



**RIGID FRAME: BASIC COLUMN REACTIONS (UNFACTORED) (k )**

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Wind_L1 Horiz	Wind_L1 Vert	Wind_R1 Horiz	Wind_R1 Vert	Wind_L2 Horiz	Wind_L2 Vert
7 *	E	1.25	3.65	1.13	3.03	10.99	29.58	-10.12	-18.84	-2.55	-16.19	-6.62	-3.49
7 *	B	-1.25	4.26	-1.13	3.34	-10.99	32.59	-0.85	-16.26	5.96	-18.91	0.17	-0.47

  

Frame Line	Column Line	Wind_R2 Horiz	Wind_R2 Vert	Seismic_L Horiz	Seismic_L Vert	Seismic_R Horiz	Seismic_R Vert	LnWind_L Horiz	LnWind_L Vert	LnWind_R Horiz	LnWind_R Vert
7 *	E	0.94	-0.84	-1.44	-0.82	1.44	0.82	-4.71	-24.90	-4.71	-24.90
7 *	B	6.98	-3.13	-1.04	0.82	1.04	-0.82	0.04	-23.59	0.04	-23.59

7 \* Frame lines: 7 8 9

**ENDWALL COLUMN: REACTIONS (UNFACTORED), ANCHOR BOLTS & BASE PLATES**

Frm Line	Col Line	Column Reactions (k )										Anc. No	Bolt D(in)	Base Plate (in)			Grout (in)
		Dead Vert	Coll Vert	Live Vert	Wind-Left Horiz	Wind-Left Vert	Wind-Right Horiz	Wind-Right Vert	Out-Of-Plane Wd P	Out-Of-Plane Wd S	Wid			Len	Thk		
10	B	0.8	0.5	4.9	0.0	-3.4	0.0	-3.4	-2.4	2.5	2	0.750	6.000	8.500	0.375	0.0	
10	C	1.7	1.2	11.7	1.7	-10.3	0.0	-1.4	-4.9	5.3	2	0.750	6.000	8.500	0.375	0.0	
10	D	1.8	1.2	11.7	0.0	-5.8	4.9	-14.7	-5.3	5.7	2	0.750	6.000	8.500	0.375	0.0	
10	E	1.0	0.5	5.1	0.0	-3.6	0.0	-3.6	-2.9	3.2	2	0.750	6.000	8.500	0.375	0.0	

**RIGID FRAME: MAXIMUM REACTIONS (FACTORED), ANCHOR BOLTS, & BASE PLATES**

Frm Line	Col Line	Load Id	Hmax H	Column Reactions (k )				Anc. No	Bolt D(in)	Base Plate (in)			Grout (in)
				V Vmax	Load Id	Hmin H	V Vmin			Wid	Len	Thk	
7 *	E	1	13.4	36.3	2	-9.4	-16.7	6	0.750	6.000	17.25	0.500	0.0
					4	-4.0	-22.7						
7 *	B	3	6.2	-0.6	1	-13.4	40.2	4	0.750	6.000	17.25	0.500	0.0
		1	-13.4	40.2	4	-0.7	-21.0						

7 \* Frame lines: 7 8 9

**BRACING REACTIONS (UNFACTORED), PANEL SHEAR**

Wall Loc	Col Line	± Reactions (k )				Panel Shear (lb/ft)
		Wind Horiz	Wind Vert	Seismic Horiz	Seismic Vert	
<del>L_EW 6 Rigid Frame At Endwall</del>						
F_SW	7,8	3.0	2.9	1.9	1.9	
		5.7	5.6	7.7	7.5	
	9,10	3.0	2.9	1.9	1.8	
		5.7	5.6	7.7	7.5	
R_EW	10	C	1.3	1.8	1.9	2.6
		D	3.7	4.9	1.9	2.6
B_SW	E	10,9	3.2	3.7	1.9	2.2
		8,7	3.2	3.8	1.9	2.3

**NOTES FOR REACTIONS**

- All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Building reactions are based on the following building data:
  - Width (ft) = 55.2
  - Length (ft) = 74.7
  - Eave Height (ft) = 28.6/24.0
  - Roof Slope (rise/12 ) = 1.0
  - Dead Load (psf ) = 4.0
  - Collateral Load (psf ) = 5.0
  - Live Load (psf ) = 49.0
  - Snow Load (psf ) = 49.0
  - Wind Speed (mph ) = 100.0
  - Wind Code = IBC 03
  - Exposure = C
  - Closed/Open = P
  - Importance - Wind = 1.10
  - Importance - Seismic = 1.00
  - Seismic Coeff (Fa\*Ss) = 0.60
- Loading conditions are:
  - DL+CL+LL
  - 0.60DL+WL1
  - 0.60DL+WR2
  - 0.60DL+LnWindL
  - 0.60DL+WR1+WS
  - 0.60DL+WP
  - 0.60DL+WL1+WS

**GENERAL NOTES**

- INFORMATION ON THIS DRAWING IS INTENDED FOR CONSTRUCTION ONLY WHEN BEARING A STEELWAY ENGINEERS SIGNED PROFESSIONAL SEAL AND WHEN FREE OF ANY NOTATIONS STATING OTHERWISE.
- FOUNDATION DESIGN AND CONSTRUCTION IS NOT THE RESPONSIBILITY OF STEELWAY BUILDING SYSTEMS.
- THE BUILDING REACTION DATA REPORTS THE LOADS WHICH THIS BUILDING PLACES ON THE FOUNDATIONS.
- THE ENDWALL WIND LOAD REACTIONS INCLUDE REACTIONS FROM ENDWALL BRACING.
- COLUMN BASE PLATES ARE DESIGNED ASSUMING A MINIMUM SPECIFIED COMPRESSIVE STRENGTH (fc') OF CONCRETE OF 2,900 P.S.I. (20 MPA) AT 28 DAYS.
- ANCHOR BOLT DIAMETER, QUANTITY AND PLACEMENT SHOULD BE AS SHOWN.
- THE EMBEDMENT OF THE ANCHOR BOLTS IN THE CONCRETE IS THE RESPONSIBILITY OF THE FOUNDATION DESIGNER. THE FRAME REACTIONS LISTED ARE THE MINIMUM LOADS TO BE DEVELOPED.
- ALL ANCHOR BOLTS ARE TO BE ASTM A307 OR EQUAL.
- ALL REACTIONS ARE IN KIPS OR KIP- FEET.
- MAXIMUM RIGID FRAME REACTIONS INCLUDE WIND AND SEISMIC REACTIONS FROM SIDEWALL BRACING.

BUILDER <b>IRISHSPAN INDUSTRIES, INC.</b>	PROJECT <b>SHIPYARD BREWING CO.</b>	LOCATION <b>PORTLAND, US.</b>	DWG NOT TO SCALE	0	ISSUED FOR INFORMATION	05/10/07	TSD
			DWG # <b>73178-R3</b>	REV.	DESCRIPTION	DATE	BY
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