

RIGID FRAME: MAXIMUM REACTIONS (FACTORED), ANCHOR BOLTS, & BASE PLATES

				Column	React	ione (k)						
Frm Line	Col Line	Load Id	Hmax H	V	Load		V Vmin	Anc No	. Bolt D(in)	Base Wid	Plate Len	(in) Thk	Grout (in)
5	1	1	9.2	36.6	_3_5	-7.3 -1.2	-17.5 -23.5	6	0.750	6.000	17.25	0.500	156
5	E	 4	7.7 -9.2	-1.3 40.0	2 5	-10.7 -3.5	21.2 -21.4	4	0.750	8.000	13.25	0.500	72.0

NOTES FOR REACTIONS

- 1. All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- 2. Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- 3. Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- 4. Building reactions are based on the following building data:

Width (fţ)	= 47.2	
Length (ft)	= 40.6	
Eave Height (ft)	= 32.3/2	8.4
Roof Slope (rise/12)	= 1.0	
Dead Load (psf ')	= 4.0	
Collateral Load (psf)	= 9.0	
Live Load (psf `)	= 49.0	
Snow Load`(psf´)	= 49.0	
Wind Speed (mph)	= 100.0	
Wind Code ` ' '	= IBC	03
Exposure	= C	
Closed/Open	= P	
Importánce — Wind	= 1.10	
Importance — Seismic	= 1.00	
Seismic Coeff (Fa*Ss)	= 0.60	
,		

5. Loading conditions are:

- DL+CL+LL DL+CL+0.75LL+0.75WL1 0.60DL+WL1 0.60DL+WR2 0.60DL+LnWndL 0.60DL+0.60CL 0.60DL+WR1+WS 0.60DL+WP
- 9 0.60DL+WL1+WS 10 DL+CL+0.75LL+0.75WL2+0.75WS 11 DL+CL+0.75LL

RIGID FRAME: BASIC COLUMN REACTIONS (UNFACTORED) (k)	
Frame ColumnDead	
Frame Column — Wind_R2—— — Seismic_L — Seismic_R— — — LnWind_L — — — — — — — — — — — — — — — — — — —	

ENDWALL COLUMN: REACTIONS (UNFACTORED), ANCHOR BOLTS & BASE PLATES

					-Column ₋	_Reacti	ons (k)	Out-0f							
Frm Line		Dead Vert	Coll Vert	Live Vert	Wind- Horiz \		Wind-F Horiz		Wd P Horiz F	Wd S		Bolt O(in)		Plate ((in) Thk	Grout (in)
4 4 6 6 6	E F E G	1.2 4.1 2.2 0.7 1.9 0.8	0.7 3.2 1.3 0.7 2.2 0.7	3.6 17.6 7.1 4.0 11.8 3.8	0.0 5.2 0.0 1.8 0.0 0.0	5.2 -19.7 -4.9 -4.9 -6.4 -2.7	2.5 0.0 0.0 0.0 3.8 0.0	-6.0 -8.5 -4.9 1.7 -13.0 -2.7	-3.7 -9.3 -6.2 -2.7 -7.1 -3.2	4.0 10.0 6.6 2.9 7.6 3.4	4	0.750 0.750 0.750 0.875 0.875 0.875	8.000 10.00 10.00 6.000 8.000 6.000	8.500 21.50 8.500 8.500 8.500 8.500	0.375 0.500 0.500 0.375 0.375 0.375	0.0 0.0 0.0 0.0 0.0 0.0

BRACING REACTIONS (UNFACTORED), PANEL SHEAR

	± Reactions (k) WindSeismic- Horz Vert Horz Vert	Panel Shear (lb/ft)
F_SW E 4,5 R_EW 6 E G	3.9 5.7 2.6 3.7 1.9 2.7 2.6 3.7 6.1 6.7 2.4 2.6 1.4 1.6 1.8 2.1 2.9 3.3 1.8 2.1 6.4 8.1 2.4 3.0	_ _

GENERAL NOTES

- 1. INFORMATION ON THIS DRAWING IS INTENDED FOR CONSTRUCTION ONLY WHEN BEARING A STEELWAY ENGINEERS SIGNED PROFESSIONAL SEAL AND WHEN FREE OF ANY NOTATIONS STATING OTHERWISE.
- 2. FOUNDATION DESIGN AND CONSTRUCTION IS NOT THE RESPONSIBILITY OF STEELWAY BUILDING SYSTEMS.
- 3. THE BUILDING REACTION DATA REPORTS THE LOADS WHICH THIS BUILDING PLACES ON THE FOUNDATIONS.
- 4. THE ENDWALL WIND LOAD REACTIONS INCLUDE REACTIONS FROM ENDWALL BRACING.
- 5. COLUMN BASE PLATES ARE DESIGNED ASSUMING A MINIMUM SPECIFIED COMPRESSIVE STRENGTH (fc') OF CONCRETE OF 2,900 P.S.I. (20 MPA) AT 28 DAYS.
- 6. ANCHOR BOLT DIAMETER, QUANTITY AND PLACEMENT SHOULD BE AS SHOWN.
- 7. THE EMBEDMENT OF THE ANCHOR BOLTS IN THE CONCRETE IS THE RESPONSIBILITY OF THE FOUNDATION DESIGNER. THE FRAME REACTIONS LISTED ARE THE MINIMUM LOADS TO BE DEVELOPED.
- 8. ALL ANCHOR BOLTS ARE TO BE ASTM A307 OR EQUAL.
- 9. ALL REACTIONS ARE IN KIPS OR KIP-FEET.

DWG NOT TO SCALE

73178-R4

DWG #

10. MAXIMUM RIGID FRAME REACTIONS INCLUDE WIND AND SEISMIC REACTIONS FROM SIDEWALL BRACING.

ISSUED FOR INFORMATION 05/10/07 TSD DESCRIPTION

PROJECT

SHIPYARD BREWING CO. LOCATION PORTLAND, US.