GENERAL STRUCTURAL NOTES

44 Quebec Street Portland, ME SI Job # 17-0017

DESIGN LIVE LOADS 2009 IBC, MUEBC

* Snow * Wind 90 mph, exp B, 3 second gust * Floor

FOUNDATION EXISTING:

* Foundations are designed without an engineer's soil investigation. Foundation design criteria was assumed for purpose of foundation design and shall be confirmed by a soils engineer, at owner's expense, prior to construction. (This procedure may require revisions to foundation design, at additional expense to the

owner, if soils engineer determines that such design criteria are inappropriate for this building site.) * Footings shall be placed on undisturbed natural soil or compacted fill tested and approved by soils

* Allowable bearing pressure = 1,500 psf. Bear on soil approved by the Soils Engineer. –typ

CONCRETE AND REINFORCEMENT:

* Concrete shall conform to applicable provisions of ACI-301 and 318.

Minimum 28 day compressive strength (F'c)

as follows: Footings:

psi w/4-6% air entrainment 4,000 Piers: Interior Slabs: 3,500 psi w/fibermesh

Exterior Slabs:

psi w/4-6% air entrainment and fiber mesh * Cement Type: I/II * Deformed reinforcement: ASTM A615 grade 60, except bars specified to be field-bent, stirrups, and ties

* Fibermesh: 100% virgin polypropylene, fibrillated fibers as manufactured by Fibermesh Co. per ASTM C-1116 type 111 4.1.3 and ASTM C-1116 performance level one, 1.5 lb. per cubic yard.

* Welded Wire Fabric (WWF): ASTM A185. See also plan.

* Typical minimum foundation reinforcing: 2 #4 top and bottom, (except as noted) continuous at

corners and steps.

* Reinforcement shall be fabricated and placed per ACI Manual of Standard Practice (ACI-315). At splices, lap bars 50 diameters unless noted otherwise.

* Minimum 2 #4 around all four sides of all openings, extend min. 2'-0 beyond openings. * Concrete cover over reinforcing: $1^{1}/_{2}$ " for concrete placed against forms; 3" for concrete placed against

earth. See also drawings.

* In continuous members, splice top bars at mid span and bottom bars over supports.

* Keep reinforcement clean and free of dirt, oil, and scale. Oil forms prior to placing reinforcement.

* Expansion Anchors shall be ICC-ES approved, installed in accordance with manufacturers specifications. In concrete: Wedge Type

WOOD FRAMING:

Dimension Lumber is designed and shall be supplied using BASE VALUES Design Criteria.
 SPF #2 and better (Maximum Moisture Content 19%) U.O.N.

Plates: Sill plates: Pressure Treated SPF or Southern Pine: "Pressure treated lumber" shall be framing material of the specified species which has been pressure treated with a decay and insect resistant solution, meeting all current standards for wood in contact with concrete, masonry, or in a wet location.

Sill plates in contact with masonry or concrete foundations, footings or slabs may be treated Timber Strand LSL (zinc borate treatment). Sodium borate treatment may also be acceptable for sill plate applications when protected from weather.

Acceptable treatment mediums for wood in contact with earth or in exterior applications include ACQ-C

and ACQ-D (Alkaline Copper Quaternary) and copper azole (CBA-A and CBA-B). DO NOT USE WOODS WHICH HAVE BEEN TREATED WITH AMMONIA BASED CARRIERS. All connectors shall meet the recommendations of the pressure treated wood manufacturer, but shall be not less than Hot Dipped Galvanized meeting requirements of ASTM A653, such as Simpson ZMAX. (G185). All screws, nails and bolts shall match hangers and other connectors, and shall meet ASTM A123

for individual connectors, and ASTM A153 for fasteners. For durability, it is our recommendation that connectors used in exposed conditions with treated lumber be stainless steel.

Do not mix galvanized and stainless products.

Do not allow aluminum to contact treated wood. Top and Bottom Plates: SPF No 2 and better

SPF U.O.N: 2 x 4 and 2 x 6 to 8'-0: stud grade

2 x 4 over 8'-0: standard and better 2x 6 over 8'-0: No. 2 and better

* All plywood and oriented strand board (OSB) sheathing shall be engineered grades with APA grade stamp indicating appropriate maximum spacing of supports

Floor sheathing: nominal ³/₄", APA Sturd-I-Floor "24" tongue & groove glued and nailed. Wall sheathing panels: 7/16" OSB or Plywood

Roof sheathing: 19/32" OSB * Nail wall sheathing <u>panels</u> with 8d commons at 4" o.c. at panel edges, and 12" o.c. intermediate framing U.N.O. BLOCK AND NAIL ALL EDGES BETWEEN STUDS. Sheathing shall be continuous from bottom plate to top plate. Cut in "L" and "T" shapes around openings. Lap sheathing over rim joists min. 4" at all floors to tie upper and lower stud walls together. Minimum height of sheathing panels shall be 16" to assure that plates are tied to studs. Use minimum 3-8d per stud and nail plates with edge nail spacing.

* Sole plate at all perimeter walls and at designated shear walls shall be nailed as for braced panels with 3-16d x 3 1/2" long box nails (coated or deformed shank) per 16". 12d nails are not acceptable.

SHEATH ALL EXTERIOR WALLS.

* Minimum nailing shall comply with IBC Table 2304.9.1 except where more or larger nailing shown on drawings.

* All roof rafters, joists, beams shall be anchored to supports with metal framing anchors.

* Double joists under partitions where joists are parallel to partitions.

* Provide continuous wall stude each side of wall openings equal to one half or greater of number of stude

interrupted by openings.

* All wall studs shall be continuous from floor to floor or from floor to roof. * Cross bridge all dimension lumber roof and floor joists at midspan or 8'-0" o.c. max and provide solid blocking or

rim joists at all joist supports and joist ends.

* Metal connectors: Simpson Strong Tie unless otherwise noted, installed with number and type of nails to achieve maximum rated capacity. Note that heavy duty and skewed hangers may require special order.

* All beams shall be braced against rotation at points of bearing.

* Drypack grout all steel beam pockets flush after beams are set.

* Unless otherwise indicated, install two lengths of solid blocking x joist depth x 12 inches long in floor framing under column loads. Columns must have a continuous load path to foundation.

* Lead holes for lag bolts shall be 60% to 70% of lag shank diameter in compliance with AITC criteria.

FIELD VERIFICATION OF EXISTING CONDITIONS:

- * Contractor shall thoroughly inspect and survey existing structure to verify conditions that affect the work shown on the
- * Contractor shall report any variations or discrepancies to the Architect before proceeding.

STRUCTURAL ERECTION AND BRACING REQUIREMENTS:

* The structural drawings illustrate the completed structure with elements in their final positions, properly supported and braced. * The contractor, in the proper sequence, shall provide proper shoring and bracing as may be required to achieve the final completed structure.

* These construction documents contain typical and representative details to assist the contractor.

* Details shown apply at all similar conditions unless otherwise indicated.

* Although due diligence has been applied to make the drawings as complete as possible, not every detail is illustrated, nor is every exceptional condition addressed.

* All proprietary connections shall be installed in accordance with the manufacturers' recommendations.

* All work shall be accomplished in a workmanlike manner and in accordance with the applicable code and local ordinances. * The general contractor is responsible for coordination of all work, including layout and dimension verification, materials coordination, shop drawing review, and the work of subcontractors.

* Any discrepancies or omissions discovered in the course of the work shall be immediately reported to the architect for

Continuation of work without notification of discrepancies relieves the architect and engineer from all consequences.

* Unless otherwise specifically indicated, the drawings do not describe methods of construction. * The contractor, in the proper sequence, shall perform or supervise all work necessary to achieve the final completed structure, and to protect the structure, workmen, and others during construction.

* Such work shall include, but not be limited to, bracing, shoring for construction equipment, shoring for excavation, formwork, scaffolding, safety devices and programs of all kinds, support and bracing for cranes and other erection equipment.

* Do not backfill against basement or retaining walls until supporting slabs and floor framing are in place and securely anchored, unless adequate bracing is provided.

* Temporary bracing shall remain in place until all floors, walls, roofs and any other supporting elements are in place.

* The architect and engineer bear no responsibility for the above items, and observation visits to the site do not in any way include inspection of them.

* These plans have been engineered for construction at one specific building site. Builder assumes <u>ALL</u> responsibility for use of these plans at Any Other building site. Plans shall not be used for construction at any other building site without specific review

* Observations of foundation reinforcing or framing required by the owner, lender, insurer, building department or any other party will be accomplished by the engineer at the owner's expense. At least 24 hours advance notice is requested.

* All slabs on grade shall be separated from adjacent structural and finish elements to allow free movement of the slab, unless specifically shown and noted otherwise.

PLANT FABRICATED / PRE-ENGINEERED WOOD FRAMING:

Trussed floor joists and roof joists shall be designed and stamped by a registered engineer to support the full dead loads and the superimposed design loads noted on the drawings.

All pre-engineered trusses shall be designed for the deflection listed:

Floor: Total Load = L/240 or 1" max. Live Load = L/600

Roof: Total Load = L/240 or 1" max. Snow Load = L/300

Stresses shall not exceed those listed in the NDS. 15% stress increase may not be used.

Web arrangement and member forces shall be determined by the fabricator.

Manufacture and installation of trusses shall comply with

ANSI/TPI 1 "National Design Standard for Metal-Plate-Connected Wood Truss Construction",

TPI HIB "Commentary and Recommendations for Handling Installing and Bracing Metal Plate Connected Wood Trusses", TPI DSB "Recommended Design Specification for Temporary Bracing of Metal Plate Connected Wood Trusses"

Calculations and shop drawings, including member sizes, lumber species and grades, and substantiating data for connector

capacities, shall be submitted to the Architect and Engineer for review prior to fabrication. Manufactured "I"-series roof and floor joists shall be by TJI/ILevel, LPI, BCI, NJI, OJ with structural wood flanges and webs, and carry Code approval for the composite section.

Bridging and blocking shall be installed according to the fabricator's requirements. Shop drawings shall be submitted to the Architect for review prior to fabrication.

Beams noted as LVL on plan shall be 1-3/4" wide Laminated Veneer Lumber beams of the depth noted on plan

Shall be plant-fabricated and manufactured by I Level, Shall have the following minimum allowable design stresses:

Fb = 2600 psi Fv = 285 psiFc (||) = 2460 psi $Fc(\perp) = 750 \text{ psi}$ E = 1900 ksiBeams noted as PSL on plan shall be plant-fabricated

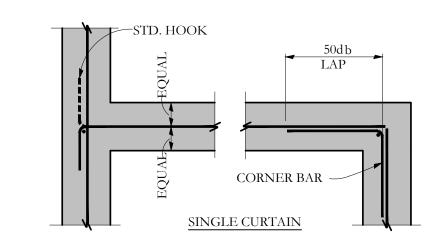
Manufactured by I Level, and have the following minimum allowable design stresses:

Fv = 290 psi $Fc(\|) = 2900 \text{ psi}$ E = 2000 ksi

AB	Anchor Rod (Bolt)	EF	Each Face	MACH	Machine	SC	Slip Critical
ADDL	Additional	EJ	Expansion Joint	MASY	Masonry	SCH	Schedule
ADJ	Adjustable	ELEV	Elevation	MATL	Material	SDST	Self Drilling Self Tapping
AFF	Above Finished Floor	ELEC	Electric (Electrical)	MAX	Maximum	SECT	Section
ALT	Alternate	ENGR	Engineer	MB	Machine bolt	SF	Square Feet
AMT	Amount	EQ	Equal	MECH	Mechanical	SHT	Sheet
ANCH	Anchor, Anchorage	EQUIP	Equipment	MEZZ	Mezzanine	SHTG	Sheathing
	Approximate	EQUIV	Equivalent	MFR	Manufacture, -er, -ed	SIM	Similar
ARCH	Architect, -ural	ES	Each Side	MIN	Minimum	SLH	Short Leg Horizontal
ATR	All Thread Rod	EST	Estimate	ML	Microllam	SLV	Short Leg Vertical
AVG	Average	E-W	East to West		(Trus-joist brand LVL)	SOG	Slab on Grade
BC	Bottom of Concrete	EXC	Excavate	МО	Masonry Opening	SP	Spaces
BL	Brick Ledge	EXP	Expansion	MTL	Metal	SPEC	Specifications
BLK	Block	EXT	Exterior	NF	Near Face	SQ	Square
BLKG	Blocking	FND	Foundation	NIC	Not In Contract	ST	Snug Tight
BM	Beam	FF	Far Face, Finished Floor	NS	Near Side	STD	Standard
ВОТ	Bottom	F-F	Face to Face	N-S	North to South	STIFF	Stiffener
BRG	Bearing	FIG	Figure	NTS	Not to Scale	STL	Steel
BW	Bottom of Wall	FL	Flush	OCJ	OSHA Column Joist		Structure, -al
СВ	Counterbore	FLG	Flange	OD	Outside Diameter	SUPT	Support
CF	Cubic Foot	FLR	Floor	OF	Outside Face	SY	Square Yard
CG	Center of Gravity	FO	Face of	ОН	Opposite Hand	SYM	Symmetrical
CIP	Cast in Place	FP	Full Penetration	OPNG	Opening	T&B	Top and Bottom
CJ	Construction Joint (Control Joint)	FS	Far Side	OPP	Opposite	T&G	Tongue and Groove
	` ,	FTG	Footing	OSB	Oriented Strand Board	TB	Top of Beam
CLG	Ceiling	GA	Gage (Gauge)	PAF	Powder Actuated Fast'n	TC	Top of Concrete
CLR	Clear	GALV	Galvanized	PC	Precast	TD	Top of Deck
CM	Construction Manager (Management)	GC	General Contractor	PCF	Pounds Per Cubic Foot	THD	Thread
CMII	Concrete Masonry Unit	GEN	General Glue laminated (Glulam)	PEN PERP	Penetration	THK	Thick, -ness
CMU	Concrete Masonry Unit	GL	<u> </u>	PERP	Perpendicular	TJ TL	Top of Joist Total Load
COL		GND	Ground Grade	PLF	Property Line		
COMB	Common Combination	GR GT	Grade Girder Truss	PLF	Pounds per Linear Foot	TPG TRANS	Topping
			Grader Truss Gypsum Board	PNL	Panel Panel Point	TW	Transverse Top of Wall
CONC CONN	Concrete Connection	HAS	Headed Anchor Stud	PS	Prestressed	TYP	Typical
CONT	Continue (Continuous)	HORIZ	Horizontal	PSF	Pounds per Square Foot	ULT	Ultimate
COORD	/	HT			Pounds per Square Inch	UNO	Unless Noted Otherwis
<u>COORD</u> CS	Coordinate, -tion Countersink	ID	Height Inside Diameter	PSI PSL	Pounds per Square Inch Parallel Strand Lumber	VERT	Vertical Otherwis
CTR	Center	IF	Inside Face	1 01	(generic term)	VERT	Verify in Field
CY	Cubic Yard	INT	Interior (Intermediate)	PT (1)	Post Tensioned	WA	Wedge Anchor
DAB	Deformed Anchor Bar	JB	Joist Bearing	PT (2)	Pressure Treated	WP	Work Point
DET	Detail	JST	Joist	PTN	Partition	WT	Weight
DEV	Develop	JT	Joint	PWD	Plywood	WWF	Welded Wire Fabric
DIAG	Diagonal	K	Kip (1,000 lbs.)	QTY	Quantity	XS	Extra Strong
DIM	Dimension	LD	Load	R	Radius	XSECT	Cross-section
DL DL	Dead Load	LL	Live Load	RE	Reference (refer to)	XXS	Double Extra Strong
DN	Down	LLH	Long Leg Horizontal	RECT	Rectangle		
DP DP	Drilled Pier	LLV	Long Leg Vertical	REINF	Reinforce, -ed, -ing	(E)	Existing
DT DT	Double Tee	LOC	Location	REQ	Required	(N)	New
DWG	Drawing	LSL	Laminated Strand	_ `	Requirement	(R)	Remove
DWL	Dowel		Lumber (generic term)	RET	Retaining		
EA	Each	LT	Light	RM	Room		
ECC	Eccentric	LVL	Laminated Veneer	RMO	Rough Masonry Opening		
	End to End	1	Lumber (generic term)	RO	Rough Opening		

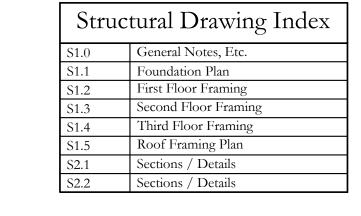
ABBREVIATIONS KEY

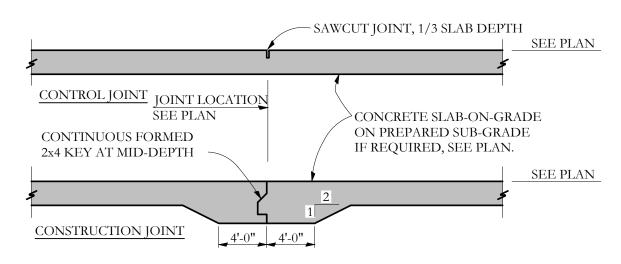
FRAM	FRAMING PLAN SYMBOLS KEY					
	WOOD POST					
0	STEEL COLUMN					
X	NUMBER OF WOOD STUDS IN POST BELOW					
A	COLUMN ABOVE THIS LEVEL					
С	COLUMN CONTINUOUS THROUGH THIS LEVEL					
-	JOIST BEARING					
	CONTINUOUS JOIST WITH INTERMEDIATE BEARING					
=	FLUSH FRAMED JOIST BEARING WITH HANGER					
	WOOD STUD BEARING WALL BELOW					
	OVER FRAMING BY OTHERS -TYP					
<u>"X"T</u>	NUMBER OF TRIM STUDS UNDER HEADER					
<u>"X"K</u>	NUMBER OF KING STUDS ADJACENT TO HEADER					



TYPICAL CONCRETE WALL INTERSECTIONS

NO SCALE





TYPICAL JOINTS AT INTERIOR SLAB-ON-GRADE NO SCALE



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