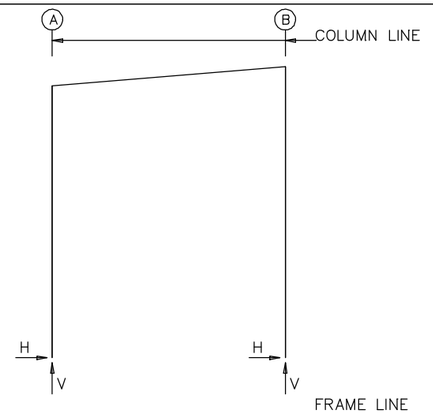


RIGID FRAME: MAXIMUM REACTIONS (FACTORED), ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Anc. No	Bolt D(in)	Base Plate (in) Wid	Len	Thk	Grout (in)
		Column Reactions (k)											
5 *	A	9	3.0	−3.4	6	−2.9	0.4	4	0.750	6.000	13.00	0.375	0.0
		1	0.6	18.2	8	3.0	−4.1						

5 * Frame lines: 5 4

RIGID FRAME: MAXIMUM REACTIONS (FACTORED), ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Anc. No	Bolt D(in)	Base Plate (in) Wid	Len	Thk	Grout (in)
		Column Reactions (k)											
2 *	A	5	4.0	0.7	6	−4.1	−2.8	4	0.750	6.000	13.00	0.375	0.0
		1	0.4	14.4	4	−1.2	−6.0						
2 *	B	7	4.3	−3.7	2	−4.1	16.9	4	0.750	6.000	13.00	0.375	0.0
		1	−2.0	22.8	5	1.4	−7.0						

2 * Frame lines: 2 1

GENERAL NOTES

1. INFORMATION ON THIS DRAWING IS INTENDED FOR CONSTRUCTION ONLY WHEN BEARING A STEELWAY ENGINEERS SIGNED PROFESSIONAL SEAL AND WHEN FREE OF ANY NOTATIONS STATING OTHERWISE.
2. FOUNDATION DESIGN AND CONSTRUCTION IS NOT THE RESPONSIBILITY OF STEELWAY BUILDING SYSTEMS.
3. THE BUILDING REACTION DATA REPORTS THE LOADS WHICH THIS BUILDING PLACES ON THE FOUNDATIONS.
4. THE ENDWALL WIND LOAD REACTIONS INCLUDE REACTIONS FROM ENDWALL BRACING.
5. COLUMN BASE PLATES ARE DESIGNED ASSUMING A MINIMUM SPECIFIED COMPRESSIVE STRENGTH (fc') OF CONCRETE OF 2,900 P.S.I. (20 MPA) AT 28 DAYS.
6. ANCHOR BOLT DIAMETER, QUANTITY AND PLACEMENT SHOULD BE AS SHOWN.
7. THE EMBEDMENT OF THE ANCHOR BOLTS IN THE CONCRETE IS THE RESPONSIBILITY OF THE FOUNDATION DESIGNER. THE FRAME REACTIONS LISTED ARE THE MINIMUM LOADS TO BE DEVELOPED.
8. ALL ANCHOR BOLTS ARE TO BE ASTM A307 OR EQUAL.
9. ALL REACTIONS ARE IN KIPS OR KIP−FEET.
10. MAXIMUM RIGID FRAME REACTIONS INCLUDE WIND AND SEISMIC REACTIONS FROM SIDEWALL BRACING.

RIGID FRAME: BASIC COLUMN REACTIONS (UNFACTORED) (k)

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Wind_L1 Horiz	Wind_L1 Vert	Wind_R1 Horiz	Wind_R1 Vert	Wind_L2 Horiz	Wind_L2 Vert
2 *	A	0.05	0.86	0.05	0.72	0.28	12.84	−1.21	−6.57	3.93	0.14	−4.13	−3.27
2 *	B	−0.05	0.91	−0.05	0.75	−1.91	21.18	−3.41	−0.85	1.43	−7.56	−0.49	2.44
5 *	A	0.02	0.93	0.02	0.79	0.53	16.51	0.34	−4.04	2.46	−4.26	−2.94	−0.15

Frame Line	Column Line	Wind_R2 Horiz	Wind_R2 Vert	Seismic_L Horiz	Seismic_L Vert	Seismic_R Horiz	Seismic_R Vert	LnWind_L Horiz	LnWind_L Vert	LnWind_R Horiz	LnWind_R Vert
2 *	A	1.01	3.44	−0.32	−0.72	0.32	0.72	2.67	−3.92	2.67	−3.92
2 *	B	4.35	−4.27	−0.28	0.72	0.28	−0.72	−2.55	−4.29	−2.55	−4.29
5 *	A	−0.81	−0.37	0.00	0.02	0.00	−0.02	2.95	−4.68	2.95	−4.68

2 * Frame lines: 2 1
5 * Frame lines: 5 4

BRACING REACTIONS (UNFACTORED), PANEL SHEAR

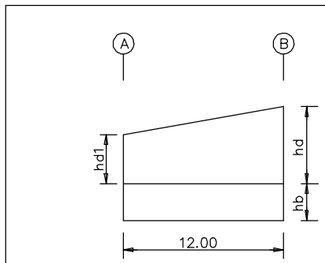
Wall Loc	Wall Line	Col Line	± Reactions (k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel Shear (lb/ft)
L_EW	5	Rigid Frame At Endwall					
F_SW	B	2 ,1	0.8	0.4	0.9	0.5	
R_EW	1	Rigid Frame At Endwall					
B_SW	A	1 ,2	0.8	0.4	0.9	0.5	

NOTES FOR REACTIONS

1. All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
2. Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
3. Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
4. Building reactions are based on the following building data:

Width (ft)	= 12.0
Length (ft)	= 73.4
Eave Height (ft)	= 14.0/15.0
Roof Slope (rise/12)	= 1.0
Dead Load (psf)	= 4.0
Collateral Load (psf)	= 5.0
Live Load (psf)	= 49.0
Snow Load (psf)	= 49.0
Wind Speed (mph)	= 100.0
Wind Code	= IBC 03
Exposure	= C
Closed/Open	= P
Importance − Wind	= 1.10
Importance − Seismic	= 1.00
Seismic Coeff (Fa*Ss)	= 0.60

5. Loading conditions are:
   1 DL+CL+LL
   2 DL+CL+0.75LL+0.75WL1
   3 DL+CL+0.75LL+0.75WL2
   4 0.60DL+WL1
   5 0.60DL+WR1
   6 0.60DL+WL2
   7 0.60DL+WR2
   8 0.60DL+LnWndL
   9 0.60DL−LnWndR



hb =BALANCE SNOW = 49.0 psf
hd =DRIFT SURCHARGE = 103 psf
hd1=DRIFT SURCHARGE = 34 psf
SNOW DRIFT DIAGRAM

DWG NOT TO SCALE

REV.	DESCRIPTION	DATE	BY
0	ISSUED FOR INFORMATION	05/10/07	TSD

BUILDER: IRISHSPAN INDUSTRIES INC
PROJECT: SHIPYARD BREWING CO.
LOCATION: PORTLAND, US.
DWG #: 73178−R1

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