

STATEMENT OF SPECIAL INSPECTIONS

PROJECT: Waterville Street Condominium Building C

LOCATION: Portland, Maine

PERMIT APPLICANT: Casco Bay Ventures, Inc

APPLICANT'S ADDRESS: 223 Woodville Road

Falmouth, Maine 04105

STRUCTURAL ENGINEER OF RECORD:

Timothy S Dean, P.E. Pinkham & Greer Engineers  
NAME FIRM

ARCHITECT OF RECORD:

Scott Teas, R.A. T FH Architects  
NAME FIRM

This Statement of Special Inspections is submitted in accordance with Section 1705.0 of the 1999 BOCA National Building Code. It includes a listing of special inspections applicable to this project as well as the name of the Special Inspector(s), and the names of other agencies intended to be retained for conducting these inspections.

The Special Inspector shall keep records of all inspections listed herein, and shall furnish inspection reports to the Code Official and to the Registered Design Professional of Record. All discrepancies shall be brought to the immediate attention of the Contractor for correction. If the discrepancies are not corrected the discrepancies shall be brought to the attention of the Code Official and to the Registered Design Professional of Record. Interim reports shall be submitted to the Code Official and to the Registered Design Professional of Record monthly, unless more frequent submissions are requested by the Code Official.

Job site safety is solely the responsibility of the Contractor. Materials and activities to be inspected are not to include the Contractor's equipment and methods used to erect or install the materials listed.

Prepared By:

Timothy S Dean, P.E.

NAME: Timothy S Dean  
SIGNATURE: [Signature]  
DATE: 8/16/03

Applicant's Authorization:

NAME: [Signature]  
SIGNATURE: [Signature]  
DATE: 8-20-03  
Casco Bay Ventures Inc

Building Code Official:



SIGNATURE \_\_\_\_\_  
DATE \_\_\_\_\_

**SCHEDULE OF SPECIAL INSPECTION SERVICES**

**PROJECT: WATERVILLE STREET CONDOMINIUMS**

PAGE 1 OF 5

MATERIAL / ACTIVITY	ITEM	SERVICE	Y/N	EXTENT (All, Sample, Other, None)	COMMENTS	AGENT #	DATE COMPLETED	REV. #
1705.3 STEEL CONSTRUCTION	1.00							
		Steel Fabrication						
		In-plant review	Y		Spec 5120 1.02B		1.2.3	
		Part A - Fabrication procedures	Y				1.2.3	
		Part B - Procedures	Y/N		SER to determine extent after completion of Part A		1.2.3	
		implementation Review	Y/N				1.2.3	
		conformance to Part A	Y/N				1.2.3	
		Review material certificates of compliance (structural steel & weld filler material)	N	ALL			1.2.3	
		Review welding of seismic-resisting system in Cat. "C" buildings	N					
		Review connections	N					
Steel Erection	1.02	Review welder certification	Y	ALL			1.2.3	
		Review materials certificates of compliance (Bolts, nuts, washers, & weld filler material)	Y	ALL			1.2.3	
		Review primary steel connections	Y	ALL			1.2.3	
	1.04	Moment connections	N					
	1.05	Shear connections	Y		VISUAL, INITIAL SAMPLE 10%		1.2.3.5	
	1.06	Bracing connections	N					
		Review welded Cat. "C" seismic connections	N					
		Review welded column splices	N					
		Review base metal testing for "T" > 1 1/2"	N					
		Review secondary steel connections						
		Girts	N					
	1.07	Steel deck	Y		VISUAL, INITIAL SAMPLE 25%		1.2.3.5	
	1.08	Lintels	Y		VISUAL, INITIAL SAMPLE 10%		1.2.3	
		Review installation of shear studs	N					
	1.09	Review Details / Steel Frame	Y		SAMPLE		1.2.3.5	

All Steel Construction Special Inspections have been completed in accordance with BOCA Section 1705.3  
Special Inspector

SCHEDULE OF SPECIAL INSPECTION SERVICES

PROJECT: WATERVILLE STREET CONDOMINIUMS

APPLICABLE TO THIS PROJECT

MATERIAL / ACTIVITY	ITEM	SERVICE	Y/N	EXTENT (All, Sample, Other, None)	COMMENTS	AGENT #	DATE COMPLETED	REV. #
1705.3 STEEL CONSTRUCTION (CONTINUED)	1.00	In-plant review						
		Part A - Fabrication procedures	N	Unless excepted by 1705.2 review fabrication Q/A procedures per 1705.2				
Light Gage Steel Framing Fabrication		Part B - Procedures implementation Review	N	SER to determine extent after completion of Part A				
		conformance to Part A						
Light Gage Steel Framing	1.10	Review material certificates of compliance (structural steel & weld filler material)	Y	ALL			1.2.3	
		Review connections	N					
Light Gage Steel Framing Installation	1.11	Review welder certification	Y	ALL			1.2.3	
	1.12	Review joist bearing connection	Y	SAMPLE (INITIAL SAMPLE 25%) VISUAL			1.2.3	
	1.13	Review joist bearing length	Y	SAMPLE (INITIAL SAMPLE 25%) VISUAL			1.2.3.5	
	1.14	Review joist bridging	Y	SAMPLE (INITIAL SAMPLE 25%) VISUAL			1.2.3.5	
	1.15	Shear connections	Y	SAMPLE (INITIAL SAMPLE 25%) VISUAL			1.2.3.5	
	1.16	Bracing connections	Y	ALL			1.2.3	
	1.17	Review Details / Steel Frame	Y	SAMPLE (INITIAL SAMPLE 25%) VISUAL			1.2.3.5	

All Steel Construction Special Inspections have been completed in accordance with BOCA Section 1705.3 Special Inspector

SCHEDULE OF SPECIAL INSPECTION SERVICES

PROJECT: WATERVILLE STREET CONDOMINIUMS

PAGE 3 OF 5

APPLICABLE TO THIS PROJECT

MATERIAL/ACTIVITY	ITEM	SERVICE	V/N	EXTENT (All, Sample, Other, None)	COMMENTS	AGENT #	DATE COMPLETED	REV. #
1705.4 CONCRETE CONSTRUCTION	2.00							
	2.01	Review materials (ACI Chapter 3)	Y	ALL			1.2.3	
Concrete Materials	2.02	Review mix design (ACI Chapter 4)	Y	ALL			1.2.3	
		Review reinforcing certification & weldability (ASTM A 706) if required	N					
Placing Reinforcement	2.03	Review condition & placement of reinforcing and prestressing steel (ACI 318 7.4-7.7)	Y	SAMPLE			1.2.3	
		Review welding of reinforcing in Cat "C" seismic-resisting systems	N					
Formwork		Review formwork (ACI 318 6.1)	N					
		Review form removal & reshoring (ACI 318 6.2)	N					
Concrete Operations	2.04	Review concrete strength tests (ACI 318 5.6)	Y	ALL			1.2.3	
		Review mix proportions and technique (ACI 318 5.2, 5.3, 5.4, & 5.8)	N					
	2.05	Review concrete placement (ACI 318 5.9 & 5.10)	Y	SAMPLE			1.2.3.5	
		Review curing technique & temperature (ACI 318 5.11, 5.12, 2.06 & 5.13)	Y	SAMPLE			1.2.3	
		Review application of prestressing force (ACI 318 18.18)	N					
		Review grouting of bonded prestressing tendons in Cat. "C" seismic-resisting systems	N					
Prestressing Operations		In-plant review	N					
		Part A - Fabrication procedures	N					
Precast Manufacturing		Part B- Procedures implementation	N					
		Review conformance to Part A	N					
Erection of Precast Concrete		Review erection of precast units	N					
		Review key reinforcement	N					
		Review key grouting	N					
		Review concrete topping	N					
		Review connections	N					

All Steel Construction Special Inspections have been completed in accordance with BOCA Section 1705.3 Special Inspector

SCHEDULE OF SPECIAL INSPECTION SERVICES

PROJECT: WATERVILLE STREET CONDOMINIUMS

PAGE 4 OF 5

APPLICABLE TO THIS PROJECT

MATERIAL/ACTIVITY	ITEM	SERVICE	Y/N	EXTENT (All, Sample, Other)	COMMENTS	AGENT #	DATE COMPLETED	REV. #
-------------------	------	---------	-----	-----------------------------	----------	---------	----------------	--------

1705.5 MASONRY CONSTRUCTION	3.00							
-----------------------------	------	--	--	--	--	--	--	--

Masonry Materials	3.01	Review materials certification Masonry units	N					
-------------------	------	---	---	--	--	--	--	--

	3.02	Review grout materials & mix design	Y				1.2.3	
--	------	-------------------------------------	---	--	--	--	-------	--

	3.03	Review mortar materials & mix design	N					
--	------	--------------------------------------	---	--	--	--	--	--

		Review strength determination Unit strength method. Review unit strengths & grout.	N					
--	--	--	---	--	--	--	--	--

		Review strength method. Review pre construction test results. Field tests during construction.	N					
--	--	--	---	--	--	--	--	--

	3.04	Grout testing Determine compressive strength	Y				5	
--	------	---	---	--	--	--	---	--

		Mortar testing Field test compressive strength ASTM C780 (Reqd. only if properly reqs of ASTM C270 are used)	N					
--	--	--	---	--	--	--	--	--

General Masonry work	3.05	Review mortar mix proportions & mixing (ACI 530.1: 2.3.2.5)	N					
----------------------	------	---	---	--	--	--	--	--

	3.06	Review grout mix proportions & mixing (ACI 530.1: 4.2.2)	Y				1.2.3	
--	------	--	---	--	--	--	-------	--

	3.07	Review general installation of mortar, grout, masonry units.	N					
--	------	--	---	--	--	--	--	--

	3.08	Review installation of horiz. vert. & joint reinforcing (incl. Location, sizes, splices, & positioning devices)	N					
--	------	---	---	--	--	--	--	--

	3.09	Review hot/cold weather procedures (ACI 530.1: 2.3.2.2, 2.3.2.3)	Y				1.2.3	
--	------	--	---	--	--	--	-------	--

		Review installation of anchorage devices (ACI 530: 4.2, 5.14)	N					
--	--	---	---	--	--	--	--	--

	3.10	Review installation of lintels	N					
--	------	--------------------------------	---	--	--	--	--	--

All Steel Construction Special Inspections have been completed in accordance with BOCA Section 1705.3 Special Inspector



**S E A M**

Structural Engineering Association of Maine

**LIST OF AGENTS**

**PROJECT: WATERVILLE STREET CONDOMINIUMS**

**STRUCTURAL ENGINEER OF RECORD: Timothy S Dean P.E. Pinkham & Greer Engineers**

*NAME*

*FIRM*

170 U. S. Route One, Falmouth, Maine 04105

*ADDRESS*

**ARCHITECT OF RECORD:**

Scott Teas R.A.

TFH Architects

*NAME*

*FIRM*

100 Commercial Street Portland, ME, 04101

*ADDRESS*

Following is the List of Agents selected for performance of Special Inspections for this project.

1. Special Inspector
2. Special Inspector
3. Special Inspector
4. Geotechnical Eng.
5. Testing Agency

Name  
Timothy S Dean P.E.  
David K Pinkham P.E.  
Kenneth I Marsh  
Matthew T. Grady P.E.

Firm  
Pinkham & Greer Engineers  
Pinkham & Greer Engineers  
Pinkham & Greer Engineers  
R. W. Gillespie & Associates  
R. W. Gillespie & Associates

# Memorandum

**Project:** Waterville Street Condominiums  
**Date:** August 13, 2003  
**To:** Mr. Michael Nugent  
 Inspection Services Manager  
 City of Portland  
 City Hall Room 315  
 389 Congress Street  
 Portland, ME 04101

**From:** Will Tinkelenberg  
 TFH Architects

**Re:** Parking Garage Ventilation

**Project No.:** 0111  
**Phone No.:** (207) 874-8700  
**Fax No.:** (207) 874-8716

**Phone No.:** (207) 775-6141  
**Fax No.:** (207) 773-0194

**Copy to:** File

Mike:

Included here is a letter from Dick Whitney, mechanical engineer, that outlines the code requirements for providing ventilation for the parking garage, along with proposed design parameters based on these requirements.

Where Mr. Whitney refers to locating the CO detector above the door to the "Utility" Room, he is actually referring to the room identified on the plans as the Maintenance Room (such that the CO detector remains located as shown on the E1-1 plan).

Please let us know if this does not address you concerns.

Thank you,

Will



Whitney Engineering, P.A.  
10 Danforth Street  
Portland, Me 04101  
(207) 874-7449  
(207) 874-7849-fax  
e-mail: whitney@maine.rr.com

Date: August 12, 2003  
CASCO BAY CONDOMINIUMS BUILDING C

To: Scott Teas, TFH Architects  
Project: Casco Bay Condominiums Building C  
Re: Garage Ventilation

Scott,

A. Code Requirements

1. BOCA National Mechanical Code 1993, International Mechanical Code 1998 ("BOCA" with new name) and ASHRAE Standard, ANSI/ASHRAE 62-1992, "Ventilation for Acceptable Indoor Air Quality" all specify 1.5 CFM/Sq. Ft. floor area for common garages serving several units in residential facilities.

2. A ventilation code per automobile applies to separate garages for each dwelling unit, not a common garage serving several units as we are looking at.

3. Ventilation requirement is for "outdoor air", that is air taken from outdoors circulated previously through the ventilation system. This means that if an exhaust fan is used, it presumes that outdoor air will enter the garage in some manner to "make-up" air exhausted. By definition in BOCA, "ventilation" as defined, includes removal of air from any space by natural or mechanical means. Note 2 in BOCA for Private Dwellings specifically requires that garage exhaust be mechanical.

4. BOCA requires that the garage exhaust:

- a. locates exhaust fan inlet in the area of heaviest concentration of contaminants
- b. exhaust directly outdoors without recirculation
- c. operate when air is required to be exhausted.

(Note: BOCA allows exhaust fan for Public Garages to not operate continuously if an automatic CO detector is set to 25 PPM and exhaust fan is capable of providing not less than 1.5 CFM per square foot garage floor area. Interpretation: BOCA prevents variable

Garage Ventilation  
Casco Bay Condominiums Building C  
Portland, Maine  
August 12, 2003

ventilation based on PPM of CO. If 25 PPM CO is detected, the exhaust fan is required to provide 1.5 CFM/Sq. Ft minimum until concentration drops below 25 PPM).  
d. operate when motor vehicles operate for a time period greater than 10 seconds (or is controlled by a CO detector)  
e. Fan exhaust discharge cannot be located to "create a nuisance" or directed onto walkways.

**B. Proposed Design to Meet Code Requirements:**

1. Provide a wall propeller or centrifugal type exhaust fan with a minimum capacity of 1.5 CFM/Square Foot garage floor area, or 1,462 square feet garage floor area x 1.5 CFM per square feet = 2,193 CFM.

2. Locate fan high as possible to pick up less dense CO-air mixture.

3. Locate fan discharge to avoid nuisance discharge.

4. Provide opening for outdoor air for make-up air to exhaust fan, for example, open doors into the garage.

5. Provide CO detector and operate fan from at detection of CO level of 25 PPM or higher. Locate CO detector above door to utility room.

If you have any questions please do not hesitate to call.

Regards,

**WHITNEY ENGINEERING, P.A.**  
*DICK WHITNEY*  
Richard P. Whitney, P.E.

The: Code Requirements 1.doc

# Transmittal Letter

Project: Waterville Street Condominiums

Project No. 0111

To: Mr. Michael Nugent

Inspection Services Manager

City Hall Room 315

389 Congress Street

Portland, Maine 04101

If enclosures are not as noted, please inform us immediately.

Date: August 7, 2003

Fax No.:

We transmit:

(x) herewith ( ) under separate cover via UPS  
 ( ) in accordance with your request

For your:

( ) approval ( ) distribution to parties ( ) information  
 ( ) Review and Comm ( ) record  
 (x) use ( ) Signature

The following:

( ) Drawings ( ) Shop Drawing Prints/Specification ( ) Samples  
 ( ) Specifications ( ) Shop Drawing Reproduces ( ) Product Literature  
 ( ) Change Order ( ) Slides

Copies	Date	Rev. No.	Description	Action
1	4/29/03		Specifications	
"	2/27/03		R. W. Gillespie Geotechnical Investigation	
"	3/10/03		Gillespie Letter re: Report Errata	
"	"		Gillespie Letter re: Slab/Footing Insulation	
"	8/6/03		City of Portland Building Code Certificate	
"	"		City of Portland BOCA Certificate	
3	8/7/03		Special Inspections Statement	

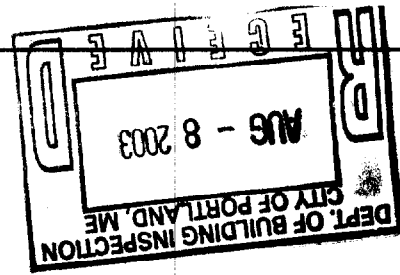
Action Code A. Action indicated on item transmitted

B. No action required  
 C. For signature and return to this office

D. For signature and forwarding as noted below under Remarks  
 E. See Remarks below

Remarks:

Copies to: (with enclosures)



TFH Architects, P. A.  
 100 Commercial Street  
 Portland Maine 04101  
 Telephone 207-775-6141  
 Fax 207-773-0194  
 By: Will Tinkelenberg

PORTLAND, MAINE  
ZONING BOARD OF APPEALS

William Nestki, Jr. Chair  
Lee Lowry, Secretary  
Andrew Braceras  
Elizabeth Bordowitz  
Julie Brady  
Tracy Decker  
Sam Sivovios

June 2, 2001

Casco Bay Ventures, Inc  
223 Woodville Road  
Falmouth, Maine 04105

RE: 3 St. Lawrence Street  
CBL: 16-F-024  
ZONE: R-6 Zone

Dear Casco Bay Ventures, Inc;

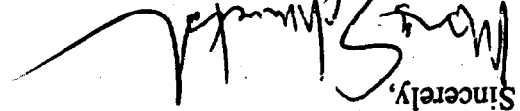
As you know, at its May 17, 2001 meeting, the Board of Appeals voted 4-1 to grant your Conditional Use Appeal. Enclosed please find a copy of the Board's decision.

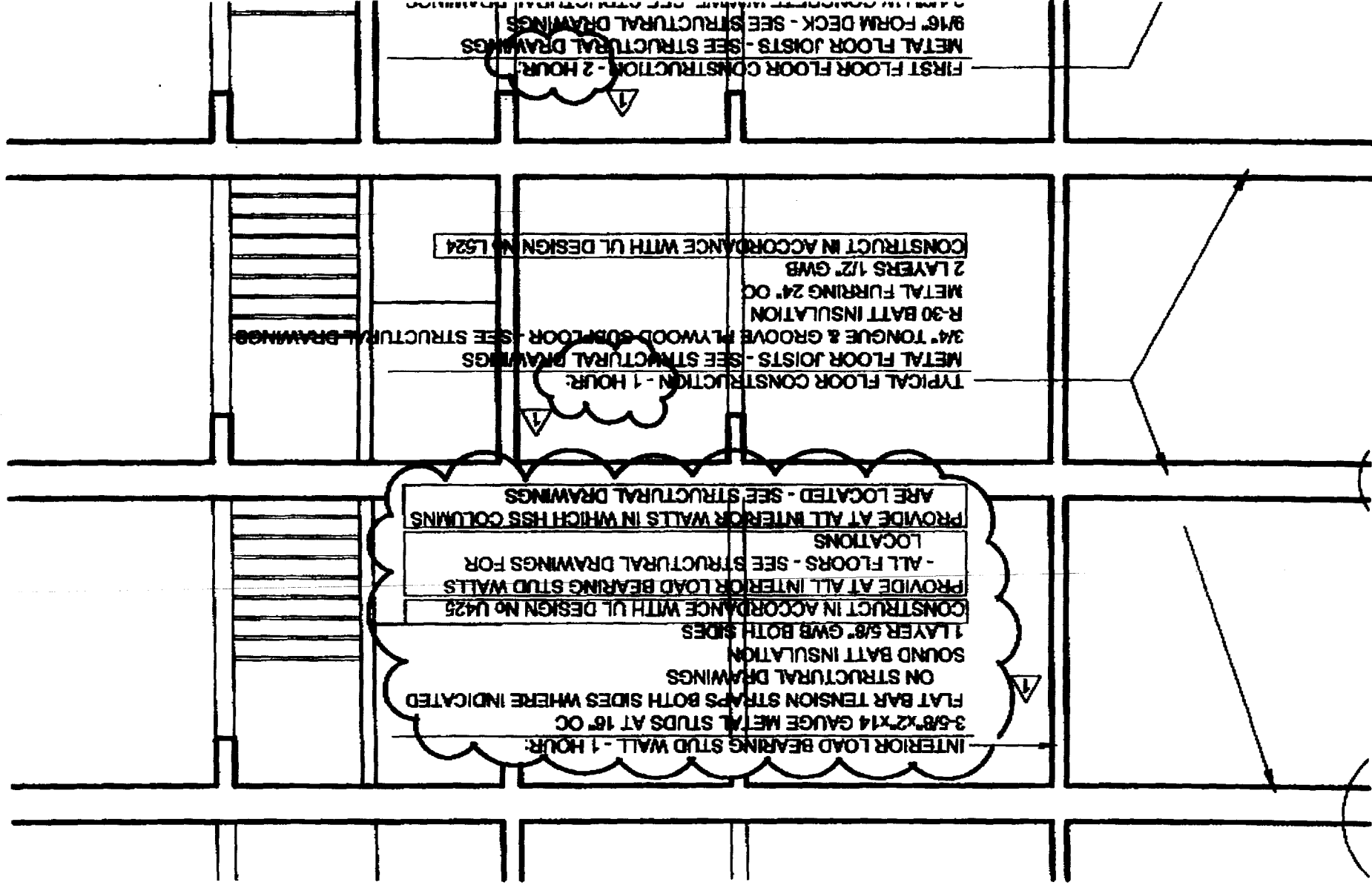
Should you have any questions please do not hesitate to contact Jodine Adams, Office Manager or myself.

M/S/ja  
Enclosure

Zoning Administrator

Marge Schmuckal

Sincerely,  




3

TOTAL P.09

7c

WILL  
TSH ARCHITECTS  
FAX 207 773 0194

In our professional opinion, these test results indicate the rail sections outlined above performed in accordance with test criteria for profiles outlined in SBCCI-99, BOCA-99, ASCE 7-95, IBC-00 and UBC-97.

6.0 Conclusion

12' PVC Rail	0.75"	0.00"	0.5"	0.00"	n/a	n/a	1.00"	0.38"
Test Identification	Maximum	Permanent	Maximum	Permanent	Maximum	Permanent	Maximum	Permanent
First Test	Second Test	Third Test	Fourth Test					

5.0 Deflection Data

12' PVC Rail	Compliant	Compliant	Compliant	Compliant
Test Identification	First Test	Second Test	Third Test	Fourth Test

4.0 Test Results

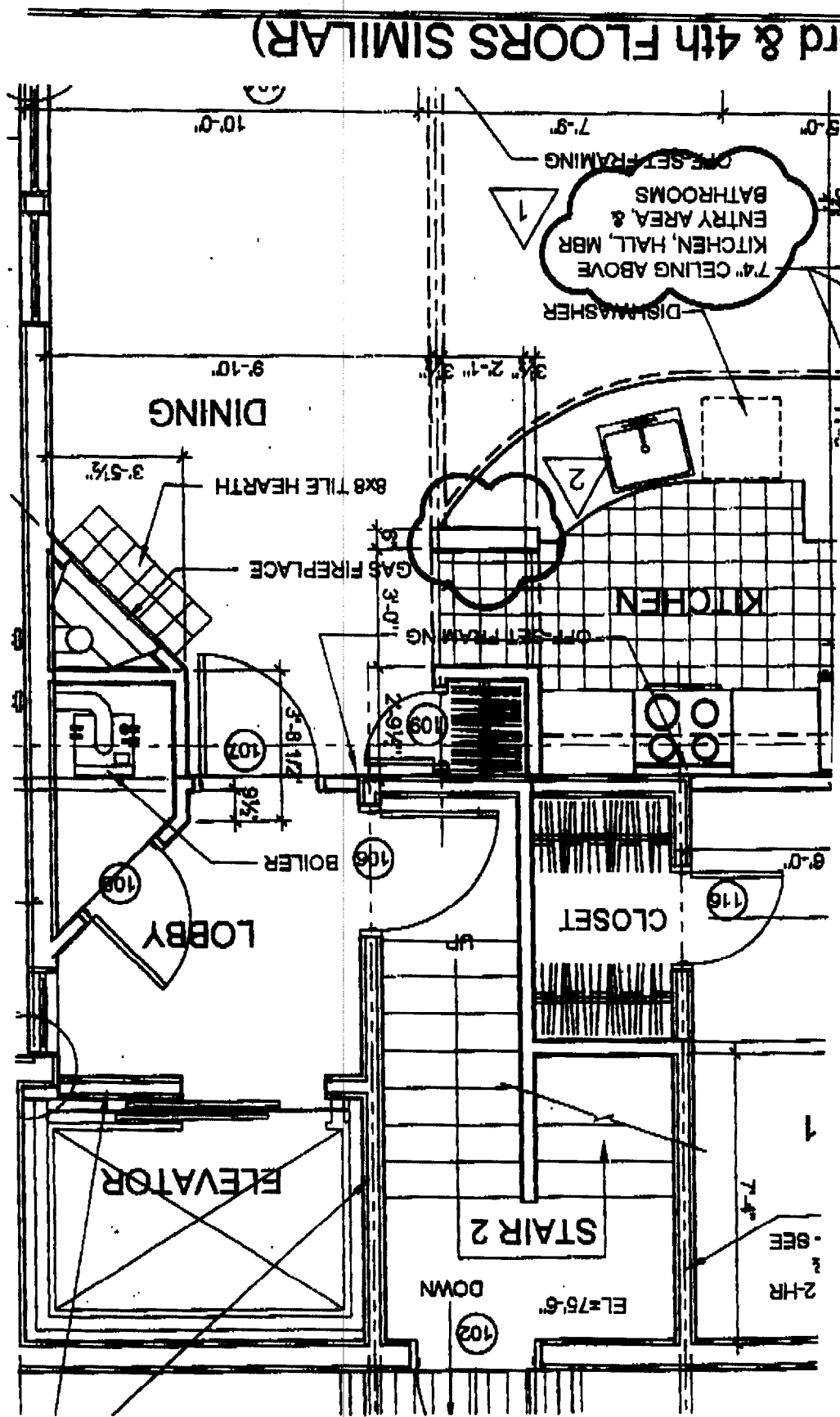
The fourth test was performed on two 4"x4" aloual wooden posts anchored to the wood deck according to criteria in SBCCI-99 section 1608.2.2.4 which states: "Loading conditions shall not be applied to produce maximum stress in each of the respective components or any of the supporting components." The wood posts anchors was performed by cutting a 4"x4" hole in the wood deck, passing the post through the hole and bolting it to the 2"x6" wood joists supporting the deck structure. Testing consisted of applying a 400-pound load at 42-inches and 36-inches from the face of the wood deck. The force was applied at a 90-degree angle with a 2-inch strap connected in line with a load application device and calibrated load cell. As the load was applied, there was visible deformation of the deck system, but there was no cracking or breaking at attachment points or separation of the mounting fixture. Upon removal of the load, the post returned to its original shape without permanent deformation.

Crane Industries  
Report No. 10285  
Page 3 of 3

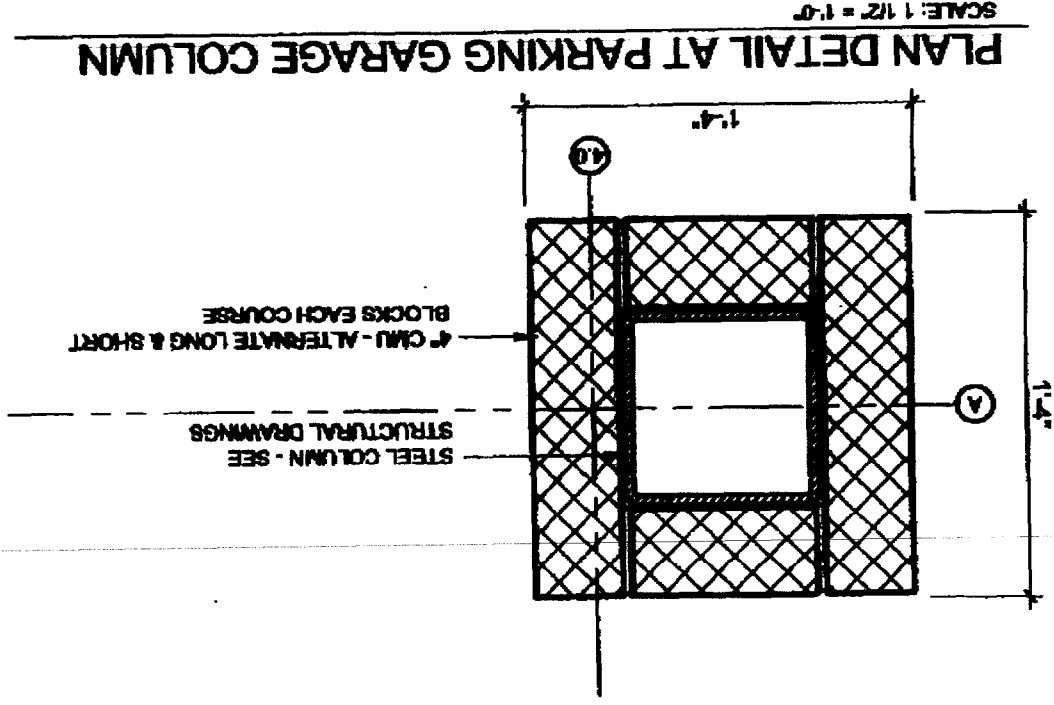
4



MODIFICATIONS SUMMARY  
△ CEILING HEIGHTS IN BATHROOMS  
REDUCED  
△ COLUMN REPLACED WITH 6" STUD  
WALL TO ACCOMMODATE PLUMBING



rd & 4th FLOORS SIMILAR)







170-U.S. Route One  
 Falmouth, Maine 04106  
 Tel: (207) 781-5242  
 Fax: (207) 781-4245

## FAX MEMORANDUM

**TO:** Will Tinkelenberg, TFH Architects

**FAX #:** 773-0194

**FROM:** Tim Dean

**DATE:** August 14, 2003

**RE:** Waterville Street

**FILE:** 02205

**# of Pages (including this one):** 1

In response to the memo from Mike Nugent, the following items address the structural comments:

5. The design is in accordance with section 7.3 of ASCE 7.
6. My calculations have been done for a roof thermal factor of 1.1 but the drawings indicate it to be 1.0. I will revise drawing S1 to include this correction. The ground snow load for this area in 1989 BOCA is 50 not 60 psf, I will correct this also.
7. The C<sub>s</sub> factor for roof slope is 1.0 for this project. The sloped roof snow load is the same as the flat roof snow load.
8. The seismic soil profile type is S<sub>1</sub>.
9. The basic structural system is a load bearing wall system. The seismic resisting system is light frame walls with shear panels.
10. The seismic analysis procedure is the equivalent lateral force procedure per section 1610.4.
11. The note regarding nailing on S6 will be revised to read "1999 BOCA".
12. The roof framing is 2x10 rafters at 16" oc as per note 1 on the roof framing plan.

**METALS & MATERIALS ENGINEERS**

1039 Industrial Court • Suwanee, Georgia 30024 • (678) 738-2000 • (678) 482-9677 • www.mme.com

**PRODUCT TESTING REPORT**

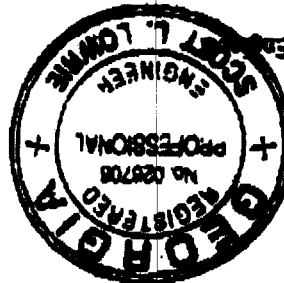
PROJECT:	Cross Industries	JOB NO.:	3021
CLIENT:	Cross Industries	LAB NO.:	10285
SAMPLE ID:	Various Quertal Systems & Components	TEST DATE:	July 26, 2001
SPECIFICATION:	IBCCI BOCA IBC USC	TOTAL PAGES:	3

Load Testing of Several Quertal Systems to the Standard Building Code Congress International (SBCCI), Building Officials Code Administration (BOCA), International Building Code (IBC) and Uniform Building Code (UBC) Specifications

Prepared for:

Cross Industries  
6685 Jimmy Carter Blvd.  
Norcross, GA 30071

Submitted By:



Scott Lomnie, PE  
Chief Product Analysts Eng

Reviewed By:

Ralph Bowman, PE  
Senior Metallurgist

MME, Inc. data, conclusions, engineering opinions and related items are the property of MME, Inc. This report, in its entirety, is the property of the client to whom it is addressed. Permission is granted to reproduce this report, provided it is reproduced in its entirety. The use of MME, Inc. or MME personnel/qualifications in any advertising or related materials requires prior written approval from MME, Inc. Reports apply only to samples tested at the time of testing and are not necessarily indicative of the quality of related or unrelated items not produced at the same time and from the same material batch.

Cross Industries  
 Report No 10285  
 Page 2 of 3

1.0 Introduction

Load testing, as requested by Cross Industries, was performed at their facility in Norcross, Georgia by the technical staff of Materials & Materials Engineers, LLC (MMER) on one 12-foot section of guardrail, constructed using the standard fabrication methods for PVC railing.

2.0 Test Samples

Standard attachments are two 1 1/2" x 5 1/2" PVC railers routed to receive top and bottom rails. The railers are attached to the wood column with four 2.5-inch deck screws per railer.

3.0 Test Procedure

MMER, Inc. performed three testing procedures on each rail system in accordance with SBCCI, BOCA and IBC specifications. For each test, the most stringent requirement from the following building codes was selected:

Building Code	Year	Applicable Section	Safety Factor
BOCA	1999	1606.4	2.5
ASCE 7	1995	4.4.2A	2.5
IBC	2000	1607.7.1	2.5
UBC	1997	Table 3	2.0
SBCCI	1999	1608.2.2	2.0

The method of test procedure selection corresponds to the following overall procedure:

The first test was performed according to criteria in the IBC-00 section 1607.7.1.1, BOCA-99 section 1606.4 and the ASCE 7-95 section 4.4.2.A which states: "Guardrail systems shall be able to resist a single concentrated load of 200 lb. applied in any direction at any point along the top of the rail." Testing consisted of applying a horizontal 500-pound load at one position at the midpoint of the rail on each guardrail. The force was applied at a 85-degree angle using a force-application device in line with a calibrated load cell. As the force was applied, there was visible bowing of the rails at their centers but no separation of material from grooves or joints and no cracking or breaking at attachment points. When the loads were released, the guardrails returned to their original configuration without excessive permanent deformation. Deflection data is given at the end of this report.

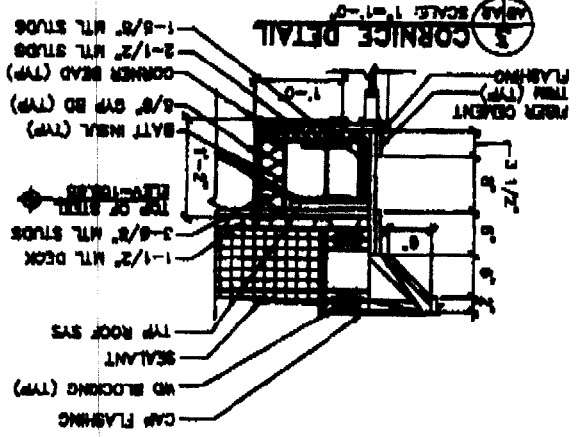
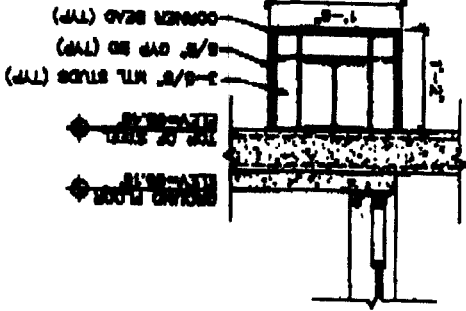
The second test was performed according to criteria in SBCCI-99 section 1608.2.2.2 which states: "Guardrail systems shall be designed and constructed for a load of 50 pounds/foot (7.10N/m) applied horizontally at the required guardrail height and a simultaneous load of 100 pounds/foot (1459 N/m) applied vertically downward at the top of the guardrail." Testing consisted of applying a 200 pound/foot vertical load utilizing 60-pound and 120-pound lead weights placed on a rigid bar taped to the top rail of the guardrail section. Simultaneously, a 100-pound/foot horizontal force was applied at the top of the guardrail along its entire length through a rigid 2" x 2" metal beam using a force application device in line with a calibrated load cell. For each test, there was visible vertical and horizontal bowing of the guardrails along their length but no separation of material from grooves or joints and no cracking or breaking at attachment points. When the loads were released, the guardrails returned to their original configuration without excessive permanent deformation. Deflection data is given at the end of this report.

The third test was performed on both the square baluster and colonial baluster according to criteria in SBCCI-99 section 1608.2.2.2 which states: "The guardrail system shall also be designed and constructed to resist a 200-pound (890N) concentrated horizontal load applied on a 1-foot square area at any point in the system including intermediate rails or other elements serving the purpose." Testing consisted of applying a 500-pound load at the center point between the top and bottom rails. The force was applied at an 80-degree angle with a 1-foot square bracket connected in line with a load application device and calibrated load cell. As the load was applied, there was visible deformation of the pickets, but there was no cracking or breaking at attachment points or separation of square tubing from grooves or joints. Upon removal of the load, the pickets returned to their original shape without permanent deformation.

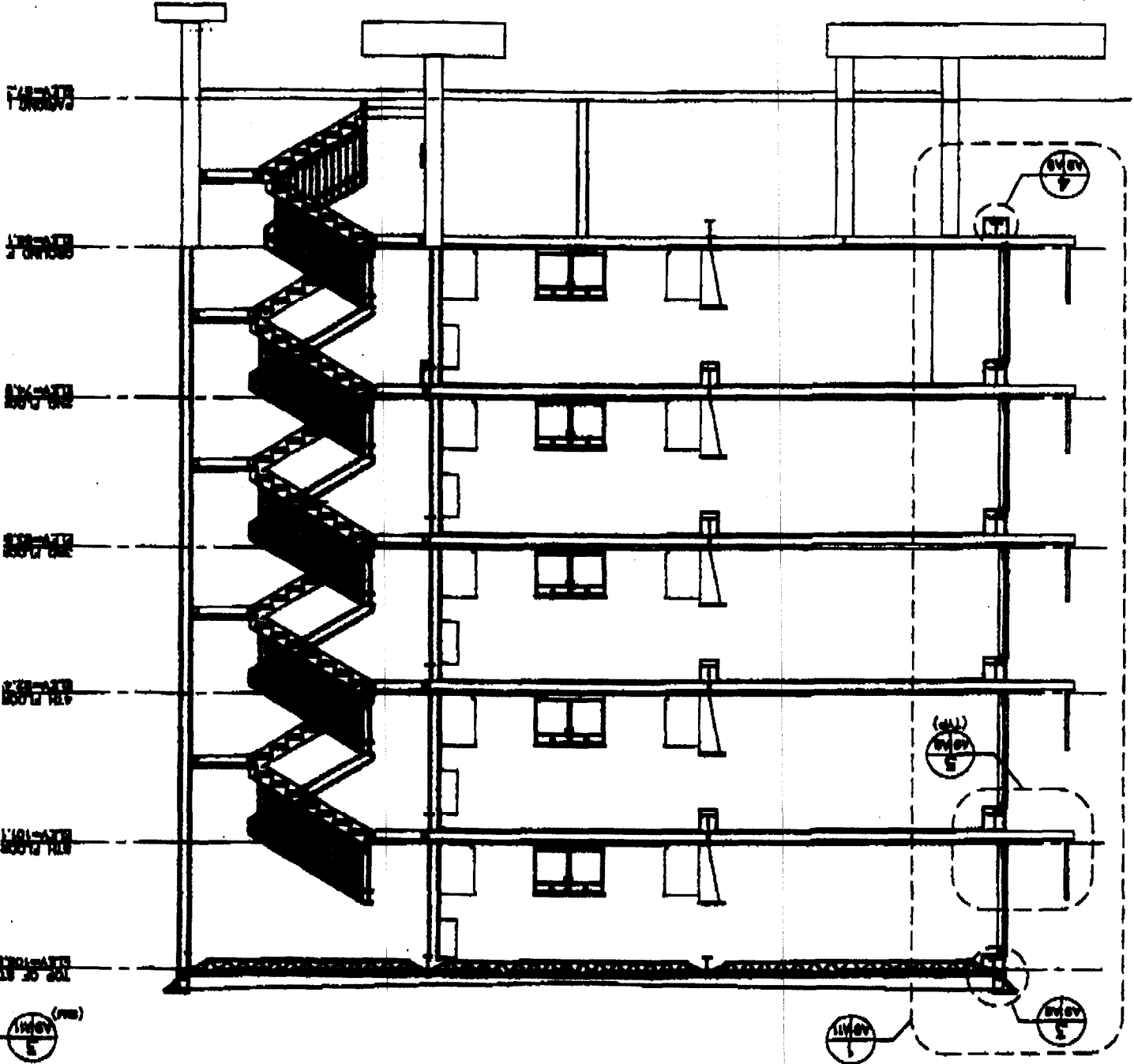
78

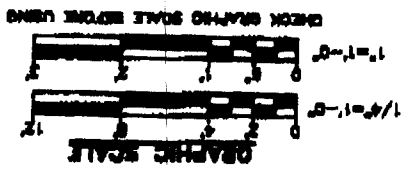
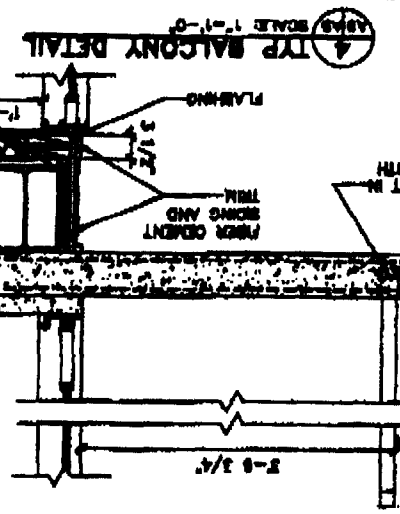
DATE/ANZ IN  
FLANK COOR  
PLANK MARK

### 4 BEAM JOINT DETAIL



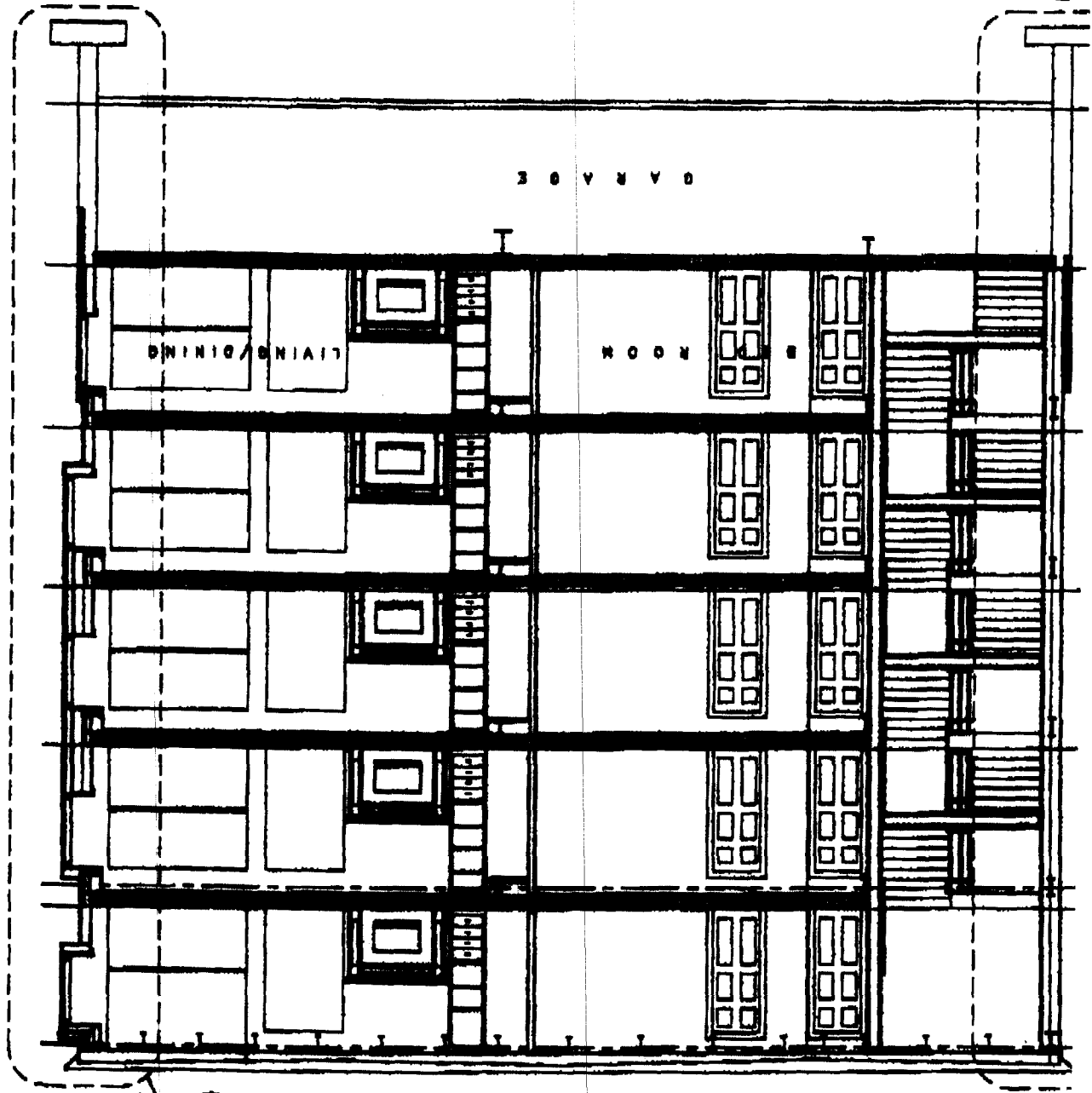
### 1 BUILDING SECTION





GENERAL NOTE:  
 HANDMADE SIZE, LOCATION AND CONSTRUCTION  
 TO BE DETERMINED BY OTHERS. COORDINATE  
 WITH CONTRACTOR AND PLUMB MANUFACTURER.

2 BUILDING SECTION  
 1/4\"/>



15 OF 28  
 A9

BUILDING SECTIONS

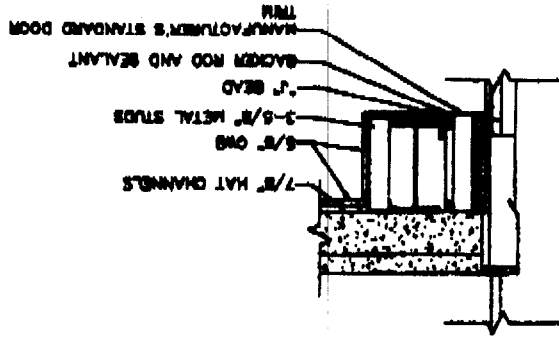
DATE	BY/DATE
ISSUED	...
REVISION	...
CHECKED	...
SCALE	...
NO.	...

5 UNIT CONDOMINIUMS  
 20-24 BRACKET STREET  
 PORTLAND, MAINE  
 RECORD OWNER  
 FEDERAL, LLC  
 20 CHESTNUT STREET, OLD ORCHARD BEACH, MAINE 04064

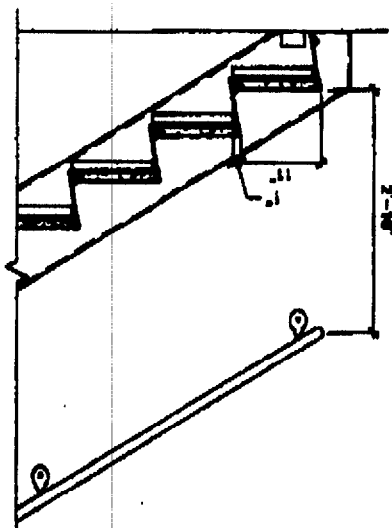
...

OAK POINT ASSOCIATES  
 100 WEST CHESTNUT STREET, SUITE 200  
 PORTLAND, MAINE 04101  
 TEL: 603.733.1111  
 FAX: 603.733.1112

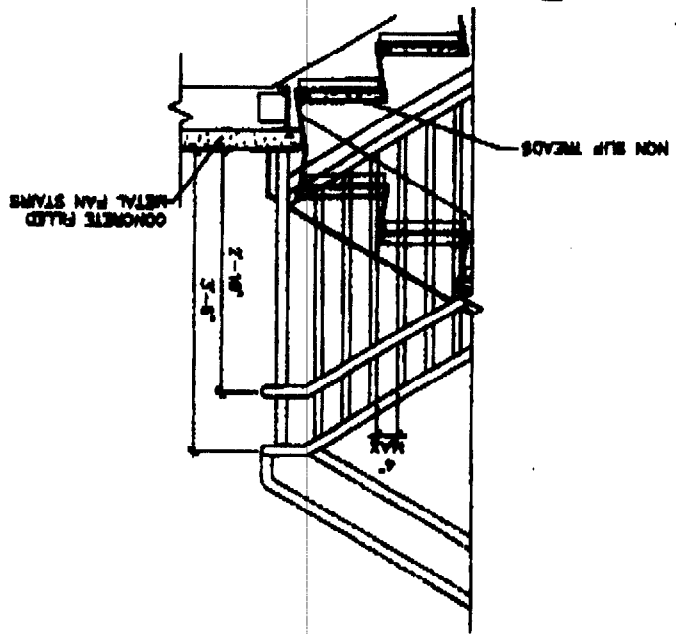
6 TYPICAL STAIR DETAIL  
ASHP SCALE 1"=1'-0"



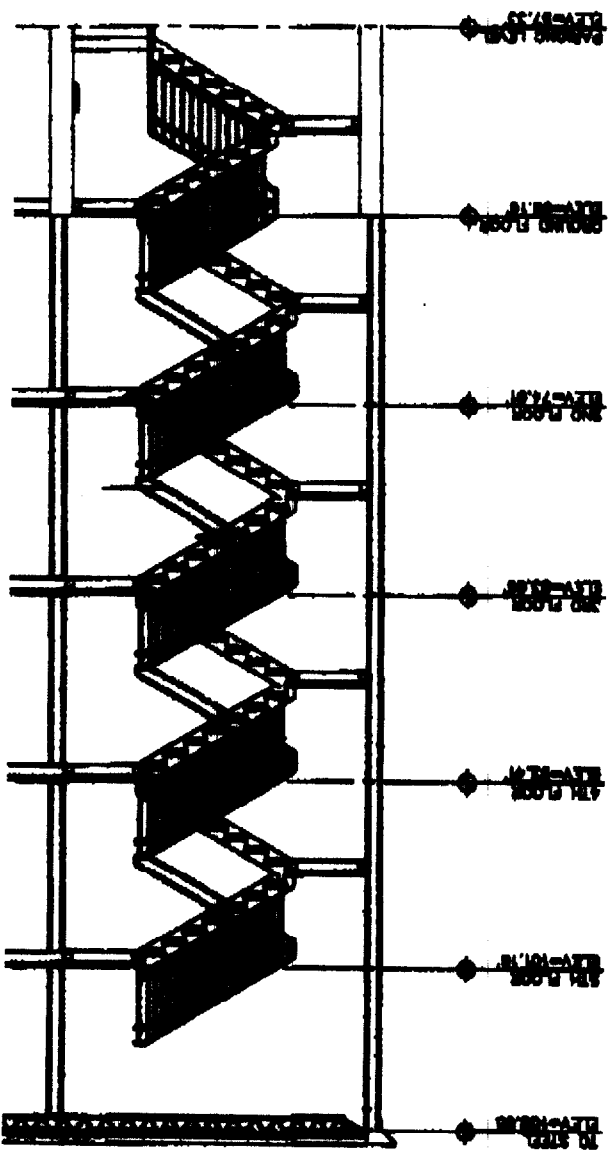
2 TYPICAL STAIR DETAIL  
ASHP SCALE 1"=1'-0"



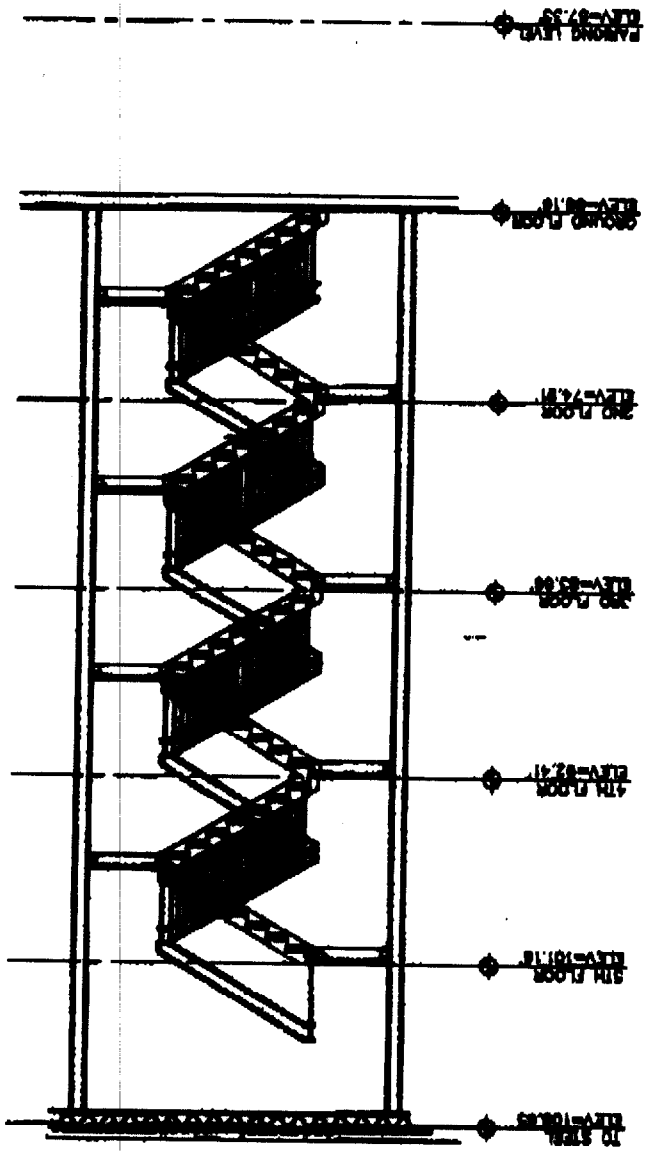
1 TYPICAL HAND RAIL DETAIL  
ASHP SCALE 1"=1'-0"



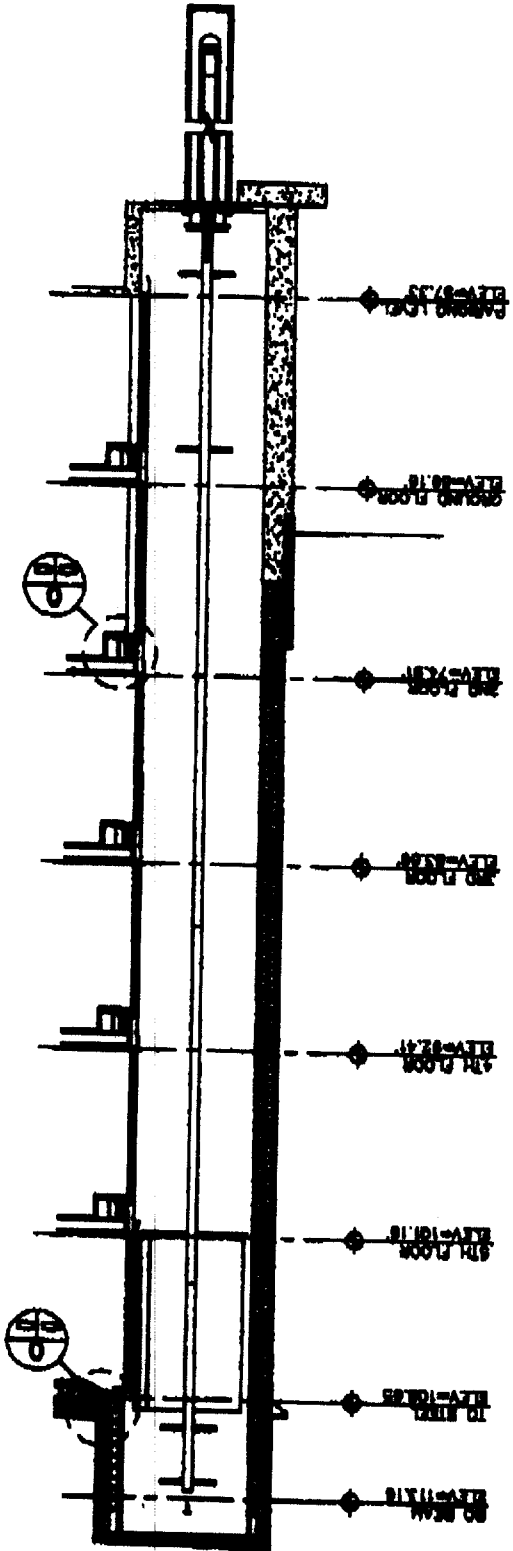
3 STAIR #1 SECTION  
ASHP SCALE 1/8"=1'-0"



4 STAIR #2 SECTION  
AS SHOWN SCALE 1/4"=1'-0"



5 ELEVATOR SECTION  
AS SHOWN SCALE 1/4"=1'-0"



OAK POINT ASSOCIATES

08/15/2003 12:04 12072834303

STAIR & ELEVATOR  
SECTIONS & DETAILS

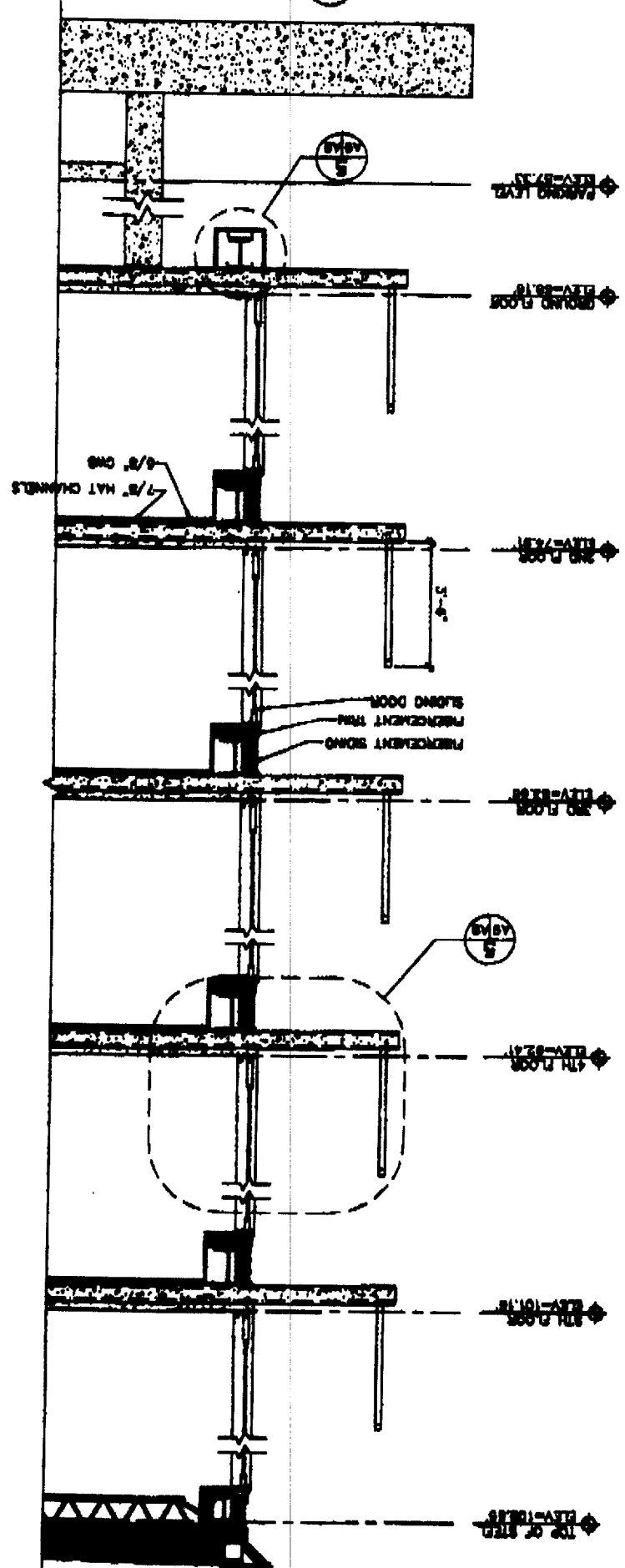
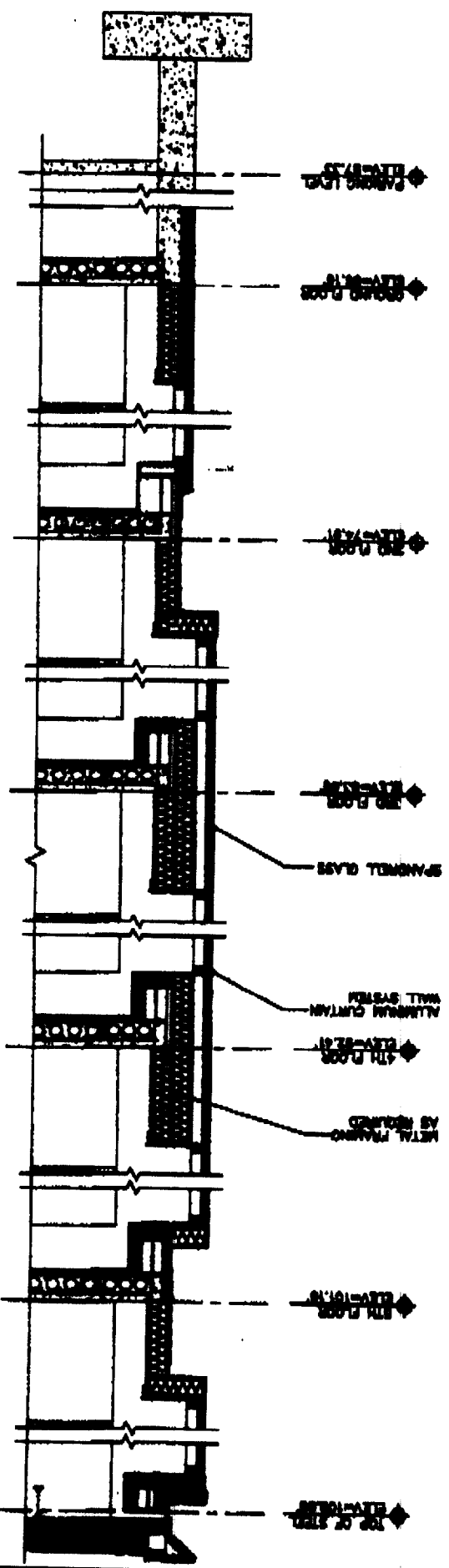
A10

18 OF 29

DATE	BY/DATE
ISSUED FOR	
REVISION	
NO.	DATE
BY	FOR
DATE	BY
NO.	DATE

**5 UNIT CONDOMINIUMS**  
20-24 BRACKETT STREET  
PORTLAND, MAINE  
RECORD OWNER  
KERRON, LLC  
20 ORCHARD STREET, 2ND FLOOR, PORTLAND, MAINE 04104

**OAK POINT ASSOCIATES**  
ARCHITECTS - ENGINEERS  
20 SUN STREET, PORTLAND, MAINE 04104





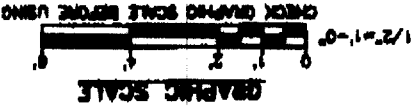
17 OF 29  
A11

WALL SECTIONS

DATE:	07/24/09
DESIGNER:	BAI
CHECKER:	BAI
SCALE:	AS SHOWN
JOB:	080433

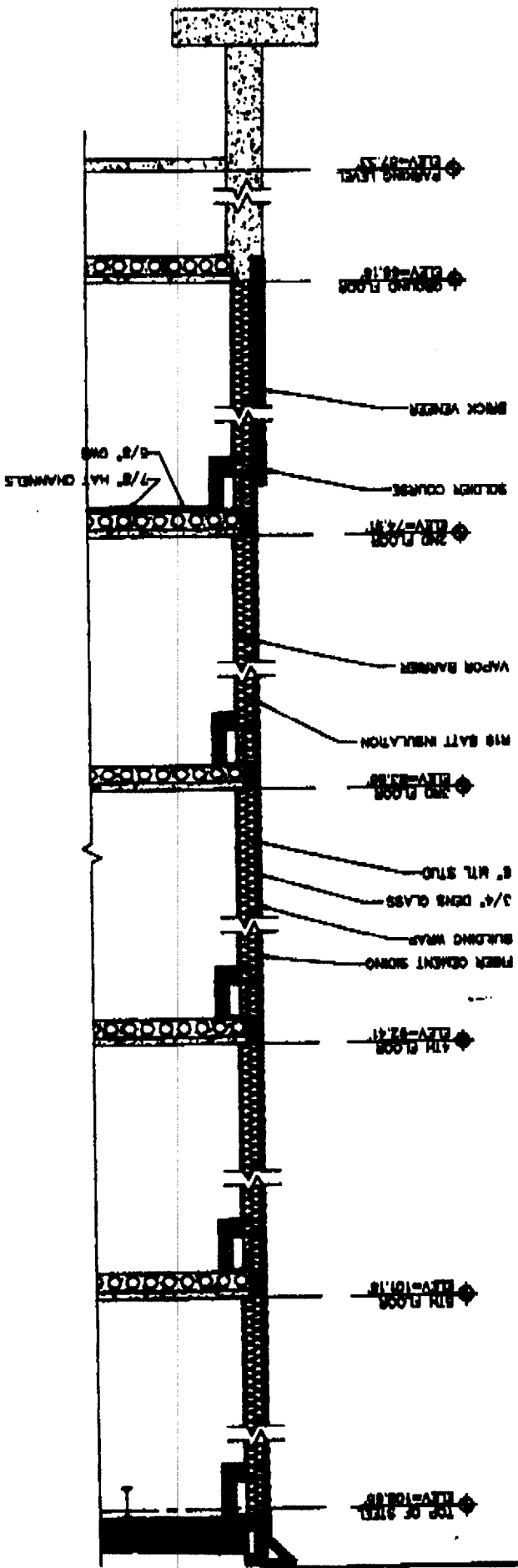
**5 UNIT CONDOMINIUMS**  
 20-24 BACCHETTI STREET  
 PORTLAND, MAINE  
 ESCROW OWNER:  
 WENDEL, LLC  
 28 CHESTNUT STREET, OLD BRIDGEWAY BEACH, MAINE 04064

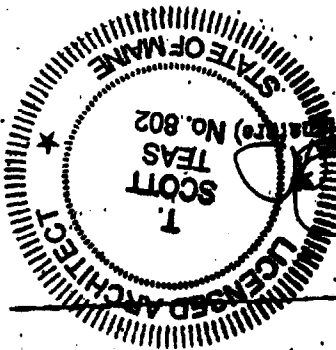
**DAK POINT ASSOCIATES**  
 100 MAIN STREET, PORTLAND, MAINE 04101



- GENERAL NOTE:**
- HANDRAIL SIZE, LOCATION AND CONSTRUCTION TO BE DETERMINED BY OTHERS. COORDINATE SLEEVE LOCATION AND FALLING CONSTRUCTION WITH CONTRACTOR AND PLANK MANUFACTURER.
  - CURTAIN WALL DESIGN BY OTHERS. CURTAIN WALL SYSTEM SHALL BE DESIGNED TO ACCOMMODATE WINDOW AND DOOR OPENINGS AS WELL AS ORIEL FRAMING WITH STRUCTURAL STEEL.

**5 WALL SECTION**  
 SCALE: 1/2"=1'-0"





(Designer Stamp & Signature) No. 802  
 T. SCOTT TEAS  
 LICENSED ARCHITECT  
 STATE OF MAINE

List Occupant loading for each room or space, designed into this Project.  
 7 Units x 3 Bed Rooms x 2 Occupants = 24  
 PER 1008.1.2. OCCUPANT LOAD = 43  
 (-Living: 1918 SF/300 = 7  
 -Upper Floors: 7125 SF/200 = 36)  
 DESIGN OCCUPANT LOAD = 43

If mixed use, what subsection of 313 is being considered? 313.1.2.  
 Is structure being considered unlimited area building? Yes  No   
 Structure has full sprinkler system? Yes  No   
 Alarm System? Yes  No   
 Sprinkler & Alarm systems must be installed according to BOCA and NFPA Standards with approval from the Portland Fire Department.

Floor Live Load Per Sq. Ft. 40  
 Basic Wind Speed (mph) 85  
 Roof Snow Load Per Sq. Ft. 42  
 Dead Load Per Sq. Ft. 20  
 Seismic Zone NA  
 Group Class II  
 Type of Construction 3A  
 Bldg. Height 45'  
 Bldg. Sq. Footage 9,093  
 Building Code and Year BOCA NBC 1999 Use Group Classification(s) R-2/5-2  
 Construction project was designed according to the building code criteria listed below.

THE BOCA NATIONAL BUILDING CODE/1999 FORTLEIGH EDITION

Address of Construction: 3 Waterville Street, Portland  
 Job Name: Waterville Street Condominiums - Building "C"  
 DATE: AUGUST 6, 2003

FROM DESIGNER: T. Scott Teas, NARB, AIA  
TFH ARCHITECTS  
 Inspector of Buildings City of Portland, Maine  
 Planning & Urban Development  
 Division of Housing & Community Services

CITY OF PORTLAND MAINE  
 389 Congress St., Rm 315  
 Portland, ME 04101  
 Tel. - 207-874-8704  
 Fax - 207-874-8716

