



- 1" RADIUS "RATHOLES" TOP AND BOTTOM FLANGE

BEAM SEE PLAN $_$

 SLIP-CRITICAL SHEAR CONNECTION 1/2" INCH MINIMUM THICK SHEAR TAB PLATE DESIGNED BY FABRICATOR. SHEAR CONNECTIONS SHALL BE PROVIDED FOR 50% GREATER CAPACITY THAN SIMPLE SPANS
1/4"X1"X(FLANGE WIDTH+1")

BACKING BARS TOP AND BOTTOM FLANGE.

STIFFENER PLATE TO MATCH BEAM FLANGE THICKNESS

NOTES:

1. NO SHOP PRIMER WITHIN 3 INCHES OF FIELD WELDS. REMOVE SHOP PRIMER (INCLUDING OVER-SPRAY) IN THE FIELD PRIOR TO WELDING IF NECESSARY.

- 2. NO SHOP PRIMER AT "FAYING" SURFACES OF SLIP-CRITICAL SHEAR CONNECTIONS. PREPARE "FAYING" SURFACES IN THE FIELD PRIOR TO ERECTION TO SATISFY CLASS "A" AS DEFINED IN AISC'S
- "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS." 3. WHERE COLUMNS HAVE WEB AND FLANGE MOMENT CONNECTIONS, THE CONTINUITY PLATES, GUSSET
- PLATES AND WELDS SHALL BE DETAILED AND DESIGNED BY FABRICATOR FOR THE WORST CONDITIONS. 4. Pf = (Af)(Fy), BASED UPON BEAM PROPERTIES.
- 5. THE FABRICATOR SHALL TAKE EXPECTED WELD SHRINKAGE INTO ACCOUNT WHEN DETAILING THE BEAM LENGTHS IN CONTINUOS RUNS. 6. FILLER METAL USED IN THE FULL PENETRATION WELDS SHALL HAVE A MINIMUM CHARPY V-NOTCH VALUE

OF 20 FT-LBS AT 40 DEGREES F FOR BUILDINGS WHICH ARE FULLY ENCLOSED AND HEATED DURING THE WELDING PROCESS AND IN SERVICE. AND 20 FT-LBS AT ZERO (0) DEGREES F FOR ALL OTHER WELDING AND SERVICE CONDITIONS.

7. THE MOMENT CONNECTION WELDS ARE TO BE CONSIDERED "PRE-QUALIFIED WELDED JOINTS". THERFORE THE STEEL ERECTOR SHALL SUBMIT PRE-QUALIFIED JOINT WELDING PROCEDURES AND JOINT DETAILS AS PER AMS D1.1 CERTIFICATE OF COMPLIANCE FOR ALL THE ELECTRODES TO BE USED, AND WELDING CERTIFICATES FOR ALL THE WELDERS.

<u>FLANGE</u>

BEAM/WIDE FLANGE COLUMN MOMENT CONNECTION

INDICATED THUS ON PLANS

