

Project: federal street

Location: steel beam

Uniformly Loaded Floor Beam

[2012 International Building Code(AISC 13th Ed ASD)]

A992-50 W10x30 x 14.67 FT

Section Adequate By: 240.3%

Controlling Factor: Deflection


 Lindsey Architects  
 Lindsey Architects  
 4 Market Place Drive Suite 201B  
 York Maine 03909

page  
 of

StruCalc Version 9.0.2.6

10/15/2016 11:01:05 PM

<b>DEFLECTIONS</b>		Center
Live Load	0.14	IN L/1225
Dead Load	0.06	in
Total Load	0.20	IN L/863
Live Load Deflection Criteria: L/360 Total Load Deflection Criteria: L/240		

<b>REACTIONS</b>		A	B
Live Load	4988 lb	4988 lb	
Dead Load	2090 lb	2090 lb	
Total Load	7078 lb	7078 lb	
Bearing Length	0.81 in	0.81 in	

<b>BEAM DATA</b>		Center
Span Length	14.67	ft
Unbraced Length-Top	0	ft

**STEEL PROPERTIES**

W10x30 - A992-50

**Properties:**

Yield Stress:	Fy =	50	ksi
Modulus of Elasticity:	E =	29000	ksi
Depth:	d =	10.5	in
Web Thickness:	tw =	0.3	in
Flange Width:	bf =	5.81	in
Flange Thickness:	tf =	0.51	in
Distance to Web Toe of Fillet:	k =	0.81	in
Moment of Inertia About X-X Axis:	Ix =	170	in <sup>4</sup>
Section Modulus About X-X Axis:	Sx =	32.4	in <sup>3</sup>
Plastic Section Modulus About X-X Axis:	Zx =	36.6	in <sup>3</sup>

**Design Properties per AISC 13th Edition Steel Manual:**

Flange Buckling Ratio:	FBR =	5.7
Allowable Flange Buckling Ratio:	AFBR =	9.15
Web Buckling Ratio:	WBR =	29.6
Allowable Web Buckling Ratio:	AWBR =	90.55
Controlling Unbraced Length:	Lb =	0 ft
Limiting Unbraced Length - for lateral-torsional buckling:	Lp =	4.84 ft
Nominal Flexural Strength w/ safety factor:	Mn =	91317 ft-lb
Controlling Equation:	F2-1	
Web height to thickness ratio:	h/tw =	29.6
Limiting height to thickness ratio for eqn. G2-2:	h/tw-limit =	53.95
Cv Factor:	Cv =	1
Controlling Equation:	G2-2	
Nominal Shear Strength w/ safety factor:	Vn =	63000 lb

**Controlling Moment:**

25960 ft-lb

7.335 ft from left support

Created by combining all dead and live loads.

**Controlling Shear:**

7078 lb

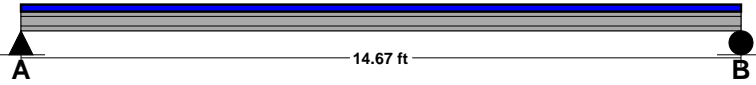
At support.

Created by combining all dead and live loads.

**Comparisons with required sections:**

	Req'd	Provided
Moment of Inertia (deflection):	49.96 in <sup>4</sup>	170 in <sup>4</sup>
Moment:	25960 ft-lb	91317 ft-lb
Shear:	7078 lb	63000 lb

**LOADING DIAGRAM**



**FLOOR LOADING**

		Side 1	Side 2
Floor Live Load	FLL =	40 psf	40 psf
Floor Dead Load	FDL =	15 psf	15 psf
Floor Tributary Width	FTW =	8 ft	9 ft
Wall Load	WALL =	0 plf	

**BEAM LOADING**

Beam Total Live Load:	wL =	680 plf
Beam Total Dead Load:	wD =	255 plf
Beam Self Weight:	BSW =	30 plf
Total Maximum Load:	wT =	965 plf

**NOTES**