. USE PT LUMBER FOR ALL EXTERIOR FRAMING AND FOR SILL PLATES ON FOUNDATION WALLS AND INTERIOR SLABS ON GRADE.

21. USE STANDARD ANCHOR BOLTS TO FASTEN PT PLATES TO FOUNDATION WALLS. USE POWER ACTUATED FASTENERS IN ALL PLATE TO SLAB CONNECTIONS.

22. USE G90 OR G185 GALVANIZED CONNECTORS (SIMPSON OR EQUAL) AND HOT DIPPED GALVANIZED NAILS FOR ALL PT CONNECTIONS.

23. WOLMANIZED PT PRODUCTS MAY NOT BE SUBSTITUTED WITH OTHER TYPES OF PT PRODUCTS.

24. WPA USE CATEGORY FOR SERVICE CONDITIONS IS UC3B. MINIMUM TREATMENT RETENTION SHALL BE 0.013 PCF.

PRESSURE TREATED LUMBER

1. PRESSURE TREATED LUMBER SHALL BE WOLMANIZED L3 OUTDOOR WOOD TREATED WITH A NON-METALLIC CARBON BASED SOLUTION SUITABLE TO EXTERIOR EXPOSED SERVICE.
2. USE PT LUMBER FOR ALL EXTERIOR FRAMING AND FOR SILL PLATES ON FOUNDATION WALLS AND INTERIOR SLABS ON GRADE.
3. USE STANDARD ANCHOR BOLTS TO FASTEN PT PLATES TO FOUNDATION WALLS. USE POWER ACTUATED FASTENERS IN ALL PLATE TO SLAB CONNECTIONS.
4. USE G90 OR G185 GALVANIZED CONNECTORS (SIMPSON OR EQUAL) AND HOT DIPPED GALVANIZED NAILS FOR ALL PT CONNECTIONS.
5. WOLMANIZED PT PRODUCTS MAY NOT BE SUBSTITUTED WITH OTHER TYPES OF PT PRODUCTS.
6. WPA USE CATEGORY FOR SERVICE CONDITIONS IS UC3B. MINIMUM TREATMENT RETENTION SHALL BE 0.013 PCF.



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THE BAY HOUSE

Madle Street

Portland, Maine

NOTE SHEET

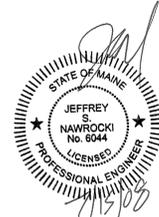
Phase 1

Scale: N/A

Commission No: 06-008

Date: July 15, 2008

REVISIONS:



DRG. NO.

SN.O

WOOD TRUSSES (SEE SN.0 FOR ADDITIONAL NOTES)

- ALL WOOD FLOOR AND/OR ROOF TRUSSES SHALL BE DESIGNED AND MANUFACTURED BY WOOD STRUCTURES, INC. OF BIDDFORD, MAINE, OR APPROVED EQUAL. ALL ASSOCIATED CONNECTION HARDWARE FOR TRUSSES SHALL ALSO BE DESIGNED AND SUPPLIED BY TRUSS MANUFACTURER. TRUSS DESIGNS ARE TO BE STAMPED BY A REGISTERED PROFESSIONAL ENGINEER RETAINED BY MANUFACTURER. CALCULATIONS AND SHOP DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW AND APPROVAL PRIOR TO START OF PRODUCTION.
- ALL WOOD TRUSSES SHALL BE BRACED IN ACCORDANCE WITH THE MANUFACTURER'S REQUIREMENTS AND IN ACCORDANCE WITH TRUSS PLATE INSTITUTE (TPI) PUBLICATION BCSI 1-03 FOR BRACING WOOD TRUSSES. PROPER BRACING OF WOOD TRUSSES DURING ERECTION IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR.
- MINIMUM GRADE FOR ANY TRUSS MEMBER SHALL BE #2.
- WOOD TRUSS MANUFACTURER SHALL CLEARLY LABEL ALL COMPRESSION WEBS WHICH REQUIRE LATERAL BRACING WITH RED TAGS OR PAINT. FAILURE TO LABEL WILL REQUIRE IN-PLACE LABELING BY TRUSS MANUFACTURER.
- WOOD TRUSS MANUFACTURER SHALL MAINTAIN A MAXIMUM PANEL POINT SPACING OF 10 FT. UNLESS WRITTEN APPROVAL FOR A MODIFICATION IS RECEIVED FROM ENGINEER.
- ROOF TRUSSES SHALL BE DESIGNED FOR:
 - TOP CHORD SNOW LOAD = 50 PSF BALANCED (UNBALANCED DESIGN PER ASCE 7-92)
 - TOP CHORD DEAD LOAD = 7 PSF
 - BOTTOM CHORD LIVE LOAD = 10 PSF
 - BOTTOM CHORD DEAD LOAD = 7 PSF
- WOOD TRUSS LAYOUT DRAWINGS SHALL BE PREPARED ON 24"x36" SHEETS AND CLEARLY LABEL ALL TRUSSES AND CONNECTORS. SHOP DRAWINGS MUST INCLUDE THE FOLLOWING INFORMATION: MEMBER SPECIES AND GRADE, MEMBER FORCES, MEMBER CSI RATIOS, ACTUAL DL AND LL DEFLECTIONS, REACTIONS, CONNECTOR PLATE REQUIREMENTS AND SIZES.
- OVERBUILD TRUSSES SHALL HAVE BOTTOM CHORDS BEVEL CUT TO MATCH THE SLOPE OF THE SUPPORTING ROOF.
- ROOF AND FLOOR TRUSSES SHALL BE DESIGNED FOR A MAXIMUM LIVE OR SNOW LOAD DEFLECTION CRITERIA OF L/480 AND TOTAL LOAD DEFLECTION CRITERIA OF L/360.
- THE "MINIMUM" MEMBER SIZE USED IN ANY ROOF TRUSS SHALL BE A 2X4. FLOOR TRUSSES SHALL ALSO USE A MINIMUM MEMBER SIZE OF 2X4 UNLESS APPROVED BY ENGINEER PRIOR TO BIDDING. THE "MINIMUM" PLATE" SIZE USED ON ANY ROOF TRUSS CONNECTION SHALL BE 3"x4", REGARDLESS OF DESIGN REQUIREMENT.
- TRUSS MANUFACTURER SHALL BE ACCEPTABLE TO THE ENGINEER. CONTRACTOR SHALL QUALIFY MANUFACTURERS WITH ENGINEER PRIOR TO BIDDING.
- PIGGY BACK TRUSSES REQUIRE SIGNIFICANT BRACING AT TOP CHORD OF THE LOWER TRUSS. CONTACT MANUFACTURER FOR REQUIREMENTS.

13. ERECTION BRACING OF WOOD TRUSSES IS SOLELY THE RESPONSIBILITY OF THE CONTRACTOR. THIS IS MEANS AND METHODS. FOLLOW TPI PUBLICATION BCSI 1-03 STRICTLY IN ADDITION TO WEB BRACING REQUIREMENTS SHOWN ON TRUSS SHOP DRAWINGS. CONSULT TRUSS MANUFACTURER OR INDEPENDENT ENGINEER IF FURTHER DESIGN ASSISTANCE IS NEEDED.

14. ALL LATERAL BRACING INSTALLED ON COMPRESSION WEBS OF TRUSSES, AS DIRECTED BY MANUFACTURER, MUST HAVE CROSS BRACING INSTALLED ON SAME WEB PLANE AT INTERVALS SHOWN ON THESE DRAWINGS AND BCSI 1-03. NOTE THAT ALTHOUGH BCSI 1-03 SUGGESTS THIS CROSS BRACING IS TEMPORARY, IT IS ALSO REQUIRED PERMANENT BRACING TO PREVENT WEBS FROM BUCKLING AS A GROUP. WHERE DIAGONAL BRACES CROSS, INTERRUPT ONE BRACE AND ADD A 4-FT. LONG 2X4 SPLICE OVER THE INTERRUPTED BRACE AND USE (8) 10d NAILS ON EACH SIDE. AS AN ALTERNATE, VEE OR CHEVRON BRACING CAN BE INSTALLED USING THE SAME QUANTITY OF DIAGONAL MEMBERS, WHICH ELIMINATES THE NEED FOR THE SPLICE. IN THE EVENT THAT TEE-BRACING OR STRONGBACKS ARE USED INSTEAD OF CONTINUOUS LATERAL BRACING, THE CROSS BRACING IS THEN ONLY A TEMPORARY ERECTION STABILITY REQUIREMENT. CONTINUOUS LATERAL BRACING MUST, HOWEVER, BE LOCATED ON TOP AND BOTTOM CHORDS ABOVE AND BELOW CROSS BRACING FOR TEMPORARY CROSS BRACING TO BE EFFECTIVE.

15. TRUSS MANUFACTURER SHALL ATTEMPT TO ALIGN ADJACENT TRUSS WEBS TO ALLOW INSTALLATION OF LATERAL BRACING. A MINIMUM OF 6 ADJACENT WEBS MUST ALIGN, OTHERWISE MANUFACTURER SHALL SPECIFY TEE-BRACING INSTEAD.

SCHEDULE OF SPECIAL INSPECTIONS

PROJECT: THE VILLAGE AT OCEAN GATE
 LOCATION: NENBURY STREET, PORTLAND, MAINE
 OWNER: THE VILLAGE AT OCEAN GATE, LLC, GFI PARTNERS, LLC
 OWNERS ADDRESS: BOSTON, MASSACHUSETTS
 ARCHITECT OF RECORD (AOR): DAVID M. WHITE, A.I.A.
 STRUCTURAL ENGINEER OF RECORD (SER): JEFFREY S. NAWROCKI, PE

THIS STATEMENT OF SPECIAL INSPECTIONS IS SUBMITTED AS A CONDITION FOR PERMIT ISSUANCE IN ACCORDANCE WITH THE SPECIAL INSPECTION REQUIREMENTS OF THE 2000 INTERNATIONAL BUILDING CODE. IT INCLUDES A SCHEDULE OF SPECIAL INSPECTION SERVICES APPLICABLE TO THIS PROJECT AS WELL AS THE NAME OF SPECIAL INSPECTORS AND THE IDENTITY OF OTHER APPROVED AGENCIES INTENDED TO BE RETAINED FOR CONDUCTING THESE SERVICES.

THE SPECIAL INSPECTOR SHALL KEEP RECORDS OF ALL INSPECTIONS AND SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, STRUCTURAL ENGINEER AND ARCHITECT OF RECORD. DISCOVERED DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR.

A FINAL REPORT OF SPECIAL INSPECTIONS BY THE SPECIAL INSPECTOR(S) DOCUMENTING COMPLETION OF ALL REQUIRED SPECIAL INSPECTIONS AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED PRIOR TO ISSUANCE OF A CERTIFICATE OF USE AND OCCUPANCY.

THE SPECIAL INSPECTOR, WHO IS GENERALLY EMPLOYED BY THE PRIMARY TESTING AGENCY, MAY USE VARIOUS INSPECTORS WHO ARE FAMILIAR WITH EACH CATEGORY OF WORK. IF SPECIAL INSPECTIONS ARE ALSO PERFORMED BY AGENTS WHO ARE NOT EMPLOYED BY PRIMARY TESTING AGENCY, EACH OF THESE ADDITIONAL SPECIAL INSPECTORS SHALL ISSUE A FINAL REPORT FOR THEIR CATEGORY OF INSPECTION. ONLY AFTER THE FINAL REPORT(S) HAS (HAVE) BEEN ISSUED BY THE SPECIAL INSPECTOR(S) CAN THE ARCHITECT AND SER ISSUE FINAL AFFIDAVITS FOR THE PROJECT COMPLETION.

JOB SITE SAFETY AND MEANS AND METHODS OF CONSTRUCTION ARE SOLELY THE RESPONSIBILITY OF THE CONTRACTOR.

PREPARED BY: *Jeffrey S. Nawrocki* 7/15/08
 JEFFREY S. NAWROCKI, P.E. DATE

STATE OF MAINE
 JEFFREY S. NAWROCKI
 No. 004
 LICENSED PROFESSIONAL ENGINEER

OWNER'S AUTHORIZATION: _____ BUILDING OFFICIAL'S AUTHORIZATION: _____
 SIGNATURE DATE SIGNATURE DATE

SCHEDULE OF SPECIAL INSPECTION SERVICES

THE FOLLOWING TABLES COMPRISE THE REQUIRED SCHEDULE OF SPECIAL INSPECTIONS FOR THIS PROJECT. THE CONSTRUCTION DIVISIONS WHICH REQUIRE SPECIAL INSPECTIONS FOR THIS PROJECT ARE AS FOLLOWS:

SOILS AND FOUNDATIONS
 CAST-IN-PLACE CONCRETE
 WOOD CONSTRUCTION
 STRUCTURAL STEEL
 REINFORCED CONCRETE MASONRY
 LIGHT GAGE STEEL FRAMING (PROPRIETARY SYSTEM, NON-BEARING WALLS ONLY)
 RAMMED AGGREGATE PIERS

INSPECTION AGENTS	FIRM	ADDRESS
1. SPECIAL INSPECTOR(S) *	TBD	TBD
2. TESTING LABORATORY	TBD	TBD
3. STRUCTURAL ENGINEER	JSN ASSOCIATES, INC.	ONE AUTUMN STREET PORTSMOUTH, NH 03801 (603) 453-8639
4. LIGHT GAGE ENGINEER	TBD	TBD

NOTE: THE INSPECTION AND TESTING AGENT SHALL BE ENGAGED BY THE OWNER OR THE OWNER'S AGENT, AND NOT BY THE CONTRACTOR OR SUBCONTRACTOR WHOSE WORK IS TO BE INSPECTED OR TESTED, ANY CONFLICT OF INTEREST MUST BE DISCLOSED TO THE BUILDING OFFICIAL, PRIOR TO COMMENCING WORK.

* THE SPECIAL INSPECTOR IS GENERALLY AN EMPLOYEE OF THE TESTING AND GEOTECHNICAL COMPANY, AND MAY BE MORE THAN ONE PERSON.

SEISMIC DESIGN CATEGORY: C
 BASIC WIND SPEED: 95 MPH
 WIND EXPOSURE CATEGORY: B

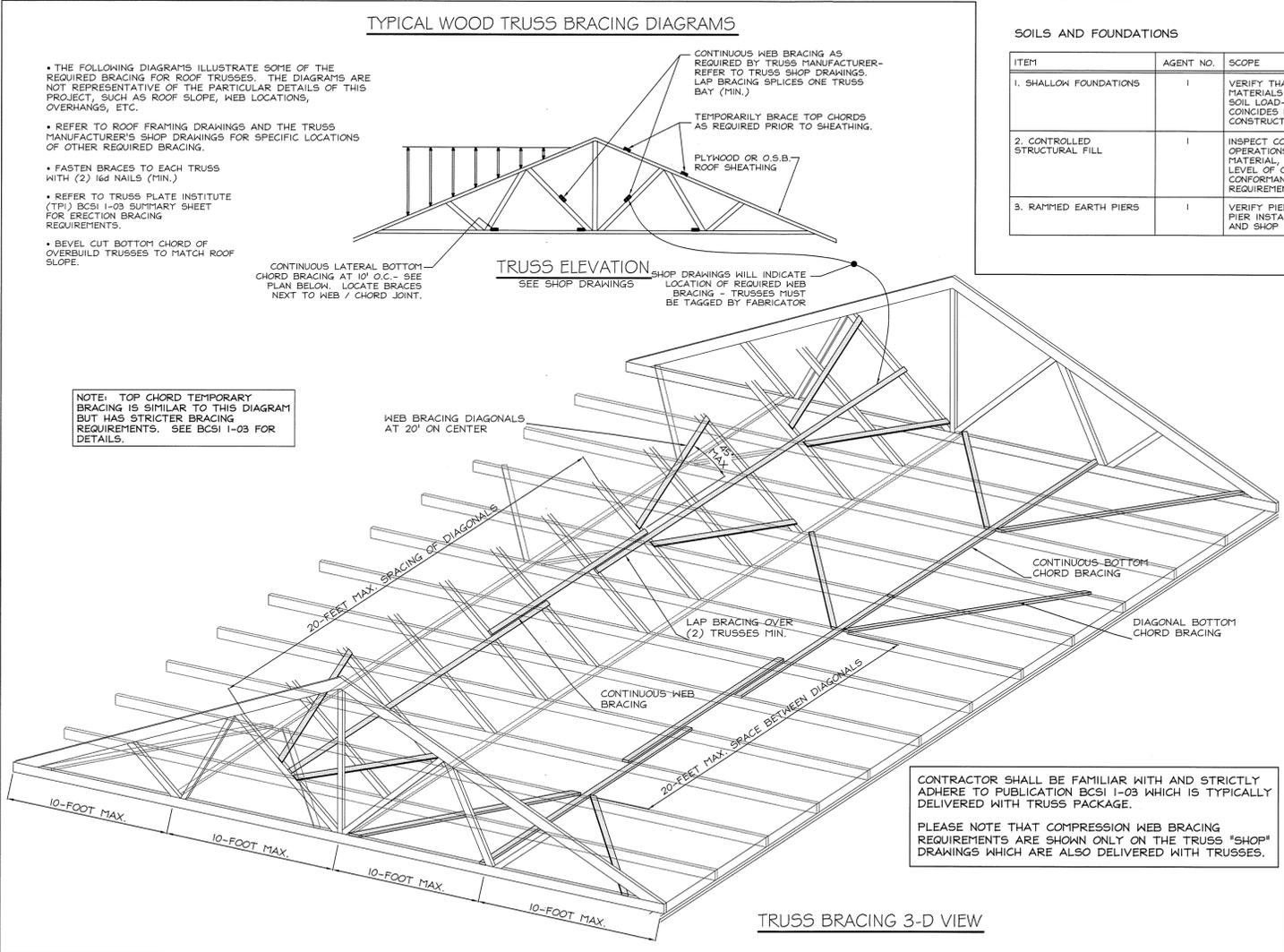
QUALIFICATIONS OF INSPECTORS AND TESTING TECHNICIANS

THE QUALIFICATIONS OF ALL PERSONNEL PERFORMING SPECIAL INSPECTION ACTIVITIES ARE SUBJECT TO THE APPROVAL OF THE BUILDING OFFICIAL. THE CREDENTIALS OF ALL INSPECTORS AND TESTING TECHNICIANS SHALL BE PROVIDED IF REQUESTED.

IT IS RECOMMENDED THAT THE PERSON ADMINISTERING THE SPECIAL INSPECTIONS PROGRAM BE A PROFESSIONAL ENGINEER EXPERIENCED IN THE DESIGN OF BUILDINGS.

SOILS AND FOUNDATIONS

ITEM	AGENT NO.	SCOPE
1. SHALLOW FOUNDATIONS	1	VERIFY THAT UNSUITABLE BEARING MATERIALS ARE REMOVED. VERIFY THE SOIL LOAD-BEARING CAPACITY COINCIDES WITH THAT IDENTIFIED IN THE CONSTRUCTION DOCUMENTS.
2. CONTROLLED STRUCTURAL FILL	1	INSPECT COMPACTED FILL OPERATIONS TO VERIFY THE FILL MATERIAL, LIFT HEIGHTS, AND LEVEL OF COMPACTION ARE IN CONFORMANCE WITH THE REQUIREMENTS OF CONSTRUCTION.
3. RAMMED EARTH PIERS	1	VERIFY PIERS ARE INSTALLED PER PIER INSTALLER SPECIFICATIONS AND SHOP DRAWINGS.



STRUCTURAL STEEL

ITEM	AGENT NO.	SCOPE
1. FABRICATOR CERTIFICATION/QUALITY CONTROL PROCEDURES	3	VERIFY THAT THE FABRICATOR MAINTAINS DETAILED FABRICATION AND QUALITY CONTROL PROCEDURES WHICH CONFORM TO THE REQUIREMENTS OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION'S QUALITY CERTIFICATION PROGRAM. AISC MEMBERSHIP SATISFIES THIS REQUIREMENT.
2. MATERIAL CERTIFICATION	1	REVIEW MILL CERTIFICATES FOR PLATES AND SHAPES. REVIEW BOLT MANUFACTURER'S CERTIFICATES OF COMPLIANCE FOR HIGH-STRENGTH BOLTS. REVIEW WELD MANUFACTURER'S CERTIFICATE OF COMPLIANCE FOR WELD FILLER MATERIAL.
3. BOLTING	1	INSPECT INSTALLATION OF HIGH-STRENGTH BOLTS FOR CONFORMANCE WITH THE SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OF A490 BOLTS* BY THE RESEARCH COUNCIL ON STRUCTURAL BOLTS, AND THE CONSTRUCTION DOCUMENTS.
4. WELDING	1	PERFORM VISUAL INSPECTION OF ALL WELDS IN ACCORDANCE WITH AWS D11. SUBMIT WELDER QUALIFICATION STATEMENTS. ADDITIONALLY, THE TESTING AGENCY (TO BE APPROVED BY JSN ASSOCIATES, INC.) MUST PERFORM A VISUAL INSPECTION OF ALL FIELD WELDS. FIELD FILLET WELDS > 5/16", MULTI-PASS WELDS, AND PARTIAL PENETRATION WELDS MUST BE SPOT TESTED AT A RATE OF ONE TEST PER MEMBER USING THE MAGNETIC PARTICLE METHOD. ONE HUNDRED PERCENT (100%) OF ALL FIELD AND SHOP FULL PENETRATION WELDS MUST BE TESTED USING THE ULTRASONIC METHOD.
5. SHEAR CONNECTORS	1	(1) VISUALLY INSPECT ALL SHEAR STUD CONNECTIONS. BEND TEST 5% OF ALL STUDS TO VERIFY CONNECTIONS USING AN APPROVED TEST METHOD. VERIFY THAT THE NUMBER AND DIAMETER OF STUDS PROVIDED CONFORMS TO THE CONSTRUCTION DOCUMENTS AND APPROVED SHOP DRAWINGS. (3) RANDOM REVIEW.
6. STRUCTURAL DETAILS	1, 3	(1) VERIFY THAT THE GENERAL GEOMETRY OF THE ERECTED STEEL FRAME CONFORMS TO THE CONSTRUCTION DOCUMENTS AND APPROVED SHOP DRAWINGS. (3) RANDOM REVIEW.

RAMMED AGGREGATE PIERS

ITEM	AGENT NO.	SCOPE
1. DESIGN REVIEW	1	REVIEW DESIGN SUBMITTAL, INCLUDING ENGINEERING CALCULATIONS, SHOP DRAWINGS, SCHEDULES AND PROPOSED QUALITY CONTROL PROCEDURES.
2. ON-SITE COORDINATION MEETING	1	ATTEND AN ON-SITE PROJECT COORDINATION MEETING WITH THE GENERAL CONTRACTOR AND THE RAMMED AGGREGATE PIER INSTALLER IMMEDIATELY PRIOR TO THE START OF WORK.
3. TEST PIER CONSTRUCTION	1	OBSERVE AND DOCUMENT INITIAL DEMONSTRATION PIER CONSTRUCTION. FLOW TESTING, AND CONSTRUCTION OF THE MODULUS LOAD TEST PIER.
4. TEST PIER PERFORMANCE	1	OBSERVE AND DOCUMENT THE PERFORMANCE OF THE MODULUS LOAD TEST TO BE INSTRUMENTED AND PERFORMED BY THE RAMMED AGGREGATE PIER INSTALLER.
5. PIER CONSTRUCTION	1	OBSERVE AND DOCUMENT PRODUCTION PIER CONSTRUCTION TO CONFIRM CONSTRUCTION OF THE PIERS TO THE REQUIRED DEPTHS AND TERMINATION CRITERIA IN ACCORDANCE WITH SPECIFIED CONSTRUCTION PROCEDURES AND CONSISTENT WITH THE PROCEDURES USED TO CONSTRUCT THE MODULUS TEST PIER.
6. POTENTIAL IMPACTS TO PIERS	1	OBSERVE AND DOCUMENT OTHER CONSTRUCTION ACTIVITIES AND DOCUMENT THE EXTENT AND POTENTIAL IMPACTS OF EXCAVATIONS THAT MAY BE COMPLETED ADJACENT TO COMPLETED IMPACT PIERS SO THAT APPROPRIATE REMEDIAL MEASURES MAY BE TAKEN TO RESTORE POTENTIALLY IMPACTED PIERS.
7. SUBGRADE SOILS	1	OBSERVE AND DOCUMENT FOUNDATION SUBGRADE EXCAVATION PROCEDURES, VERIFY THE PRESENCE AND LOCATION OF SUPPORTING PIERS, AND THE SATISFACTORY PREPARATION AND PROTECTION OF THE SUBGRADE SOILS AS REQUIRED BY THE PROJECT SPECIFICATIONS.

LIGHT GAGE STEEL FRAMING (PROPRIETARY SYSTEM)

ITEM	AGENT NO.	SCOPE
1. FABRICATOR CERTIFICATION	3	VERIFY THAT THE FABRICATOR MAINTAINS DETAILED FABRICATION AND QUALITY CONTROL PROCEDURES WHICH CONFORM TO THE REQUIREMENTS OF THE AMERICAN IRON AND STEEL INSTITUTE, COLD FORMED STEEL DESIGN MANUAL.
2. MATERIAL CERTIFICATION	1	(1) REVIEW MATERIAL GRADE USED FOR COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS. VERIFY THAT MEMBER SIZES INCLUDING DEPTH, FLANGE WIDTHS, AND MATERIAL GAGE COPY WITH CONSTRUCTION DOCUMENTS.
3. CONNECTIONS	1	VERIFY THAT CONNECTIONS COMPLY WITH CONSTRUCTION DOCUMENTS AND SHOP DRAWINGS. VERIFY THAT SIZE AND QUANTITY OF LIGHT GAGE FASTENERS COMPLY WITH CONSTRUCTION DOCUMENTS.
4. FRAMING AND DETAILS	1, 3, 4	(1) VERIFY THAT FRAMING CONFIGURATION IS AS SPECIFIED ON THE CONSTRUCTION DOCUMENTS, (3),(4) RANDOM OBSERVATION.

CAST-IN-PLACE CONCRETE

ITEM	AGENT NO.	SCOPE
1. MIX DESIGN	3	REVIEW FOR COMPLIANCE WITH CONSTRUCTION DOCUMENTS.
2. MATERIAL CERTIFICATION	3	REVIEW FOR COMPLIANCE WITH CONSTRUCTION DOCUMENTS.
3. REINFORCEMENT INSTALLATION	1, 3	(1) REVIEW THE INSTALLATION OF THE REINFORCING STEEL FOR COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS AND THE APPROVED SHOP DRAWINGS. REVIEW FOR 25% OF FOOTINGS, 50% OF FROST WALLS. (3) RANDOM REVIEW OF CONSTRUCTION PROCEDURE.
4. FORMWORK GEOMETRY	1	REVIEW GEOMETRY FOR COMPLIANCE WITH THE STRUCTURAL CONSTRUCTION DOCUMENTS. CONDUCT REVIEW WHEN REINFORCING STEEL INSTALLATION IS BEING REVIEWED.
5. CONCRETE PLACEMENT	1	INSPECT THE PLACEMENT OF CONCRETE FOR CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS. TEST SLUMP AND TEMPERATURE OF EACH BATCH. TEST AIR CONTENT WHEN COMPRESSIVE STRENGTH TEST SPECIMENS ARE MOULDED.
6. EVALUATION OF CONCRETE STRENGTH	1	OBTAIN ONE SET OF (4) STANDARD CYLINDERS FOR EACH COMPRESSIVE STRENGTH TEST. TEST ONE SPECIMEN AT 7-DAYS, (2) AT 28-DAYS, AND RETAIN ONE IN RESERVE FOR LATER TESTING IF REQUIRED.
7. CURING AND PLACEMENT	1	IN COLD WEATHER, TEST CYLINDERS SHALL BE FIELD CURED. ADDITIONAL CYLINDERS SHALL BE TAKEN AND LABORATORY CURED PER ACI REQUIREMENTS.

REINFORCED CONCRETE MASONRY

ITEM	AGENT NO.	SCOPE
1. MATERIAL CERTIFICATION	3	REVIEW CERTIFICATES OF COMPLIANCE FOR MASONRY UNITS, MORTAR MIX DESIGNS AND STRENGTH TESTS, GROUT DESIGNS AND STRENGTH TESTS, AND MANUFACTURER'S CATALOG DATA FOR JOINT REINFORCING AND METAL ACCESSORIES.
2. MIXING OF MORTAR AND GROUT	1	INSPECT THE PROPORTIONING AND MIXING OF MORTAR AND GROUT FOR CONFORMANCE WITH ACI 530.1-99, SECTION 2.1 AND 2.4, AND THE CONSTRUCTION DOCUMENTS.
3. INSTALLATION OF MASONRY	1, 3	(1) INSPECT THE PLACEMENT OF MORTAR AND MASONRY UNITS FOR CONFORMANCE WITH ACI 530.1-99, SECTION 3.3, AND THE CONSTRUCTION DOCUMENTS. (3) RANDOM OBSERVATION.
4. REINFORCEMENT INSTALLATION	1, 3	(1) INSPECT THE SIZE, CONDITION, LOCATION, AND PLACEMENT OF REINFORCEMENT FOR CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS AND ACI 530-99, SECTION 3.4. (3) RANDOM OBSERVATIONS.
5. GROUTING OPERATIONS	1	INSPECT THE PLACEMENT OF GROUT (INCLUDING GROUT VIBRATION) FOR CONFORMANCE WITH ACI 530.1-99, SECTION 3.5 AND THE CONSTRUCTION DOCUMENTS.
6. WEATHER PROTECTION	1	INSPECT MASONRY PLACEMENT AND PROTECTION FOR CONFORMANCE WITH ACI 530.1-99, SECTION 1.6 AND THE CONSTRUCTION DOCUMENTS.
7. EVALUATION OF MASONRY STRENGTH	3	DETERMINE STRENGTH BY THE UNIT STRENGTH METHOD IN CONFORMANCE WITH ACI 530.1-99, SECTION 1.4. REVIEW MANUFACTURER'S TEST DATA AND CERTIFICATES FOR MASONRY UNITS, GROUT, MORTAR, AND REINFORCING.
8. CONNECTIONS	1	VERIFY THAT CONNECTIONS OF THE MASONRY UNITS TO STRUCTURAL MEMBERS ARE PROVIDED WHERE INDICATED IN THE CONSTRUCTION DOCUMENTS.

WOOD CONSTRUCTION

ITEM	AGENT NO.	SCOPE
1. TRUSS FABRICATOR CERTIFICATION/QUALITY CONTROL PROCEDURES	3	VERIFY THAT THE FABRICATOR MAINTAINS DETAILED FABRICATION AND QUALITY CONTROL PROCEDURES WHICH CONFORM TO THE REQUIREMENTS OF THE TRUSS PLATE INSTITUTE (TPI) AND WOOD TRUSS COUNCIL OF AMERICA (WTCA).
2. MATERIAL GRADING	1, 3	(1) REVIEW SPECIES AND GRADES OF LUMBER USED TO ENSURE CONFORMANCE WITH CONSTRUCTION DOCUMENTS. REVIEW TRUSS MEMBERS TO ENSURE CONFORMANCE WITH TRUSS ENGINEERING AND SHOP DRAWINGS. (3) RANDOM OBSERVATION.
3. CONNECTIONS	1, 3	(1) VERIFY THAT ROOF TRUSS AND OTHER WOOD FRAME CONNECTIONS COMPLY WITH CONSTRUCTION DOCUMENTS AND SHOP DRAWINGS. (3) RANDOM OBSERVATION.
4. FRAMING DETAILS	1, 3	(1) VERIFY THAT FRAMING CONFIGURATION AND ALIGNMENT OF WALL FRAMING BELOW FLOOR AND ROOF FRAMING IS AS SPECIFIED ON THE CONSTRUCTION DOCUMENTS. VERIFY TEMPORARY AND PERMANENT TRUSS BRACING TO CONFORM WITH TPI PUBLICATION BCSI 1-03. (3) RANDOM OBSERVATION.
5. OTHER	1, 3	(1) VERIFY THAT FASTENING OF ALL LATERAL LOAD RESISTING ELEMENTS SUCH AS SHEAR WALLS AND DIAPHRAGMS ARE IN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS. (3) RANDOM OBSERVATION.

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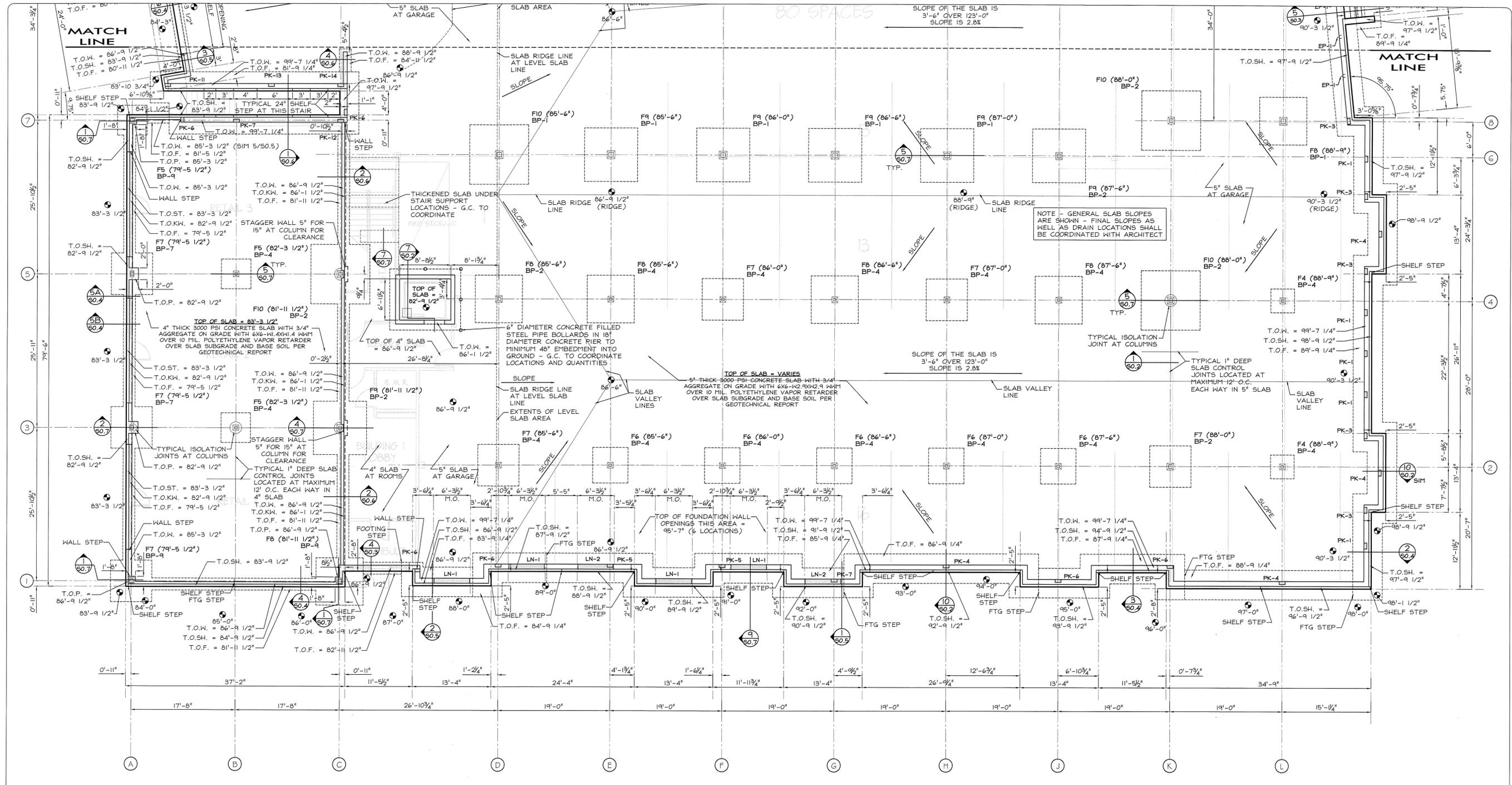
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THE BAY HOUSE
 Middle Street
 Portland, Maine

NOTE SHEET
 Phase 1
 Scale: N/A
 Commission No: 06-008
 Date: July 15, 2008

REVISIONS:

DRG. NO.
 SN.1



FOOTING SCHEDULE		
FTG	SIZE	REINFORCING
F3	3'x3'x12"	[3] #5 BARS
F4	4'x4'x14"	[4] #5 BARS
F5	5'x5'x16"	[5] #6 BARS
F6	6'x6'x18"	[6] #6 BARS
F7	7'x7'x20"	[7] #6 BARS
F8	8'x8'x22"	[8] #7 BARS
F9	9'x9'x24"	[9] #7 BARS
F10	10'x10'x26"	[10] #8 BARS

PROVIDE NUMBER OF BARS IN EACH DIRECTION, SPACED EVENLY, TIED IN MAT, AT 3" CLEAR FROM BOTTOM OF FOOTING (U.N.O.) - TOP STEEL SHALL BE 2" CLEAR FROM TOP OF FOOTING WHERE REQUIRED - FOOTING SIZES ARE BASED ON A 3000 PSF BEARING PRESSURE

- NOTES:**
- G.C. SHALL BE RESPONSIBLE FOR VERIFYING ALL DIMENSIONS. G.C. MAY CONTACT ENGINEER IF DIMENSIONAL CLARIFICATION IS NEEDED DUE TO SCALE OF DRAWINGS.
 - UNDER SLAB AND THROUGH WALL UTILITIES TO BE COORDINATED BY CONTRACTOR. SEE 5/50.2 FOR REINFORCING AT WALL OPENINGS.
 - BACKFILL ALL WALLS SIMULTANEOUSLY, BOTH SIDES, TO MAXIMUM HEIGHT POSSIBLE.
 - REFER TO GEOTECHNICAL REPORT FOR ALL INFORMATION REGARDING EXCAVATION, BACKFILL, SUBGRADE PREPARATION, STRUCTURAL FILL, DRAINAGE, ETC. NOTE THAT A MINIMUM OF 12" OF 3/4" CRUSHED STONE UNDERLAIN BY NON-MOVEN GEOTEXTILE FABRIC IS REQUIRED PER GEOTECHNICAL REPORT.
 - BUILDING BEARS DIRECTLY ON FOUNDATIONS. PROVIDE A SMOOTH AND LEVEL SURFACE AT ALL BEARING LOCATIONS.
 - MAINTAIN MINIMUM 4'-6" FROST COVER FROM GRADE TO BOTTOM OF FOOTING AT ALL EXTERIOR FOOTING LOCATIONS. SEE GEOTECHNICAL REPORT.
 - G.C. SHALL COORDINATE ALL FINAL SLAB SLOPE AND INTERIOR FLOOR DRAIN REQUIREMENTS WITH ARCHITECT.
 - GRADE ELEVATIONS ARE BASED ON ARCHITECTURAL ELEVATION OF 100'-0" AT CIVIL DATUM OF 43.20±.

FOUNDATION PLAN - BUILDING 1
 Scale: 1/8" = 1'

LEGEND:

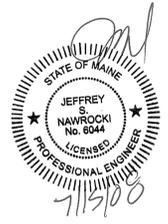
- T.O.S. = TOP OF SLAB ELEVATION
- T.O.W. = TOP OF WALL ELEVATION
- T.O.ST. = TOP OF WALL STEP
- T.O.SH. = TOP OF SHELF ELEVATION
- T.O.K.W. = TOP OF KEYWAY
- T.O.P. = TOP OF FOOTING ELEVATION
- BP-# = BASE PLATE DESIGNATION ("T.B.D.")
- LN-# = LINTEL DESIGNATION
- EP-# = EMBEDDED PLATE DESIGNATION
- F# (#) = FOOTING DESIGNATION AND TOP OF FOOTING ELEVATION
- ⊙ = TOP OF GRADE/SLAB ELEVATION

BEAM POCKET SCHEDULE					
POCKET	BEAM	DEPTH/WIDTH	T.O. POCKET	T.O. BEAM	WIDTH
PK-1	W8x10	7-7/8 / 4	98'-11 3/8"	99'-7 1/4"	12"
PK-2	W10x12	9-7/8 / 4	98'-9 3/8"	99'-7 1/4"	12"
PK-3	W12x14	11-7/8 / 4	98'-7 3/8"	99'-7 1/4"	12"
PK-4	W12x19	12-1/8 / 4	98'-7 1/8"	99'-7 1/4"	12"
PK-5	W14x22	13-3/4 / 5	98'-5 1/2"	99'-7 1/4"	12"
PK-6	W16x26	15-3/4 / 5.5	98'-3 1/2"	99'-7 1/4"	12"
PK-7	W16x31	15-7/8 / 5.5	98'-3 3/8"	99'-7 1/4"	12"
PK-8	W21x44	20-5/8 / 6.5	97'-10 5/8"	99'-7 1/4"	12"
PK-9	W24x55	23-5/8 / 7	97'-7 5/8"	99'-7 1/4"	12"
PK-10	W24x76	23-7/8 / 9	97'-7 3/8"	99'-7 1/4"	16"
PK-11	W27x84	26-3/4 / 10	97'-4 1/2"	99'-7 1/4"	16"
PK-12	W30x99	29-5/8 / 10-1/2	97'-1 5/8"	99'-7 1/4"	16"
PK-13	W21x44	20-5/8 / 6.5	97'-9 1/4"	99'-5 7/8"	12"
PK-14	W27x84	26-3/4 / 10	97'-3 1/8"	99'-5 7/8"	16"

ALL POCKETS SHALL HAVE A 1/2" THICK X 6" WIDE BEARING PLATE X POCKET WIDTH. SEE DETAIL 10/50.2.

NOTE: FOOTING DIMENSIONS MAY CHANGE TO ACCOMMODATE THE GEOMETRY REQUIREMENTS OF THE RAMMED AGGREGATE PIERS AT APPLICABLE LOCATIONS. THIS SHALL BE DETERMINED BY PIER DESIGNER AND LATER MODIFIED BY JSN IF REQUIRED.

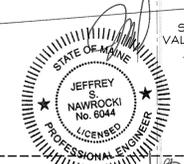
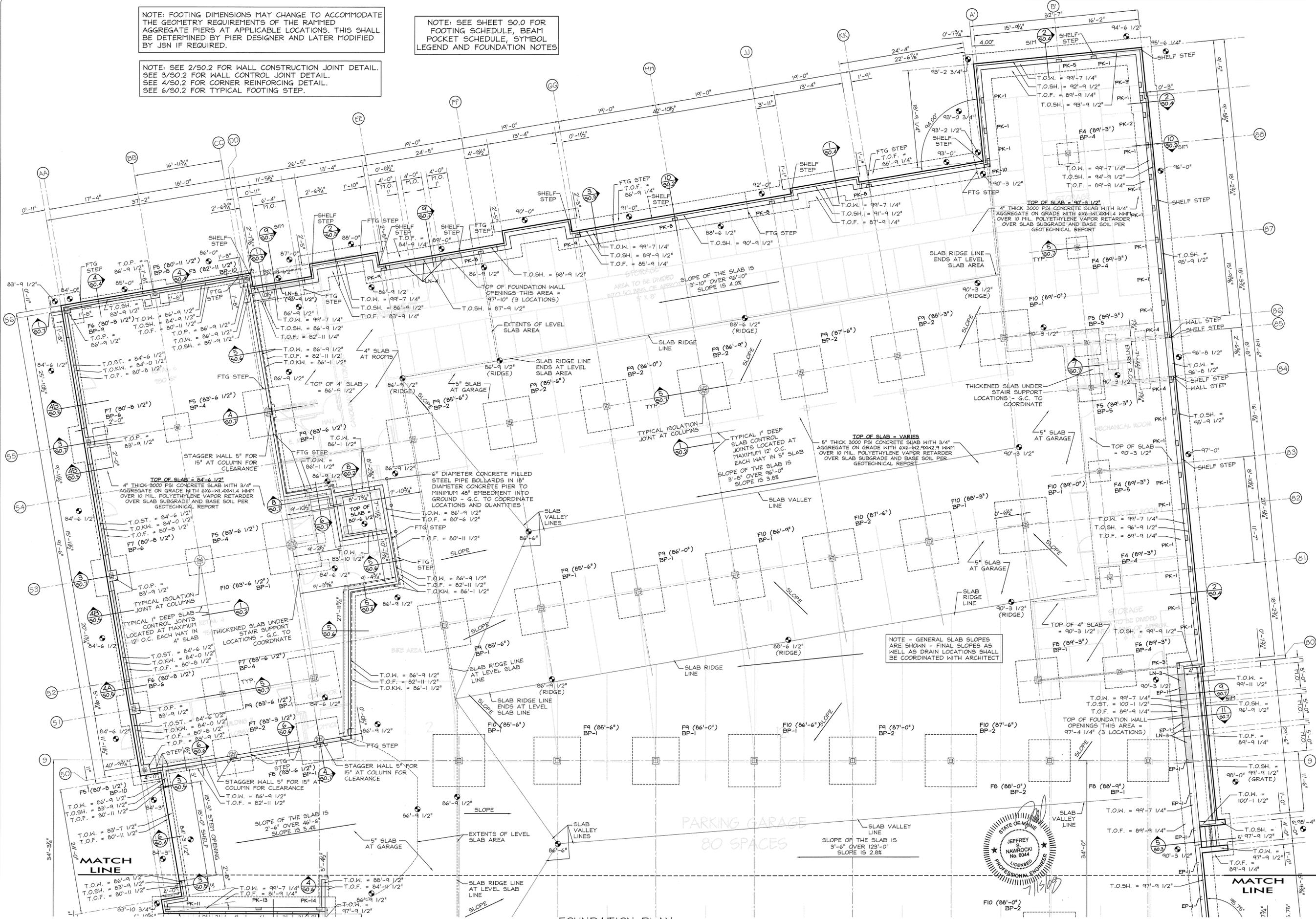
NOTE: SEE 2/50.2 FOR WALL CONSTRUCTION JOINT DETAIL. SEE 3/50.2 FOR WALL CONTROL JOINT DETAIL. SEE 4/50.2 FOR CORNER REINFORCING DETAIL. SEE 6/50.2 FOR TYPICAL FOOTING STEP.



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NOTE: SEE SHEET 50.0 FOR FOOTING SCHEDULE, BEAM POCKET SCHEDULE, SYMBOL LEGEND AND FOUNDATION NOTES



FOUNDATION PLAN - BUILDING 2
Scale: 1/8" = 1'

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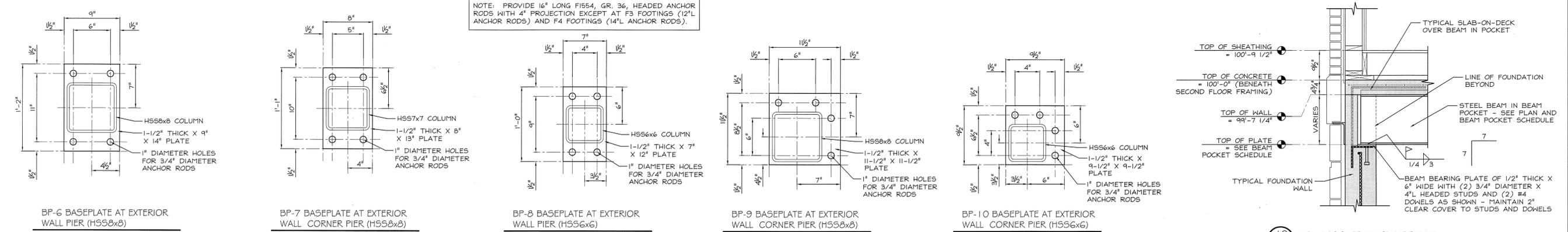
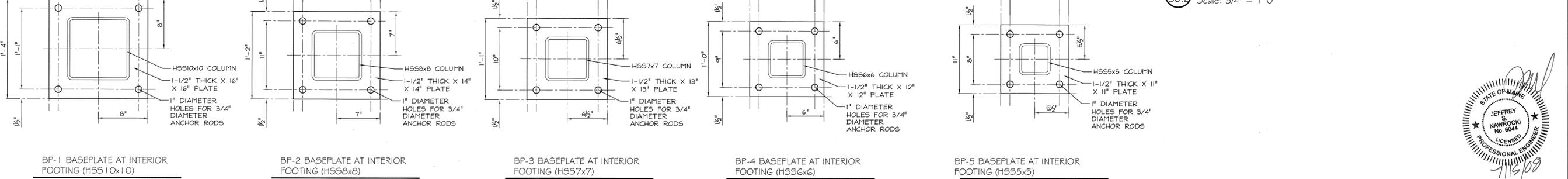
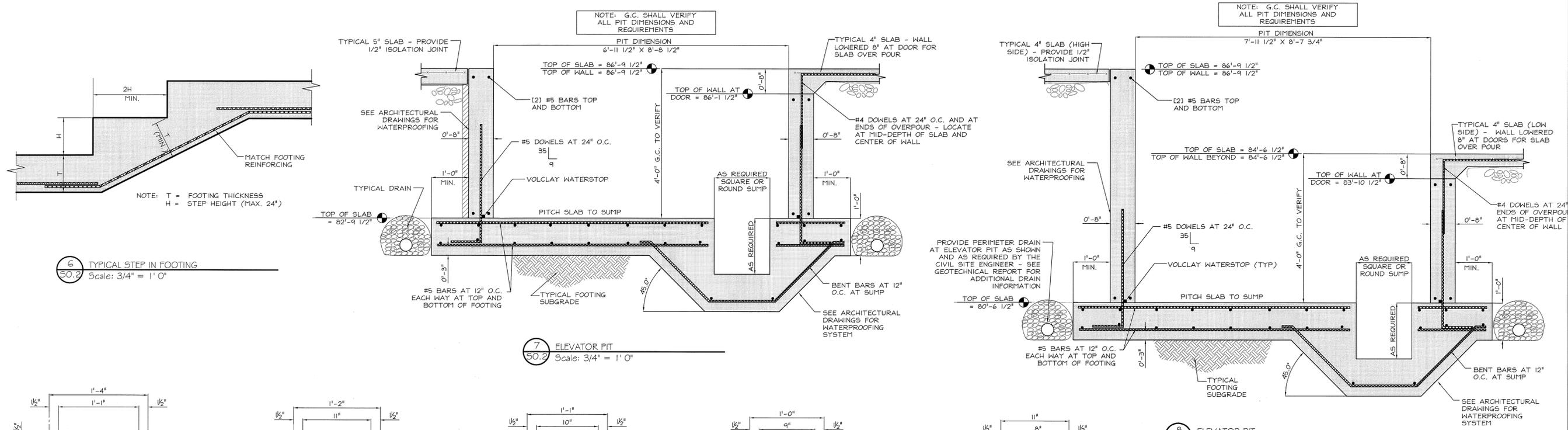
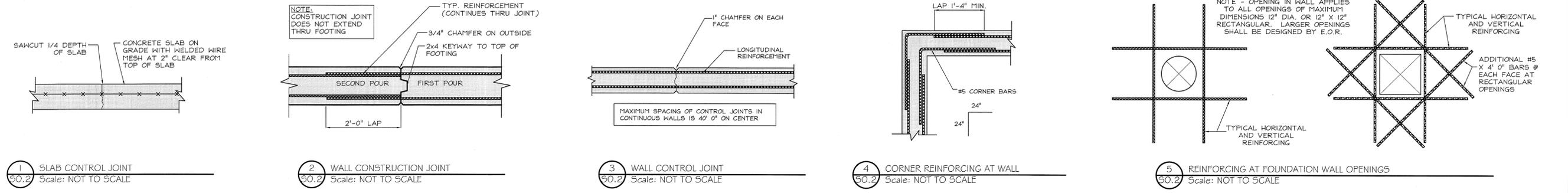
THE BAY HOUSE
Middle Street
Portland, Maine

FOUNDATION PLAN
Phase 1 / Building 2
Scale: 1/8" = 1'
Commission No: 06-008
Date: July 15, 2008

REVISIONS:

DRG. NO.

50.1



STATE OF MAINE
JEFFREY S. NAWROCKI
No. 6046
LICENSED PROFESSIONAL ENGINEER
7/15/08

