

DISPLAY THIS CARD ON PRINCIPAL FRONTAGE OF WORK CITY OF PORTLAND

BUILDING INSPECTION

PERMIT

Permit Number: 031195

Please Read Application And Notes, if Any, Attached

This is to certify that Promenade East/TBD Construction

has permission to Install 3 Antennas & Base Station Equipment for 4G LTE Network

AT 320 Eastern Promenade 015 A002001

provided that the person or persons, firm or corporation accepting this permit shall comply with all of the provisions of the Statutes of the State and of the Ordinances of the City of Portland regulating the construction, maintenance and use of buildings and structures, and of the application on file in this department.

Apply to Public Works for street line and grade if nature of work requires such information.

Notification inspection must be given and when permission procured before this building or part thereof is leased or otherwise used-in. HOUR NOTICES REQUIRED.

A certificate of occupancy must be procured by owner before this building or part thereof is occupied.

OTHER REQUIRED APPROVALS

Fire Dept. UHM

Health Dept. _____

Appeal Board _____

Other _____

Department Name

Alvin King 10/17/23
Director - Building & Inspection Services

PENALTY FOR REMOVING THIS CARD

City of Portland, Maine - Building or Use Permit

389 Congress Street, 04101 Tel: (207) 874-8703, Fax: (207) 874-8716

Permit No: 03-1195	Date Applied For: 09/29/2003	CBL: 015 A002001
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Location of Construction: 320 Eastern Promenade	Owner Name: Promenade East	Owner Address: 320 Eastern Promenade	Phone: () 773-5531
Business Name:	Contractor Name: TBD Construction	Contractor Address: Westview Drive Sanford	Phone: (207) 651-0408
Lessee/Buyer's Name	Phone:	Permit Type: Additions - Multi Family	

Proposed Use: Multi Family High Rise/80 Units-Promenade East w/3 antennae & base station for PCS network	Proposed Project Description: Install 3 Antennas & Base Station Equipment for PCS Network
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Dept: Zoning **Status:** Approved **Reviewer:** Marge Schmuckal **Approval Date:** 10/03/2003
Note: C of O on file for the 80 D.U. **Ok to Issue:**

Dept: Building **Status:** Pending **Reviewer:** Mike Nugent **Approval Date:**
Note: **Ok to Issue:**

Dept: Fire **Status:** Approved **Reviewer:** Lt. MacDougal **Approval Date:** 10/06/2003
Note: **Ok to Issue:**

Comments:

10/14/2003-mjn: Guard design? Roof loads, Special inspections

OEST Associates, Inc.

• engineers
• architects
• surveyors
• construction
managers

343 Gorham Road • South Portland, ME 04106-2317 • TEL (207) 761-1770 • FAX (207) 774-1246
E- mail: mail@oest.com • Web Site: www.oest.com

October 14, 2003

City of Portland
Michael Nugent
389 Congress Street
Portland Maine 04101

SUBJECT: Portland Maine Wireless Communications Projects

Dear Michael:

This letter will hopefully clear up any questions that you raised in a phone conversation with me on 14 October 2003.

Promenade East

1. The answer to the question that you raised about the railing configuration around the platform as shown on our drawings is Use group U. This allows the railing to be as shown. The whole equipment platform is enclosed in a locked wood fence compound.
2. The analysis of the existing roof for capacity is shown in the calculation package as submitted on pages 2 thru 5.

Doubletree Hotel

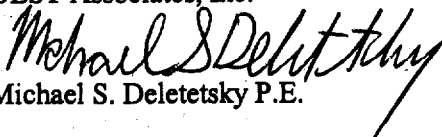
1. The analysis of the existing penthouse for capacity is shown in the calculation package as submitted on pages 4 and 5.

Deering Pavilion

1. The analysis of the existing roof for capacity is shown in the calculation package as submitted on pages 5 and 10 (Concrete Wall Design (ACI 381-99)).

The work for the field welds shall be visually inspected by an AWS Certified Weld Inspector prior to welding for all the projects that are submitted for your review. If you have any questions please feel free to call me.

Sincerely,
OEST Associates, Inc.


Michael S. Deletetsky P.E.

MSD:lam

M:\390 LCC, International\City of Portland\101403.doc

City of Portland, Maine - Building or Use Permit Application

389 Congress Street, 04101 Tel: (207) 874-8703, Fax: (207) 874-8716

Permit No: 03-1195	Issue Date:	CBL: 015 A002001
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Location of Construction: 320 Eastern Promenade	Owner Name: Promenade East	Owner Address: 320 Eastern Promenade	Phone: 773-5531
Business Name:	Contractor Name: TBD Construction	Contractor Address: Westview Drive Sanford	Phone: 2076510408
Lessee/Buyer's Name	Phone:	Permit Type: Additions - Multi Family	Zone: <i>Rb</i>

Past Use: Multi Family High Rise/80 Units- Promenade East	Proposed Use: Multi Family High Rise/80 Units- Promenade East	Permit Fee: \$516.00	Cost of Work: \$55,000.00	CEO District: 1
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Legal use: 80 D.M.

Proposed Project Description: Install 3 Antennas & Base Station Equipment for PCS Network	FIRE DEPT: <input checked="" type="checkbox"/> Approved <input type="checkbox"/> Denied	INSPECTION: Use Group: <i>1</i> Type: <i>2</i> <i>10/17/03</i>
	Signature: <i>[Signature]</i>	Signature: <i>[Signature]</i>

PEDESTRIAN ACTIVITIES DISTRICT (P.A.D.)	
Action: <input type="checkbox"/> Approved <input type="checkbox"/> Approved w/Conditions <input type="checkbox"/> Denied	Signature: _____ Date: _____

Permit Taken By: gad	Date Applied For: 09/29/2003	Zoning Approval	
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<p>1. This permit application does not preclude the Applicant(s) from meeting applicable State and Federal Rules.</p> <p>2. Building permits do not include plumbing, septic or electrical work.</p> <p>3. Building permits are void if work is not started within six (6) months of the date of issuance. False information may invalidate a building permit and stop all work..</p>	<p>Special Zone or Reviews</p> <input type="checkbox"/> Shoreland <input type="checkbox"/> Wetland <input type="checkbox"/> Flood Zone <input type="checkbox"/> Subdivision <input checked="" type="checkbox"/> Site Plan <i>attached & signed</i> Maj <input type="checkbox"/> Minor <input type="checkbox"/> MM <input type="checkbox"/> Date: <i>10/3/03</i>	<p>Zoning Appeal</p> <input type="checkbox"/> Variance <input type="checkbox"/> Miscellaneous <input type="checkbox"/> Conditional Use <input type="checkbox"/> Interpretation <input type="checkbox"/> Approved <input type="checkbox"/> Denied Date: _____	<p>Historic Preservation</p> <input checked="" type="checkbox"/> Not in District or Landmark <input type="checkbox"/> Does Not Require Review <input type="checkbox"/> Requires Review <input type="checkbox"/> Approved <input type="checkbox"/> Approved w/Conditions <input type="checkbox"/> Denied Date: _____
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CERTIFICATION

I hereby certify that I am the owner of record of the named property, or that the proposed work is authorized by the owner of record and that I have been authorized by the owner to make this application as his authorized agent and I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in the application is issued, I certify that the code official's authorized representative shall have the authority to enter all areas covered by such permit at any reasonable hour to enforce the provision of the code(s) applicable to such permit.

SIGNATURE OF APPLICANT	ADDRESS	DATE	PHONE
RESPONSIBLE PERSON IN CHARGE OF WORK, TITLE		DATE	PHONE

031195

All Purpose Building Permit Application

If you or the property owner owes real estate or personal property taxes or user charges on any property within the City, payment arrangements must be made before permits of any kind are accepted.

Location/Address of Construction: ³²⁰ <u>340 Promenade East</u>		
Total Square Footage of Proposed Structure <u>200 sq ft</u>	Square Footage of Lot <u>Leasing 200 sq ft</u>	
Tax Assessor's Chart, Block & Lot Chart# <u>15</u> Block# <u>A00</u> Lot# <u>2</u>	Owner: <u>Promenade East Condo Assoc.</u>	Telephone: <u>773-5531</u>
Lessee/Buyer's Name (If Applicable) <u>United States Cellular</u>	Applicant name, address & telephone: <u>U.S. Cellular</u> <u>482 Congress Street</u> <u>Portland, Me. 04103</u>	Cost Of Work: \$ <u>55,000</u> Fee: \$ <u>516.00</u>
Current use: <u>Condominiums</u>		
If the location is currently vacant, what was prior use: _____		<u>Will bring lease in office</u>
Approximately how long has it been vacant: _____		
Proposed use: <u>Installation of 3 Antennas + Base Station Equipment</u>		
Project description: _____		
Contractor's name, address & telephone: <u>TBD</u>		
Who should we contact when the permit is ready: <u>Ed Shaw</u>		
Mailing address: _____		
We will contact you by phone when the permit is ready. You must come in and pick up the permit and review the requirements before starting any work, with a Plan Reviewer. A stop work order will be issued and a \$100.00 fee if any work starts before the permit is picked up. PHONE: <u>771-9992</u> <u>Cell</u> <u>xl</u>		

IF THE REQUIRED INFORMATION IS NOT INCLUDED IN THE SUBMISSIONS THE PERMIT WILL BE AUTOMATICALLY DENIED AT THE DISCRETION OF THE BUILDING/PLANNING DEPARTMENT, WE MAY REQUIRE ADDITIONAL INFORMATION IN ORDER TO APPROVE THIS PERMIT.

I hereby certify that I am the Owner of record of the named property, or that the owner of record authorizes the proposed work and that I have been authorized by the owner to make this application as his/her authorized agent. I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in this application is issued, I certify that the Code Official's authorized representative shall have the authority to enter all areas covered by this permit at any reasonable hour to enforce the provisions of the codes applicable to this permit.

Signature of applicant: <u>Edward A. Shaw</u>	Date: <u>9/15/03</u>
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This is NOT a permit, you may not commence ANY work until the permit is issued.
If you are in a Historic District you may be subject to additional permitting and fees with the Planning Department on the 4th floor of City Hall



LCC International
482 Congress Street
Suite 502
Portland, ME 04101
(207) 771-9992 (Office)
(207) 771-9993 (Fax)

September 15, 2003

Building Department
389 Congress Street
Portland, Me 04101

To Whom It May Concern:

United States Cellular Corporation (USCC) received a License authorizing them to provide service in Cumberland County. USCC has since hired LCC International to accomplish that objective. LCC is currently locating, leasing, and receiving zoning approvals and acquiring building permits for several approved locations in Portland.

Promenade East Condominium Association has been chosen as one of the approved locations. USCC is proposing to place (3) three cellular antennas on top of the building located at 340 Promenade East in Portland, Maine. The Antennas and Base Station Equipment will be located as shown on the attached drawing.

USCC has already received an Exemption from Site Plan Review and would now like to receive a building permit to do the work as proposed.

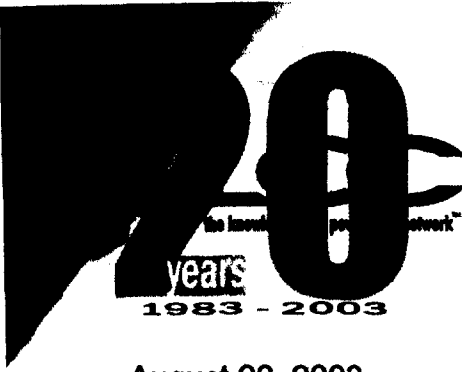
We would appreciate consideration at the earliest possible date. In addition, I would appreciate it if you, or someone on your staff, would contact me if any additional material or information is required at this stage of the process.

Thank you for your consideration of this application.

Respectfully submitted,

A handwritten signature in black ink, appearing to read 'Edward A. Shaw'. The signature is fluid and cursive, with a long horizontal line extending to the right.

Edward A. Shaw
LCC International Inc.
482 Congress Street
Suite 502
Portland, ME 04101



LCC International, Inc.
482 Congress Street
Portland, ME 04101
(207) 771-9992 (Office)
(207) 771-9993 (Fax)

August 26, 2003

*Lease is forthcoming
OK'd By Marge
Ed Shaw*

Promenade East Condominiums
340 Eastern Promenade
Portland, ME 04101

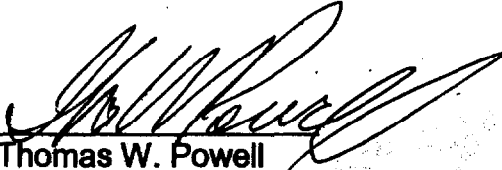
Re: Site # 853332.1


Dear Pauline R. Daniels,

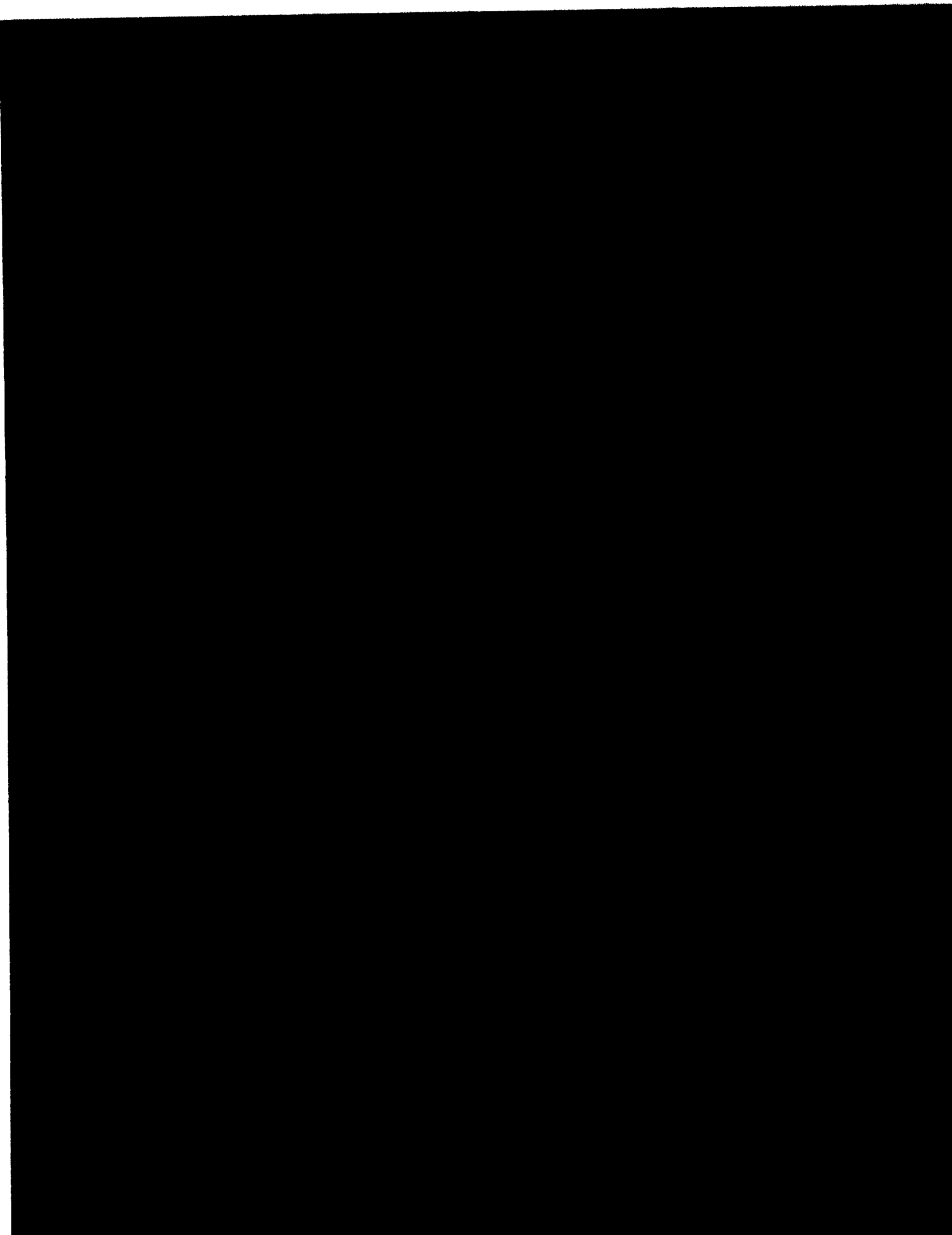
LCC has been contracted by US Cellular to design, develop and deploy their new PCS network in York, Cumberland and Sagadahoc Counties. As part of LCC's scope of work, we negotiate leases on behalf of our client, to secure space to construct repeater sites. When we reach a point in lease negotiations where both parties (land owner and US Cellular) are in agreement on lease terms and language, we initiate the building permit process to determine if and when the lease will commence.

At this time we are in good faith negotiations with you and feel we will consummate our deal on your property in a relatively short period. We would like to request your permission to apply for all regulatory approvals required (including, but not limited to; zoning permit, building permit, FAA and FCC filings) to build our repeater site on your property. By granting your approval, you are not contractually binding yourself to any lease contract that has not been fully executed to date. Please signify your consent to grant LCC permission to file for necessary permits for constructing a repeater site on your property by signing below.

Thank You,


Thomas W. Powell
Program Manager
LCCI, Inc.

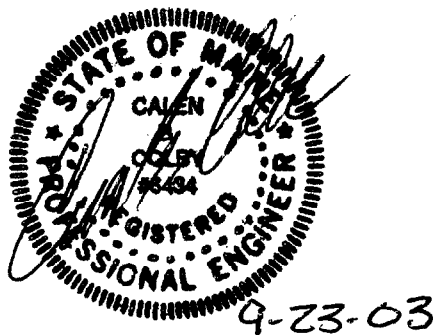

Pauline R. Daniels
Office Administrator / ~~Officer~~ Ed
Promenade East Condominiums



EASTERN PROMENADE CONDOMINIUMS
Portland, Maine

**CELLULAR TELEPHONE ANTENNA SUPPORT
STRUCTURAL CALCULATIONS**

SUBMITTAL #1



Prepared for:

LCC International, Inc.
482 Congress Street, Suite 502
Portland, ME 04101

Prepared by:

OEST Associates, Inc.
343 Gorham Rd.
South Portland, ME 04106-2317

September 22, 2003



LCC International
482 Congress Street
Suite 502
Portland, ME 04101
(207) 771-9992 (Office)
(207) 771-9993 (Fax)

October 3, 2003

Building Department
389 Congress Street
Portland, Me 04101

To Whom It May Concern:

United States Cellular Corporation (USCC) received a License authorizing them to provide service in Cumberland County. USCC has since hired LCC International to accomplish that objective. LCC is currently locating, leasing, and receiving zoning approvals and acquiring building permits for several approved locations in Portland.

Doubletree Hotel has been chosen as one of the approved locations. USCC is proposing to place (6) six cellular antennas on top of a twenty-foot Monopole placed on the roof of the building located at 1230 Congress Street in Portland, Maine. The Antennas and Base Station Equipment will be located as shown on the attached drawing.

USCC has already received an Exemption from Site Plan Review and would now like to receive a building permit to do the work as proposed.

We would appreciate consideration at the earliest possible date. In addition, I would appreciate it if you, or someone on your staff, would contact me if any additional material or information is required at this stage of the process.

Thank you for your consideration of this application.

Respectfully submitted,

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Edward A. Shaw
LCC International Inc.
482 Congress Street
Suite 502
Portland, ME 04101

OEST ASSOCIATES, INC.

PREPARED BY J. Walker DATE 9/22/03 PROJECT NO. _____
 CALCULATIONS CHECKED BY _____ DATE _____ SHEET NO. 1 OF 6
 SUBJECT Size Antenna Frame Ballast

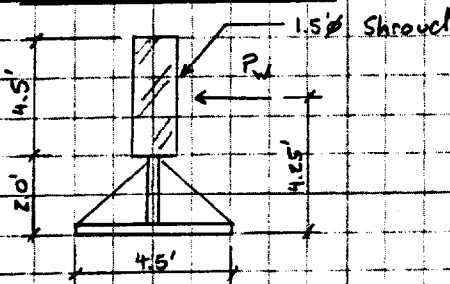
Calculate Required Antenna Frame Ballast

Find Design Wind Load: [TIA/EIA-222]

$$H = 135' \quad K_z = \left(\frac{135'}{55'}\right)^{2/7} = 1.50$$

$$V = 65 \text{ mph} \quad q_z = 0.00256(1.5)(65 \text{ mph})^2 = 27.7 \text{ psf}$$

Find Antenna Load:



$$\text{Aspect Ratio} = \frac{4.5'}{1.5'} = 3 \quad \therefore C_A = 0.8 \quad [\text{Tbl 3}]$$

$$G_H = 0.65 + \frac{0.6}{\left(\frac{135'}{55'}\right)^{1/4}} = 1.14$$

$$P_w = 27.7 \text{ psf}(1.14)(0.8)\left[1.5'(4.5')\right] = 170.5'$$

$$M_{af} = 170.5'(4.25') = 725' \text{ -ft}$$

Calculate Frame Weight:

L4x4 Base	⇒	4(4.5')(6.6 p/f)	=	119'
L3x3 Brace	⇒	4(3.0')(4.9 p/f)	=	59'
2 3/8" Pipe	⇒	6.5'(3.65 p/f)	=	24'
GH Beam	⇒	4.5'(5.4 p/f)	=	24'
Antenna	⇒	(1) FVG5	=	18'
			=	244'

Calculate Req'd Ballast:

$$M_R = 1.5(725' \text{ -ft}) = 1088' \text{ -ft} = (W + 244')\left(\frac{4.5'}{2}\right) \Rightarrow W = 240'$$

$$4' \times 8' \times 16' \text{ Solid CMU Block} \Rightarrow W = \frac{3 \frac{5}{8}''(7 \frac{5}{8}'')(15 \frac{3}{4}'')}{12^3} (115 \text{ pcf}) = 28.7' / \text{block}$$

$$\frac{240'}{28.7' / \text{block}} = 8.4$$

Use (9) 4" x 8" x 16" Solid CMU Blocks

Calculate Roof Load:

$$RL = \frac{244' + 9(28.7')}{(4.5')^2} = 25 \text{ psf}$$

OEST ASSOCIATES, INC.

PREPARED BY J. Walker DATE 9/15/03 PROJECT NO. 39003.01
 CALCULATIONS CHECKED BY _____ DATE _____ SHEET NO. 2 OF 6
 SUBJECT Check Existing Roof

Calculate Roof Loads:

Dead \Rightarrow 4" Slab = 50 psf
 Membrane = 1 psf
 Ballast = 5 psf
 Ceiling = 5 psf
 Joists = 8 psf
 Misc = 9 psf
 = 78 psf

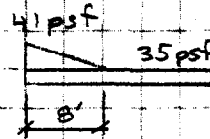
Snow \Rightarrow $P_f = 0.7(50 \text{ psf}) = 35 \text{ psf}$

Drift \Rightarrow $Y_s = 0.13(50 \text{ psf}) + 14 = 20.5 \text{ psf}$

$$h_d = 0.43 \sqrt[3]{25' \sqrt{50 \text{ psf} + 10}} - 1.5 = 2.0'$$

$$W_d = 4(2.0') = 8.0'$$

$$P_d = 2.0'(20.5 \text{ psf}) = 41 \text{ psf}$$



Check Roof Joists:

14" Deep Composite Joists $w_{cap} = 125 \text{ psf @ } 26.25' \text{ Spacing}$ [See Attached Sheet]

$$w_{TL} = 125 \text{ psf}(4.1') = 513 \text{ plf} \qquad M = \frac{513 \text{ plf}(26.25')^2}{8} = 51,176 \text{ #-ft}$$

$$V = \frac{513 \text{ plf}(26.25')}{2} = 7,246 \text{ #}$$

Estimate @ 17'-2" \Rightarrow $w_m = \frac{8(51,176 \text{ #-ft})}{(17.1')^2} = 1,400 \text{ plf}$

$$w_v = \frac{2(7,246 \text{ #})}{17.1'} = 848 \text{ plf} \quad \Leftarrow \text{ Controls}$$

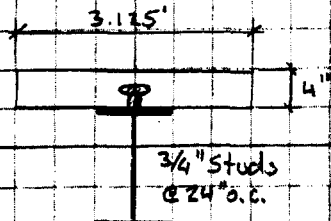
$$P_{all} = \frac{848 \text{ plf}}{4.1'} = 206 \text{ psf}$$

OEST ASSOCIATES, INC.

PREPARED BY J. Walker
 CALCULATIONS CHECKED BY _____
 SUBJECT Check Existing Roof

DATE 9/15/03 PROJECT NO. 390.03.01
 DATE _____ SHEET NO. 3 OF 6

Check Composite Roof Beam



W12x27
 $d = 11.96''$
 $A = 7.95 \text{ in}^2$
 $I = 204 \text{ in}^4$
 $S = 34.2 \text{ in}^3$

$$f'_c = 3,000 \text{ PSI} \therefore E_c = 3,122 \text{ ksi} \quad \& \quad n = 9.3$$

$$\text{Transformed Width} = \frac{3.125'(12)}{9.3} = 4.0''$$

Find \bar{x} :

$$\bar{x} = \frac{4''(4'')(2'') + 7.95 \text{ in}^2 (4'' + \frac{11.96''}{2})}{4''(4'') + 7.95 \text{ in}^2} = 4.65''$$

Find I_{Trans} :

$$I_{\text{slab}} = \frac{4''(4'')^3}{12} + 4''(4'')(4.65'' - 2'')^2 = 134 \text{ in}^4$$

$$I_{\text{beam}} = 204 \text{ in}^4 + 7.95 \text{ in}^2 \left[\frac{11.96''^2}{12} - (4.65'' - 4'')^2 \right] = 430 \text{ in}^4$$

$$= 564 \text{ in}^4$$

Find S_{Trans} :

$$S_{\text{slab}} = \frac{564 \text{ in}^4}{4.65''} = 121 \text{ in}^3$$

$$S_{\text{beam}} = \frac{564 \text{ in}^4}{(11.96'' + 4'') - 4.65''} = 49.9 \text{ in}^3$$

Find $S_{\text{effective}}$:

$$V_u = \text{MW of } \cdot 0.85(3 \text{ ksi})(3.25 \cdot 12)(4'')(0.5) = 191 \text{ K}$$

$$\cdot 36 \text{ ksi}(7.95 \text{ in}^2)(0.5) = 143 \text{ K} \quad \leftarrow \text{Controls}$$

$$V_u = 6(11.5 \text{ K}) = 69 \text{ K}$$

ASDM T6I4.17

$$S_{\text{eff}} = 34.2 \text{ in}^3 \sqrt{\frac{69 \text{ K}}{143 \text{ K}}} (49.9 \text{ in}^3 - 34.2 \text{ in}^3) = \underline{45.1 \text{ in}^3}$$

OEST ASSOCIATES, INC.

PREPARED BY J. Walker DATE 9/15/03 PROJECT NO. 392.03.01
 CALCULATIONS CHECKED BY _____ DATE _____ SHEET NO. 4 OF 6
 SUBJECT Check Existing Roof

Check Composite Roof Beam Cont:

Calculate Live Load Capacity:

Check #1

$$M_{DL} = \frac{78 \text{ psf} (172') (25')^2}{8} = 52,400 \text{ K-#}$$

$$f_{DPL} = \frac{52.4 \text{ K-#} (12)}{34.2 \text{ ksi}} = 18.4 \text{ ksi}$$

$$f_{DCL} = 0.9 (36 \text{ ksi}) - 18.4 \text{ ksi} = 14 \text{ ksi}$$

$$M_{LL} = 14 \text{ ksi} (45.1 \text{ in}^3) = 52.6 \text{ K-#}$$

Check #2:

$$M_{TL} = 0.66 (36 \text{ ksi}) (45.1 \text{ in}^3) = 89.3 \text{ K-#}$$

$$M_{LL} = 89.3 \text{ K-#} - 52.4 \text{ K-#} = \underline{36.9 \text{ K-#}} \leftarrow \text{Controls}$$

Fixed Live Load:

$$W_{LL} = \frac{8 (36.9 \text{ K-#})}{(25')^2} = 472 \text{ plf}$$

$$P_{LL} = \frac{472 \text{ plf}}{7.2/2} = \underline{55 \text{ psf}}$$

Check Roof Load:

Note: Roof Ballast Will be Removed Under Antenna Frame $\therefore P_a = 25 \text{ psf} - 5 \text{ psf} = 20 \text{ psf}$

$$\text{Roof Reserve} = 55 \text{ psf} - 35 \text{ psf} = \underline{20 \text{ psf}} \approx P_a = 20 \text{ psf} \quad \text{OK}$$

OEST ASSOCIATES, INC.

PREPARED BY J. Walker DATE 9/15/03 PROJECT NO. 390.03.01
 CALCULATIONS CHECKED BY _____ DATE _____ SHEET NO. 5 OF 6
 SUBJECT Check Existing Penthouse Roof

Check Penthouse Roof Joists

$$V_{max} = 7,246^{\#} \quad [\text{See page \#2}]$$

$$\text{Estimate Capacity @ 21.5' } \Rightarrow \quad w_v = \frac{2(7,246^{\#})}{21.5'} = 674 \text{ plf}$$

$$P_{int} = \frac{674 \text{ plf}}{4.25'} = \underline{159 \text{ psf}}$$

Check Penthouse Roof Beam

$$W14 \times 30 \Rightarrow \quad M_{cap} = 0.66(26^{\text{ksi}})(41.9 \text{ in}^3) = 83.0 \text{ K-ft}$$

$$w_{cap} = \frac{8(83.0 \text{ K-ft})}{(23')^2} = 1,255 \text{ plf}$$

$$P_{cap} = \frac{1,255 \text{ plf}}{21.5'/2} = \underline{117 \text{ psf}} \quad \leftarrow \text{Controls Roof}$$

Estimate Penthouse Roof Weight:

3" Slab	=	38 psf
Membrane	=	1 psf
Ballast	=	5 psf
Ceiling	=	5 psf
Joists	=	8 psf
Misc	=	5 psf
	=	<u>62 psf</u>

Check Roof Load:

Note: Roof Ballast Will be Removed Under Antenna Frame $\therefore P_a = 25 \text{ psf} - 5 \text{ psf} = 20 \text{ psf}$

$$\text{Roof Reserve} = 117 \text{ psf} - 62 \text{ psf} - 35 \text{ psf} = \underline{20 \text{ psf}} \approx P_a = 20 \text{ psf} \quad \textcircled{OE}$$

OEST ASSOCIATES, INC.

PREPARED BY J. Walker

DATE 9/15/03

PROJECT NO. 390.03.01

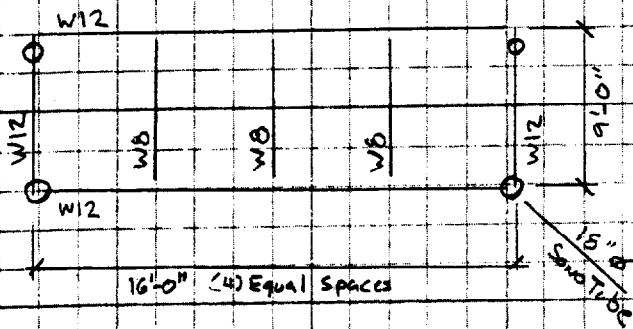
CALCULATIONS CHECKED BY _____

DATE _____

SHEET NO. 6 OF 6

SUBJECT Platform Framing Design

Design Equipment Platform:



Design Loads:

DL = 10 psf

Equip = 2,400 # @ 30" x 42" = 275 psf

Live = 40 psf

Grating => Use 1 1/4" x 3/16" Steel Bearing Bars @ 1 7/8" o.c. $w_{cap} = 370 \text{ psf}$

WB => L = 9' $w = DL = 10 \text{ psf} (4.0') = 40 \text{ plf}$
 $= LL = 275 \text{ psf} (") = 1,100 \text{ plf}$

Try W6x10 $M = \frac{1,140 \text{ plf} (9')^2}{8} = 11,543 \text{ #-ft}$ $f_b = \frac{11,543 \text{ #-ft} (12)}{7.81 \text{ in}^3} = 17.7 \text{ ksi}$ (OK)

$V = \frac{1,140 \text{ plf} (9')}{2} = 5,130 \text{ #}$ $f_v = \frac{5,130 \text{ #}}{7.89 \text{ in}^2 (0.11')} = 3.8 \text{ ksi}$ (OK)

W12 => L = 16' $w = DL = 10 \text{ psf} (9 1/2') = 45 \text{ plf}$
 $= LL = 275 \text{ psf} (") = 1,238 \text{ plf}$

Try W12x19 $M = \frac{1,283 \text{ plf} (16')^2}{8} = 41,056 \text{ #-ft}$ $f_b = \frac{41,056 \text{ #-ft} (12)}{21.5 \text{ in}^3} = 23.2 \text{ ksi}$ (OK)

$V = \frac{1,283 \text{ plf} (16')}{2} = 10,264 \text{ #}$ $f_v = \frac{10,264 \text{ #}}{12.16 \text{ in}^2 (0.235')} = 3.6 \text{ ksi}$ (OK)

Sand Tube => $P = DL = 10 \text{ psf} (9 1/2') (16 1/2') = 360 \text{ #}$
 $= 1.76 \text{ ft}^2 (8') (150 \text{ psf}) = 2,112 \text{ #}$
 $= LL = 1/2 (3) (2,400 \text{ #}) = 3,600 \text{ #}$
 $= 6,072 \text{ #}$

$F_p = \frac{6,072 \text{ #}}{1.76 \text{ ft}^2} = 3,450 \text{ psf}$ < Add (2) More Tubes to Control Settlement

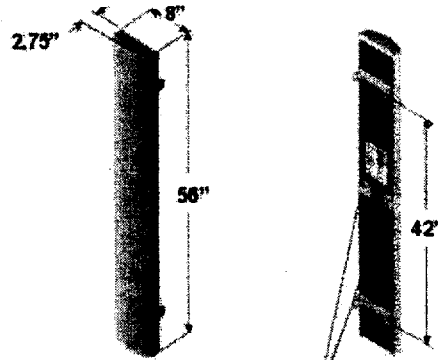


FR65-17-XXDP DualPol® Polarization 1850 MHz - 1990 MHz



Electrical Specifications

Azimuth Beamwidth	65°
Elevation Beamwidth	7°
Gain	16.5 dBi (14.4 dBd)
Polarization	Dual Linear Slant ($\pm 45^\circ$)
Port-to-Port Isolation	≥ 30 dB
Front-to-Back Ratio	≥ 30 dB
Electrical Downtilt Options	0°, 2°, 4°
VSWR	1.35:1 Max
Connectors	2; 7-16 DIN (female)
Power Handling	250 Watts CW
Passive Intermodulation	≤ -150 dBc [2 x 20 W (+ 43 dBm)]
Lightning Protection	Chassis Ground



RF CONNECTORS



Mechanical Specifications

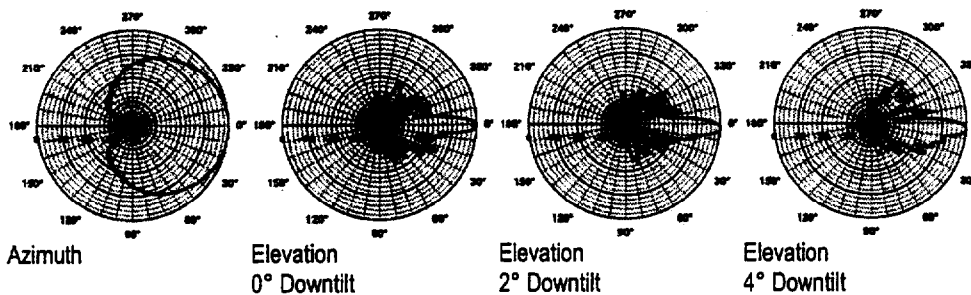
Dimensions (L x W x D)	56 in x 8 in x 2.75 in (142 cm x 20.3 cm x 7.0 cm)
Rated Wind Velocity	150 mph (241 km/hr)
Equivalent Flat Plate Area	3.1ft ² (.29 m ²)
Front Wind Load @ 100 mph (161 kph)	90 lbs (400 N)
Side Wind Load @ 100 mph (161 kph)	31 lbs (139 N)
Weight	18 lbs (8.2 kg)

Mounting Options

MTG-P00-10, MTG-S02-10, MTG-DXX-20*, MTG-CXX-10*, MTG-C02-10, MTG-TXX-10*

Note: *Model number shown represents a series of products. See Mounting Options section for specific model number.

Patterns



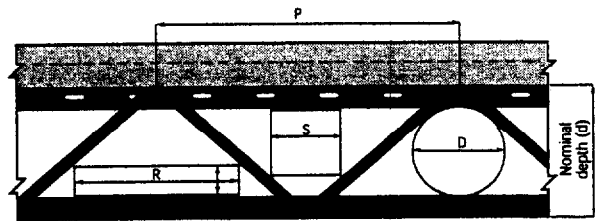
Revised 04/05/02

Maximum Clear Spans

8"	23'-2"	22'-8"	22'-4"	21'-2"	20'-10"
10"	26'-4"	25'-1"	25'-1"	24'-0"	23'-4"
12"	29'-8"	27'-10"	27'-10"	26'-6"	25'-11"
14"	32'-6"	30'-4"	30'-4"	28'-10"	28'-3"
16"	35'-2"	33'-0"	33'-0"	31'-2"	30'-4"
18"	37'-9"	35'-4"	35'-4"	33'-5"	32'-7"
20"	40'-4"	37'-7"	37'-7"	35'-6"	34'-6"
22"	42'-8"	39'-11"	39'-11"	37'-7"	36'-6"
24"	45'-2"	42'-0"	42'-0"	39'-6"	38'-5"

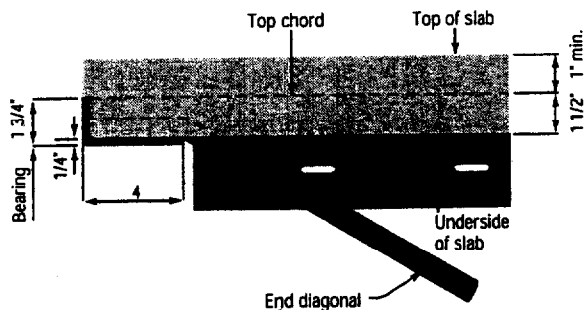
*Note: Total floor depth = hambro® depth plus slab thickness loads
Loads are psf.

Maximum Web Opening

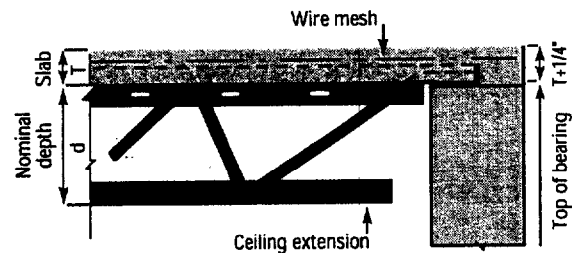


d	P	D	S	R
8"	17'	4"	4"	6" X 3"
10"	18'	6"	5"	6" X 4"
12"	19'	7 1/2"	6"	6" X 6"
14"	21'	8"	6"	8" X 4"
16"	24'	10"	7"	10" X 4"
18"	24'	11"	8"	10" X 6"
20"	24'	12"	8"	12" X 6"
22"	24'	12"	9"	12" X 6"
24"	24'	13"	10"	12" X 6"

Standard Shoe*

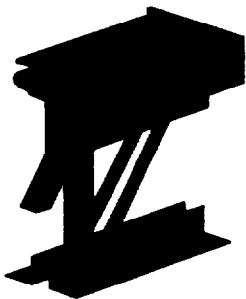


Typical Bearing Detail



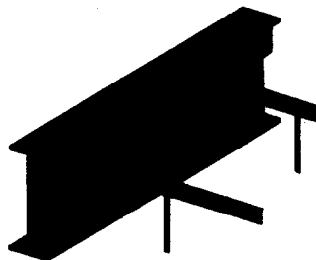
*All dimensions are approximate

Additional Systems and Accessories



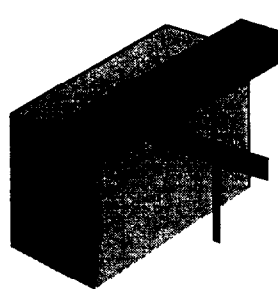
LH Series

This series features a top chord "S" made of two hambro® sections. Hambro® composite long span floors provide greater economy for heavy service loads and longer spans. Joist depths range from 20" to 48" with spans up to 65'. Details are available from your hambro® representative.



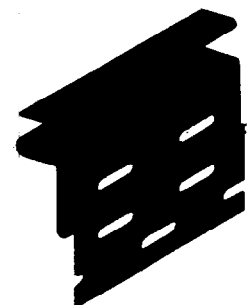
Rollbar Angle (RA) on Steel Beam

This hambro® accessory is designed to be field attached to the top flange of a steel beam running parallel to a hambro® joist. Rollbar Angle (RA) is slotted similar to the hambro® top chord "S" to accommodate ROLLBARS®.



RA on Walls

This hambro® accessory is fastened at the top of walls and is slotted similar to the hambro® top chord "S" to accommodate ROLLBARS®.



Hanger Plate

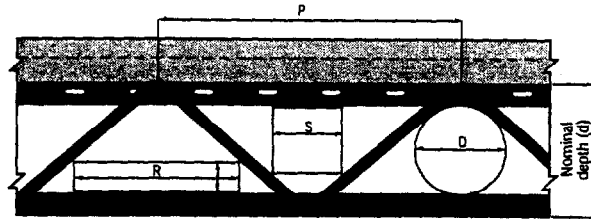
Shop attached slotted plate for thicken slab areas.

Maximum Clear Spans

8"	23'-2"	22'-8"	22'-4"	21'-2"	20'-10"
10"	26'-4"	25'-1"	25'-1"	24'-0"	23'-4"
12"	29'-8"	27'-10"	27'-10"	26'-6"	25'-11"
14"	32'-6"	30'-4"	30'-4"	28'-10"	28'-3"
16"	35'-2"	33'-0"	33'-0"	31'-2"	30'-4"
18"	37'-9"	35'-4"	35'-4"	33'-5"	32'-7"
20"	40'-4"	37'-7"	37'-7"	35'-6"	34'-6"
22"	42'-8"	39'-11"	39'-11"	37'-7"	36'-6"
24"	45'-2"	42'-0"	42'-0"	39'-6"	38'-5"

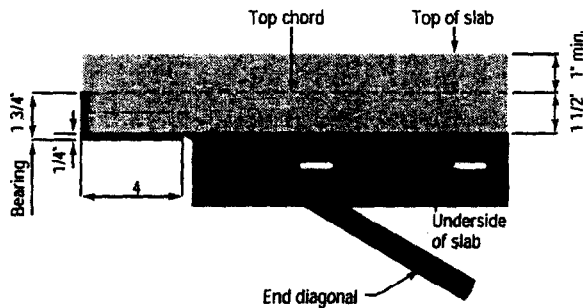
*Note: Total floor depth = hambro® depth plus slab thickness loads
Loads are psf.

Maximum Web Opening

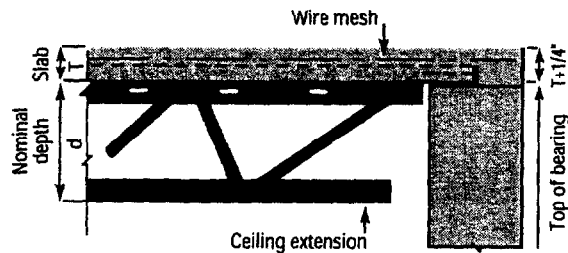


d	P	D	S	R
8"	17"	4"	4"	6" X 3"
10"	18"	6"	5"	6" X 4"
12"	19"	7 1/2"	5"	8" X 4"
14"	21"	8"	6"	8" X 4"
16"	24"	10"	7"	10" X 4"
18"	24"	11"	8"	10" X 6"
20"	24"	12"	9"	12" X 6"
22"	24"	12"	9"	12" X 6"
24"	24"	13"	10"	12" X 6"

Standard Shoe*

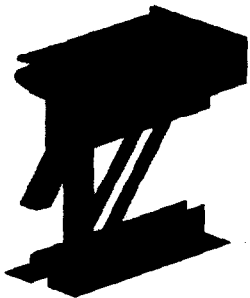


Typical Bearing Detail



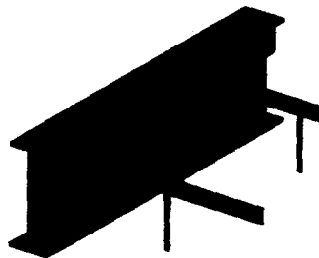
*All dimensions are approximate

Additional Systems and Accessories



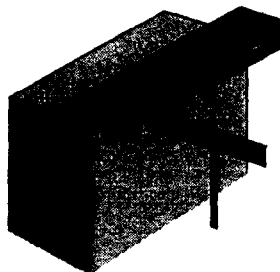
S Series

This series features a top chord "S" made of hambro sections. This hambro composite long span floor provides greater economy for heavy service joists and longer spans. Joist depths range from 20" to 48" with spans up to 65'. Details are available from your hambro representative.



Rollbar Angle (RA) on Steel Beam

This hambro accessory is designed to be field attached to the top flange of a steel beam running parallel to a hambro joist. Rollbar Angle (RA) is slotted similar to the hambro top chord "S" to accommodate ROLLBARS.



RA on Walls

This hambro accessory is fastened to the top of walls and is slotted similar to the hambro top chord "S" to accommodate ROLLBARS.



Hanger Plate

Shop attached slotted plate for thickened slab areas.



FR65-17-XXDP

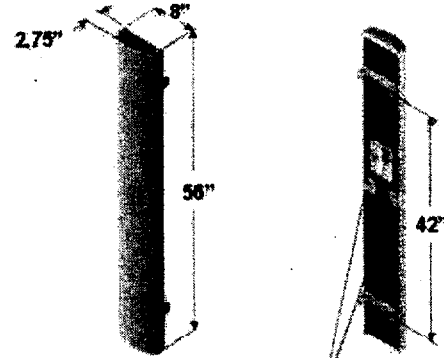
DualPol® Polarization

1850 MHz - 1990 MHz

OptiFill™

Electrical Specifications

Azimuth Beamwidth	65°
Elevation Beamwidth	7°
Gain	16.5 dBi (14.4 dBd)
Polarization	Dual Linear Slant ($\pm 45^\circ$)
Port-to-Port Isolation	≥ 30 dB
Front-to-Back Ratio	≥ 30 dB
Electrical Downtilt Options	0°, 2°, 4°
VSWR	1.35:1 Max
Connectors	2; 7-16 DIN (female)
Power Handling	250 Watts CW
Passive Intermodulation	≤ -150 dBc [2 x 20 W (+ 43 dBm)]
Lightning Protection	Chassis Ground



RF
CONNECTORS

Mechanical Specifications

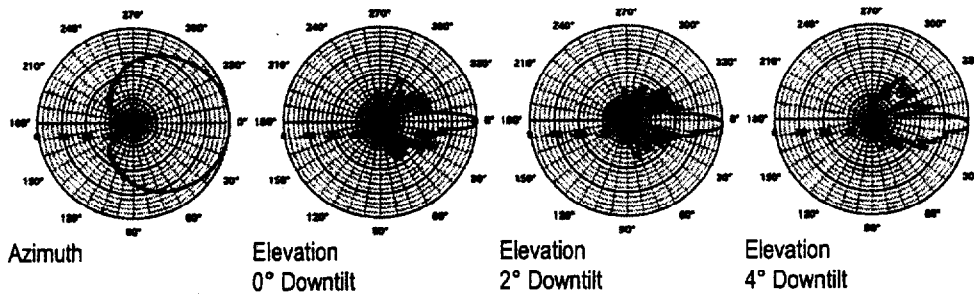
Dimensions (L x W x D)	56 in x 8 in x 2.75 in (142 cm x 20.3 cm x 7.0 cm)
Rated Wind Velocity	150 mph (241 km/hr)
Equivalent Flat Plate Area	3.1ft ² (.29 m ²)
Front Wind Load @ 100 mph (161 kph)	90 lbs (400 N)
Side Wind Load @ 100 mph (161 kph)	31 lbs (139 N)
Weight	18 lbs (8.2 kg)

Mounting Options

MTG-P00-10, MTG-S02-10, MTG-DXX-20*, MTG-CXX-10*, MTG-C02-10, MTG-TXX-10*

Note: *Model number shown represents a series of products. See Mounting Options section for specific model number.

Patterns



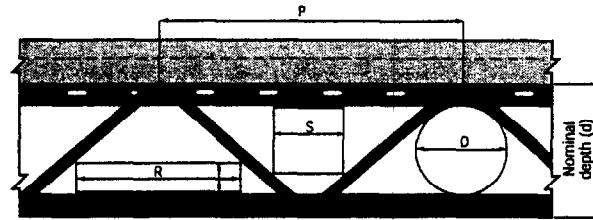
Revised 04/05/02

Maximum Clear Spans

8"	23'-2"	22'-8"	22'-4"	21'-2"	20'-10"
10"	26'-4"	25'-1"	25'-1"	24'-0"	23'-4"
12"	29'-8"	27'-10"	27'-10"	26'-6"	25'-11"
14"	32'-6"	30'-4"	30'-4"	28'-10"	28'-3"
16"	35'-2"	33'-0"	33'-0"	31'-2"	30'-4"
18"	37'-9"	35'-4"	35'-4"	33'-5"	32'-7"
20"	40'-4"	37'-7"	37'-7"	35'-6"	34'-6"
22"	42'-8"	39'-11"	39'-11"	37'-7"	36'-6"
24"	45'-2"	42'-0"	42'-0"	39'-6"	38'-5"

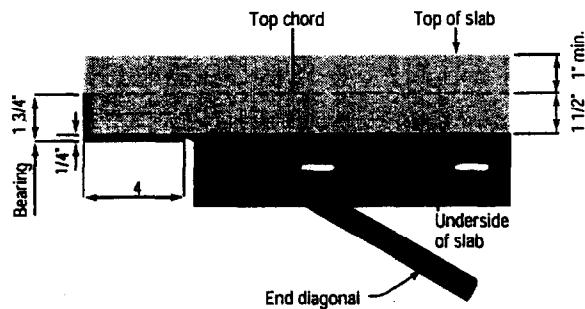
*Note: Total floor depth = hambro® depth plus slab thickness loads
Loads are psf.

Maximum Web Opening

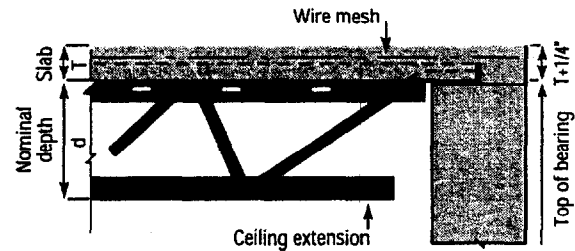


D	P	D	S	R
8"	17"	4"	4"	6" X 3"
10"	18"	6"	5"	6" X 4"
12"	19"	7 1/2"	5"	8" X 4"
14"	21"	8"	6"	8" X 4"
16"	24"	10"	8"	10" X 4"
18"	24"	11"	8"	10" X 6"
20"	24"	12"	9"	12" X 6"
22"	24"	12"	9"	12" X 6"
24"	24"	13"	10"	12" X 6"

Standard Shoe*



Typical Bearing Detail



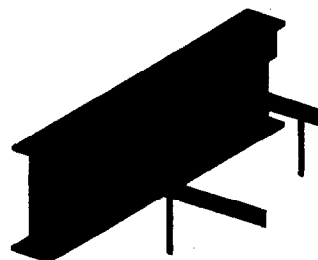
*All dimensions are approximate

Additional Systems and Accessories



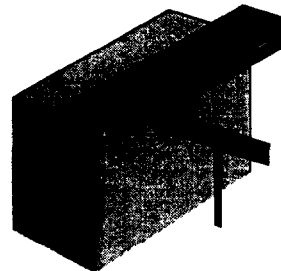
LH Series

This series features a top chord "S" made of two hambro® sections. Hambro® composite long span floors provide greater economy for heavy service loads and longer spans. Joist depths range from 20" to 48" with spans up to 65'. Details are available from your hambro® representative.



Rollbar Angle (RA) on Steel Beam

This hambro® accessory is designed to be field attached to the top flange of a steel beam running parallel to a hambro® joist. Rollbar Angle (RA) is slotted similar to the hambro® top chord "S" to accommodate ROLLBARS®.



RA on Walls

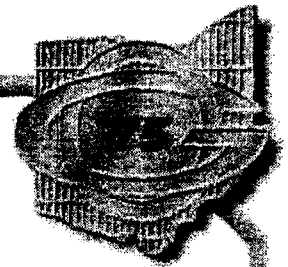
This hambro® accessory is fastened at the top of walls and is slotted similar to the hambro® top chord "S" to accommodate ROLLBARS®.



Hanger Plate

Shop attached slotted plate for thicken slab areas.

LIGHT DUTY WELDED STEEL W SERIES



Ohio Gratings is a major stocking and fabricating distributor of electroforge welded carbon steel bar grating. Bearing bars range from $\frac{3}{4}$ " x $\frac{3}{16}$ " through $2\frac{1}{2}$ " x $\frac{3}{16}$ " in $\frac{1}{8}$ " increments. Bearing bar spacings of $1\frac{3}{16}$ ", $\frac{15}{16}$ " and $\frac{11}{16}$ " are available with cross rods on 4" or 2" centers. The bearing bar surface may be provided plain, or with serrations for maximum skid resistance.

Electroforge welded steel grating is ideal for pedestrian traffic, and for light, rubber pneumatic tired rolling traffic (carts, dollies and hand trucks). For other rolling loads (forklifts, cars, trucks, etc.) see the Heavy Duty Steel Grating section, page 47.

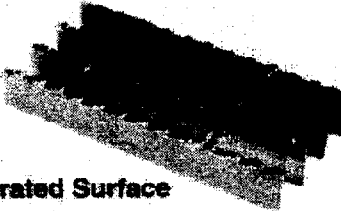
How to Specify:

1. Grating: Light Duty Welded Steel W Series by Ohio Gratings, Inc., or approved equal.
2. Bearing Bars: Rectangular Bar on $1\frac{3}{16}$ " centers maximum. (Note: Other spacings may be specified at the discretion of the architect/engineer.)
3. Cross Bars: Electroforge welded at right angles to bearing bars at 4" centers maximum. (Note: 2" cross bar centers may be specified at the discretion of the architect/engineer.)
4. Surface: Plain. (Note: A serrated surface may be specified for maximum skid resistance.)
5. Loading: Grating to carry a pedestrian loading equal to 100# per square foot over the required clear span with deflection not to exceed $\frac{1}{4}$ ". (Note: Alternate loading requirements may be specified at the discretion of the architect/engineer.)
6. Finish: (Galvanized or manufacturer's standard black paint at the discretion of the architect/engineer.)
7. Fabrication and Tolerances: In accordance with the NAAMM Metal Bar Grating Manual.

Plain Surface



Serrated Surface



Stainless Steel

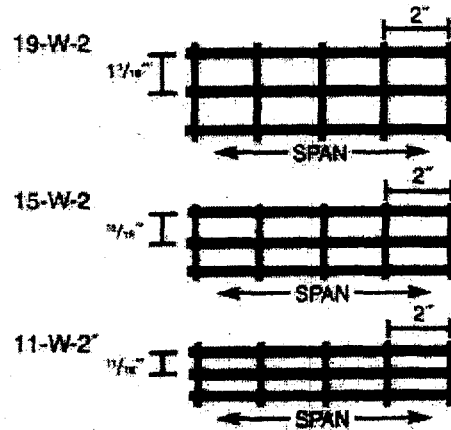
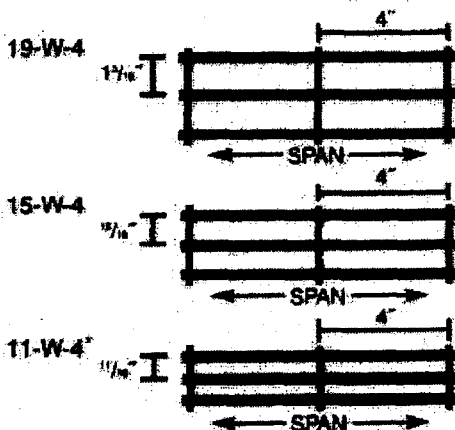
(Shown for comparison only; not available from Ohio Gratings)



For those areas requiring the corrosion resistance of stainless steel, Ohio Gratings stocks 1 " x $\frac{3}{16}$ ", $1\frac{1}{4}$ " x $\frac{3}{16}$ " and $1\frac{1}{2}$ " x $\frac{3}{16}$ " 19-SGSS-4 Type 304 Swaged Stainless Steel grating (see page 93). Swaged Stainless eliminates the warping, twisting and discoloration inherent in the electroforging process, and provides an excellent choice for those severe industrial corrosion applications. The SGSS Series is also available in ADA (July 1991) compliant spacings.

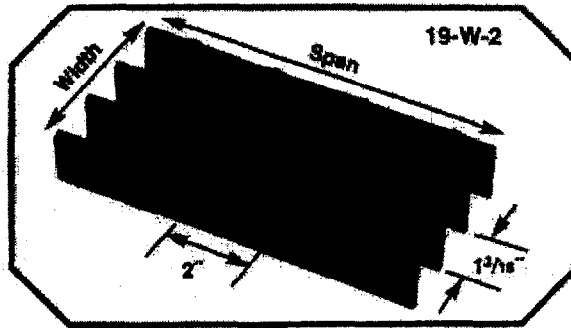
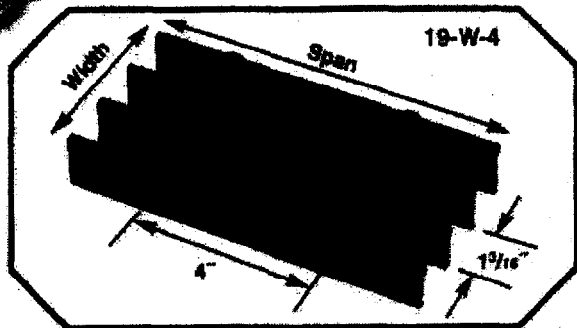
Note: For complete CSI Three-Part Section Format guide specification information, refer to page 113.

Grating Types - Light Duty Welded Steel W Series



Note: Conforms with the spacing requirements of ADA (July 1991) when installed with the elongated opening perpendicular to the dominant direction of travel. See inside Front Cover for further information.

LIGHT DUTY WELDED STEEL 19-W-4/19-W-2



Load Table 19-W-4/19-W-2

Bar Size, inches	Ped Span, inches	Wt. Lbs. Sq. Ft.	Sec. Prop Sx ² , In ²	Ix ² , In ⁴	Clear Span																
					2'-0"	2'-6"	3'-0"	3'-6"	4'-0"	4'-6"	5'-0"	5'-6"	6'-0"	6'-6"	7'-0"	8'-0"					
3/4 x 3/16	46	5.67	0.176	0.067	U	533	241	227	174	135											
					D	0.069	0.165	0.224	0.304	0.387											
					C	533	426	395	305	246											
					D	0.078	0.124	0.179	0.244	0.317											
1 x 3/16	51	5.15	0.201	0.105	U	625	264	251	197	158											
					D	0.078	0.118	0.165	0.220	0.285											
					C	625	507	471	367	292											
					D	0.089	0.135	0.190	0.255	0.329											
1 x 3/16	57	7.35	0.316	0.168	U	947	656	621	508	377	187										
					D	0.074	0.116	0.165	0.228	0.296	0.377	0.467									
					C	947	756	632	541	474	421	378									
					D	0.080	0.125	0.174	0.238	0.299	0.362	0.432									
1 1/4 x 3/16	67	9.03	0.493	0.308	U	1440	947	899	737	570	292	237	184								
					D	0.060	0.093	0.134	0.182	0.238	0.301	0.373	0.451	0.535							
					C	1440	1184	987	848	740	658	592	538	493							
					D	0.048	0.074	0.107	0.148	0.191	0.241	0.298	0.360	0.428							
1 1/2 x 3/16	77	10.94	0.711	0.533	U	2132	1384	947	805	593	421	341	282	237	202						
					D	0.060	0.078	0.112	0.152	0.199	0.261	0.310	0.378	0.447	0.525						
					C	2132	1705	1421	1218	1066	947	853	775	711	658						
					D	0.040	0.062	0.089	0.122	0.169	0.221	0.268	0.320	0.388	0.463						
1 3/4 x 3/16	87	12.82	0.987	0.698	U	2921	1927	1329	1137	827	625	497	404	334	287	241					
					D	0.042	0.067	0.095	0.133	0.179	0.241	0.290	0.358	0.426	0.502	0.588					
					C	2921	2384	1984	1695	1461	1261	1100	969	869	799	738					
					D	0.024	0.037	0.052	0.071	0.095	0.122	0.152	0.185	0.221	0.262	0.307	0.357	0.412	0.472		
2 x 3/16	96	14.30	1.263	1.263	U	3788	2425	1684	1257	947	749	605	501	421	358	309	257				
					D	0.037	0.058	0.084	0.114	0.148	0.199	0.233	0.292	0.335	0.394	0.456	0.526				
					C	3788	3032	2526	2185	1896	1684	1516	1378	1263	1168	1083	1003	947			
					D	0.030	0.047	0.067	0.091	0.119	0.151	0.189	0.225	0.268	0.315	0.365	0.420	0.477			
2 1/4 x 3/16	106	16.87	1.898	1.798	U	4788	3129	2178	1625	1200	947	757	625	526	447	384	327	287	241		
					D	0.032	0.050	0.070	0.095	0.125	0.159	0.200	0.239	0.285	0.335	0.390	0.450	0.515			
					C	4788	3887	3222	2711	2326	2000	1756	1566	1426	1300	1192	1100	1020	947		
					D	0.026	0.041	0.056	0.075	0.099	0.128	0.162	0.200	0.242	0.289	0.340	0.395	0.454			
2 1/2 x 3/16	113	17.55	1.974	2.457	U	5921	3788	2632	1933	1480	1170	947	783	658	561	483	421	370			
					D	0.030	0.047	0.067	0.091	0.119	0.151	0.189	0.225	0.268	0.315	0.365	0.420	0.477			
					C	5921	4737	3847	3283	2860	2532	2288	2103	1974	1822	1682	1548	1480			
					D	0.024	0.037	0.054	0.073	0.095	0.121	0.148	0.180	0.215	0.262	0.292	0.331	0.381			

U - Safe uniform load in pounds/sq. ft.
C - Safe concentrated load in pounds/ft. grating width.
D - Deflection in inches

Loads and deflections given in this table are theoretical, and are based on a unit stress of 18,000 psi.

% Open Area*		
Bars	1/8"	3/16"
4" cc	65%	77%
2" cc	76%	71%

*Based on 10.105 bars/ft. of grating width. Bearing bars 1 3/16" c.c. Add .8 lbs./sq. ft. for 19-W-2.
Notes: Grating for spans to the left of the heavy line have a deflection less than 1/4" for uniform loads of 100 lbs./sq. ft. This is the maximum deflection to afford pedestrian comfort and can be exceeded for other types of load at the discretion of the engineer. The actual Ped (pedestrian) Span under this condition is shown above for each size of grating. When serrated grating is specified, the depth of grating required for a specific load will be 1/4" greater than that shown in these tables.

19-W-4/19-W-2 Panel Width Chart (in.) Dimensions Are Out-to-Out of Bearing Bars**

No. of Bars	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
3/16" Bars	1 1/2	2 1/8	3 1/4	4 1/8	5 1/8	6 1/4	7 1/4	8 1/2	9 1/2	10 3/4	11 3/4	12 3/4	13 3/4	14 3/4	15 3/4
1/2" Bars	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31
3/8" Bars	19 1/2	20 1/2	21 1/2	22 1/4	23 1/4	24 1/2	25 1/2	26 1/2	27 1/2	28 1/2	29 1/2	30 1/2	31 1/2	32 1/2	33 1/2

**Deduct 1/8" for 1/8" bearing bars. Standard panel widths indicated in blue.