

# Single 5-1/4" x 7" VERSA-LAM® 1.8 2750 SP

BC CALC® Design Report



Dry | 7' 0" Column Freestanding

September 8, 2017 14:11:02

Build 6080

Job Name:

20' RIDGE BEAM-LEFT SIDE

Address:

5 Monument Street City, State, Zip: PORTLAND, ME

Customer:

HANCOCK LUMBER

Code reports: ESR-1040

File Name: BC CALC Project Description: Designs\CL01

Specifier:

**GUY DOYON** Designer:

Company: HANCOCK LUMBER COMPANY

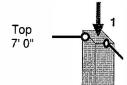
Misc:

Column

5.25" Freestanding

				FIAG	Dead	SHOW WANTE	HOU! LIVE
<b>Load Summary</b>		F:					
Tag Description	Load Type	Start	End	100%	90%	115% 160%	125%
1	Conc Pt (lbs)	00-00-00	00-00-0	0.2.000	2 320	11 960	

Bracing	Elevation	Sheathing
Гор	07-00-00	
Base	00-00-00	



Controls Summary	Value	% Allowable	Duration	Load Case
Axial Compression	n/a	15.9%	115%	2
Sienderness Batio	16	32%	n/a	n O

#### **Cautions**

Design does not consider perpendicular to grain stress on the sill plate or other supporting member.

A generic column cap was used in the analysis of the column. Make sure to install and size the cap. BC Calc does not perform shear wall or connection design for in-plane load transfer.

#### **User Notes**

**COLUMN WHERE 2 RIDGES JOIN AT WALL** 

## **Disclosure**

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

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Not to scale



# Triple 1-3/4" x 16" VERSA-LAM® 2.0 3100 SP

Roof Beam\RB02

Dry | 1 span | No cantilevers | 0/12 slope

September 8, 2017 14:02:05

BC CALC® Design Report

Report

Build 6080

Job Name: 20' RIDGE BEAM-LEFT SIDE

Address: 5 Monument Street ER-C City, State, Zip: PORTLAND, ME

Customer: HANCOCK LUMBER

Code reports: ESR-1040

File Name: BC CALC Project Description: Designs\RB02

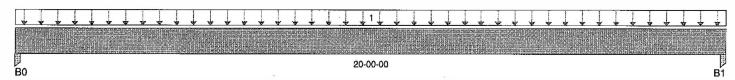
Specifier:

Designer: GUY DOYON

Company: HANCOCK LUMBER COMPANY

Misc:

12



Total Horizontal Product Length = 20-00-00

Reaction Summary	(Down / Uplift) (lbs)				75.	=
Bearing	Live	Dead	Snow	Wind	Roof Live	
B0, 5-1/8"		1,243 / 0	8,000 / 0			
B1, 5-1/8"	(4	1,243 / 0	8,000 / 0		*	

				Live	Dead	Snow	Wind Roof Li	ve Trib.
Load Summary								
•	Load Type F	Ref. Start	End	100%	90%	115%	160% 125%	
1 Standard Load l	Unf. Area (ib/ft^2) L	_ 00-00-00	20-00-00		10	80		10-00-00

Controls Summary	Value	% Allowable	Duration	Case	Location = 12
Pos. Moment	42,908 ft-lbs	66.6%	115%	4	10-00-00
End Shear	7,616 lbs	41.5%	115%	4	01-09-02
Total Load Defl.	L/289 (0.8")	62.3%	n/a	4	10-00-00
Live Load Defl.	L/334 (0.693")	71.9%	n/a	5	10-00-00
Max Defl.	0.8"	80%	n/a	4	10-00-00
Span / Depth	14.5	n/a	n/a	0	00-00-00
Squash Blocks	Valid			_	

_	_	25		% Allow	% Aflow	
<u>Beari</u>	ng Supports	Dim. (L x W)	Value	Support	Member	Material
B0	Post	5-1/8" x 5-1/4"	9,243 lbs	11.5%	45.8%	Versa-Lam 1.7
B1	Post	5-1/8" x 5-1/4"	9,243 lbs	11.5%	45.8%	Versa-Lam 1.7

## **Cautions**

For roof members with slope (1/4)/12 or less final design must ensure that ponding instability will not occur.

For roof members with slope (1/2)/12 or less final design must account for Rain-on-Snow surcharge load.

## **Notes**

Design meets Code minimum (L/180) Total load deflection criteria. Design meets Code minimum (L/240) Live load deflection criteria. Design meets arbitrary (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Design based on Dry Service Condition.



Build 6080

Job Name:

# Triple 1-3/4" x 16" VERSA-LAM® 2.0 3100 SP

## Roof Beam\RB02

Dry | 1 span | No cantilevers | 0/12 slope

September 8, 2017 14:02:05

BC CALC® Design Report

20' RIDGE BEAM-LEFT SIDE

Address: 5 Monument Street City, State, Zip: PORTLAND, ME

HANCOCK LUMBER Customer:

Code reports: ESR-1040 File Name: BC CALC Project Description: Designs\RB02

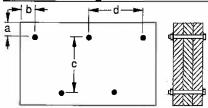
Specifier:

Designer: **GUY DOYON** 

HANCOCK LUMBER COMPANY Company:

Misc:

## Connection Diagram



a minimum = 2" c = 12"b minimum = 2-1/2"d = 24"

Bolts are assumed to be Grade A307 or Grade 2 or higher.

Member has no side loads.

Connectors are: 1/2 in. Staggered Through Bolt

#### **Disclosure**

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## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP

Roof Beam\RB01

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope

September 8, 2017 13:57:17

Wind Roof Live

Trib.

**Build 6080** 

Duliu 0000

Code reports:

Job Name: 9' RIDGE BEAM-RIGHT SIDE

Address: 5 Monument Street
City, State, Zip: PORTLAND, ME
Customer: HANCOCK LUMBER

ESR-1040

File Name: BC CALC Project Description: Designs\RB01

Specifier:

Designer: GUY DOYON

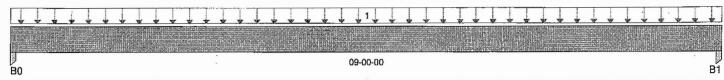
Dead

Live

Company: HANCOCK LUMBER COMPANY

Misc:

12



Total Horizontal Product Length = 09-00-00

Reaction Summary (	Down / Uplift) (lbs)				110000	
Bearing	Live	Dead	Snow	Wind	Roof Live	
B0, 3-1/2"		538 / 0	3,960 / 0		72	
B1, 3-1/2"		538 / 0	3,960 / 0			

Lo	ad Summary										
Tag	Description		Load Type	R	ef. Start	End	100%	90%	115%	160% 125%	
1	Standard Load	48	Unf. Area (lb/ft^2)	L	00-00-00	09-00-00		10	80		11-00-00

<b>Controls Summary</b>	Value	% Allowable	Duration	Case	Location
Pos. Moment	9,117 ft-lbs •	56.8%	115%	4	04-06-00
End Shear	3,415 lbs	47%	115%	4	01-01-00
Total Load Defl.	L/428 (0.239")	42%	n/a	4	04-06-00
Live Load Defl.	L/486 (0.211")	49.3%	n/a	5	04-06-00
Max Defl.	0.239"	23.9%	n/a	4	04-06-00
Span / Depth	10.8	n/a	n/a	0	00-00-00
Squash Blocks	Valid				

Beari	ng Supports_	Dim. (L x W)	Value	% Allow Support	% Allow Member	Material
B0	Post	3-1/2" x 3-1/2"	4,498 lbs	n/a	49%	Unspecified
B1	Post	3-1/2" x 3-1/2"	4,498 lbs	n/a	49%	Unspecified

#### Cautions

For roof members with slope (1/4)/12 or less final design must ensure that ponding instability will not occur.

For roof members with slope (1/2)/12 or less final design must account for Rain-on-Snow surcharge load.

### Notes

Design meets Code minimum (L/180) Total load deflection criteria. Design meets Code minimum (L/240) Live load deflection criteria. Design meets arbitrary (1") Maximum Total load deflection criteria. Calculations assume member is fully braced. Design based on Dry Service Condition.



# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP

Roof Beam\RB01

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope

September 8, 2017 13:57:17

**Build 6080** 

Job Name:

9' RIDGE BEAM-RIGHT SIDE

Address: 5 Monument Street
City, State, Zip: PORTLAND, ME
Customer: HANCOCK LUMBER

Code reports: ESR-1040

File Name: BC CALC Project

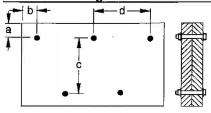
Description: Designs\RB01 Specifier:

Designer: GUY DOYON

Company: HANCOCK LUMBER COMPANY

Misc:

## **Connection Diagram**



a minimum = 2" c = 5-1/2" b minimum = 2-1/2" d = 24"

Bolts are assumed to be Grade A307 or Grade 2 or higher.

Member has no side loads.

Connectors are: 1/2 in. Staggered Through Bolt

## **Disclosure**

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**Build 6080** 

Job Name:

Address:

# Triple 2 x 8 SPF #2 Dry | 1 span | No cantilevers | 0/12 slope

Wall Header\FB02

September 8, 2017 13:44:22

BC CALC® Design Report



6'-4" HEADER FOR WINDOWS 5 Monument Street City, State, Zip: PORTLAND, ME

HANCOCK LUMBER Customer:

Code reports: **NLGA** 

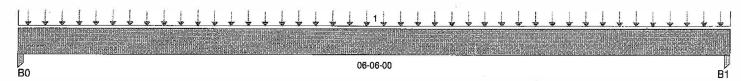
File Name: BC CALC Project Description: Designs\FB02

Specifier:

Designer: **GUY DOYON** 

HANCOCK LUMBER COMPANY Company:

Misc:



Total Horizontal Product Length = 06-06-00

Reaction Summary (Down / Uplift) (lbs)							
Bearing	Live	Dead	Snow	Wind	Roof Live		
B0, 3-1/2"	30/1	317 / 0	2,340 / 0		40		
B1, 3-1/2"		317 / 0	2.340 / 0				

				Live	Dead	Snow	Wind Roof Live	Trib.
Load Summary	Lond Time	Def Steet	F	1000/	000/	4450/	1000/ 4850/	
Tag Description	Load Type	Ref. Start	End	100%	90%	115%	160% 125%	
1 Standard Load	Unf. Area (lb/ft^2)	L 00-00-00	06-06-00	<del></del>	10	80		09-00-00

<b>Controls Summary</b>	Value	% Allowable	Duration	Case	Location
Pos. Moment	3,730 ft-lbs	94%	115%	1	03-03-00
End Shear	1,924 lbs	57%	115%	1	00-10-12
Total Load Defl.	L/999 (0.122")	n/a	n/a	1	03-03-00
Live Load Defl.	L/999 (0.108")	n/a	n/a	2	03-03-00
Max Defl.	0.122"	n/a	□ n/a	1	03-03-00
Span / Depth	10	n/a	n/a	0	00-00-00
Squash Blocks	Valid				

Beari	ng Supports	Dim. (L x W)	Value	% Allow Support	% Allow Member	Material
B0	Post	3-1/2" x 3-1/2"	2,657 lbs	n/a	51%	Unspecified
B1	Post	3-1/2" x 3-1/2"	2,657 lbs	n/a	51%	Unspecified

## **Cautions**

Member is not fully supported at post B0. A connector is required at this bearing. Member is not fully supported at post B1. A connector is required at this bearing.

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets arbitrary (0.5") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Design based on Dry Service Condition.

The analysis of solid sawn wood members is in accordance with the NDS and is limited to the output shown above. All other support and design for these products, including but not limited to notching, connections, installation, and engineer/architect certification is the responsibility of the project's design professional of record.

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