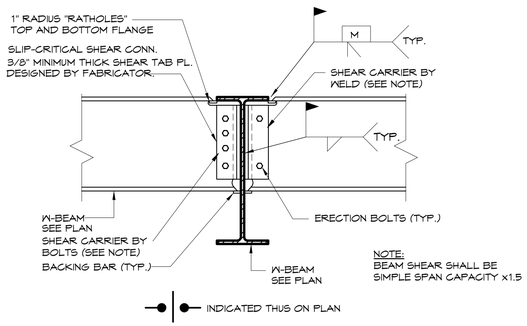
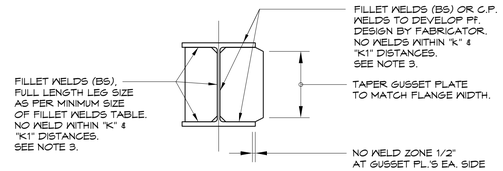
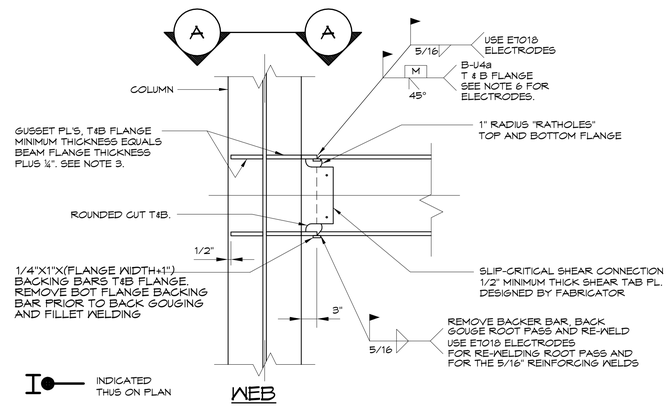


BEAM TO BEAM
TYPICAL MOMENT CONNECTION

A 3/4" = 1'-0"



SECTION A
1/2" = 1'-0"

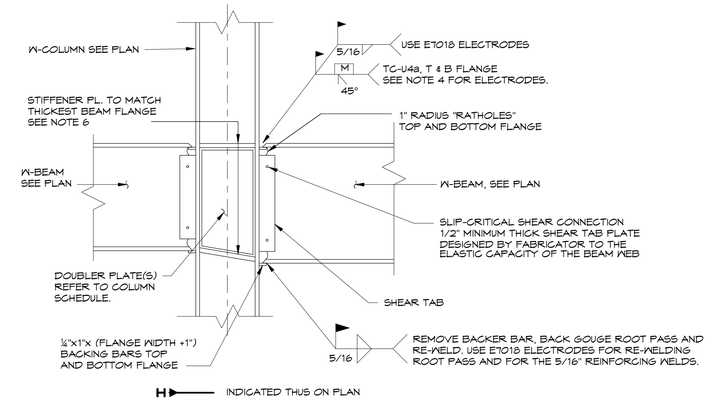


WEAK AXIS SEISMIC MOMENT CONNECTION DETAIL

NOTES:

1. NO SHOP PRIMER WITHIN 3 INCHES OF FIELD WELDS. REMOVE SHOP PRIMER (INCLUDING OVER-SPRAY) IN THE FIELD PRIOR TO WELDING IF NECESSARY.
2. NO SHOP PRIMER AT "FAYING" SURFACES OF SLIP-CRITICAL SHEAR CONNECTIONS. PREPARE "FAYING" SURFACES IN THE FIELD PRIOR TO ERECTION TO SATISFY CLASS "A" AS DEFINED IN AISC'S "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS."
3. THE FABRICATOR SHALL TAKE EXPECTED WELD SHRINKAGE INTO ACCOUNT WHEN DETAILING THE BEAM LENGTHS IN CONTINUOS RUNS.
4. FILLER METAL USED IN THE FULL PENETRATION WELDS SHALL HAVE A MINIMUM CHARPY V-NOTCH VALUE OF 20 FT-LBS.
5. THE MOMENT CONNECTION WELDS ARE TO BE CONSIDERED "PRE-QUALIFIED WELDED JOINTS". THEREFORE THE STEEL ERECTOR SHALL SUBMIT PRE-QUALIFIED JOINT WELDING PROCEDURES AND JOINT DETAILS AS PER AWS D1.1 CERTIFICATE OF COMPLIANCE FOR ALL THE ELECTRODES TO BE USED, AND WELDING CERTIFICATES FOR ALL THE WELDERS.
6. AT ALL MOMENT CONNECTIONS PROVIDE STIFFENER PLATES. THE PLATE THICKNESS SHALL MATCH THE THICKNESS OF THE THICKER BEAM FLANGE.
7. REFER TO THE COLUMN SCHEDULE FOR DOUBLER PLATE LOCATIONS AND THICKNESS (IF ANY).

B 3/4" = 1'-0"



BEAM/WIDE FLANGE COLUMN SEISMIC MOMENT CONNECTION

NOTES:

1. NO SHOP PRIMER WITHIN 3 INCHES OF FIELD WELDS. REMOVE SHOP PRIMER (INCLUDING OVER-SPRAY) IN THE FIELD PRIOR TO WELDING IF NECESSARY.
2. NO SHOP PRIMER AT "FAYING" SURFACES OF SLIP-CRITICAL SHEAR CONNECTIONS. PREPARE "FAYING" SURFACES IN THE FIELD PRIOR TO ERECTION TO SATISFY CLASS "A" AS DEFINED IN AISC'S "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS."
3. THE FABRICATOR SHALL TAKE EXPECTED WELD SHRINKAGE INTO ACCOUNT WHEN DETAILING THE BEAM LENGTHS IN CONTINUOS RUNS.
4. FILLER METAL USED IN THE FULL PENETRATION WELDS SHALL HAVE A MINIMUM CHARPY V-NOTCH VALUE OF 20 FT-LBS.
5. THE MOMENT CONNECTION WELDS ARE TO BE CONSIDERED "PRE-QUALIFIED WELDED JOINTS". THEREFORE THE STEEL ERECTOR SHALL SUBMIT PRE-QUALIFIED JOINT WELDING PROCEDURES AND JOINT DETAILS AS PER AWS D1.1 CERTIFICATE OF COMPLIANCE FOR ALL THE ELECTRODES TO BE USED, AND WELDING CERTIFICATES FOR ALL THE WELDERS.
6. AT ALL MOMENT CONNECTIONS PROVIDE STIFFENER PLATES. THE PLATE THICKNESS SHALL MATCH THE THICKNESS OF THE THICKER BEAM FLANGE.
7. REFER TO THE COLUMN SCHEDULE FOR DOUBLER PLATE LOCATIONS AND THICKNESS (IF ANY).

C 1/8" = 1'-0"

STATE OF MAINE
RIMANTAS M. VEITAS
No. 9477
LICENSED PROFESSIONAL ENGINEER

Reviewed for Code Compliance
Inspection Division
Date: 10/27/17

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Project:
0 HANCOCK STREET

0 HANCOCK STREET
PORTLAND, MAINE

Revisions:
08-11-17 Issued for Foundation Permit
08-31-17 Permit Set
09-15-17 Steel Bid Set
09-22-17 Pile Bid Set
09-29-17 Concrete Bid Set
10-06-17 Issued for Permit

Scale:

Date: 08/11/17

Typical Details

S204